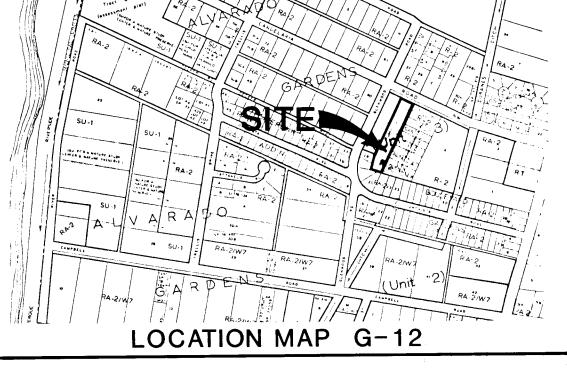


SOMBRA DEL RIO SUBDIVISION

INDEX TO DRAWINGS

- 1. COVER
- 2. PLAT
- 3. GRADING/DRAINAGE PLAN
- 4. GRADING/DRAINAGE PLAN
- 5. UTILITY PLAN/PROFILE
- 6. UTILITY PLAN/PROFILE

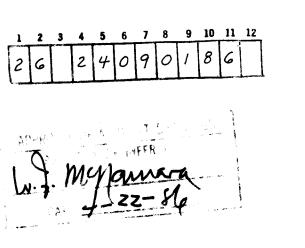


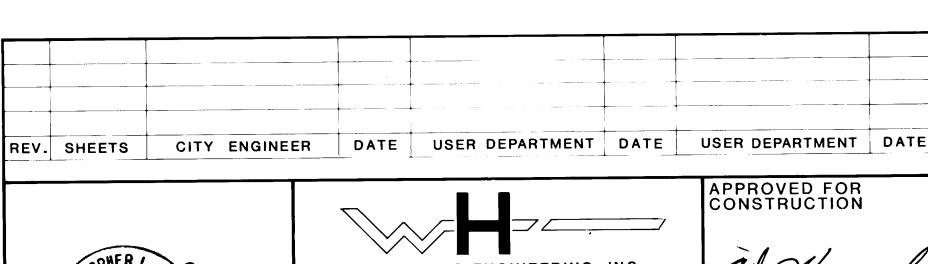
NOTICE TO CONTRACTORS

- 1) All work detailed on these plans to be performed under contract shall, except as otherwise stated or provided for hereon, be constructed in accordance with the New Mexico Standard Specifications for Public Works Construction 1979 Edition (referred to herein as the Standard Specifications) and the Contract Documents for Public Works Contract 85-1.
- 2) Two (2) working days prior to any excavation, contractor must contact Line Locating Service, 765-1234, for location of existing utilities.
- 3) Prior to construction, the contractor shall excavate and verify the Borizontal and vertical locations of all obstructions. Should a conflict exist, the contractor shall notify the engineer or surveyor so that the conflict can be resolved with minimum amount of delay.

THE FOLLOWING NOTES APPLY WHEN CHECKED:

- All utilities and utility service lines shall be installed prior to paving.
- Backfill compaction shall be according to specified street use. ART.-RES.
- igtherapsize Tack coat requirements shall be determined by the City Engineer.
- Sidewalks and wheelchair ramps within the curb returns shall be constructed wherever a new curb return is constructed
- If curb is depressed for a drivepad or a handicap ramp, the drivepad or ramp shall be constructed prior to acceptance of the curb and gutter.
- All storm drainage facilities shall be completed prior to final acceptance.







WEISS-HINES ENGINEERING, INC.
1100-B ALVARADO N.E.
ALBUQUERQUE, NEW MEXICO 87110
(505) 266-3444

PROJECT NO.

2409

SHEET ' O

2 3 4 5 6 7 8 9 10 11 29 2 6 2 4 0 9 0 Z 8 6

DEL RIO SUBDIVISION
ARIA BOULEVARD N.W.



SCOPE:

The proposed improvements, comprised of 16 condominiums, private access roadway, parking. landscaping, and utilities are located on Candelaria Blvd. N.W. just east of Glennwood Road. The front (north) portion of the site will be drained to a catch basin located on the south side of Candelaria approximately 150' west of the site. From conversations with City Hydrology, it was determined that discharge to the pump station on the extreme west end of Candelaria (at Riverside drain) has adequate capacity to empty the Candelaria storm sewer without flood hazard to private property. All other site drainage will be retained by backyard infiltration or by "Turfstone" ponds located in the private access roadway. All roof tops and impervious surfaces will drain to the "Turfstone" pond area. The developer has elected to pond the back south portion of the site rather than drain to Candelaria because of the large amount of fill required (average of 3') and the need for extensive retaining wall construction.

The present site is undeveloped except for a small home and pump house on the north side (to be demolished.) The existing home site drains to Candelaria. The remainder of the site ponds and infiltrates locally.

The intent of this plan is to show:

- a) Grading relationships between the existing ground elevations and proposed finished elevations in order to facilitate positive drainage to designated discharge points.
- b) The extent of proposed site improvements, including buildings, walks and pavement.
- c) The flow rate/volume of rainfall runoff across or around these improvements and methods of handling these flows to meet City requirements for drainage management.
- d) The relationship of onsite improvements with existing neighboring property to insure an orderly transition between proposed and surrounding grades.

GENERAL NOTES:

LEGAL:
E/2 Lot G, Alvarado Gardens, Albuquerque, New Mexico, MRGCD Map G-12.

SURVEYOR:
Steve Walker, R.L.S., June 22, 1984.

B.M.:
City of Albuquerque BM (8-G13), " \Box " at S. end of S.W. curb return on Rio Grande Blvd. and Candelaria Road. Elevation = 4968.09.

 $\underline{\text{T.B.M.:}}$ Nail in power pole at sidewalk near N.E. property corner, Elevation =

Gila Clay Loam (Ge), Hydrologic Class B.

FLOOD HAZARD:

No flood hazard areas are located in the vicinity.

Site is not affected by off-site flows.

EROSION CONTROL:

Site is bordered by adobe and wood fences on east side which will prevent movement of eroded sediments during construction. Two-ft. high silt fences will be installed during site construction to prevent sediment from leaving site on the west and south sides. A 6"-high driveway berm will be placed across the entrance from Candelaria during construction to control sediments on the north side.

CALCULATIONS:

Based on a pre-design meeting with City of Albuquerque Hydrology, in 11-84, the following criteria was established.

> a. Allowable discharge from site to be determined by analysis of downstream capacity. On-site retention will be considered if no other solution exist.

Calculations are based on the City of Albuquerque D.P.M. Manual, Vol. II for the 100 year-6 hour storm, using the Rational Formula to compare the existing and proposed runoff rates.

RESUBMITTAL 3-12-85 NOTES ADDED TO POND VOLUME CALCULATIONS.

Pond Volume Calculations (cont'd)

Three percolation tests run on Ge soils at the nearby Pawidol subdivision by Albuquerque Testing Labs indicate that the average infiltration rate was 0.05 cu.ft. per sq.ft. of pond surface area. Rate at Sombra del Rio will be much higher because turfstone pond is underlain by sand and gravel. Use 0.05 rate to be conservative. Calculation to show that pond will drain in 24 hours:

0.05 cu.ft./hr x7500 sq.ft.(full pond)x4 hrs = 1500 cu.ft. 0.05 cu.ft./hr x3750 sq.ft.(half full)x8 hrs = 1500 cu.ft. 0.05 cu.ft./hr x2500 sq.ft.(third full)x12hrs = $\frac{1500 \text{ cu.ft.}}{4,500} > \frac{4,200 \text{ o.k.}}{4,200}$

RATIONAL METHOD- Q = CIA

Area of site: 69,700 sq.ft.= 1.6 Ac. North = 0.4 Ac., South = 1.2 Ac.

Run-off Coefficient:

Existing site:	Developed Site:
A imp. = .04 Ac % imp. = 2 % "C" = 0.36 (DPM 22.2 C-1) North side "C" = 0.42 South side "C" = 0.34	A imp. = 0.66 Ac % imp. = 41 % "C" = 0.52 (DPM 22.2 C-1) North side "C" = 0.70 South side "C" = 0.44 (Use 0.50 to account for Turfstone)

Rainfall Intensity:

 $I = P_6 (6.84) Tc^{-0.51} = 4.65$ " per hour where $P_6 = 2.2$ " (DPM 22.2 D-1) Tc = 10 minutes

Existing Condition:	<u>Developed Condition:</u>
Q100 = $\frac{\text{North Side}}{(0.42(4.65))(.40)}$	Q100 = $\frac{\text{North Side}}{(0.70)(4.65)}(0.40)$
= 0.8 cfs	= 1.3 cfs
V100 = (0.8)(5 Tc)(60 sec/in)/2	V100 = (1.3)(50)(60)/2
= 1,200 cu. ft.	= 1,950 cu. ft.
Q100 = $\frac{\text{South Side}}{(0.34)(4.65)(1.2)}$	Q100 = $\frac{\text{South Side}}{(.50)(4.65)(1.2)}$
= 1.9 cfs	= 2.8 cfs
V100 = (1.9)(50)(60)/2	V100 = (2.8)(50)(60)/2
= 2,845 cu. ft.	= 4,200 cu. ft.
<u>Total</u>	<u>Total</u>
Q100 = 2.7 cfs	Q100 = 4.1 cfs
V100 = 4,045 cu. ft.	V100 = 6,150 cu. ft.

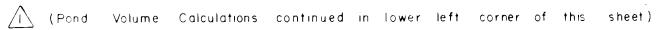
SUMMARY:

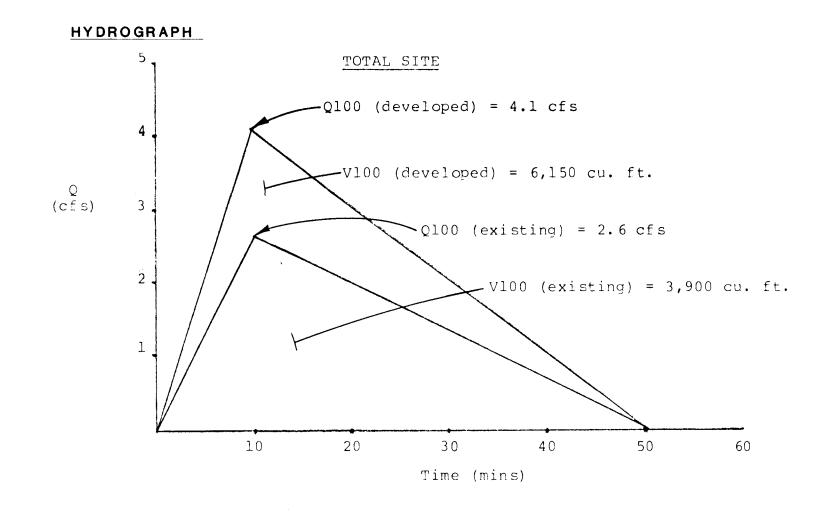
 $\triangle Q100 = (4.1)-(2.7) = 1.4 \text{ cfs (increase)}$ $\Delta V100 = (6,150)-(4,045) = 2,105$ cu. ft. (increase)

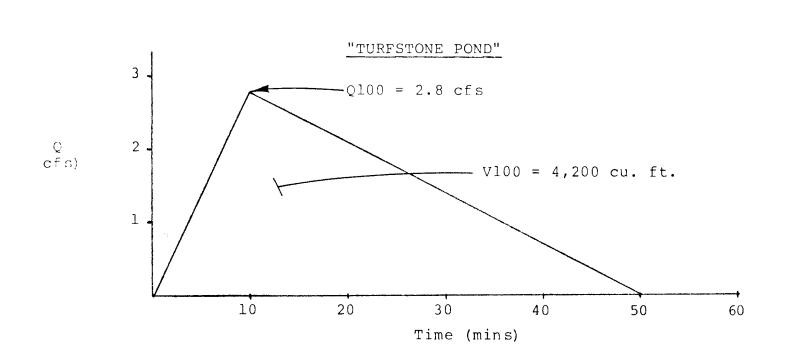
POND VOLUME CALCULATIONS

The drainage of the site is divided into 3 areas and will be handled as follows:

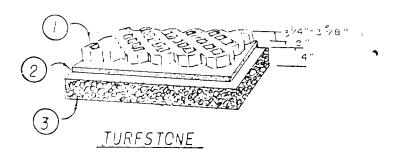
1) All backyard areas, throughout subdivision will infiltrate rainfall directly. 2) The North portion will drain (free discharge) to Candelaria Blvd. 3) Therefore, only buildings and front areas of the south portion of site must be ponded. Required V100 for complete retention of south-side flows is somewhat less than 4,200 cu. ft. calculated above. By planimeter and assuming a maximum depth of 0.6' for "Turfstone" pond at elevation = 67.0, available pond volume = 4,500 cu. ft. To improve infiltration capacity of "Turfstone" pond, a series of 7'-deep, 6"diameter gravel-filled holes will be constructed at locations shown on







DETAILS



1 Turfstone

② 2" layer graded sand

 \odot 4" of 3/4" crusher run gravel

INSTALLATION NOTES -

1. Overexcavate sub-soil 6" below finished grade and replace, compacted to 95% max.dry density(ASTM D-1557)

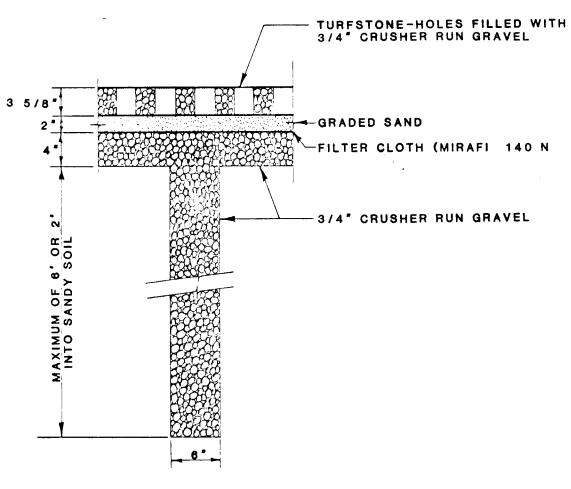
2. Use 4" of 3/4" crusher run, which should be compacted with a vibrating compactor

3. Install the levelling bed of a 2" layer of graded sand, levelled. Do not compact.

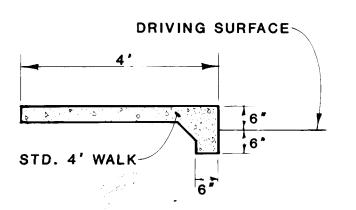
4. Install Turfstone over sand, leaving a space of 1/8" between each paver.

5. Turfstone holes filled with 3/4" crusher run gravel.

TURFSTONE INSTALLATION



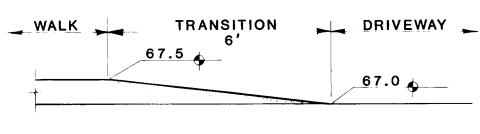
SEEPAGE HOLE



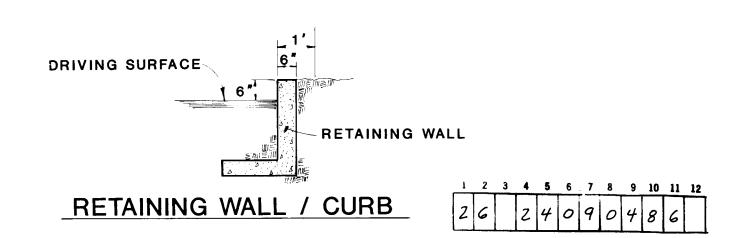
2" ASPHALT 6" SUBBASE COMPACTED SUBGRADE

TURNED DOWN WALK

ON SITE PAVING SECTION



SIDEWALK TRANSITION



DEL



