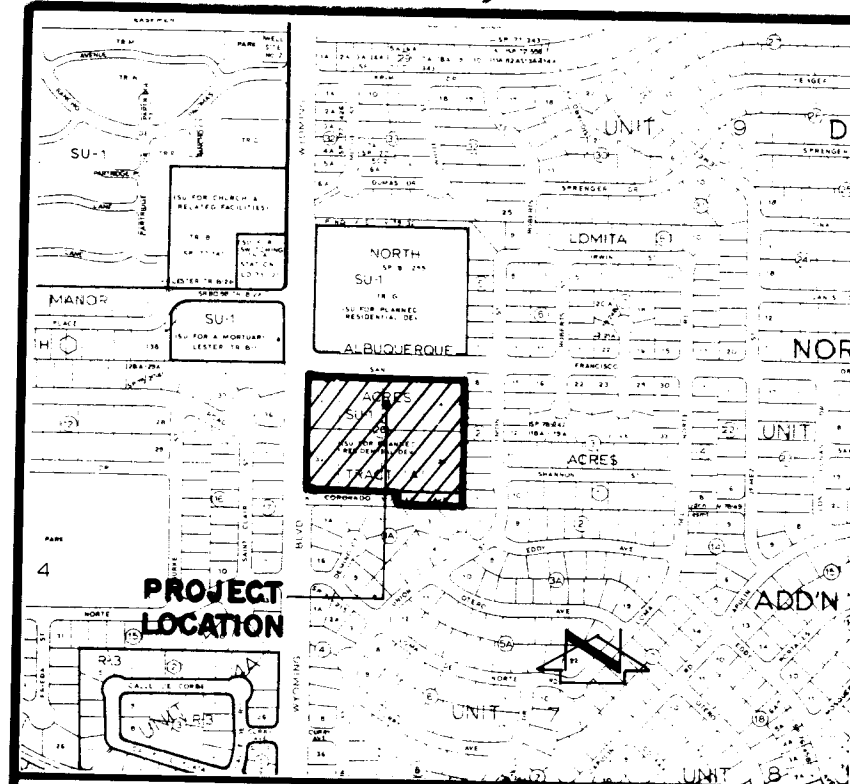


SHEET 1 OF 8

VICINITY MAP D-19
SCALE 1" = 800'

KEYSTONE PARK ADDITION A REPLAT OF LOTS 1-4 & 29-32, TRACT A, UNIT A NORTH ALBUQUERQUE ACRES ALBUQUERQUE, NEW MEXICO SEPTEMBER, 1985

THE UNDERSIGNED OWNERS OF THE LAND SHOWN HEREON DO HEREBY CONSENT TO THE SUBDIVISION OF SAID LAND IN THE MANNER SHOWN ON THIS PLAT AND DO HEREBY DEDICATE THE PUBLIC RIGHTS-OF-WAY, PRIVATE WAY AND GRANT UTILITY EASEMENTS INCLUDING THE RIGHTS OF INGRESS AND EGRESS AND THE RIGHT TO MAINTAINING THE SAME.

Thomas E. Fielder
THOMAS E. FIELDER
9/26/85
Date

ACKNOWLEDGEMENT
STATE OF NEW MEXICO)
COUNTY OF BERNILLO) SS
5-6-89

The foregoing instrument was acknowledged before me this
25th day of September, 1985

Charles E. Calap
Notary Public

APPROVALS:
Richard P. Green 1-15-86
Planning Director, City of Albuquerque, N.M. Date
Jon E. Eidsgaard 12-30-85
City Engineer, City of Albuquerque, N.M. Date
Frank J. Gagny 1-13-86
City Engineer, City of Albuquerque, N.M. Date
Frank J. Gagny 1-13-86
City Engineer, City of Albuquerque, N.M. Date
Robert A. Hume 12-30-85
Traffic Engineer, City of Albuquerque, N.M. Date
James J. Davis 12-30-85
City Engineer, City of Albuquerque, N.M. Date
W. H. Chitt 02/28/85
City Engineer, City of Albuquerque, N.M. Date
Rene E. Hume 10-3-85
City Engineer, City of Albuquerque, N.M. Date
John J. Myers 12/10/85
City Engineer, City of Albuquerque, N.M. Date
Mike Anderson 12-12-85
City Engineer, City of Albuquerque, N.M. Date
Wayne J. Hume 10-18-85
City Engineer, City of Albuquerque, N.M. Date

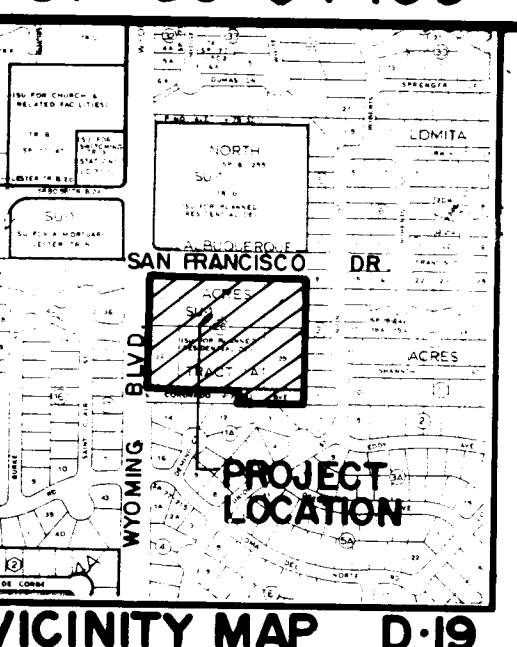
CERTIFICATION
I, Thomas T. Mann, Jr., a registered Land Surveyor under the laws of the State of New Mexico, do hereby certify that this plat was prepared by me or under my supervision, shows all easements noted in a title report prepared by J.A.S. LIFE-TITLE COMPANY on April 1, 1985 (1147890-11). This plat meets the minimum requirements of the Uniform Subdivision Act and the Uniform Subdivision Ordinance, and is true and correct to the best of my knowledge and belief.

Thomas T. Mann, Jr.
THOMAS T. MANN, JR.
9/26/85
Date

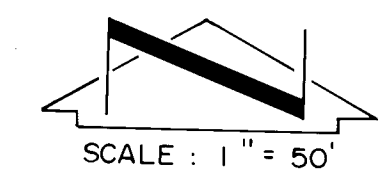
ACKNOWLEDGEMENT
STATE OF NEW MEXICO)
COUNTY OF BERNILLO) SS
5-6-89

The foregoing instrument was acknowledged before me on this
25th day of September, 1985

Charles E. Calap
Notary Public
SP-86-18
DRB-85-750-1

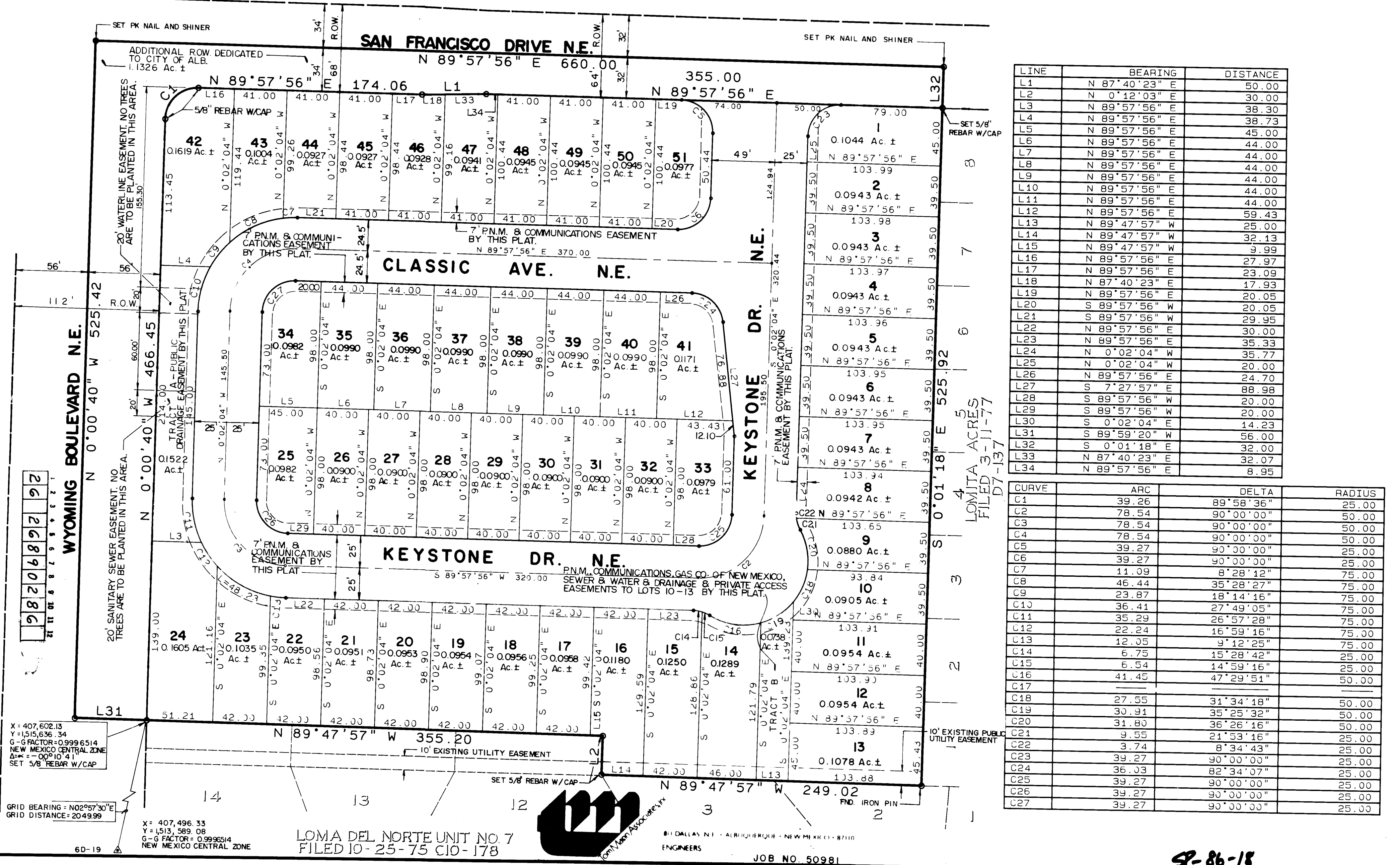
VICINITY MAP D-19
SCALE 1" = 800'

KEYSTONE PARK ADDITION A REPLAT OF LOTS 1-4 & 29-32, TRACT A, UNIT A NORTH ALBUQUERQUE ACRES ALBUQUERQUE, NEW MEXICO SEPTEMBER, 1985



NORTH ALBUQUERQUE ACRES TRACT G FILED 10-19-81 C18-200

- NOTES:
1. Property corners have been set as indicated with cap. L.S. No. 6401.
 2. 0.22 miles of half-width and 0.25 miles of full-width street created.
 3. All distances are ground distances.
 4. Site located within Section 21, T11N, R2E, N.M.P.M.
 5. Bearing base is plat of "Loma Del Norte, Unit 7" filed October 22, 1976, Book 610, Page 178. Grid to Ground Rotation Factor = 0.00006.
 6. Access to Wyoming Blvd. and San Francisco Dr. from Lots 1, 24 and 42-51 is prohibited.
 7. Tract A is to be utilized for drainage purposes. Maintenance of the drainage facilities shall be by the lot owners of the subdivision.
 8. The Public Service Company of New Mexico (P.N.M.) and Communications easements shown hereon are reserved for overhead distribution lines for pole top utilities and buried distribution lines, conduits and pipes for underground utilities where shown or indicated and include the right of ingress and egress for construction and maintenance, and the right to trim interfering trees and shrubs. Also included are easements for the right to install power and communication service lines along side of lot lines.



LINE	BEARING	DISTANCE
L1	N 87°40'23" E	30.00
L2	N 0°12'03" E	30.00
L3	N 88°57'56" E	38.30
L4	N 88°57'56" E	38.73
L5	N 88°57'56" E	45.00
L6	N 88°57'56" E	44.00
L7	N 88°57'56" E	44.00
L8	N 88°57'56" E	44.00
L9	N 88°57'56" E	44.00
L10	N 88°57'56" E	44.00
L11	N 88°57'56" E	44.00
L12	N 88°57'56" E	59.43
L13	N 88°47'57" W	23.08
L14	N 89°47'57" W	25.00
L15	N 88°57'56" E	32.13
L16	N 88°57'56" E	27.57
L17	N 88°57'56" E	23.08
L18	N 87°40'23" E	17.93
L19	N 88°57'56" E	20.05
L20	S 88°57'56" W	20.05
L21	S 88°57'56" W	29.95
L22	N 88°57'56" E	30.00
L23	N 88°57'56" E	35.77
L24	N 0°02'04" W	20.00
L25	N 88°57'56" E	24.70
L26	S 7°27'57" E	88.98
L27	S 88°57'56" W	20.00
L28	S 88°57'56" W	20.00
L29	S 88°57'56" W	14.83
L30	S 0°02'04" E	56.00
L31	S 88°57'56" W	38.00
L32	S 0°01'18" E	32.07
L33	N 87°40'23" E	8.95
L34	N 88°57'56" E	8.95

CURVE	ARC	DELTA	RADIUS
C1	39.26	89°58'36"	25.00
C2	78.54	90°00'00"	50.00
C3	78.54	90°00'00"	50.00
C4	78.54	90°00'00"	50.00
C5	39.27	90°00'00"	25.00
C6	39.27	90°00'00"	25.00
C7	11.09	8°28'12"	75.00
C8	46.44	35°28'27"	75.00
C9	23.87	18°14'16"	75.00
C10	36.41	27°49'05"	75.00
C11	35.29	26°57'28"	75.00
C12	22.24	16°59'16"	75.00
C13	12.08	8°12'25"	75.00
C14	6.75	4°29'42"	25.00
C15	6.54	4°39'16"	25.00
C16	41.45	47°29'51"	50.00
C17			
C18	27.55	31°34'18"	50.00
C19	30.93	35°25'32"	50.00
C20	31.80	36°26'16"	50.00
C21	3.58	2°13'16"	25.00
C22	3.74	8°34'43"	25.00
C23	39.27	90°00'00"	25.00
C24	36.33	82°34'07"	25.00
C25	39.27	90°00'00"	25.00
C26	39.27	90°00'00"	25.00
C27	39.27	90°00'00"	25.00

EROSION CONTROL MEASURES

1. THE CONTRACTOR SHALL ENSURE THAT NO SOIL ERODES FROM THE SITE INTO PUBLIC RIGHT-OF-WAY OR ONTO PRIVATE PROPERTY. THIS CAN BE ACHIEVED BY CONSTRUCTING TEMPORARY BERMS AT THE PROPERTY LINES AND WETTING THE SOIL TO KEEP IT FROM BLOWING.
2. THE CONTRACTOR SHALL PROMPTLY CLEAN UP ANY MATERIAL EXCAVATED WITHIN THE PUBLIC RIGHT-OF-WAY SO THAT THE EXCAVATED MATERIAL IS NOT SUSCEPTIBLE TO BEING WASHED DOWN THE STREET.
3. THE CONTRACTOR SHALL SECURE "TOPSOIL DISTURBANCE PERMIT" PRIOR TO BEGINNING CONSTRUCTION.

CONSTRUCTION NOTES

1. TWO (2) WORKING DAYS PRIOR TO ANY EXCAVATION, CONTRACTOR MUST CONTACT LINE LOCATING SERVICE 765-1234, FOR LOCATION OF EXISTING UTILITIES.
2. PRIOR TO CONSTRUCTION, THE CONTRACTOR SHALL EXCAVATE AND VERIFY THE HORIZONTAL AND VERTICAL LOCATION OF ALL POTENTIAL OBSTRUCTIONS. SHOULD A CONFLICT EXIST, THE CONTRACTOR SHALL NOTIFY THE ENGINEER SO THAT THE CONFLICT CAN BE RESOLVED WITH A MINIMUM AMOUNT OF DELAY.
3. ALL WORK ON THIS PROJECT SHALL BE PERFORMED IN ACCORDANCE WITH APPLICABLE FEDERAL, STATE AND LOCAL LAWS, RULES AND REGULATIONS CONCERNING CONSTRUCTION SAFETY AND HEALTH.
4. ALL CONSTRUCTION WITHIN PUBLIC RIGHT-OF-WAY SHALL BE PERFORMED IN ACCORDANCE WITH APPLICABLE CITY OF ALBUQUERQUE STANDARDS AND PROCEDURES.

PROJECT BENCHMARK

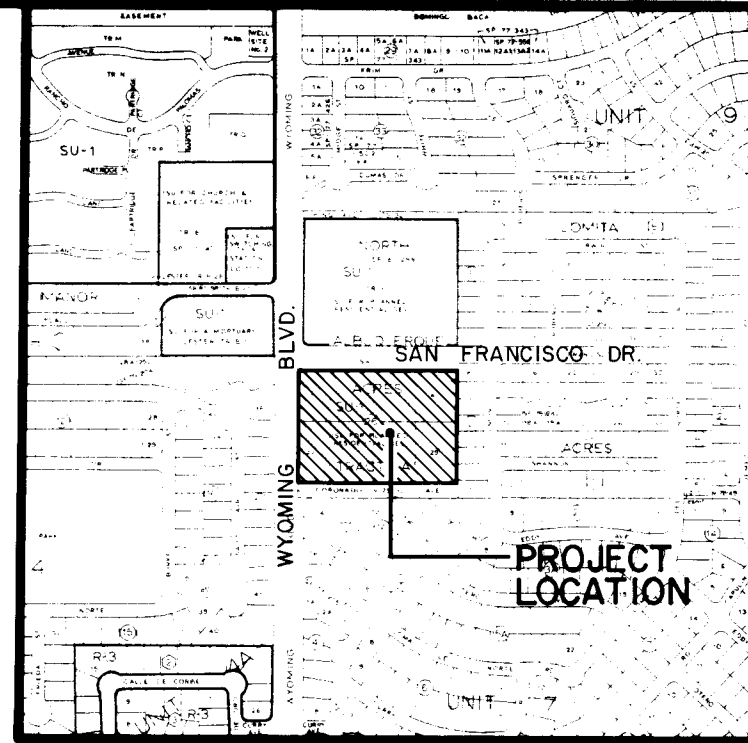
A STANDARD ACS BENCHMARK TABLE "G-19-1978" SET IN TOP OF A CONCRETE POST FLUSH WITH THE GROUND LOCATED AT THE NORTHWEST CORNER OF THE INTERSECTION OF SAN FRANCISCO DRIVE, N.E. & WYOMING BOULEVARD, N.E.
ELEVATION = 5405.54 FEET (M.S.L.D.)

LEGAL DESCRIPTION

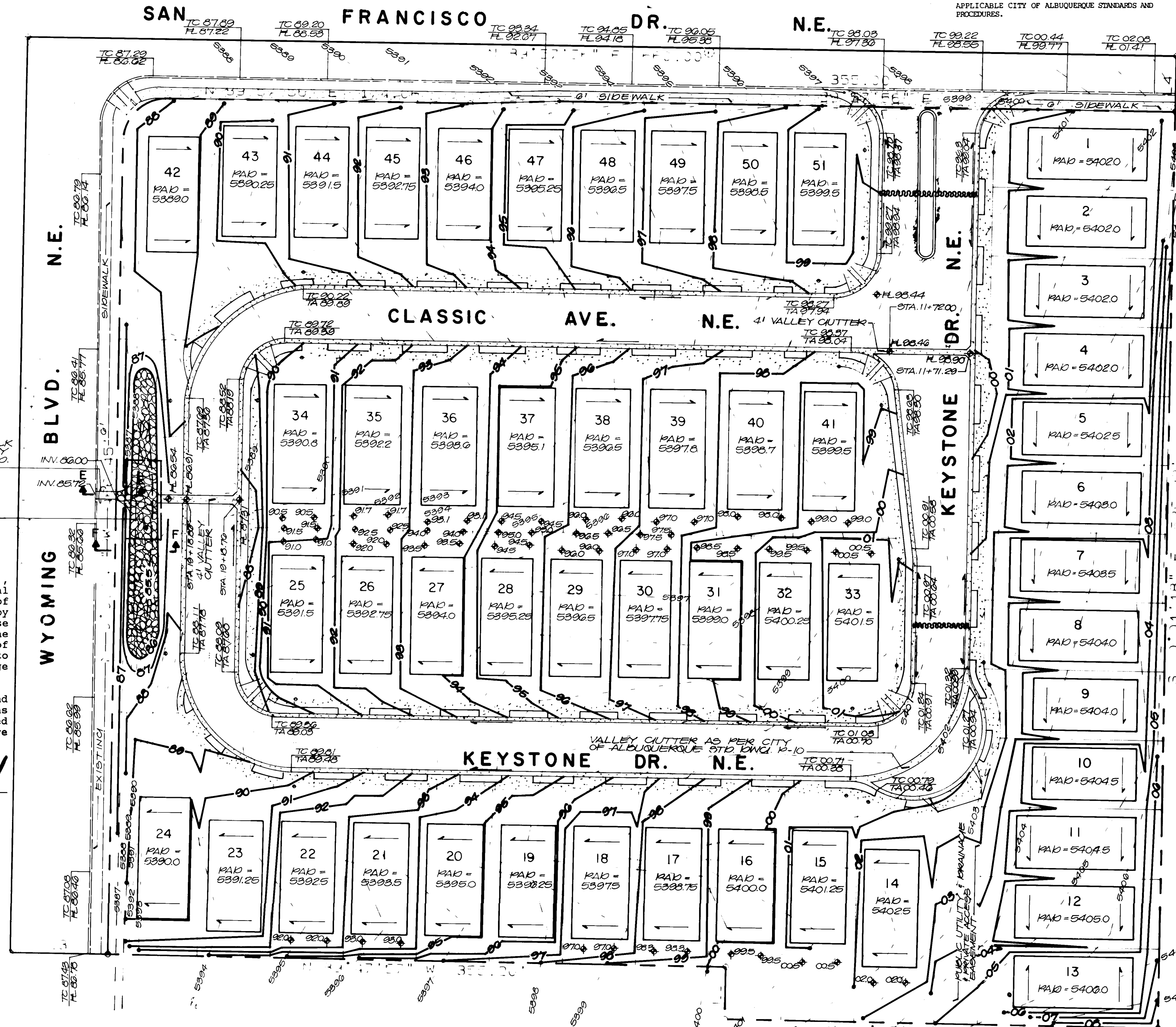
LOTS 1-4 & 20-32, TRACT A, UNIT A, NORTH ALBUQUERQUE ACRES

LEGEND

- 5400 --- EXISTING CONTOUR
- PROPOSED CONTOUR
- EXISTING SPOT ELEVATION
- PROPOSED SPOT ELEVATION
- SWALE
- PROPOSED CONCRETE
- PROPOSED ASPHALT PAVING
- TC 99.22 TOP OF CURB
- HL 98.55 FLOWLINE
- WATERBLOCK



VICINITY MAP
SCALE: 1" = 800'



The following items concerning the Keystone Park South Drainage Plan are contained herein:

1. Vicinity Map
2. Grading Plan
3. Calculations

The proposed improvements, as shown by the Vicinity Map, are located on the south side of San Francisco Drive N.E. and east of Wyoming Boulevard N.E. At the present time, the site is undeveloped. The adjoining parcels are developed.

As shown by Panel 10 of the National Flood Insurance Program Flood Boundary and Floodway Map for the City of Albuquerque, New Mexico, this site does not lie within a designated Flood Hazard Zone. The existing site is higher than Wyoming Boulevard to the west and San Francisco Drive to the north. The land to the east of the project site is higher than the project site, but concrete block walls prevent runoff from entering the project site. The land to the south is approximately parallel to the project site, but again block walls prevent stormwater runoff from entering the project site; therefore, offsite flows are negligible.

The Grading Plan shows 1) existing and proposed grades indicated by spot elevations and contours at 1'-0" intervals, 2) the continuity between existing and proposed elevations, 3) the limit and character of existing improvements, and 4) the limit and character of proposed improvements. As shown by this plan, the proposed improvements consist of the construction of residences, streets, sidewalks and associated improvements. Street grades for San Francisco Drive N.E. have been established in SDN No. 213. At present the site drains from east to west. The proposed grading will continue the flow in an east-to-west direction. Flow will be collected by the streets and routed to the west and into a ponding area before exiting into Wyoming Boulevard. The pond area will detain low flows and reduce the peak flow rates during large storms, thereby benefiting flow conditions downstream in San Francisco Drive. Stormwater runoff will continue across Wyoming Boulevard and in a westerly direction down San Francisco Drive. As the flows approach Interstate Highway No. 25, there is an area designated as a Zone A2 Flood Area. This zone could have flooding to a depth of one foot in a 100-year, 6-hour rainfall event.

Individual lots are graded to drain to the street in front of each lot. Grading in conjunction with block walls will direct flows to the front of each unit. Cross-lot drainage easements will not be necessary.

The calculations which appear below analyze both the existing and developed conditions for the 100-year, 6-hour rainfall event and for the 10-year, 6-hour rainfall event. The Rational Method has been used for this analysis in accordance with the City of Albuquerque Development Process Manual, Volume II. As shown by these calculations, the flow from the existing condition has been increased to 209 cfs. The increase in flow as a result of development is less than 5%, which is a relatively small amount. The calculations for the 10-year, 6-hour rainfall event show a depth of flow in San Francisco Drive N.E. of 0.62 feet in both the existing and developed conditions, which shows that the proposed development will have no impact on the street flows. These calculations show the worst condition of street flows in San Francisco Drive N.E. At the present time, construction is under way for two drop inlets in San Francisco Drive at the Wyoming Boulevard intersection. These inlets will collect water from San Francisco Drive and divert it out of the basin and to the north. The lot to the north have the capacity to divert more water out of the basin than the increased runoff generated by the development of Keystone Park South. This pattern of runoff is consistent with the pre-design conference recap which accompanies this submittal.

CALCULATIONS - KEYSTONE PARK SOUTH

Ground Cover Information

From SCS Bernallillo County Soil Survey, Plates 11 & 12:
Rc Budo-Tijeras Complex
Hydrologic Soil Group B

Rational Method

Discharge: $Q = CIA$
where C varies
 $1 = P_2 (6.84) T_p^{-0.51} = 4.97 \text{ in./hr.}$
 $P_2 = 2.35 \text{ in. (DN Plate 22.2 D-1)}$
 $T_p = 10 \text{ min. (minimum)}$
 $A = \text{area, acres}$

Volume: $V = C_p A (1/12)$
where C varies
 $P_2 = 2.35 \text{ in. (DN Plate 22.2 D-1)}$
 $A = \text{area, acres}$

Existing Condition

Total = 305,700 sf = 7.02 Ac
 $A_{imp} = 0 \text{ sf; } \% \text{ impervious} = 0\%$
 $C_{imp} = 0.34 \text{ (DN Plate 22.2 C-1)}$
 $Q_{100} = CIA = 0.34(4.97)(7.02) = 12 \text{ cfs}$
 $V_{100} = C_p A = 0.34(2.35/12)(305,700) = 20,350 \text{ cf}$

Developed Condition

Total = 305,700 sf = 7.02 Ac
 $A_{imp} = 190,000 \text{ sf; } \% \text{ impervious} = 62\%$
 $C_{imp} = 0.64 \text{ (DN Plate 22.2 C-1)}$
 $Q_{100} = CIA = 0.64(4.97)(7.02) = 22 \text{ cfs}$
 $V_{100} = C_p A = 0.64(2.35/12)(305,700) = 38,310 \text{ cf}$
Ypond = $1/2(2.64)(1,280)(0.50) + 1/2(4.02)(2,640)(1.0)$
 $= 4,315 \text{ cf}$

Comparison

$\Delta Q_{100} = 22 - 12 = 10 \text{ cfs (increase)}$
 $\Delta V_{100} = 38,310 - 20,350 = 17,960 \text{ cf (increase)}$

Gate Capacity for Type 'A' Storm Inlet (Two grates)

$Q = 13 \text{ cfs per each (DN Plate 22.3 D-5)}$
Capacity = 26 cfs
 $Q = 1.49/n A^{2/3} s^{1/2}$ (Manning Equation)
 $n = \text{slope} = 0.0332$
 $d = \text{depth of water above flowline} = 0.67$

$Q = 1.49/n A^{2/3} s^{1/2}$ (Manning Equation)
 $n = \text{slope} = 0.02$
 $d = \text{depth} = 0.625$
Capacity = $f(w, s, n, d) = 22.5 \text{ cfs}$

Flow Capacity

Capacity = 15 cfs
Depth = 18"

Hydraulic Analysis of Sidewalk Culvert

$Q = 1.49/n A^{2/3} s^{1/2}$ (Manning Equation)
 $w = \text{bottom width} = 4'$
 $n = \text{slope} = 0.022$
 $n = \text{roughness coefficient} = 0.015 \text{ (concrete)}$
 $d = \text{side slope} = 0 \text{ (vertical)}$
 $d = \text{depth} = 0.625$
Capacity = $f(w, s, n, d) = 22.5 \text{ cfs}$

CALCULATIONS - 10-YEAR, 6-HOUR RAINFALL EVENT

Ground Cover Information

From SCS Bernallillo County Soil Survey, Plate 11 & 12:
Rc Budo-Tijeras Complex
Hydrologic Soil Group B

Rational Method

Discharge: $Q = CIA$
where C varies
 $1 = P_2 (6.84) T_p^{-0.51} = 1.61 \text{ in./hr.}$
 $P_2 = 2.35 \text{ in. (DN Plate 22.2 D-1)}$
 $T_p = 40 \text{ min}$
 $A = \text{area, acres}$

Volume: $V = C_p A (1/12)$
where C varies
 $P_2 = 2.35 \text{ in. (DN Plate 22.2 D-1)}$
 $A = \text{area, acres}$

Existing Condition

Total = 6,387,500 sf = 147 Ac
 $A_{imp} = 2,871,800 \text{ sf; } \% \text{ impervious} = 45\%$
 $C_{imp} = 0.58 \text{ (DN Plate 22.2 C-1)}$
 $Q_{100} = CIA = 0.58(1.61)(147) = 130 \text{ cfs}$
 $V_{100} = C_p A = 0.58(2.35/12)(6,387,500) = 450,850 \text{ cf}$

Developed Condition

Total = 6,387,500 sf = 147 Ac
 $A_{imp} = 3,066,000 \text{ sf; } \% \text{ impervious} = 48\%$
 $C_{imp} = 0.58 \text{ (DN Plate 22.2 C-1)}$
 $Q_{100} = CIA = 0.58(1.61)(147) = 137 \text{ cfs}$
 $V_{100} = C_p A = 0.58(2.35/12)(6,387,500) = 475,440 \text{ cf}$

Comparison

$\Delta Q_{100} = 137 - 130 = 7 \text{ cfs (increase)}$
 $\Delta V_{100} = 475,440 - 450,850 = 24,590 \text{ cf (increase)}$

Street Cross-Section of San Francisco Drive

$Q = 1.49/n A^{2/3} s^{1/2}$ (Manning Equation)
 $w = \text{bottom width} = 48'$
 $n = \text{slope} = 0.024$
 $n = \text{roughness coefficient} = 0.017$
 $d = \text{side slope} = 0 \text{ (vertical)}$
 $d = \text{depth}$
 $Q = f(w, s, n, d)$
Depth existing condition = 0.62'
Depth developed condition = 0.62'

CALCULATIONS - TOTAL BASIN

Ground Cover Information

From SCS Bernallillo County Soil Survey, Plates 11 & 12:
Rc Budo-Tijeras Complex
Hydrologic Soil Group B

Rational Method

Discharge: $Q = CIA$
where C varies
 $1 = P_2 (6.84) T_p^{-0.51} = 2.45 \text{ in./hr.}$
 $P_2 = 2.35 \text{ in. (DN Plate 22.2 D-1)}$
 $T_p = 40 \text{ min}$
 $A = \text{area, acres}$

Volume: $V = C_p A (1/12)$
where C varies
 $P_2 = 2.35 \text{ in. (DN Plate 22.2 D-1)}$
 $A = \text{area, acres}$

Existing Condition

Total = 6,387,500 sf = 147 Ac
 $A_{imp} = 2,871,800 \text{ sf; } \% \text{ impervious} = 45\%$
 $C_{imp} = 0.58 \text{ (DN Plate 22.2 C-1)}$
 $Q_{100} = CIA = 0.58(2.45)(147) = 209 \text{ cfs}$
 $V_{100} = C_p A = 0.58(2.35/12)(6,387,500) = 687,990 \text{ cf}$

Developed Condition

Total = 6,387,500 sf = 147 Ac
 $A_{imp} = 3,066,000 \text{ sf; } \% \text{ impervious} = 48\%$
 $C_{imp} = 0.58 \text{ (DN Plate 22.2 C-1)}$
 $Q_{100} = CIA = 0.58(2.45)(147) = 209 \text{ cfs}$
 $V_{100} = C_p A = 0.58(2.35/12)(6,387,500) = 725,514 \text{ cf}$

Comparison

$\Delta Q_{100} = 209 - 209 = 0 \text{ cfs (increase)}$
 $\Delta V_{100} = 725,514 - 687,990 = 37,524 \text{ cf (increase)}$

AS-BUILT CERTIFICATION

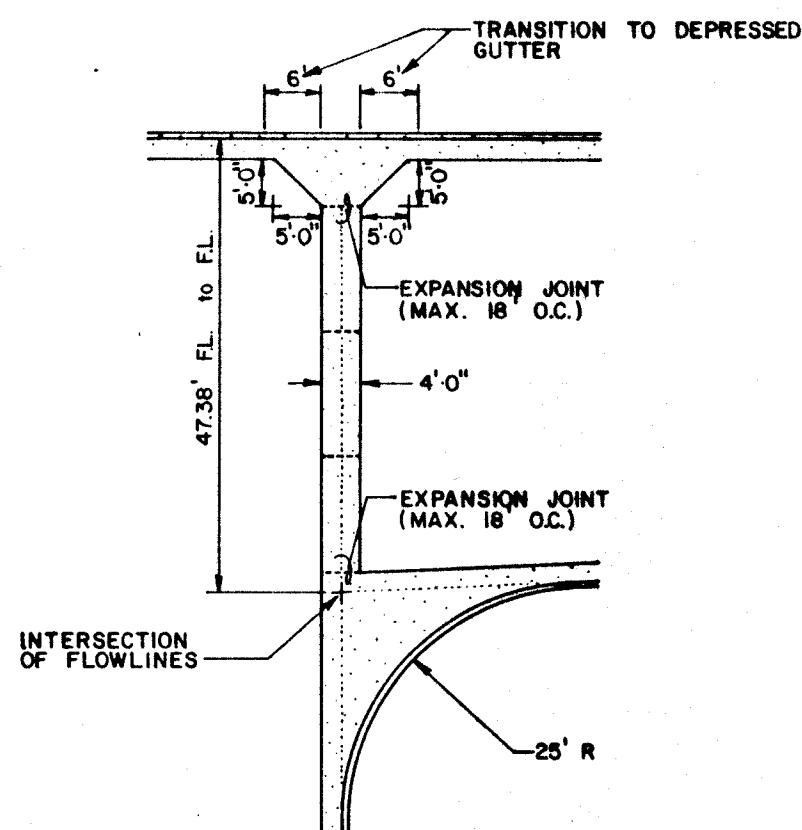
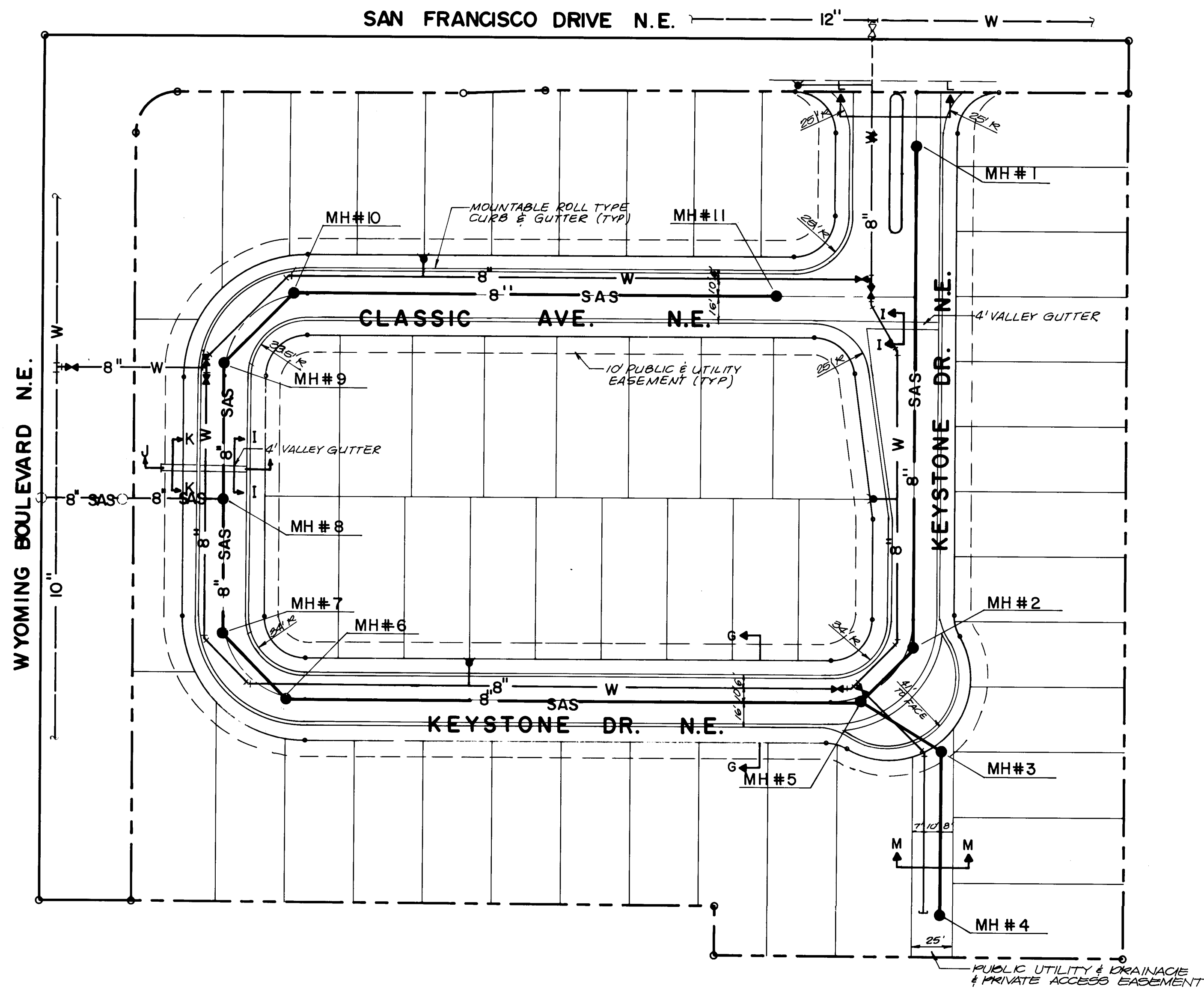
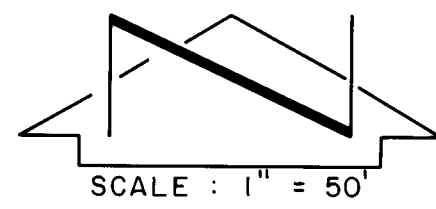
I, Thomas T. Mann, Jr., Registered Professional Engineer in the State of New Mexico, do hereby certify that these drawings represent the "as-built" conditions of this project to the best of my knowledge and belief.

All vertical and horizontal dimensions should be filed verified prior to use on future projects.

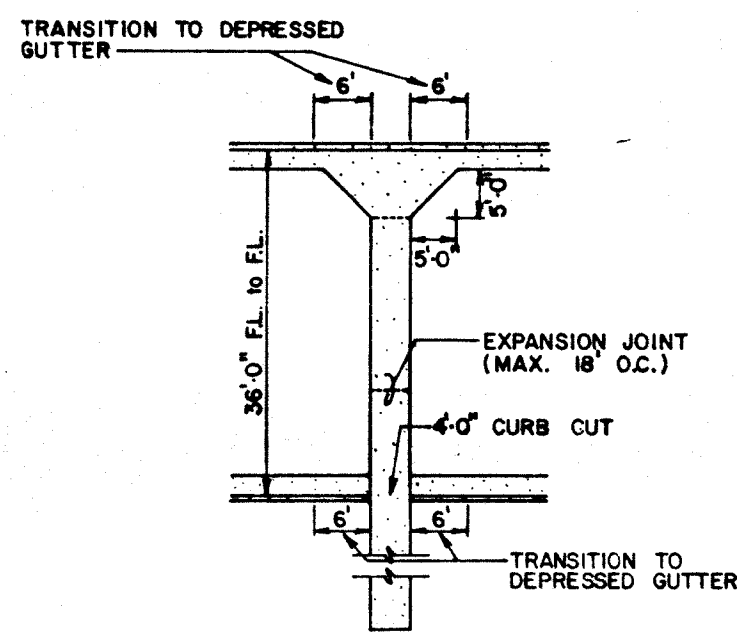
Thomas T. Mann, Jr. Date
NNPE & LS No. 3792



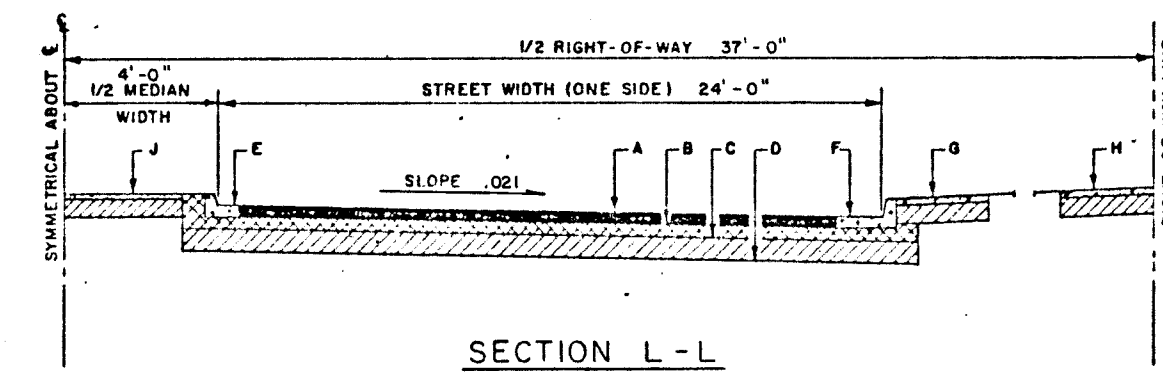
GRADING & DRAINAGE PLAN KEYSTONE PARK SOUTH



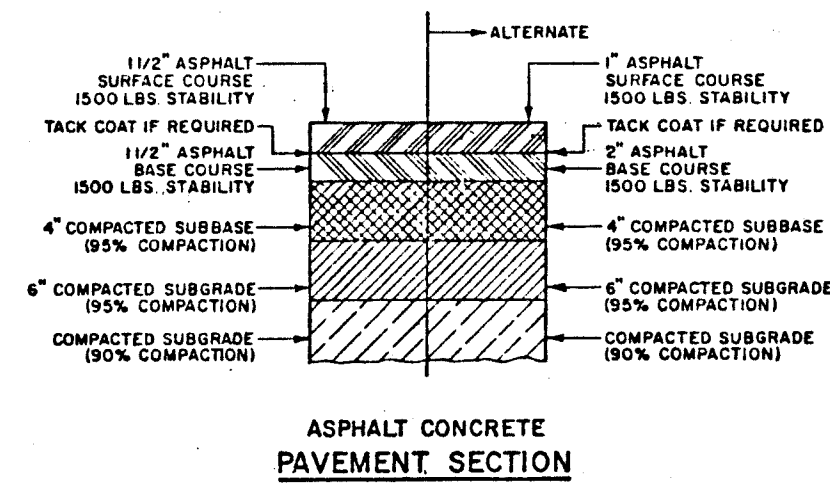
DETAIL SCALE: 1" = 20'



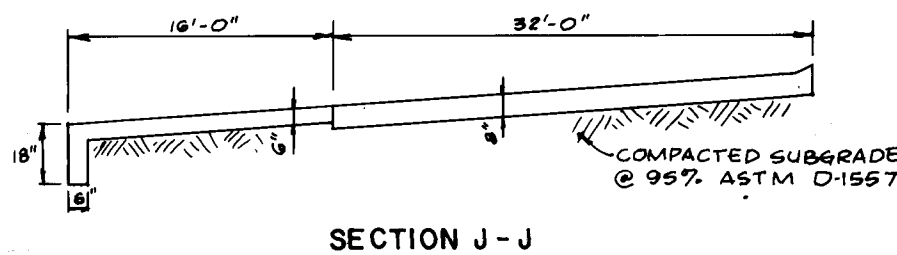
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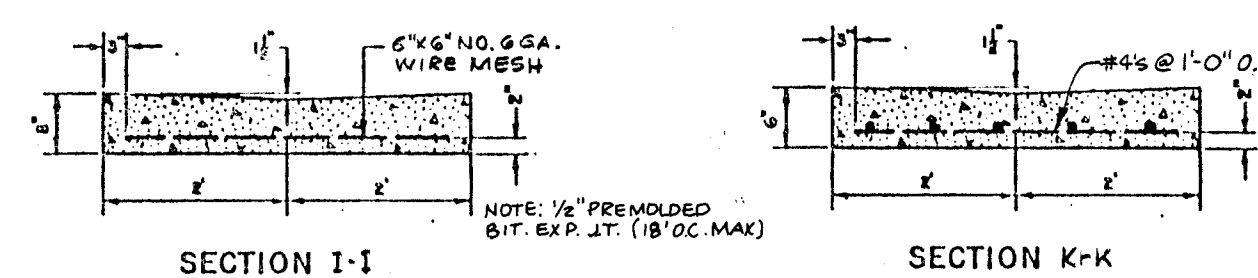
SECTION L-L



ASPHALT CONCRETE PAVEMENT SECTION

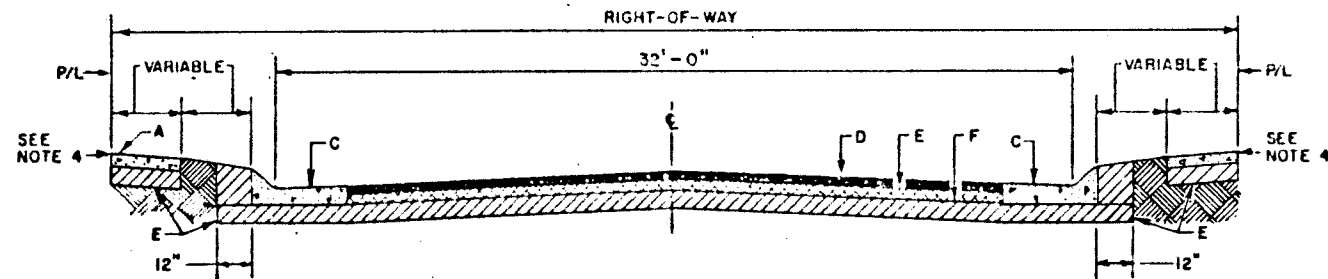


SECTION J-J

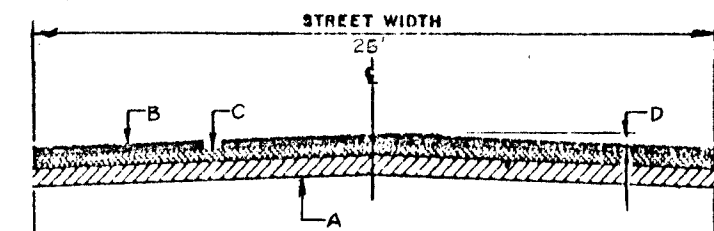


SECTION I-I

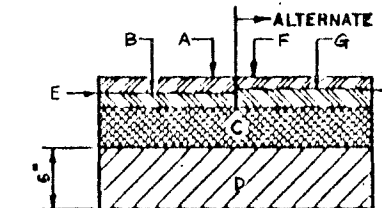
SECTION K-K



SECTION G-G



SECTION M-M

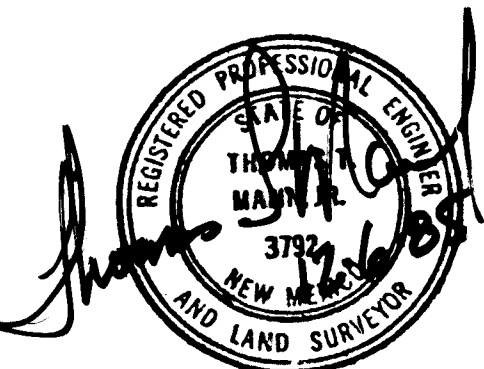


PRIVATE ACCESS EASEMENT

- GENERAL NOTES:**
1. ALL SUBGRADE COMPACTION FOR C & G WILL EXTEND 12" MIN. ON EITHER SIDE OF C & G OR CURB SECTION.
 2. SUBGRADE PREPARATION UNDER SIDEWALK AND DRIVEWAYS SHALL BE INCLUDED WITH THE PARTICULAR ITEM.
 3. FINISH GRADE AT PROPERTY LINE SHALL BE 0.67' MIN. ABOVE TOP OF CURB.
 4. SLOPE EASEMENT REQUIREMENTS WILL BE SHOWN ON PROJECT CONSTRUCTION PLANS.
 5. TRANSVERSE SLOPE FOR PAVEMENT SHALL BE 2% TYPICAL.
- CONSTRUCTION NOTES:**
- A. ASPHALT CONC. PAVEMENT.
 - B. A.T.B. OR C.T.B.
 - C. COMPACTED SUBBASE.
 - D. COMPACTED SUBGRADE.
 - E. C & G. MOUNTABLE MEDIAN.
 - F. C & G. MOUNTABLE ROLL TYPE.
 - G. SIDEWALK ADJACENT TO CURB NON-STANDARD, VARIANCE REQUIRED.
 - H. SIDEWALK AT STANDARD SETBACK.
 - I. MEDIAN. TO BE LANDSCAPED.
 - J. 1/2" EXPANSION JOINT MATERIAL.

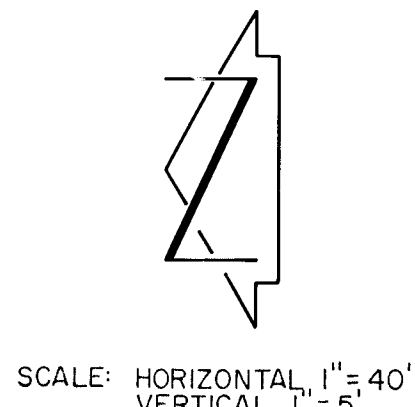
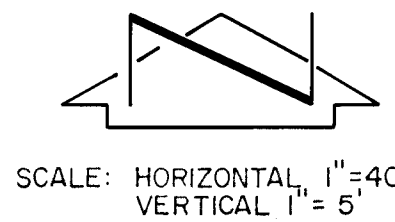
- GENERAL NOTES:**
1. CROWN ON STREETS SHALL BE AS FOLLOWS:
a. 32' STREET x 4"
b. 40' STREET x 5"
c. LESS THAN 32' STREET PAVEMENT SLOPE + 2%.
 2. ALL SUBGRADE COMPACTION FOR C & G WILL EXTEND 12" MIN. ON EITHER SIDE OF C & G OR CURB SECTION.
 3. SUBGRADE PREPARATION UNDER SIDEWALK AND DRIVEWAYS SHALL BE INCLUDED WITH THE PARTICULAR ITEM.
 4. FINISH GRADE AT PROPERTY LINE SHALL BE 0.67' MIN. ABOVE TOP OF CURB.
- CONSTRUCTION NOTES:**
- A. SIDEWALK AT STANDARD SETBACK.
 - C. MOUNTABLE ROLL C & G.
 - D. A.C. OR P.C. PAVEMENT.
 - E. 4" COMPACTED SUBBASE.
 - F. 6" COMPACTED SUBGRADE.

- GENERAL NOTES:**
1. REDUCE CROWN TO FLAT SECTION AT INTERSECTIONS ONLY WHERE REQUIRED FOR DRAINAGE.
 2. REDUCE NORMAL CROWN TO HALF CROWN AT THROUGH INTERSECTIONS WHERE DRAINAGE ACROSS ROADWAY IS NOT REQUIRED.
 3. CARRY NORMAL CROWN THROUGH "T" INTERSECTIONS WHERE DRAINAGE ACROSS ROADWAY IS NOT REQUIRED.
- CONSTRUCTION NOTES:**
- A. 6" COMPACTED SUBGRADE.
 - B. ASPHALT CONCRETE.
 - C. COMPACTED SUBBASE.
 - D. CROWN (4" TYPICAL MEASURED FROM EDGE OF PAVEMENT).
- GENERAL NOTES:**
1. FOR TYPICAL STREET SECTIONS CROWN IS MEASURED FROM LIP OF CURB TO 4' OF STREET OR FROM LIP OF CURB TO LIP OF CURB ON STREETS WITH MEDIANS. FOR NON-TYPICAL STREET SECTIONS, MAINTAIN A MINIMUM 2% CROSS SLOPE FROM HIGH POINT TO EDGE OF PAVEMENT EXCEPT IN NO CROWN SECTIONS AND TRANSITIONS TO NO CROWN SECTIONS (SEE DWG. P-11).
- CONSTRUCTION NOTES:**
- A. 1 1/2" SURFACE COURSE, 1500 LBS. STABILITY.
 - B. 1 1/2" BASE COURSE, 1500 LBS. STABILITY.
 - C. 4" COMPACTED SUBBASE.
 - D. COMPACTED SUBGRADE.
 - E. TACK COAT.
 - F. 1" SURFACE COURSE, 1500 LBS. STABILITY.
 - G. 2" BASE COURSE, 1500 LBS. STABILITY.



APPROVED FOR CONSTRUCTION
CITY ENGINEER DATE

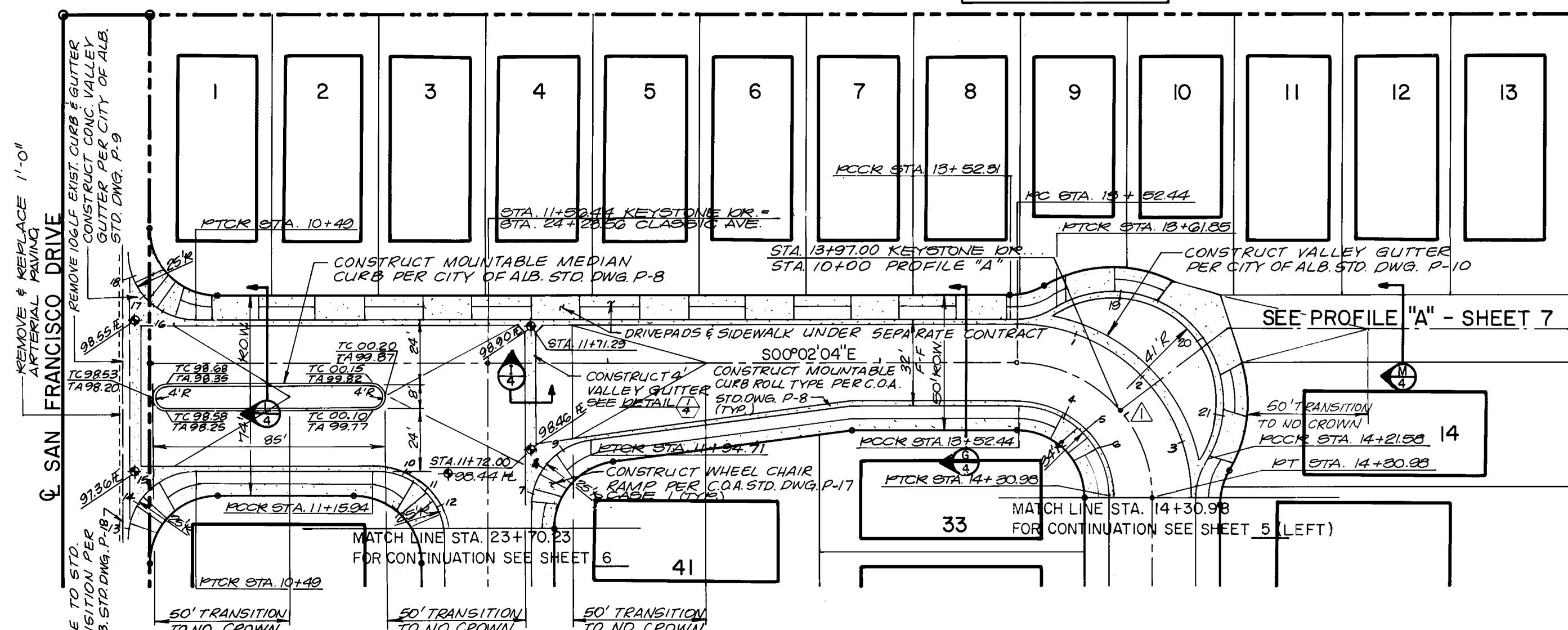
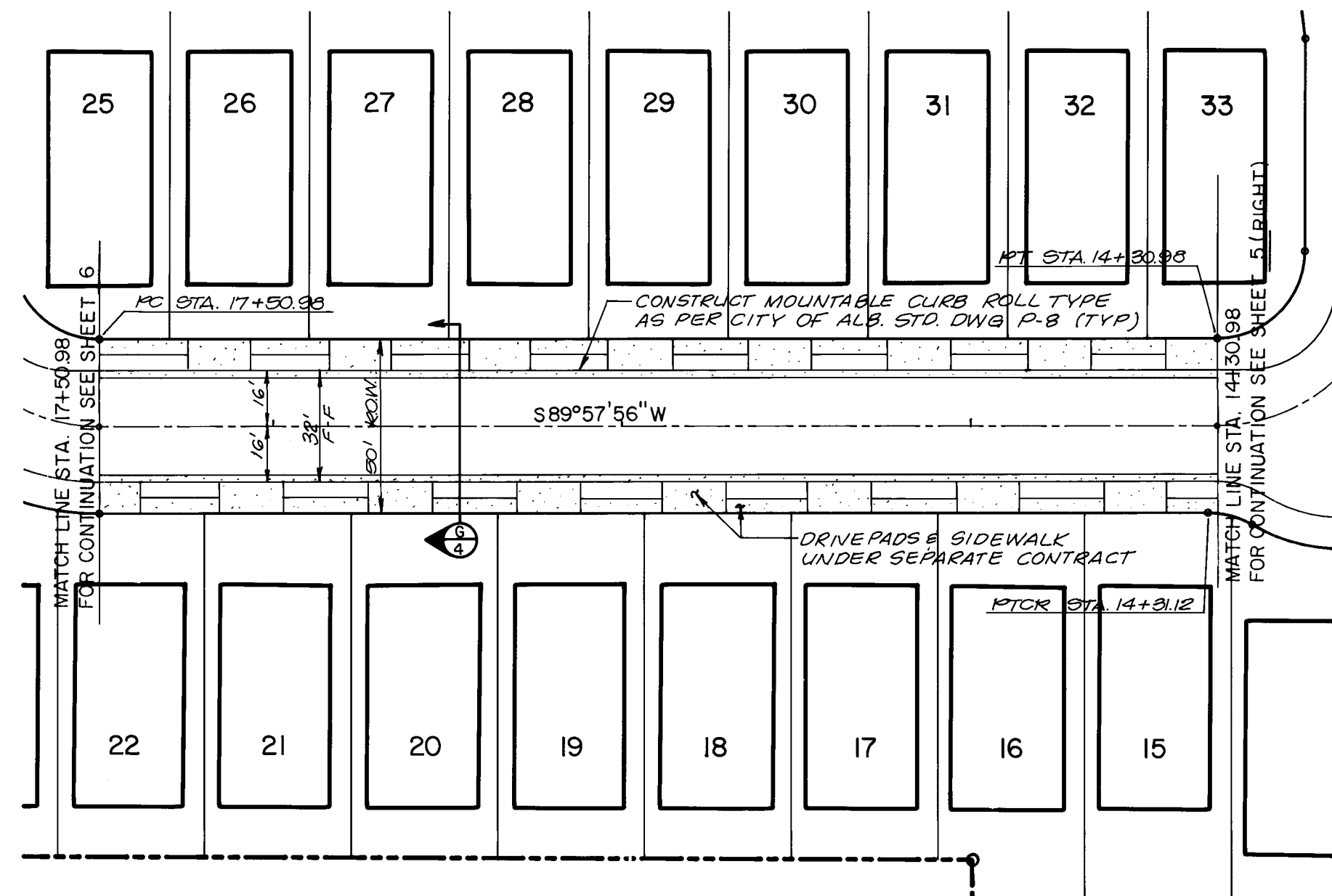
CITY OF ALBUQUERQUE MUNICIPAL DEVELOPMENT DEPARTMENT ENGINEERING DIVISION					
TITLE: KEYSTONE PARK SOUTH UTILITY SITE PLAN DETAILS					
APPROVALS	ENGINEER	DATE	APPROVALS	ENGINEER	DATE
City Engineer	<i>[Signature]</i>	12/23/05	Liquid Waste	<i>[Signature]</i>	12/26/05
A.C.E. - Design	<i>[Signature]</i>	12/10/05	Traffic	<i>[Signature]</i>	12/16/05
A.C.E. - Hydrology	<i>[Signature]</i>	12/10/05	Water	<i>[Signature]</i>	12/16/05
DRAWING NO.	2689	MAP NO.	D-19	SHEET	4 OF 8



SCALE: HORIZONTAL 1"=40'
VERTICAL 1"=5'

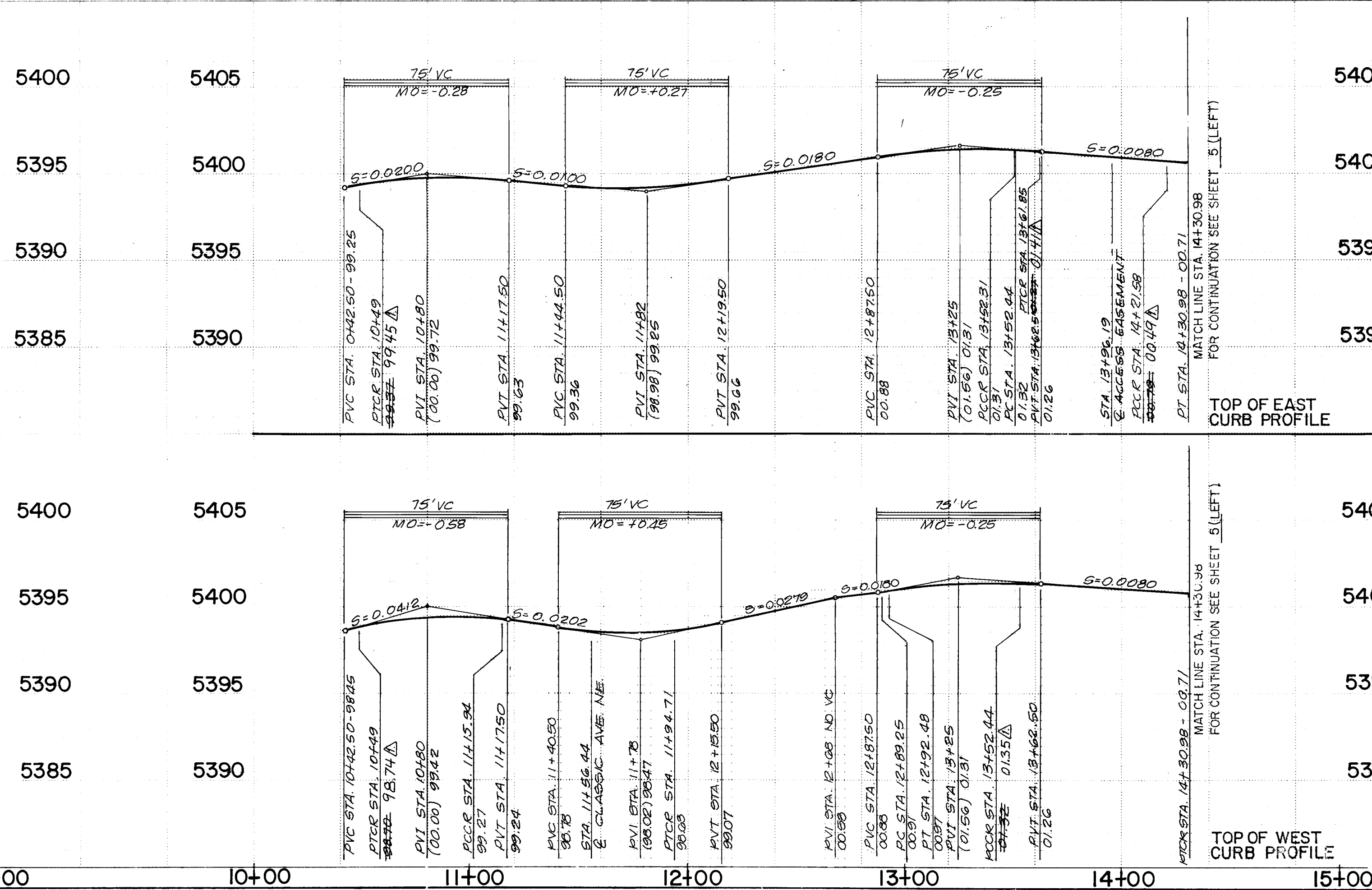
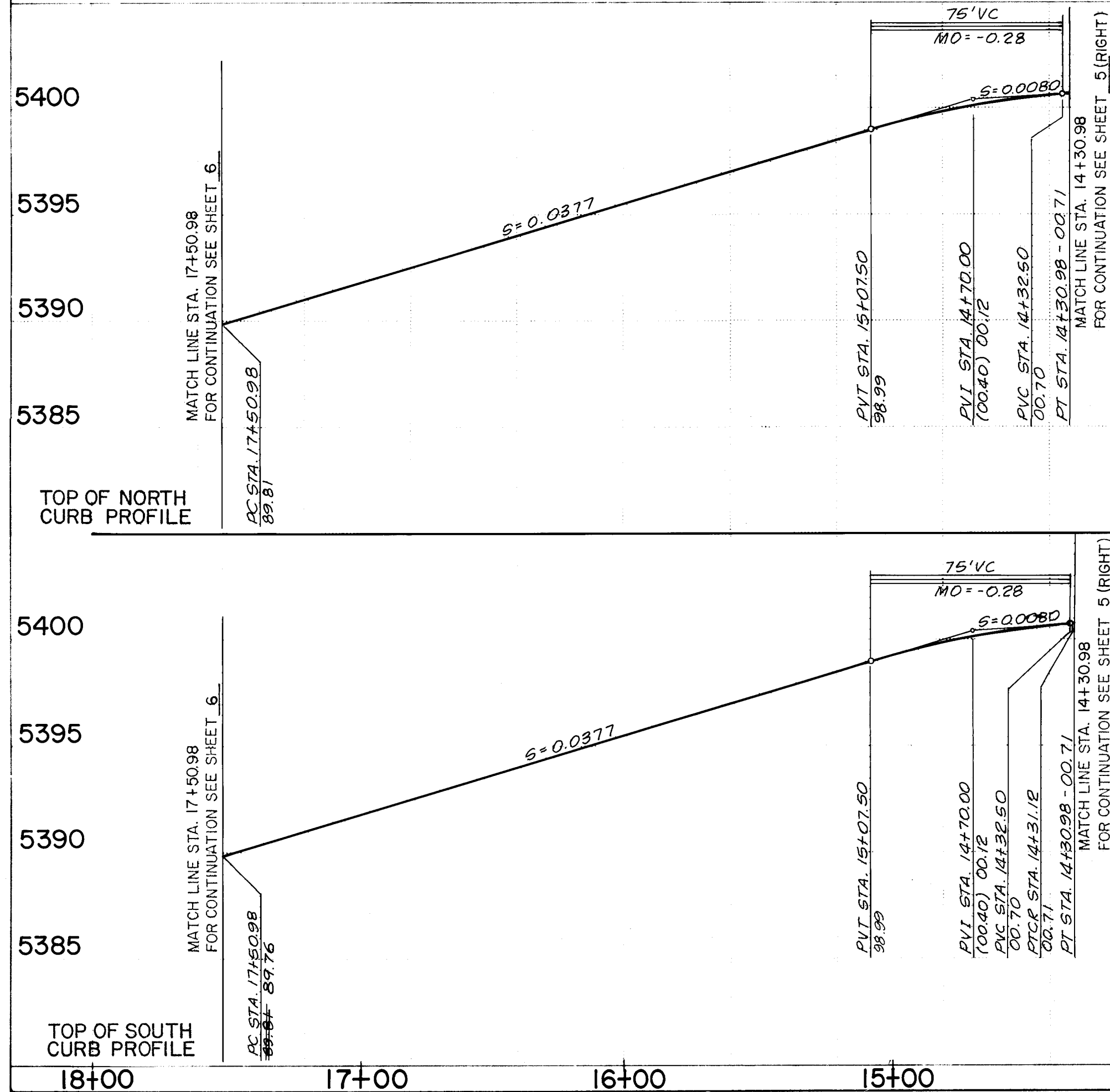
TC	FL
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2	00.66
3	00.52
4	01.17
5	00.95
6	00.87
7	98.38
8	98.46
9	98.55
10	99.07
11	98.87
12	98.67
13	97.81
14	98.11
15	98.41
16	99.58
17	99.76
18	99.95
19	01.56
20	01.39
21	01.21

CURVE DATA NO. 1
Δ = 90°00'00"
R = 50.00'
L = 78.54'



KEYSTONE DRIVE NE

KEYSTONE DRIVE NE



- STREET PAVING IMPROVEMENTS**
- ALL SLOPES AS SHOWN ON PROFILES ARE BASED ON CENTERLINE STATIONING.
 - WHERE REMOVAL OF EXISTING CURB & GUTTER, SIDEWALK OR PAVEMENT IS REQUIRED, THE CONTRACTOR SHALL SAWCUT AND/OR REMOVE TO THE NEAREST JOINT. CURB & GUTTER SHOWN AS EXISTING AND NOT TO BE REMOVED UNDER THIS CONTRACT WHICH IS DAMAGED OR DISPLACED BY THE CONTRACTOR AT THE CONTRACTOR'S EXPENSE.
 - ALL DIMENSIONS OF CURB & GUTTER RETURNS ARE SHOWN TO FACE OF CURB.

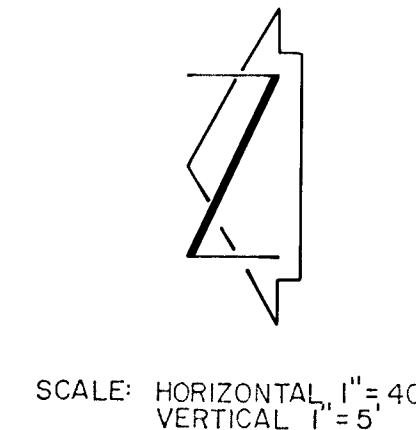
ENGINEER'S SEAL		SURVEY INFORMATION		BENCH MARKS		AS BUILT INFORMATION	
		FIELD NOTES		A STANDARD ACS BRASS TABLET STAMPED		CONTRACT INFORMATION	
		NO.	BY	DATE	NO.	BY	DATE
REVISIONS		REMARKS		REMARKS		REMARKS	
NO.		DATE		DATE		DATE	
DESIGNED BY		K.L.S.		DATE 12-85		DATE 12-85	
DRAWN BY		L.B.D.		DATE 12-85		DATE 12-85	
CHECKED BY		T.T.M.		DATE 12-85		DATE 12-85	

CITY OF ALBUQUERQUE
MUNICIPAL DEVELOPMENT DEPARTMENT
ENGINEERING DIVISION

TITLE: KEYSTONE PARK SOUTH
PAVING IMPROVEMENTS PLAN & PROFILE
STATION 10+00 — STATION 17+50.98

APPROVALS	ENGINEER	DATE	APPROVALS	ENGINEER	DATE
City Engineer	<i>[Signature]</i>	12/10/85	Liquid Waste	N/A	12/10/85
A.C.E. - Design	<i>[Signature]</i>	12/10/85	Traffic	S.A. Parker	12/10/85
A.C.E. - Hydrology	<i>[Signature]</i>	12/10/85	Water	N/A	12/10/85

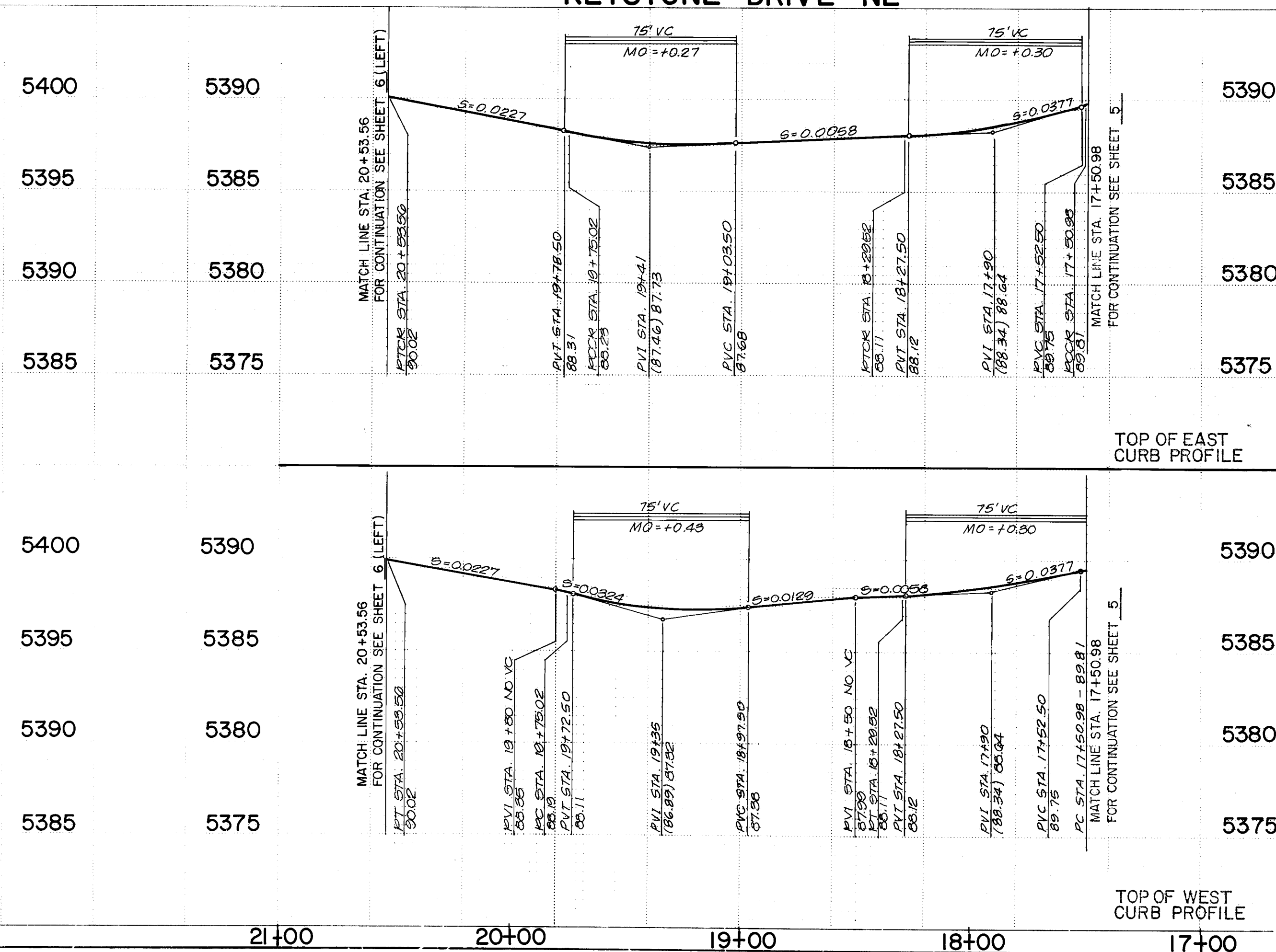
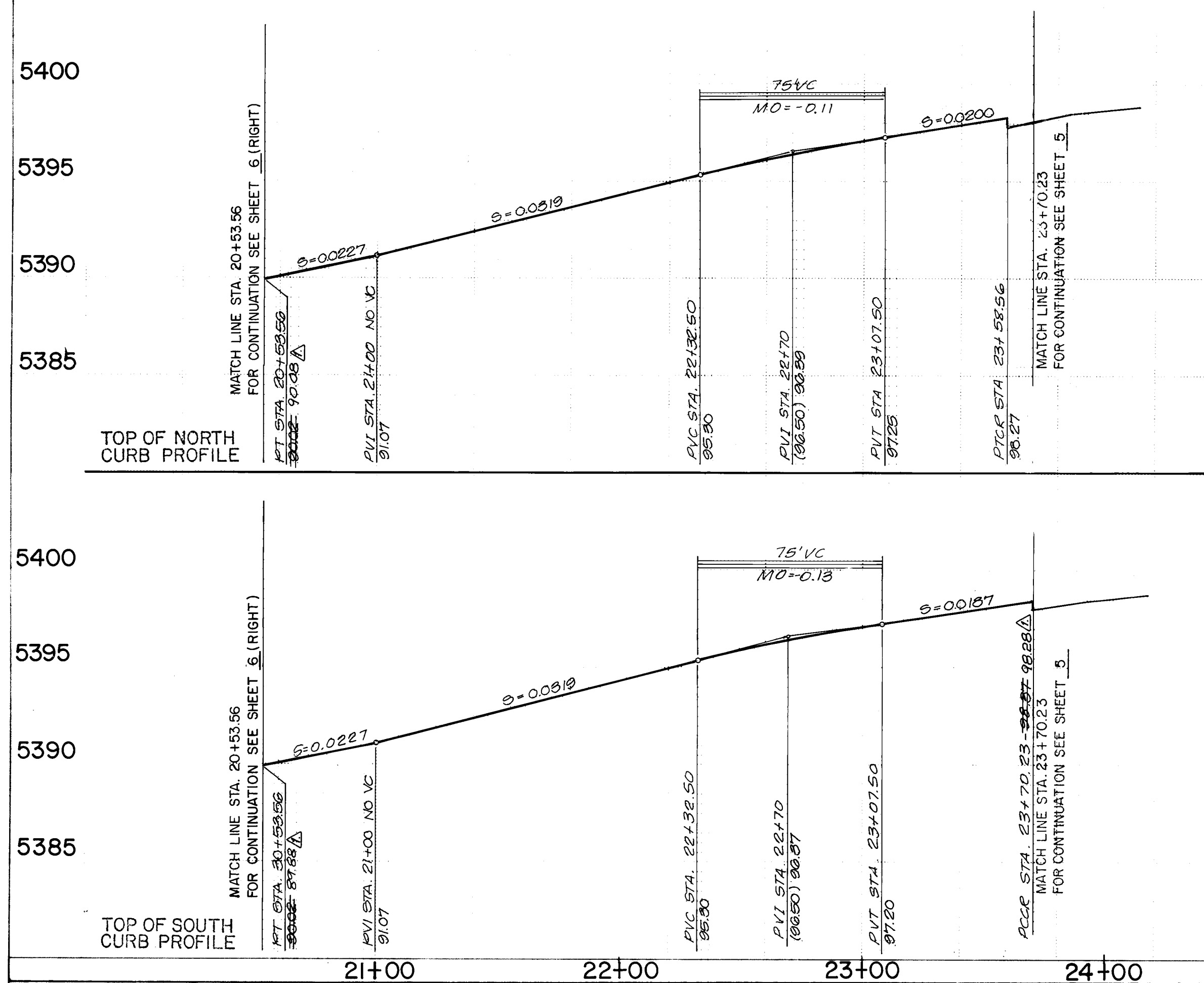
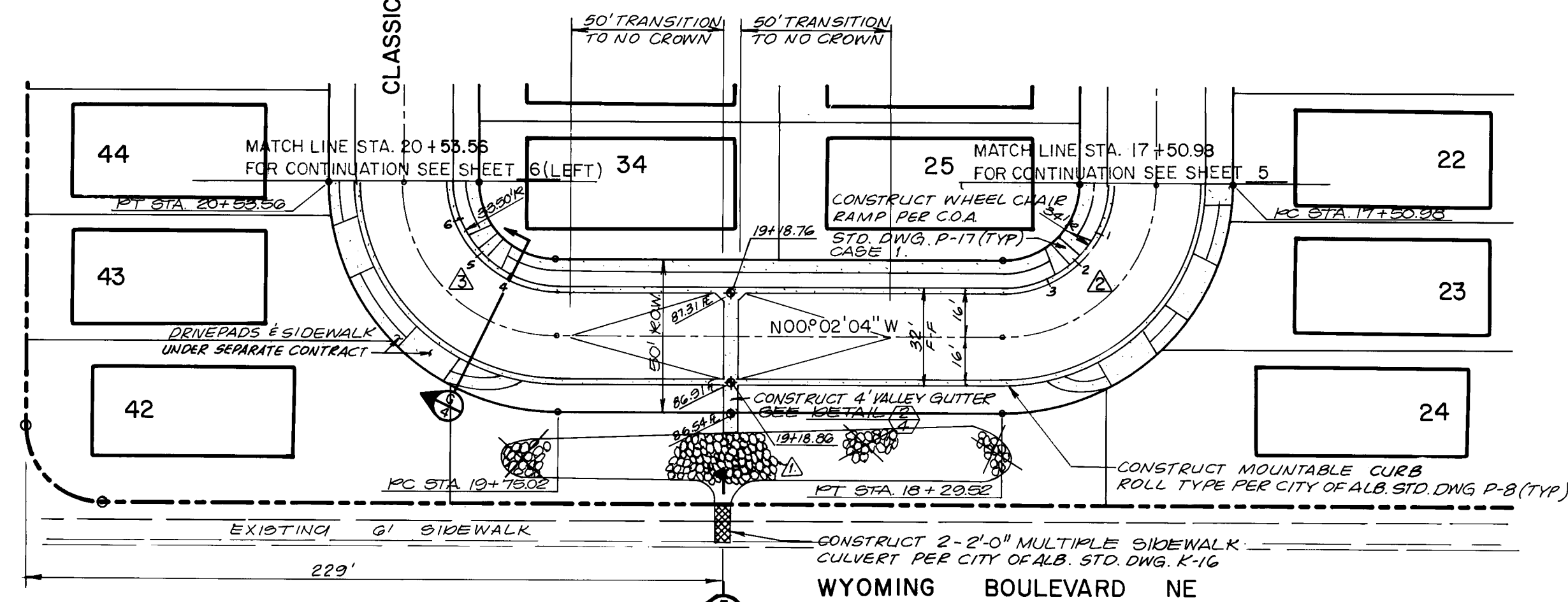
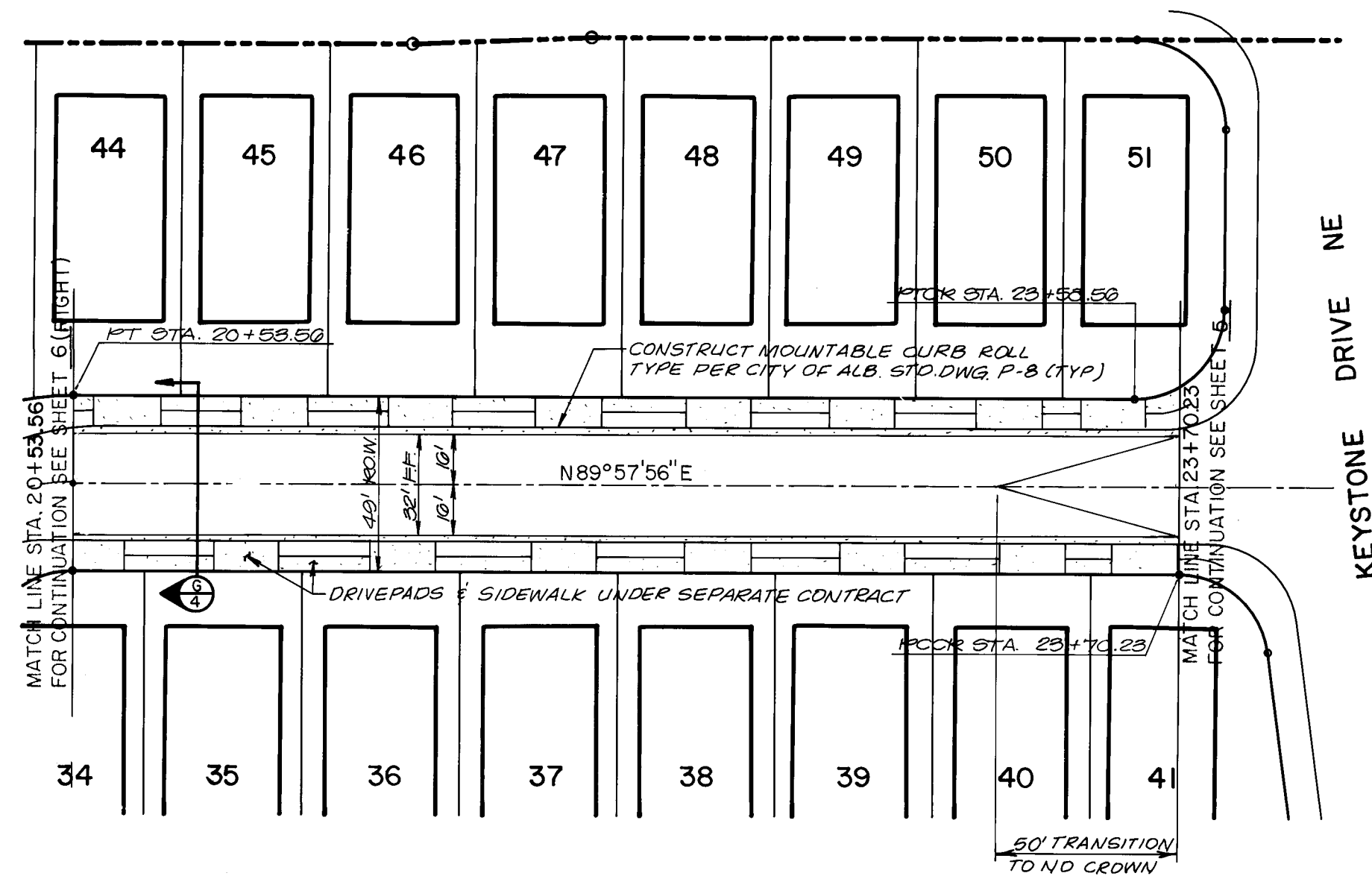
DRAWING NO. 2689 MAP NO. D-19 SHEET 5 OF 8



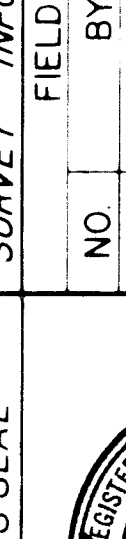
	TC	FL
1	89.14	88.81
2	88.63	88.30
3	88.29	87.96
4	88.68	88.35
5	89.12	88.79
6	89.57	89.24

℄ CURVE DATA NO. 
Δ = 90°00'00"
R = 50.00'
L = 78.54'

C CURVE DATA NO. 
$\Delta = 90^{\circ}00'00''$ $R = 50.00'$ $L = 78.54'$



1. ALL SLOPES AS SHOWN ON PROFILES ARE BASED ON CENTERLINE STATIONING.
2. WHERE REMOVAL OF EXISTING CURB & GUTTER, SIDEWALK OR PAVEMENT IS REQUIRED, THE CONTRACTOR SHALL SAWCUT AND/OR REMOVE TO THE NEAREST JOINT. CURB & GUTTER SHOWN AS EXISTING AND NOT TO BE REMOVED UNDER THIS CONTRACT WHICH IS DAMAGED OR DISPLACED BY THE CONTRACTOR AT THE CONTRACTOR'S EXPENSE.
3. ALL DIMENSIONS OF CURB & GUTTER RETURNS ARE SHOWN TO FACE OF CURB.

ENGINEER'S SEAL		SURVEY INFORMATION		BENCH MARKS		AS BUILT INFORMATION	
		FIELD NOTES		A STANDARD ACS BRASS TABLET STAMPED		CONTRACT INFORMATION	
		NO	BY	DATE	WORK	DATE	
NO.		DATE	REMARKS		INSPECTED BY		
10-08-85		Revised from as-built information		FIELD CHECKED BY L. VALDEZ			
				VERIFICATION BY			
				CORRECTED BY E.N.			
				DATE 12/08/85			
				MICRO-FILM INFORMATION			
				ELEVATION = 5403.54 (MISD)		RECORDED BY	
						NO	

CITY OF ALBUQUERQUE MUNICIPAL DEVELOPMENT DEPARTMENT ENGINEERING DIVISION					
TITLE: KEYSTONE PARK SOUTH PAVING IMPROVEMENTS PLAN & PROFILE STATION 17+50.98 — STATION 23+70.23					
APPROVAL'S	ENGINEER	DATE	APPROVAL'S	ENGINEER	DATE
City Engineer	<i>[Signature]</i>	12/15/85	Liquid Waste	N/A BR	12/16/85
A C E - Design	<i>[Signature]</i>	12/10/85	Traffic	D. A. Pankos	12/16/85
A C E -Hydrology	<i>[Signature]</i>	12/10/85	Water	N/A BR	12/16/85

DRAWING NO.	2689	MAP NO.	D-19	SHEET OF	6 8
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SCALE: HORIZONTAL 1"=40'
VERTICAL 1"=5'

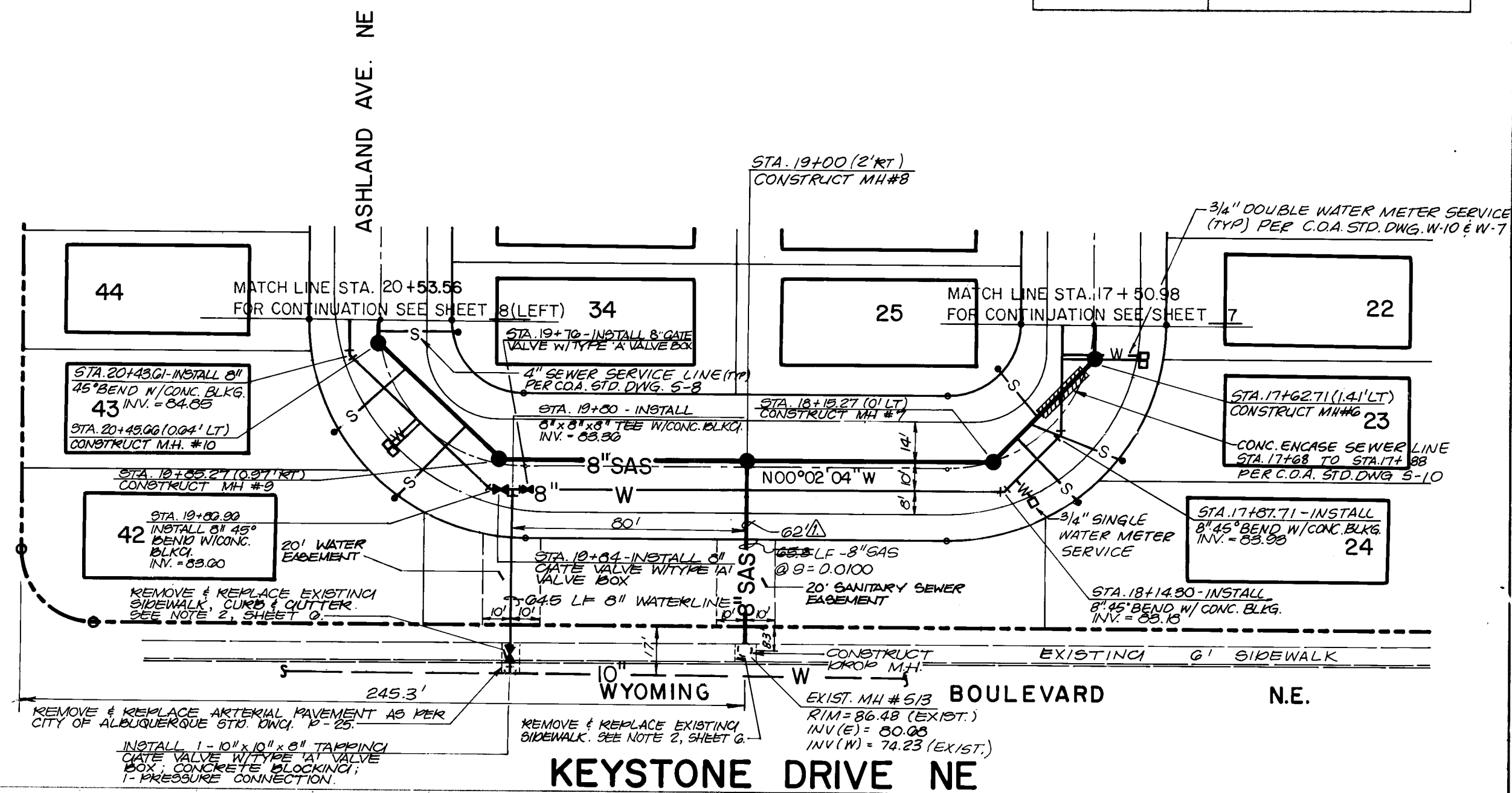
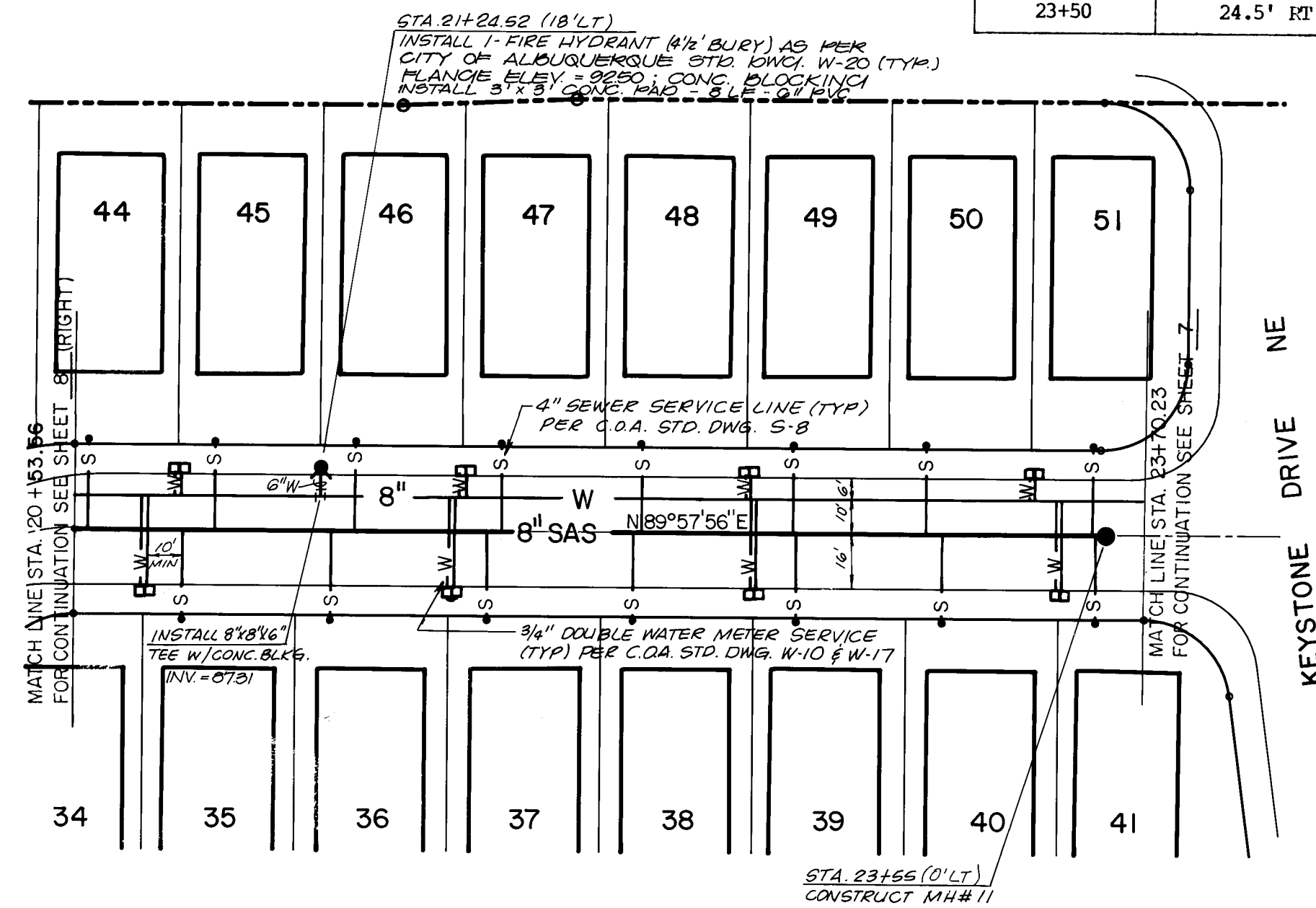
METERED SERVICE LINE STATIONING	
STATION	LOCATION
20+83.52	18' LT OF CENTERLINE
20+73.56	18' RT OF CENTERLINE
21+61.56	18' RT OF CENTERLINE
21+65.52	18' LT OF CENTERLINE
22+47.52	18' LT OF CENTERLINE
22+49.56	18' RT OF CENTERLINE
23+29.52	18' LT OF CENTERLINE
23+37.56	18' RT OF CENTERLINE

4" SANITARY SEWER SERVICE STATIONING		
STATION	LOCATION	INVERT AT PROPERTY LINE
20+58	24.5' LT OF CENTERLINE	86.10
20+84	24.5' RT OF CENTERLINE	86.90
20+94	24.5' LT OF CENTERLINE	87.30
21+27	24.5' RT OF CENTERLINE	88.30
21+35	24.5' LT OF CENTERLINE	88.60
21+72	24.5' RT OF CENTERLINE	89.70
21+76	24.5' LT OF CENTERLINE	89.90
22+15	24.5' RT OF CENTERLINE	91.10
22+16	24.5' LT OF CENTERLINE	91.10
22+58	24.5' RT OF CENTERLINE	92.10
22+60	24.5' LT OF CENTERLINE	92.00
22+98	24.5' RT OF CENTERLINE	93.00
23+03	24.5' LT OF CENTERLINE	93.10
23+48	24.5' RT OF CENTERLINE	94.00
23+50	24.5' LT OF CENTERLINE	93.90

SCALE: HORIZONTAL 1"=40'
VERTICAL 1"=5'

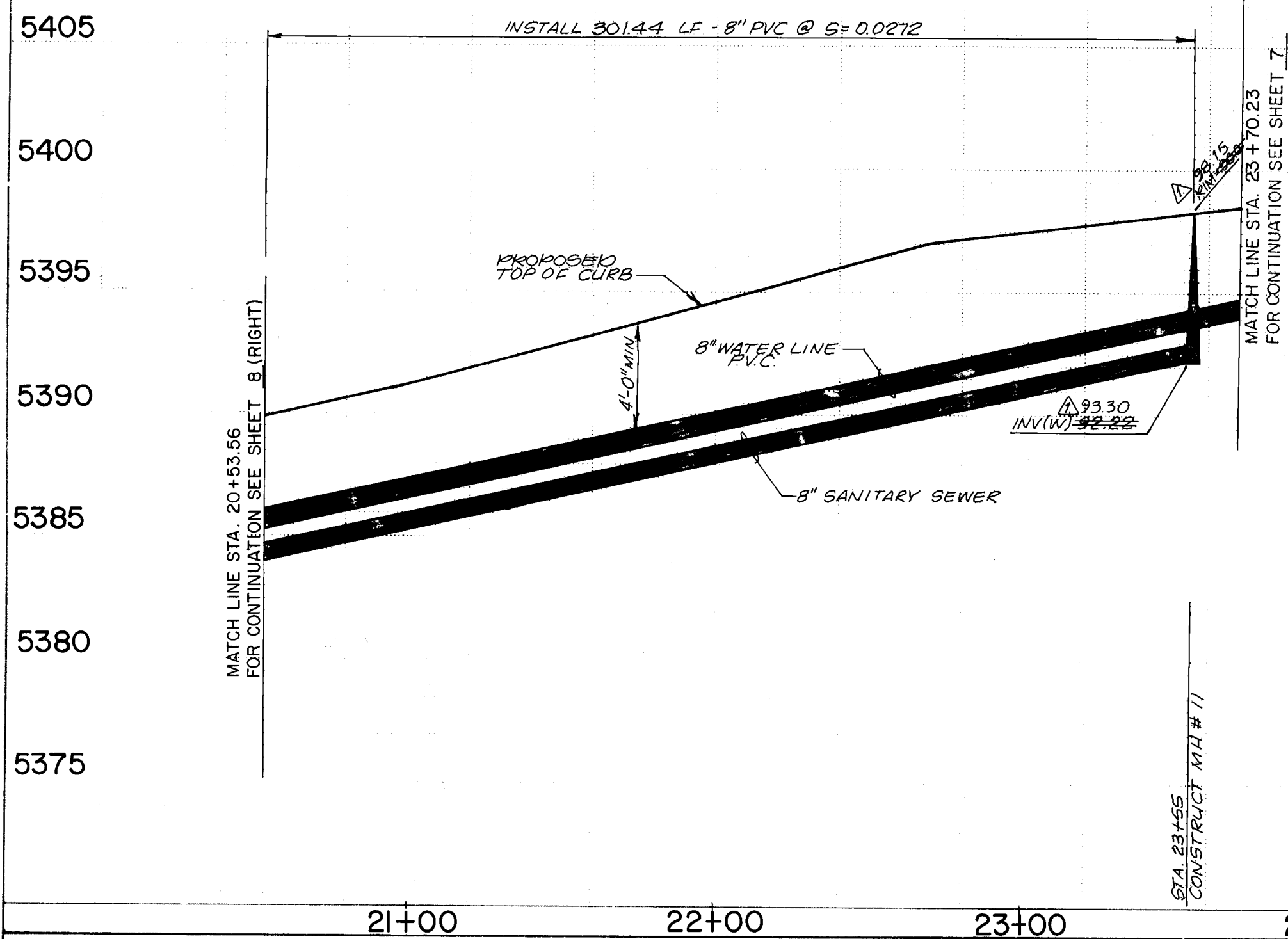
4" SANITARY SEWER SERVICE STATIONING		
STATION	LOCATION	INVERT AT PROPERTY LINE
17+90	25' RT OF CENTERLINE	84.60
17+92	25' LT OF CENTERLINE	84.60
18+00	25' LT OF CENTERLINE	84.20
20+07	25' LT OF CENTERLINE	84.90
20+32	25' LT OF CENTERLINE	85.50
20+50	25' RT OF CENTERLINE	85.90

METERED SERVICE LINE STATIONING	
STATION	LOCATION
17+59.02	18' LT OF CENTERLINE
20+14.98	18' LT OF CENTERLINE

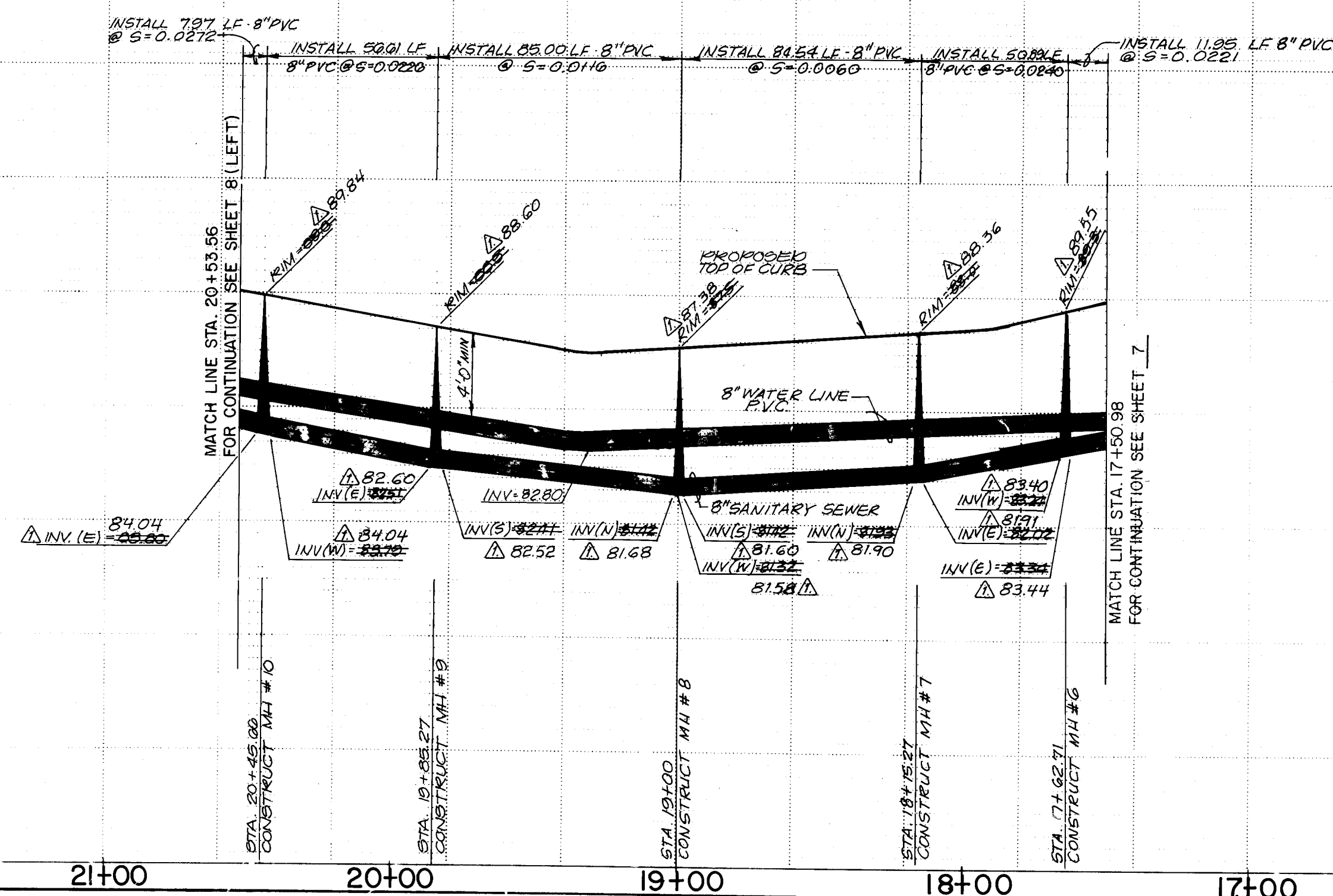


CLASSIC AVENUE NE

KEYSTONE DRIVE NE



5405	5400
5400	5395
5395	5390
5390	5385
5385	5380
5380	5375
5375	5370



SANITARY SEWER SYSTEM

- ALL STATIONING IS BASED ON STREET CENTERLINE UNLESS INDICATED OTHERWISE.
 - ALL SEWER PIPE SHALL BE P.V.C. (SDR 35).
 - SLOPES SHOWN ON THE PROFILES ARE BASED ON TRUE DISTANCES.
 - ALL MANHOLES SHALL BE 4-FOOT DIAMETER TYPE "E" AS PER CITY OF ALBUQUERQUE STANDARD DRAWING S-2 UNLESS OTHERWISE NOTED.
 - SEWER SERVICE LATERALS SHALL BE PROVIDED WITH ELECTRONIC MARKER DISCS AS PER CITY OF ALBUQUERQUE STANDARD DRAWING S-9.
- #### WATER DISTRIBUTION SYSTEM
- ALL STATIONING IS BASED ON STREET CENTERLINE.
 - WATER MAIN SHALL BE P.V.C. (C-900) OR AS APPROVED BY THE CITY OF ALBUQUERQUE WATER RESOURCES DIVISION.
 - WATER LINE SHALL HAVE A MINIMUM COVER OF 4'-0" (TOP OF CURB TO TOP OF PIPE).
 - WATER LINE SHALL BE LOCATED 10'-0" FROM AND PARALLEL TO STREET CENTERLINE UNLESS OTHERWISE NOTED.
 - CONTRACTOR SHALL FIELD VERIFY THE HORIZONTAL AND VERTICAL LOCATION OF EXISTING WATERLINES.

AS BUILT INFORMATION		BENCH MARKS		SURVEY INFORMATION		ENGINEER'S SEAL	
CONTRACT NO.	100-100000-000	DATE	12/10/85	NO.	1	REGISTERED PROFESSIONAL ENGINEER	STATE OF NEW MEXICO
INSPECTOR	W. L. VALDEZ	DATE	12/10/85	BY	W. L. VALDEZ	THOMAS T. HARRIS	372
FIELD NO.	100-100000-000	DATE	12/10/85	REMARKS	Revised from as built drawings	DESIGN	DATE 12-85
DATE	12/10/85	DATE	12/10/85	NO.	1	DESIGNED BY	K.L.S.
DATE	12/10/85	DATE	12/10/85	NO.	1	DRAWN BY	L.B.D.
DATE	12/10/85	DATE	12/10/85	NO.	1	CHECKED BY	T.T.M.

CITY OF ALBUQUERQUE MUNICIPAL DEVELOPMENT DEPARTMENT ENGINEERING DIVISION					
TITLE: KEYSTONE PARK SOUTH SANITARY SEWER & WATER LINE IMPROVEMENTS P&P STATION 17+50.98 - STATION 23+70.23					
APPROVALS	ENGINEER	DATE	APPROVALS	ENGINEER	DATE
City Engineer	W. L. Valdez	12/10/85	Liquid Waste	B. R. J.	12/10/85
A.C.E. - Design	W. L. Valdez	12/10/85	Traffic	B. R. J.	12/10/85
A.C.E. - Hydrology	W. L. Valdez	12/10/85	Water	B. R. J.	12/10/85
DRAWING NO.	2689	MAP NO.	D-19	SHEET	8 OF 8