· L E G E N D · MATERIALS CONCRETE LINES SUBDIVISION BOUNDARY PROPERTY LINE (PLAN) PROPERTY LINE (SECTION) CENTERLINE EASEMENT LINE MATCH LINE SECTION CUT LINE EARTHWORK CONTOUR LINE SPOT ELEVATION PROJECT / PHASE BOUNDARY DIRECTION OF FLOW MISCELLANEOUS UTILITIES GAS LINE UNDERGROUND TELEPHONE UNDERGROUND ELECTRICAL STORM DRAIN STORM DRAIN MANHOLE STORM DRAIN INLET SANITARY SEWER SANITARY SEWER LINE SANITARY SEWER MANHOLE SAS SERVICE CONNECTIONS SAS CAP OR PLUG **ENCASEMENT** WATER WATER LINE WATER SERVICE CONNECTIONS GATE VALVE FIRE HYDRANT BUTTERFLY VALVE REDUCER WATER PRESSURE ZONE BOUNDARY WATER FITTINGS CAPS AND PLUGS MISCELLANEOUS CHAINLINK FENCE FIELD FENCE COMMON YARD WALL RETAINING WALL POWER OR TELEPHONE POLE

CONSTRUCTION PLANS

PINNACLE VIEW ESTATES UNIT

ALBUQUERQUE, NEW MEXICO FEBRUARY, 1992

INDEX OF DRAWINGS

SHEET	DESCRIPTION					
1.	Cover Sheet, Vicinity Map, Le gend, General Notes and Index of Drawings					
2.	Final Plat					
3.	Grading Plan					
4.	Drainage Plan, Calculations, Grading Notes and Sections					
	ti all Dlam					

Composite Site Plan

Sections and Details

Traffic Control Details

PAVING IMPROVEMENTS PLAN AND PROFILE Narcisco Street N.E. & Pine Butte Road N.E. Blue Quail Road N.E. & Setter Drive N.E.

WATER AND SANITARY SEWER EXTENSIONS

Narcisco Street N.E. & Pine Butte Road N.E.

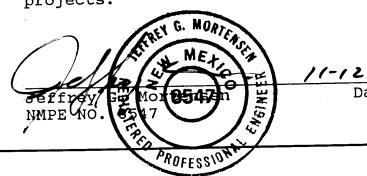
Blue Quail Road N.E. & Setter Drive N.E.

PUBLIC DRAINAGE

Drainage Channel Plan and Profile



I, Jeffrey G. Mortensen, Registered Professional Engineer in the State of New Mexico, do hereby certify that this "asbuilt" information was obtained by me or under my supervision and represents the "as-built" conditions of this project, and is true and correct to the best of my knowledge and belief. All vertical and horizontal dimensions should be field verified prior to use on future



VICINITY MAP

CONSTRUCTION COMMENCES. THE ENGINEER HAS CONDUCTED ONLY PRELIMINARY INVESTIGATION OF THE LOCATION, DEPTH, SIZE, OR TYPE OF EXISTING UTILITY LINES, PIPELINES, OR UNDERGROUND UTILITY LINES. THIS INVESTIGATION IS NOT CONCLUSIVE, AND MAY NOT BE COMPLETE, THEREFORE, MAKES NO REPRESENTATION PERTAINING THERETO, AND ASSUMES NO RESPONSIBILITY OR LIABILITY THEREFOR. THE DAMAGE CAUSED BY ITS FAILURE TO LOCATE, IDENTIFY AND PRESERVE ANY AND ALL EXISTING UTILITIES, PIPELINES, AND UNDERGROUND UTILITY LINES. IN PLANNING AND CONDUCTING EXCAVATION, THE CONTRACTOR SHALL COMPLY WITH STATE STATUTES, MUNICIPAL AND LOCAL ORDINANCES, RULES AND REGULATIONS, IF ANY, PERTAINING TO

1. ALL WORK DETAILED ON THESE PLANS TO BE PERFORMED UNDER CONTRACT SHALL, EXCEPT AS OTHERWISE STATED OR PROVIDED FOR HEREON, BE CONSTRUCTED IN ACCORDANCE WITH THE CITY OF ALBUQUERQUE STANDARD SPECIFICATIONS -PUBLIC WORKS CONSTRUCTION -

2. TWO (2) WORKING DAYS PRIOR TO ANY EXCAVATION, CONTRACTOR MUST CONTACT NEW MEXICO ONE CALL SYSTEM, 260-1990, FOR LOCATION OF EXISTING UTILITIES.

3. IF ANY UTILITY LINES, PIPELINES, OR UNDERGROUND UTILITY LINES ARE SHOWN ON THESE DRAWINGS, THEY ARE SHOWN IN AN APPROXIMATE MANNER ONLY, AND SUCH LINES

MAY EXIST WHERE NONE ARE SHOWN. IF ANY SUCH EXISTING LINES ARE SHOWN, THE

LOCATION IS BASED UPON INFORMATION PROVIDED BY THE OWNER OF SAID UTILITY, AND THE INFORMATION MAY BE INCOMPLETE, OR MAY BE OBSOLETE BY THE TIME

LOCATION

SHOULD A CONFLICT EXIST BETWEEN THESE PLANS AND ACTUAL FIELD CONDITIONS, THE CONTRACTOR SHALL PROMPTLY NOTIFY THE ENGINEER IN WRITING SO THAT THE CONFLICT CAN BE RESOLVED WITH A MINIMUM AMOUNT OF DELAY FOR ALL PARTIES.

5. THE CONTRACTOR SHALL MAINTAIN ACCESS TO ADJACENT PROPERTIES DURING

ALL WORK ON THIS PROJECT SHALL BE PERFORMED IN ACCORDANCE WITH APPLICABLE FEDERAL, STATE AND LOCAL LAWS, RULES AND REGULATIONS CONCERNING SAFETY AND

7. CONTRACTOR SHALL COMPLY WITH SECTION 19 OF THE "STANDARD SPECIFICATIONS".

8. ALL UTILITIES AND UTILITY SERVICE LINES SHALL BE INSTALLED PRIOR TO PAVING.

9. BACKFILL COMPACTION SHALL BE ACCORDING TO SPECIFIED STREET USE.

10. TACK COAT REQUIREMENTS SHALL BE DETERMINED DURING CONSTRUCTION BY THE PROJECT

11. SIDEWALKS AND WHEELCHAIR RAMPS WITHIN THE CURB RETURNS SHALL BE CONSTRUCTED WHEREVER A NEW CURB RETURN IS CONSTRUCTED.

12. IF CURB IS DEPRESSED FOR A DRIVEPAD OR A HANDICAP RAMP, THE DRIVEPAD OR RAMP SHALL BE CONSTRUCTED PRIOR TO ACCEPTANCE OF THE CURB AND GUTTER.

13. ALL STORM DRAINAGE FACILITIES SHALL BE COMPLETED PRIOR TO FINAL ACCEPTANCE.

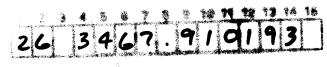
14. CONTRACTOR SHALL COORDINATE WITH THE WATER SYSTEM DIVISION FOR THE EXECUTION OF THE VALVE SHUT OFF PLAN, NOT LESS THAN THREE (3) WORKING DAYS IN ADVANCE OF ANY WORK THAT MAY AFFECT THE EXISTING PUBLIC WATER UTILITIES. ONLY WATER SYSTEM DIVISION PERSONNEL SHALL OPERATE EXISTING VALVES. REFER TO SECTION 18 OF THE SPECIFICATION.

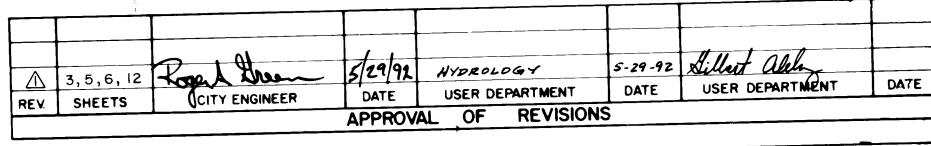
15. CONTRACTOR SHALL NOTIFY THE ENGINEER NOT LESS THAN SEVEN (7) DAYS PRIOR TO STARTING WORK IN ORDER THAT THE ENGINEER MAY TAKE NECESSARY MEASURES TO INSURE THE PRESERVATION OF SURVEY MONUMENTS. CONTRACTOR SHALL NOT DISTURB PERMANENT SURVEY MONUMENTS WITHOUT THE CONSENT OF THE ENGINEER AND SHALL NOTIFY THE ENGINEER AND BEAR THE EXPENSE OF REPLACING ANY THAT MAY BE DISTURBED WITHOUT PERMISSION. REPLACEMENT SHALL BE DONE ONLY BY THE ENGINEER. WHEN A CHANGE IS MADE IN THE FINISHED ELEVATION OF THE PAVEMENT OF ANY ROADWAY IN WHICH A PERMANENT SURVEY MONUMENT IS LOCATED, CONTRACTOR SHALL, AT HIS OWN EXPENSE, ADJUST THE MONUMENT COVER TO THE NEW GRADE UNLESS OTHERWISE SPECIFIED. REFER TO SECTION 4.4 OF THE SPECIFICATIONS.

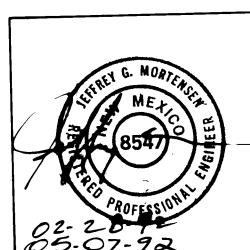
16. THREE (3) WORKING DAYS PRIOR TO BEGINNING CONSTRUCTION THE CONTRACTOR SHALL SUBMIT TO THE CONSTRUCTION CO-ORDINATION DIVISION A DETAILED CONSTRUCTION SCHEDULE. TWO (2) WORKING DAYS PRIOR TO CONSTRUCTION THE CONTRACTOR SHALL OBTAIN A BARRICADING PERMIT FROM THE CONSTRUCTION CO-ORDINATION DIVISION. CONTRACTOR SHALL NOTIFY BARRICADE ENGINEER (768-2551) PRIOR TO OCCUPYING AN INTERSECTION. CONTRACTOR MUST REFER TO SECTION 19 OF THE STANDARD SPECIFICATION FOR TRAFFIC CONTROL.

17. ALL STREET STRIPING ALTERED OR DESTROYED SHALL BE REPLACED IN KIND BY CONTRACTOR TO LOCATION AND IN KIND AS EXISTING OR AS INDICATED BY THIS PLAN











JEFF MORTENSEN & ASSOCIATES, INC

3467.91

SHEET I OF 12

6-23-92 C.E.

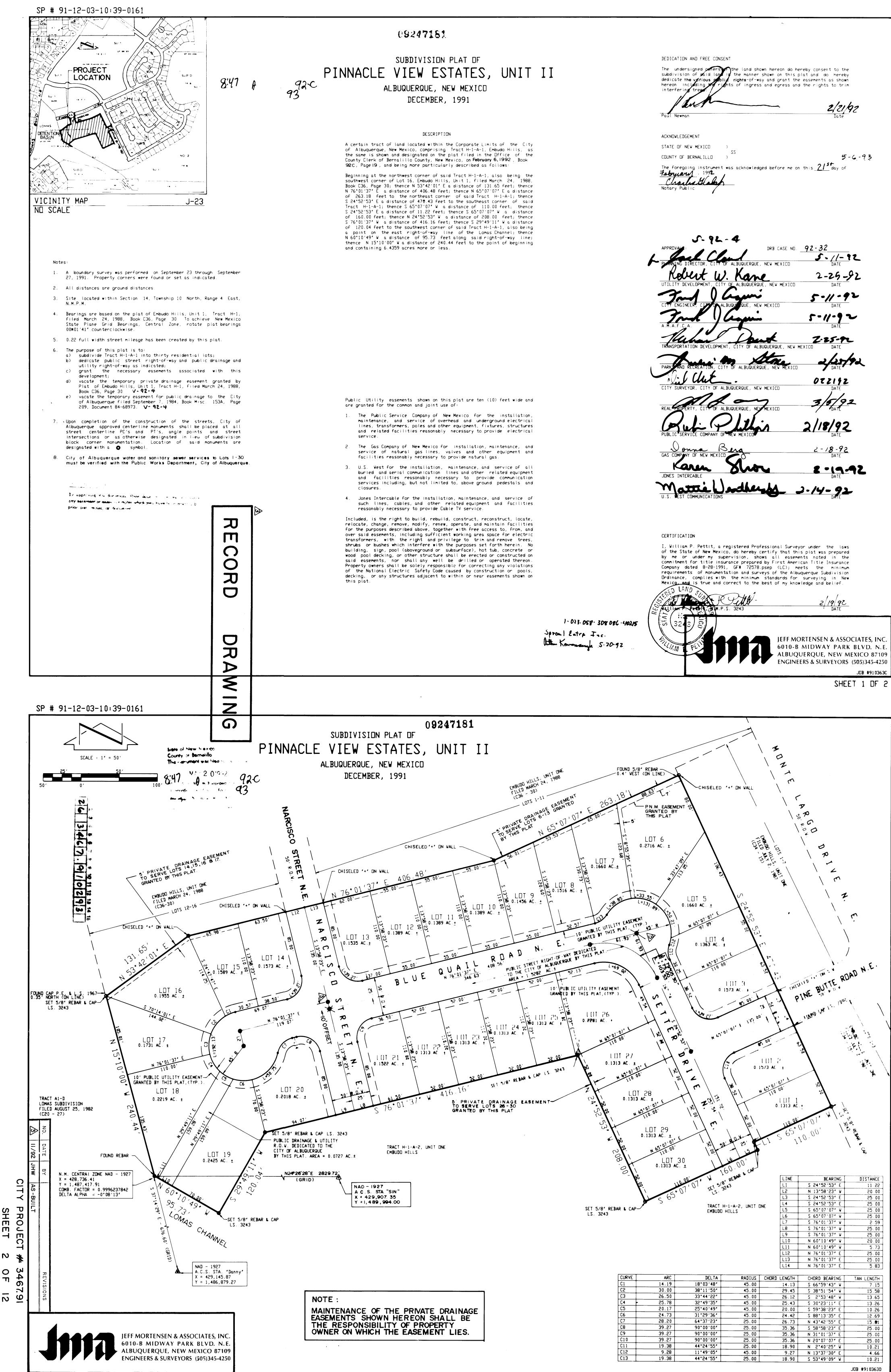
APPROVED FOR

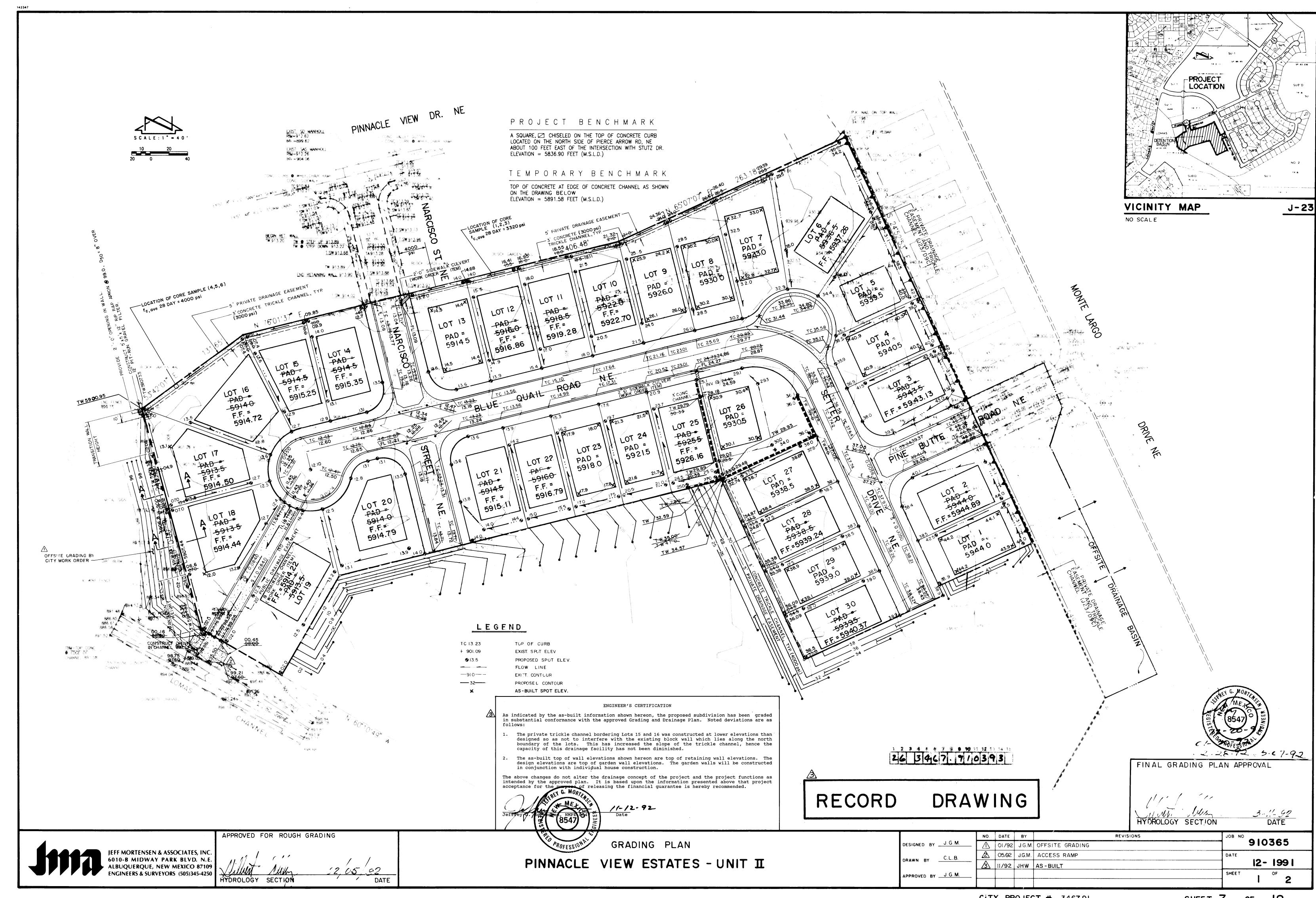
CONSTRUCTION

DRAWING RECORD

CAUTION:

THESE DRAWINGS DO NOT INCLUDE NECESSARY COMPONENTS FOR CONSTRUCTION SAFETY WHICH SHALL REMAIN THE RESPONSIBILITY OF THE CONTRACTOR.





DRAINAGE PLAN

The following items concerning the Pinnacle View Estates Unit II Grading and Drainage Plan are contained hereon:

- 1. Vicinity Map
- 2. Grading Plan 3. Calculations
- 4. Grading Notes 5. Typical Sections

As shown by the Vicinity Map, the site is located within the Embudo Hills Area, immediately east of the Lomas Detention Basin. The site is bounded on the north by Pinnacle View Estates-Unit I, on the east by existing residential development, and the south by the Upper Lomas Channel. The Upper Lomas Channel drains from southeast to northwest and discharges to the Lomas Detention Basin which, as stated above, is situated to the west of the site.

As shown by Panel 31 of 50 of the National Flood Insurance Program Flood Insurance Rate Maps for the City of Albuquerque, New Mexico, dated October 13, 1984, this site does not lie within a designated flood hazard zone. This site does lie adjacent to areas of designated flooding characterized by constructed drainage improvements. These areas are the Lomas Detention Basin and the Upper Lomas Channel. According to the above referenced mapping, 100-year flows are confined to these facilities.

The Grading Plan shows 1) existing and proposed grades indicated by spot elevations and contours at 1'0" intervals, 2) the limit and character of the existing improvements, 3) the limit and character of the proposed improvements, and 4) continuity between existing and proposed grades. As shown by this plan, the site presently slopes from east to west toward the Lomas Detention Basin. Minor offsite flows are received from Pine Butte Road and a portion of seven residential lots which discharge to Pine Butte Road. This offsite contributing area is shown on the preceding Grading Plan. The runoff generated by this small offsite area is quantified in the calculations hereon. These offsite flows will be accepted and conveyed through the site within the developed roadway system. The streets will drain in a westerly direction to the Blue Quail Road N.E. cul-de-sac. Runoff will exit the cul-de-sac by a public concrete drainage channel. This channel will discharge to the Upper Lomas Channel just upstream of its discharge to the Lomas Detention Basin. The calculations shown hereon indicate that the streets and the channel will both have sufficient capacity to convey the 100-year developed runoff calculated for this site and the offsite basin previously identified.

The discharge of developed runoff from the lots to the public streets is encouraged. In several instances, however, the rear yard portion of the lots will need to drain to a private drainage easement. The private drainage easement will lie along the rear lot line of the lots. A private drainage facility designated as a trickle channel will be constructed within this easement. The trickle channel will accept the runoff generated by the rear yard portion of the site and will convey that runoff to public right-of-way where it will discharge through a sidewalk culvert. The need for this cross-lot drainage in the rear yard areas is necessary so that the rear yards do not require additional filling, thereby creating an adverse grade situation with adjacent residential lots. The majority of the adjoining lots which could be impacted are already developed residentially. This decision has been made in view of the recent awareness and sensitivity to grade separations at property lines.

The calculations which appear hereon analyze both the existing and developed conditions for the 100-year, 6-hour rainfall event. The Rational Method has been used to calculate the peak rate of discharge while the SCS Method has been used to quantify the volume of runoff generated. Both Methods have been used in accordance with the City of Albuquerque Development Process Manual, Volume II, coupled with the Mayor's Emergency Rule dated October 14, 1986. As shown by these calculations, the development of this site will increase the amounts of runoff generated within the project. The design flow for the hydraulic computations has been based upon the 100-year developed peak discharge plus the 100-year offsite flow, as calculated. The hydraulic computations demonstrate that the streets will have capacity to convey this design flow. For the purpose of analysis, the minimum slope has been utilized for calculation. The public drainage channel hydraulics have been analyzed for capacity using the Manning Equation for Open Channel Flow and by the Weir Equation for the channel entrance conditions. Both sets of calculations indicate that the structure will have adequate capacity to receive and convey the runoff generated by this site and the designated offsite basin. Lastly, the capacity of the private trickle channel has been analyzed. The private trickle channel, under its minimum slope conditions of 1%, will have a capacity of slightly greater than 8 cfs. The maximum number of lots discharging to the trickle channel is 8. This represents approximately 27% of the site. By proportion, that portion of the site which could discharge through this channel, in a worst case scenario, would be approximately 6 cfs, thereby demonstrating that the trickle channel will have adequate capacity to handle the runoff it will receive. The runoff received by the trickle channel will be conveyed to a point of discharge to public right-of-way. From that point, the runoff will be carried within the streets to the previously mentioned public drainage channel. The outfall for this site is the Upper Lomas Channel.

The lots shown on the grading plan will result from the subdivision of Tract H-1-A-1, Unit 1, Embudo Hills DRB-91-321. The property to the south of these lots shall be designated as Tract H-1-A-2, Unit 1910/ / FER GRADING HE Embudo Hills. The development of Tract H-1-A-1 will constitute the first 30 lots of an overall development which is shown by Amended Site Development Plan. The Amended Site Development Plan will reduce the total number of lots collectively for the two tracts. Each tract will discharge its runoff to the Lomas Channel. The development of Tract H-1-A-2 shall be required to discharge its runoff directly to the channel and shall not discharge its runoff via Narcisco Street to Blue Quail Road. The discharge of runoff from Tract H-1-A-2 to Blue Quail Road would overtax the capacity of the public drainage channel entrance conditions. Because of this, a separate channel will be required to drain the latter development. The further subdivision of Tract H-1-A-2 will require a separate submittal to more thoroughly address the hydrologic and hydraulic characteristics of that development.

RECORD

DRAWING

JMA #910365

HYDROLOGIC CALCULATIONS Ground Cover Information

From SCS Bernalillo County Soil Survey, Plate 32: Te - Tesajo-Millett Stony Sandy Loams Hydrologic Soil Group: A Existing Pervious CN = 54 (DPM Plate 22.2 C-3

Pasture or Range Land: fair condition) Developed Pervious CN = 39 (DPM Plate 22.2 C-3 Open Space: good condition)

Time of Concentration/Time to Peak

 $T_C = 0.0078 L^{0.77}/S^{0.385}$ (Kirpich Equation)

 $T_D = T_C = 10 \text{ min.}$

Point Rainfall

 $P_6 = 2.55$ in. (DPM Plate 22.2 D-1)

Rational Method

Discharge: Q = CiA

where C varies $i = P_6 (6.84) T_C^{-0.51} = 5.39 in/hr$ $P_6 = 2.55 \text{ in (DPM Plate 22.2D-1)}$ $T_C = 10 \text{ min (minimum)}$ A = area, acres

SCS Method

Volume: V = 3630(DRO) A

Where DRO = Direct runoff in inches A = area. acres

Existing Condition

 $A_{total} = 278,800 \text{ sf} = 6.4 \text{ Ac}$ Undeveloped area = 278,800 sf (1.0)C = 0.40 (Weighted average per Emergency Rule, 1/14/86) $Q_{100} = CiA = 0.40(5.39)(6.4) = 13.8 cfs$ % impervious = -0-% Composite CN = 54 (DPM Plate 22.2 C-3) DRO = 0.10 in (DPM Plate 22.2 C-4) $V_{100} = 3630 \text{ (DRO)A} = 2300 \text{ cf}$

Developed Condition

 $A_{total} = 278,800 \text{ sf} = 6.4 \text{ Ac}$ Roof area = 60,000 sf (0.22)Paved area = 105,200 sf (0.38)Landscaped area = 113,600 sf (0.40)C = 0.66 (Weighted average per Emergency Rule, 1/14/86) $Q_{100} = CiA = 0.66(5.39)(6.4) = 22.8 cfs$ % impervious = 60 % Composite CN = 74 (DPM Plate 22.2 C-3) DRO = 0.65 in (DPM Plate 22.2 C-4) $V_{100} = 3630 \text{ (DRO)A} = 15,100 \text{ cf}$

<u>Offsite Basin</u>

 $A_{total} = 22,600 \text{ sf} = 0.52 \text{ Ac}$ Roof area = 9200 sf (0.41)Paved area = 4400 sf (0.19)Landscaped area = 9000 sf (0.40)C = 0.65 (Weighted average per Emergency Rule, 1/14/86) $Q_{100} = CiA = 0.65(5.39)(0.52) = 1.8 cfs$

 $\Delta Q_{100} = 22.8 - 13.8 = 9 \text{ cfs (increase)}$

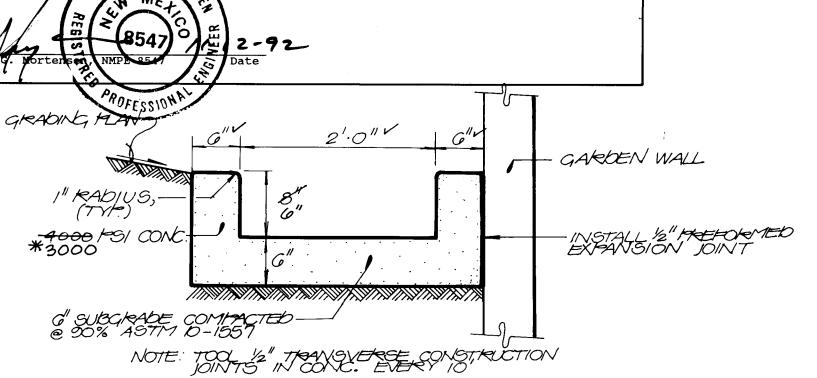
$\Delta V_{100} = 15,100 - 2300 = 12,800 \text{ cf (increase)}$ ENGINEER'S CERTIFICATION

in substantial conformance with the approved Grading and Drainage Plan. Noted deviations are as The private trickle channel bordering Lots 15 and 16 was constructed at lower elevations than designed so as not to interfere with the existing block wall which lies along the north boundary of the lots. This has increased the slope of the trickle channel, hence the

As indicated by the as-built information shown hereon, the proposed subdivision has been graded

capacity of this drainage facility has not been diminished. The as-built top of wall elevations shown hereon are top of retaining wall elevations. The design elevations are top of garden wall elevations. The garden walls will be constructed

rainage concept of the project and the project functions as is based upon the information presented above that project the financial guarantee is hereby recommended.



HYDRAULIC COMPUTATIONS

1. DESIGN FLOW

 $Q_{Design} = Q_{100} + Q_{Offsite} = 24.6 \text{ cfs} \frac{25}{2} \text{ cfs}$

2. STREET CAPACITY

a. Minimum Slope (0.0050)

Using DPM Plate 22.3 D-1 for S = 0.0050 and D = 0.70, $Q_{1/2} = 26$ cfs and v = 3.5 fps

 $Q_{capacity} = 2 \times Q_{1/2} = 52 \text{ cfs}$

 $Q_{Design} = 25 \text{ cfs}$ Then $Q_{1/2} = 12$ cfs, D = 0.52 feet, and v = 2.6 cfs

b. Maximum Slope (0.0800)

Using DPM Plate 22.3 D-1 for S = 0.0800 and D = 0.70, $Q_{1/2} > 70 \text{ cfs}$

 $Q_{Design} = 25 \text{ cfs},$ Then $Q_{1/2} = 12$ cfs, D = 0.35 feet, and v = 6.5 cfs

 $D \times V = 2.3 << 6.5$

3. CHANNEL HYDRAULICS

 $Q = (1.49/n)AR^2/3S^{1/2}$ (Manning Equation) Let n = 0.013 $A = w \times d = 19 \times 2 = 38 \text{ sf}$ w = 20 - 2(0.50) = 19d = 2 feet (minimum) R = A/p = 38/23 = 1.65 $R^{2/3} = 1.40$ S = 0.0840

$Q_{capacity} = 1768 \text{ cfs} >> Q_{100}$

 $Q_{100} = 25 \text{ cfs}$ d = 0.15 feet v = 9 fps

 $s^{1/2} = 0.29$

4. CHANNEL ENTRANCE HYDRAULICS

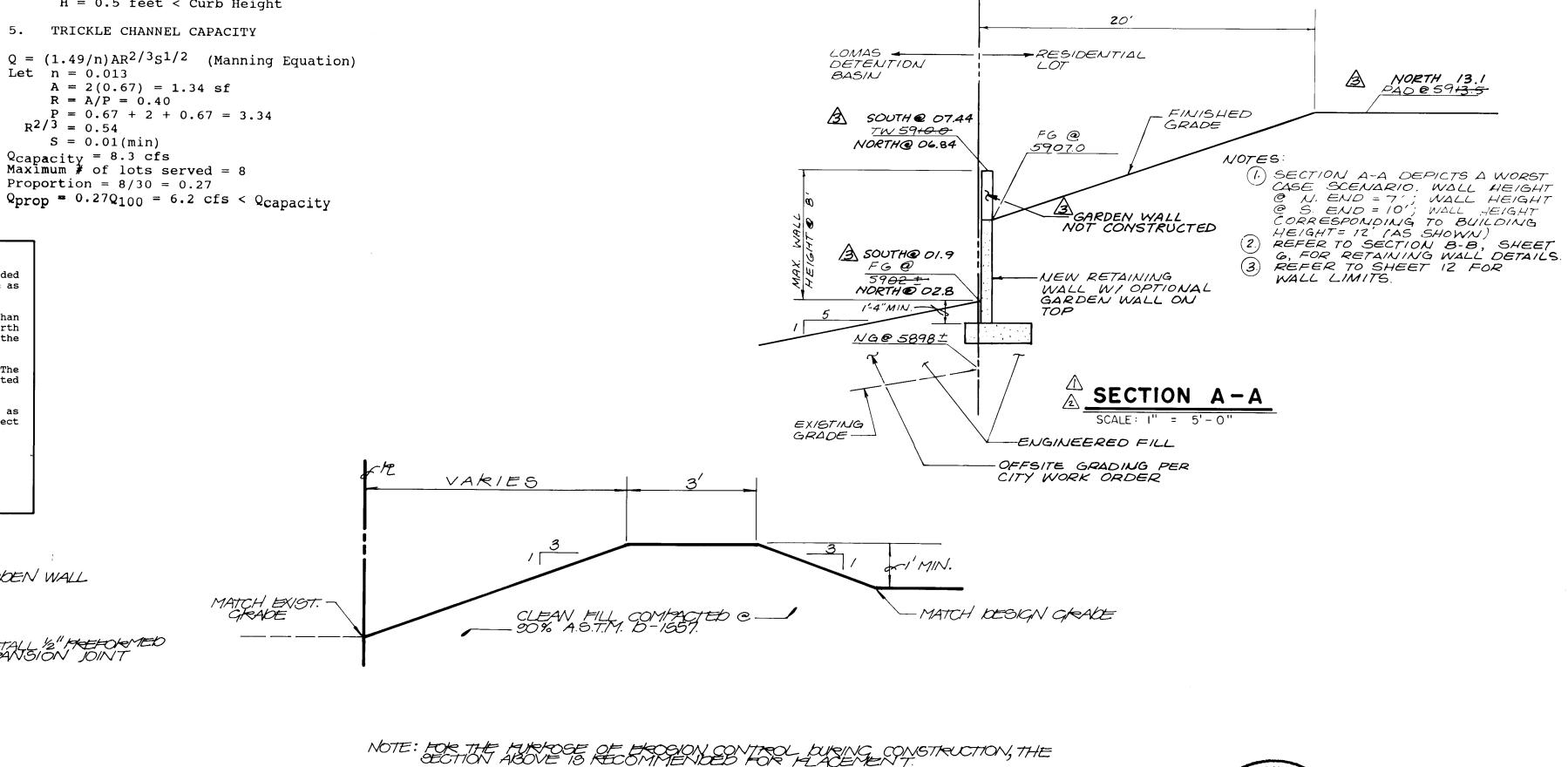
 $Q = 3.33LH^{3/2}$ (Weir Equation) Where L = 19H = 0.67 (curb height) Qcapacity = 35 cfs 0 Opesign = 25 cfs, $H^{3/2} = Q/(3.33L)$ $H^{3/2} = 23/(3.33)(19) = 0.40$ H = 0.5 feet < Curb Height

5. TRICKLE CHANNEL CAPACITY

 $Q = (1.49/n)AR^{2/3}S^{1/2}$ (Manning Equation) Let n = 0.013A = 2(0.67) = 1.34 sfR = A/P = 0.40P = 0.67 + 2 + 0.67 = 3.34 $R^{2/3} = 0.54$ S = 0.01(min)Qcapacity = 8.3 cfs
Maximum # of lots served = 8 Proportion = 8/30 = 0.27

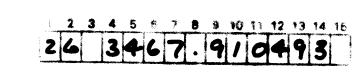
GRADING NOTES:

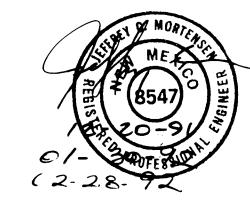
- 1. ALL FILL SHALL BE COMPACTED TO A MINIMUM OF 90% ASTM D-1557; HOUSE PADS SHALL BE COMPACTED AT 95%
- 2. THE PAD ELEVATIONS SHOWN HEREON ARE FOR ROUGH GRADING PURPOSES.
- FINISHED FLOOR ELEVATIONS MAY VARY FROM THE PAD ELEVATIONS AND WILL BE DETERMINED AS A FUNCTION OF INDIVIDUAL HOUSE DESIGN.
- FINISHED FLOOR ELEVATIONS SHOULD BE ESTABLISHED AT A MINIMUM OF 6 INCHES ABOVE PAD ELEVATIONS; DEVIATIONS FROM THESE GUIDELINES MUST BE BASED ON THE RECOMMENDATIONS AND/OR DESIGN OF A COMPETENT DESIGN
- NO CROSS-LOT DRAINAGE WILL BE ALLOWED UNLESS PROVIDED FOR BY PRIVATE DRAINAGE EASEMENT.
- RETAINING WALLS SHALL BE CONSTRUCTED BY THE DEVELOPER.
- YARD (GARDEN) WALLS SHALL BE CONSTRUCTED BY THE LOT OWNER OR ITS BUILDER.
- THE FINISHED GRADING OF EACH LOT SHALL BE ACCOMPLISHED BY THE LOT OWNER OR ITS BUILDER. RUNOFF SHOULD BE DIRECTED TO THE STREETS OR DRAINAGE EASEMENT (IF AVAILABLE).
- 9. MAXIMUM SLOPES SHALL BE 3:1; MINIMUM SLOPES SHALL BE 1%.
- 10. TWO (2) WORKING DAYS PRIOR TO ANY EXCAVATION, CONTRACTOR MUST CONTACT NEW MEXICO ONE CALL SYSTEM, 260-1990, FOR LOCATION OF EXISTING UTILITIES.
- 11. IF ANY UTILITY LINES, PIPELINES, OR UNDERGROUND UTILITY LINES ARE SHOWN ON THESE DRAWINGS, THEY ARE SHOWN IN AN APPROXIMATE MANNER ONLY, AND SUCH LINES MAY EXIST WHERE NONE ARE SHOWN. IF ANY SUCH EXISTING LINES ARE SHOWN, THE LOCATION IS BASED UPON INFORMATION PROVIDED BY THE OWNER OF SAID UTILITY, AND THE INFORMATION MAY BE INCOMPLETE, OR MAY BE OBSOLETE BY THE TIME CONSTRUCTION COMMENCES. THE ENGINEER HAS CONDUCTED ONLY PRELIMINARY INVESTIGATION OF THE LOCATION, DEPTH, SIZE, OR TYPE OF EXISTING UTILITY LINES, PIPELINES, OR UNDERGROUND UTILITY LINES. THIS INVESTIGATION IS NOT CONCLUSIVE, AND MAY NOT BE COMPLETE. THEREFORE, MAKES NO REPRESENTATION PERTAINING THERETO, AND ASSUMES NO RESPONSIBILITY OR LIABILITY THEREFOR. THE CONTRACTOR SHALL INFORM ITSELF OF THE LOCATION OF ANY UTILITY LINE, PIPELINE, OR UNDERGROUND UTILITY LINE IN OR NEAR THE AREA OF THE WORK IN ADVANCE OF AND DURING EXCAVATION WORK. THE CONTRACTOR IS FULLY RESPONSIBLE FOR ANY AND ALL DAMAGE CAUSED BY ITS FAILURE TO LOCATE, IDENTIFY AND PRESERVE ANY AND ALL EXISTING UTILITIES, PIPELINES, AND UNDERGROUND UTILITY LINES. IN PLANNING AND CONDUCTING EXCAVATION, THE CONTRACTOR SHALL COMPLY WITH STATE STATUTES, MUNICIPAL AND LOCAL ORDINANCES. RULES AND REGULATIONS, IF ANY, PERTAINING TO THE LOCATION OF THESE LINES AND FACILITIES.
- 12. THE CONTRACTOR SHALL ENSURE THAT NO SOIL ERODES FROM THE SITE INTO PUBLIC RIGHT-OF-WAY OR ONTO PRIVATE PROPERTY. THIS CAN BE ACHIEVED BY CONSTRUCTING TEMPORARY BERMS AT THE PROPERTY LINES AND WETTING THE SOIL TO KEEP IT FROM BLOWING.
- 13. THE CONTRACTOR SHALL PROMPTLY CLEAN UP ANY MATERIAL EXCAVATED WITHIN THE PUBLIC RIGHT-OF-WAY SO THAT THE EXCAVATED MATERIAL IS NOT SUSCEPTIBLE TO BEING WASHED DOWN THE STREET.
- riangle 14. The contractor shall secure "topsoil disturbance permit" prior to beginning construction. An excavation PERMIT IS REQUIRED FOR ALL WORK WITHIN PUBLIC RIGHT-OF-WAY.
- 15. CONTRACTOR SHALL CONTACT MR. GLENN JURGENSON, STORM DRAIN MAINTENANCE, C.O.A., 9805 CENTRAL AVENUE N.E., (505) 291-6214 PRIOR TO COMMENCEMENT AND FOLLOWING COMPLETION OF ALL GRADING WITHIN THE LOMAS DETENTION BASIN AND LOMAS CHANNEL RIGHT-OF-WAY.



A TYPICAL TRICKLE CHANNEL SECTION

* MOST OF THE CHANNEL WAS CONSTRUCTED W/ 3000 PSI CONCRETE; LOCATIONS OF CORE SAMPLES & TEST RESULTS SHOWN ON TYPICAL INTERIM EROSION CONTROL BERM SECTION







DRAINAGE PLAN, CALCULATIONS, GRADING NOTES & SECTION PINNACLE VIEW ESTATES - UNIT II

	NO.	DATE	BY	REVISIONS	JOB NO.			
SIGNED BYJ.G. M	\triangle	01/92	J.G.M.	OFFSITE GRADING	1	9103	365	
AWN BY J.M.C.	2	02/92	JGM	REVISE SECTION B-B	DATE			
AWN BY	<u>A</u>	11/92	JHW	AS - BUILT]	12-	2-1991	
PROVED BY J.G.M.					SHEET		OF	
						2	2	>

