MATERIALS CONCRETE RIP · RAP **LINES** SUBDIVISION BOUNDARY PROPERTY LINE (PLAN) PROPERTY LINE (SECTION) CENTERLINE EASEMENT LINE MATCH LINE SECTION CUT LINE **EARTHWORK** CONTOUR LINE SPOT ELEVATION PROJECT / PHASE BOUNDARY DIRECTION OF FLOW MISCELLANEOUS UTILITIES GAS LINE UNDERGROUND TELEPHONE UNDERGROUND ELECTRICAL STORM DRAIN STORM DRAIN MANHOLE STORM DRAIN INLET SANITARY SEWER SANITARY SEWER LINE SANITARY SEWER MANHOLE SAS SERVICE CONNECTIONS SAS CAP OR PLUG **ENCASEMENT** -===-WATER WATER LINE WATER SERVICE CONNECTIONS GATE VALVE FIRE HYDRANT **BUTTERFLY VALVE** REDUCER WATER PRESSURE ZONE BOUNDARY WATER FITTINGS CAPS AND PLUGS **ELBOW** CROSS TEE **MISCELLANEOUS** CHAINLINK FENCE FIELD FENCE COMMON YARD WALL RETAINING WALL POWER OR TELEPHONE POLE \Box

CONSTRUCTION PLANS FOR

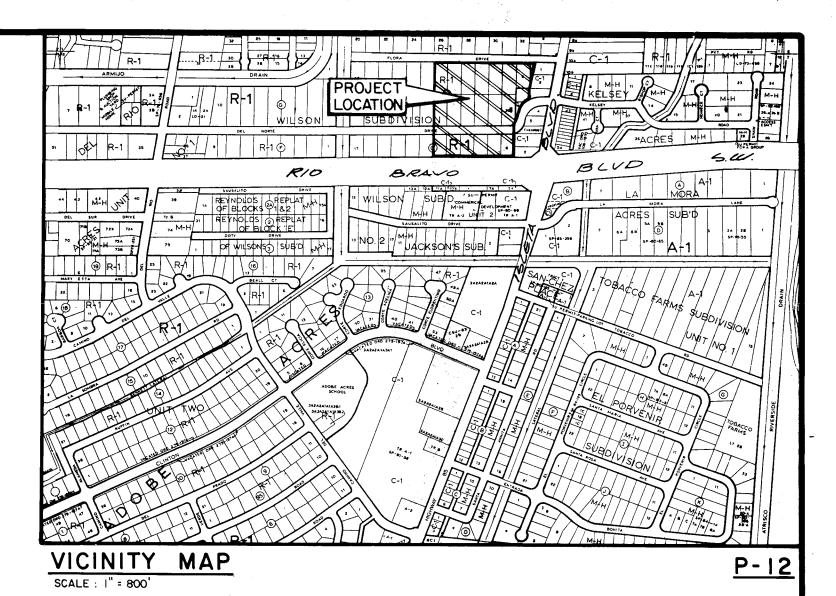
RIO BRAVO SHOPPING CENTER WATER & SANITARY SEWER LINE EXTENSIONS

BERNALILLO COUNTY, NEW MEXICO JULY, 1988

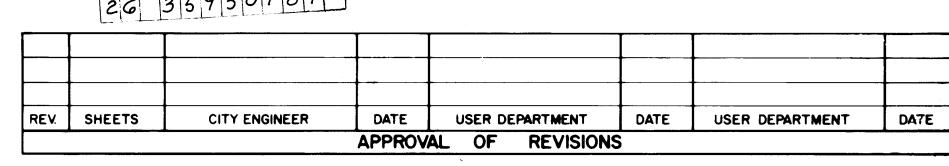
SHEET NO.	INDEX OF DRAWINGS
1.	COVER SHEET, INDEX OF DRAWINGS, VICINITY MAP, GENERAL NOTES, LEGEND
2.	FINAL PLAT
3 4.	GRADING AND DRAINAGE PLAN
5.	WATER AND SANITARY SEWER SITE PLAN AND BASE LINE PLAN
6.	LINE "A" SANITARY SEWER AND WATER LINE PLAN AND PROFILE
7.	LINE "B" WATER LINE PLAN AND PROFILE
8.	VALVE SHUT-OFF PLAN/TRAFFIC CONTROL DETAILS

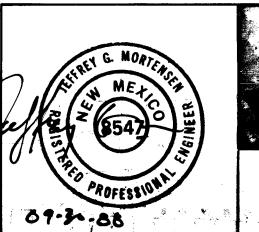
A RECORD DRAWING

I, Jeffrey G. Mortensen, Registered Professional Engineer in the State of New Mexico, do hereby certify that these drawings represent the "as-built" conditions of this project as determined from survey information provided by Stephen E. Walker, NMPS No. 6401, Walker Surveying Company. All vertical and horizontal dimensions should be field verified prior to use on future projects.



- 1. ALL WORK DETAILED ON THESE PLANS TO BE PERFORMED UNDER CONTRACT SHALL, EXCEPT AS OTHERWISE STATED OR PROVIDED FOR HEREON, BE CONSTRUCTED IN ACCORDANCE WITH THE CITY OF ALBUQUERQUE STANDARD SPECIFICATIONS - PUBLIC WORKS CONSTRUCTION - 1986. (STANDARD SPECIFICATIONS)
- 2. TWO (2) WORKING DAYS PRIOR TO ANY EXCAVATION, CONTRACTOR MUST CONTACT LINE LOCATING SERVICE, 765-1234, FOR LOCATION OF EXISTING UTILITIES.
- 3. IF ANY UTILITY LINES, PIPELINES, OR UNDERGROUND UTILITY LINES ARE SHOWN ON THESE DRAWINGS, THEY ARE SHOWN IN AN APPROXIMATE MANNER ONLY, AND SUCH LINES MAY EXIST WHERE NONE ARE SHOWN. IF ANY SUCH EXISTING LINES ARE SHOWN THE LOCATION IS BASED UPON INFORMATION PROVIDED BY THE OWNER OF SAID UTILITY, AND THE INFORMATION MAY BE INCOMPLETE, OR MAY BE OBSOLETE BY THE TIME CONSTRUCTION COMMENCES. THE ENGINEER HAS UNDERTAKEN NO FIELD VERIFICATION OF THE LOCATION, DEPTH, SIZE, OR TYPE OF EXISTING UTILITY LINES, PIPELINES, OR UNDERGROUND UTILITY LINES, MAKES NO REPRESENTATION PERTAINING THERETO, AND ASSUMES NO RESPONSIBILITY OR LIABILITY THEREFOR. THE CONTRACTOR SHALL INFORM ITSELF OF THE LOCATION OF ANY UTILITY LINE, PIPELINE, OR UNDERGROUND UTILITY LINE IN OR NEAR THE AREA OF THE WORK IN ADVANCE OF AND DURING EXCAVATION WORK. THE CONTRACTOR IS FULLY RESPONSIBLE FOR ANY AND ALL DAMAGE CAUSED BY ITS FAILURE TO LOCATE, IDENTIFY AND PRESERVE ANY AND ALL EXISTING UTILITIES, PIPELINES, AND UNDERGROUND UTILITY LINES. IN PLANNING AND CONDUCTING EXCAVATION, THE CONTRACTOR SHALL COMPLY WITH STATE STATUTES, MUNICIPAL AND LOCAL ORDINANCES, RULES AND REGULATIONS, IF ANY, PERTAINING TO THE LOCATION OF THESE LINES AND FACILITIES.
- 4. SHOULD A CONFLICT EXIST BETWEEN THESE PLANS AND ACTUAL FIELD CONDITIONS, THE CONTRACTOR SHALL PROMPTLY NOTIFY THE ENGINEER IN WRITING SO THAT THE CONFLICT CAN BE RESOLVED WITH A MINIMUM AMOUNT OF DELAY FOR ALL PARTIES.
- 5. THE CONTRACTOR SHALL MAINTAIN ACCESS TO ADJACENT PROPERTIES DURING CONSTRUCTION.
- 6. ALL WORK ON THIS PROJECT SHALL BE PERFORMED IN ACCORDANCE WITH APPLICABLE FEDERAL, STATE AND LOCAL LAWS, RULES AND REGULATIONS CONCERNING SAFETY AND HEALTH.
- 7. CONTRACTOR SHALL COMPLY WITH SECTION 19 OF THE "STANDARD SPECIFICATIONS "
- 8. ALL UTILITIES AND UTILITY SERVICE LINES SHALL BE INSTALLED PRIOR TO PAVING.
- 9. BACKFILL COMPACTION SHALL BE ACCORDING TO SPECIFIED
- 10. TACK COAT REQUIREMENTS SHALL BE DETERMINED BY THE CITY
- 11. SIDEWALKS AND WHEELCHAIR RAMPS WITHIN THE CURB RETURNS SHALL BE CONSTRUCTED WHEREVER A NEW CURB RETURN IS
- 12. IF CURB IS DEPRESSED FOR A DRIVEPAD OR A HANDICAP RAMP, THE DRIVEPAD OR RAMP SHALL BE CONSTRUCTED PRIOR TO ACCEPTANCE OF THE CURB AND GUTTER.
- 13. ALL STORM DRAINAGE FACILITIES SHALL BE COMPLETED PRIOR TO FINAL ACCEPTANCE.
- 14. CONTRACTOR SHALL COORDINATE WITH THE WATER SYSTEM DIVISION FOR THE EXECUTION OF THE VALVE SHUT OFF PLAN, NOT LESS THAN TWO (2) WORKING DAYS IN ADVANCE OF ANY WORK THAT MAY AFFECT THE EXISTING PUBLIC WATER UTILITIES.





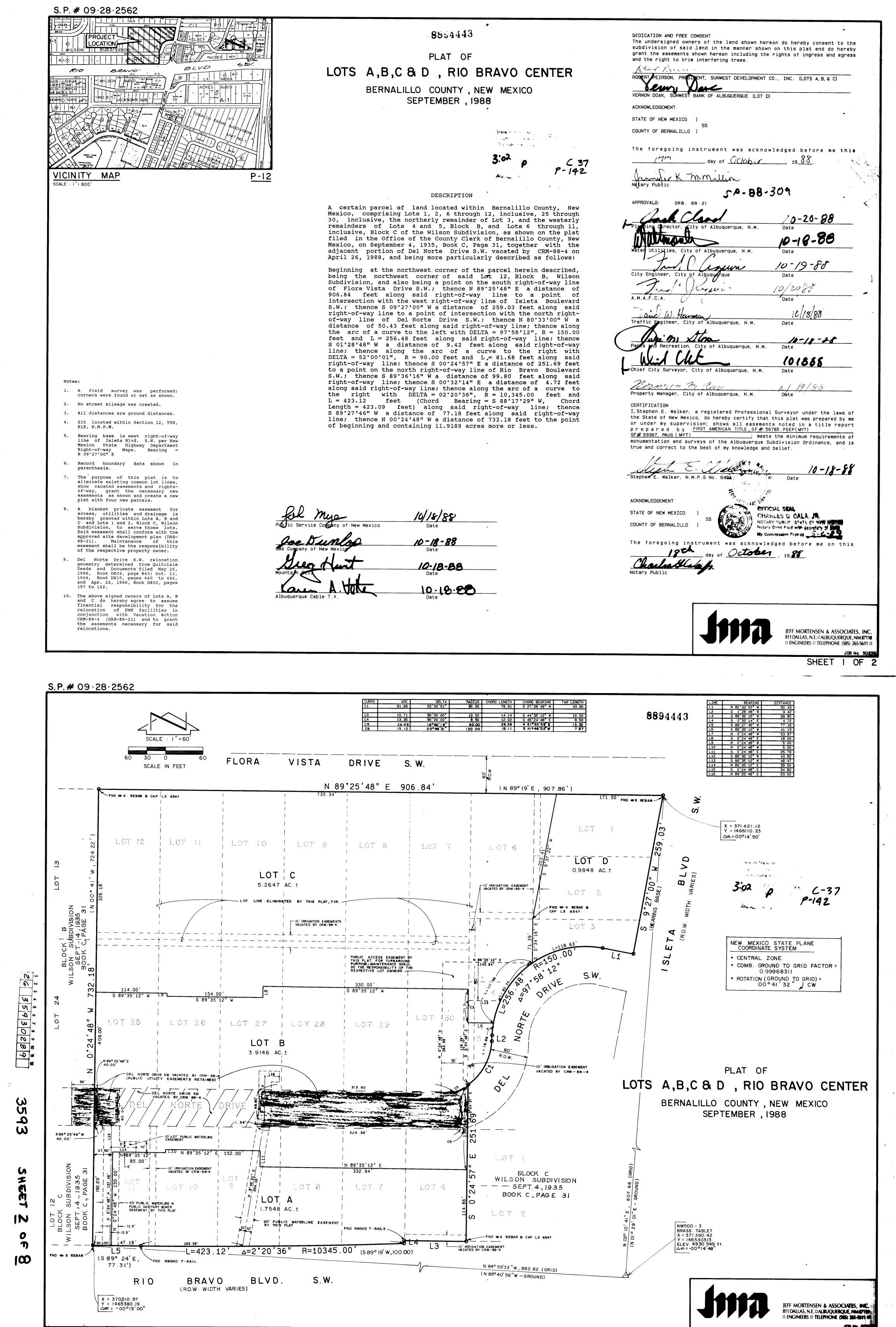


CONSTRUCTION

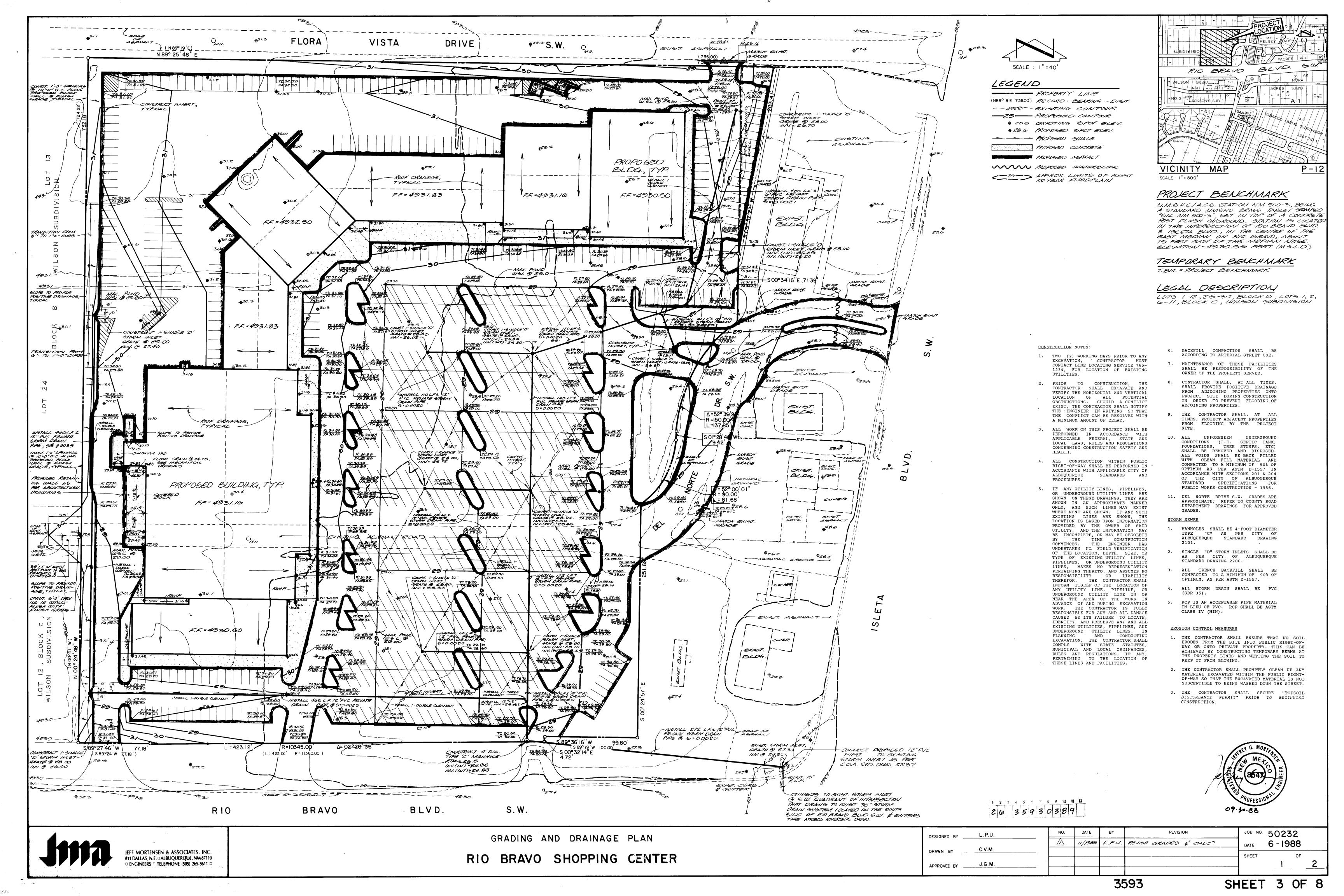
SHEET I OF 8

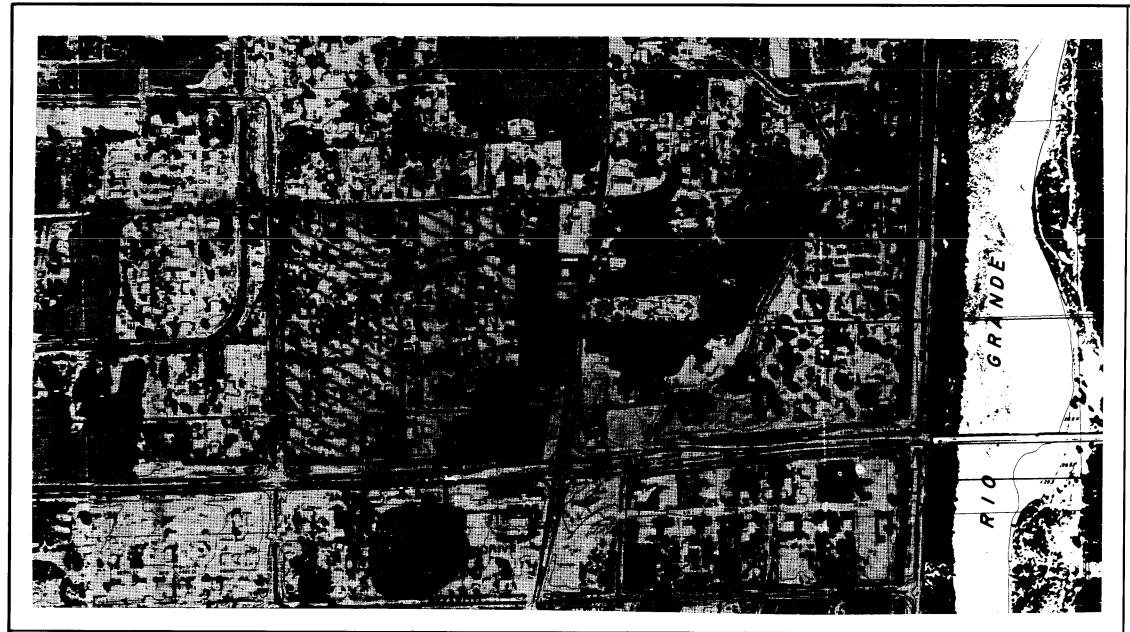
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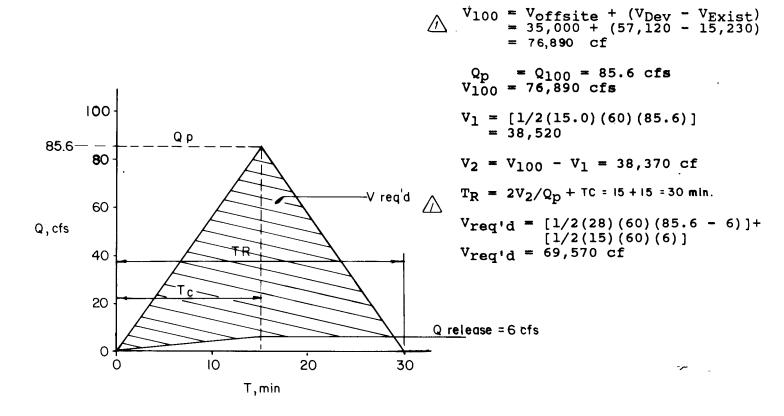


SHEET 2 OF





OFF-SITE CONTRIBUTING AREA



HYDROGRAPH

DRAINAGE PLAN

The following items concerning the Rio Bravo/Isleta Shopping Center Drainage Plan are contained hereon: 1. Vicinity Map 2. Grading Plan

Calculations

As shown by the Vicinity Map, this site is located on the northwest quadrant of the intersection of Rio Bravo Boulevard S.W. and Isleta Boulevard S.W. Flows from this site is undeveloped. Much of the surrounding area is also developed, thereby making this a modification to the existing site. As shown by Panel 40 of the National Flood Insurance Program Flood Boundary and Floodway Maps for the City of Albuquerque, Bernalillo County, this site lies within an AH Flood Zone. Because of this, the finished floor elevation of the proposed buildings have been established at least 1.5' above the corresponding flood elevation of 4929.0 shown on the aforementioned flood map. At present, runoff generated by the site is confined within an existing depression located within the site. Some offsite flows enter the site along the north. south, east and west property lines quantified hereon will be accepted and conveyed through the project site as discussed

The grading plan shows 1) existing and proposed grades indicated by spot elevations and contours at 1'0" intervals, 2) continuity between existing and proposed grades, and 3) the limit and character of the proposed improvements. As shown by this plan, the proposed improvements consist of the construction of a new shopping center along with adjacent paving and landscaping. Flows generated by the proposed improvements will be routed from north to south to a series of proposed detention ponds sized to handle the difference between the developed 100-year runoff and the existing 100-year runoff generated by the proposed improvements and the anticipated offsite volume of runoff. The proposed detention ponds will be connected to an existing storm drain system located within the intersection of Rio Bravo Boulevard S.W. and Isleta Boulevard S.W. From that point, runoff will flow east within the underground storm drain system to the Alameda Drain which is the outfall for this site. The capacity of the aforementioned storm drain system has been determined to allow runoff generated by the proposed improvements at a controlled rate. Based upon the fact that this site is an infill site, the proximity of downstream facilities, and the elimination of the flooding problem, the controlled rate of discharge from this site is appropriate.

The Calculations which appear hereon analyze both the existing and developed conditions for the 100-year, 6-hour rainfall event. The Rational Method has been used to quantify the peak rate of discharge and the SCS Method has been used to quantify the volume of runoff. Both Methods have been used in accordance with the City of Albuquerque Development Process Manual, Volume II, and the Mayor's Emergency Rule adopted January 14, 1986. As shown by these calculations, the peak runoff generated by the proposed improvements will increase by approximately 6 cfs.

CALCULATIONS

Ground Cover Information

From SCS Bernalillo County Soil Survey, Plate 40, VbA - Vinton sandy loam & Va - Vinton loamy sand Hydrologic Soil Group: B Existing Pervious CN = 70 (DPM Plate 22.2 C-2 Pasture or Range Land: fair condition)
Developed Pervious CN = 61 (DPM Plate 22.2 C-2

Time of Concentration/Time to Peak

 $T_C = 0.0078 L^{0.77}/S^{0.385}$ (Kirpich Equation) $T_p = T_c = 10 \text{ min.}$

Point Rainfall

 $P_6 = 2.26 \text{ in. (DPM Plate 22.2 D-1)}$

Discharge: Q = CiA

Rational Method

where C varies e C varies $i = P_6$ (6.84) $T_C = 0.51 = 4.78$ in/br $P_6 = 2.26$ in (DPM Plate 22.2D-1) $T_C = 10$ min (minimum) A = area, acres

SCS Method

Volume: V = 3630(DRO) AWhere DRO = Direct runoff in inches A = area, acres

Existing Condition

Atotal = 456,770 sf = 10.49 Ac Roof area = 47,590 sf (0.10) Paved area = 68,500 sf (0.15) Dirt area = 150,000 sf (0.33)Landscaped area = 190,680 sf (0.42)C = 0.47 (Weighted average per Emergency Rule, 1/14/86) $Q_{100} = CiA = (0.47)(4.78)(10.49) = 23.6 cfs$ $A_{imp} = 116,090 sf; % impervious = 25 %$ Composite CN = 71 (DPM Plate 22.2 C-3) DRO = 0.4 in (DPM Plate 22.2 C-4) $V_{100} = 3630 \text{ (DRO)A} = 15,230 \text{ cf}$

Developed Condition

Atotal = 456,770 sf = 10.49 Ac Roof area = 110,500 sf (0.24) Paved area = 288,770 sf (0.63)Landscaped area = 57,500 sf (0.13)C = 0.85 (Weighted average per Emergency Rule, 1/14/86) $Q_{100} = CiA = (0.85)(4.78)(10.49) = 42.6 cfs$ Aimp = 399,270 sf; % impervious = 87 % Composite CN = 93 (DPM Plate 22.2 C-3) DRO = 1.50 in (DPM Plate 22.2 C-4) V100 = 3630 (DRO)A = 57,120 cf

 $V_{\text{pond}} = \frac{1}{2} \left[\frac{A_{28.0} + A_{29.0}}{A_{28.00}} (29.0 - 28.0) + \frac{A_{29.0} + A_{29.8}}{A_{29.00}} (29.80 - 29.0) \right]$ $= \frac{1}{2} \left[\frac{(0+140,690)}{(0+140,690)} (1.0) + \frac{(0+6000)}{(0+140)} (0.8) + \frac{(0+3000)}{(0+140)} (0.8) \right]$ $= \frac{73,645}{2} \quad \text{Vrequired} = \frac{69,570}{2} \quad \text{cf} \quad \text{(By hydrograph analysis)}$

Offsite Flows

 $A_{total} = 1,050,000 \text{ sf} = 24.1 \text{ Ac}$ C = 0.47 (Based on Onsite Existing Conditions) $T_C = 15 \text{ min}$ Where s = 0.007L = 1,600 fti = 3.80 in/hr $Q_{100} = CiA = (0.47)(3.80)(24.1) = 43 cfs$ Composite CN = 71 (Based on Onsite Existing Conditions) DRO = 0.40 in (Based on Onsite Existing Conditions) V100 = 3630 (DRO)A = 35.000 cf

Drainage Basin Contributing to Existing Storm System (Included Proposed Development & Offisgte Flows)

 $A_{total} = 2,175,000 \text{ sf} = 49.93 \text{ Ac}$ Roof area = 288,600 sf = (0.13) Paved area = 665,400 sf = (0.31)Landscaped area = 327,800 sf = (0.15)C = 0.61 (Based on Onsite Existing Conditions) $T_C = 15 \text{ min}$ Where s = 0.007L = 1,600 fti = 3.80 in/hr $Q_{100} = CiA = (0.61)(3.80)(49.93) = 116 cfs$ Capacity of 30" Storm Drain System

$Q = CA \sqrt{2gh} = 48 cfs$

Where C = 0.7A = 3.14 sf (Area of 30" storm drain outlet downsized to 24" Dia) h = 4930 - 4922.4 = 7.6 ft (NMSH Asbuilts - ST - (m) - 4008 (202+203)

Qallowable = 48/49.9 = 1.0 cfs/Acre Capacity of 18" Lateral (Outlet for Proposed Development)

Q = 6 cfs (Feild's Hydraulics Calculator for Gravity Flow in Pipes Manning Formula) Where s = 0.003 (NMSH Asbuilts - St - (m) - 4008 (202 + 203)

n = 0.013Qallowable = $\frac{6}{34.6}$ cfs = 0.17 cfs/acre

<u>Comparison</u>

 $\Delta Q_{100} = 42.6 - 24.6 = 18 \text{ cfs (increase)}$ $\Delta V_{100} = 57,120 - 15,230 = 41,890 \text{ cf (increase)}$

CONSTRUCTION NOTES:

- 1. TWO (2) WORKING DAYS PRIOR TO ANY EXCAVATION, CONTRACTOR MUST CONTACT LINE LOCATING SERVICE 765-1234, FOR LOCATION OF EXISTING
- 2. PRIOR TO CONSTRUCTION, THE CONTRACTOR SHALL EXCAVATE AND VERIFY THE HORIZONTAL AND VERTICAL LOCATION OF ALL POTENTIAL OBSTRUCTIONS. SHOULD A CONFLICT EXIST, THE CONTRACTOR SHALL NOTIFY THE ENGINEER IN WRITING SO THAT THE CONFLICT CAN BE RESOLVED WITH A MINIMUM AMOUNT OF DELAY.
- 3. ALL WORK ON THIS PROJECT SHALL BE PERFORMED IN ACCORDANCE WITH APPLICABLE FEDERAL, STATE AND LOCAL LAWS, RULES AND REGULATIONS CONCERNING CONSTRUCTION SAFETY AND HEALTH.
- 4. ALL CONSTRUCTION WITHIN PUBLIC RIGHT-OF-WAY SHALL BE PERFORMED IN ACCORDANCE WITH APPLICABLE CITY OF ALBUQUERQUE STANDARDS AND

PROCEDURES 5. IF ANY UTILITY LINES, PIPELINES. OR UNDERGROUND UTILITY LINES ARE SHOWN ON THESE DRAWINGS, THEY ARE SHOWN IN AN APPROXIMATE MANNER ONLY, AND SUCH LINES MAY EXIST WHERE NONE ARE SHOWN. IF ANY SUCH EXISTING LINES ARE SHOWN, THE LOCATION IS BASED UPON INFORMATION PROVIDED BY THE OWNER OF SAID UTILITY, AND THE INFORMATION MAY BE INCOMPLETE, OR MAY BE OBSOLETE BY THE TIME CONSTRUCTION COMMENCES. THE ENGINEER HAS UNDERTAKEN NO FIELD VERIFICATION OF THE LOCATION, DEPTH, SIZE, OR TYPE OF EXISTING UTILITY LINES, PIPELINES, OR UNDERGROUND UTILITY LINES. MAKES NO REPRESENTATION PERTAINING THERETO, AND ASSUMES NO RESPONSIBILITY OR LIABILITY THEREFOR. THE CONTRACTOR SHALL INFORM ITSELF OF THE LOCATION OF ANY UTILITY LINE, PIPELINE, OR UNDERGROUND UTILITY LINE IN OR NEAR THE AREA OF THE WORK IN ADVANCE OF AND DURING EXCAVATION WORK. THE CONTRACTOR IS FULLY RESPONSIBLE FOR ANY AND ALL DAMAGE CAUSED BY ITS FAILURE TO LOCATE, IDENTIFY AND PRESERVE ANY AND ALL EXISTING UTILITIES, PIPELINES, AND UNDERGROUND UTILITY LINES. IN PLANNING AND CONDUCTING EXCAVATION, THE CONTRACTOR SHALL COMPLY WITH STATE STATUTES, MUNICIPAL AND LOCAL ORDINANCES, RULES AND REGULATIONS, IF ANY, PERTAINING TO THE LOCATION OF THESE LINES AND FACILITIES.

6. BACKFILL COMPACTION SHALL BE ACCORDING TO ARTERIAL STREET USE. MAINTENANCE OF THESE FACILITIES SHALL BE RESPONSIBILITY OF THE

OWNER OF THE PROPERTY SERVED.

- CONTRACTOR SHALL, AT ALL TIMES, SHALL PROVIDE POSITIVE DRAINAGE FROM ADJOINING PROPERTIES ONTO PROJECT SITE DURING CONSTRUCTION IN ORDER TO PREVENT FLOODING OF ADJOINING PROPERTIES.
- THE CONTRACTOR SHALL, AT ALL TIMES, PROTECT ADJACENT PROPERTIES FROM FLOODING BY THE PROJECT
- 10. ALL UNFORESEEN UNDERGROUND CONDITIONS (I.E. SEPTIC TANK, FOUNDATIONS, TREE STUMPS, ETC) SHALL BE REMOVED AND DISPOSED. ALL VOIDS SHALL BE BACK FILLED WITH CLEAN FILL MATERIAL AND COMPACTED TO A MINIMUM OF 90% OF OPTIMUM AS PER ASTM D-1557 IN ACCORDANCE WITH SECTIONS 201 & 204 OF THE CITY OF ALBUQUERQUE STANDARD SPECIFICATIONS FOR PUBLIC WORKS CONSTRUCTION - 1986. 11. DEL NORTE DRIVE S.W. GRADES ARE
- APPROXIMATE: REFER TO COUNTY ROAD DEPARTMENT DRAWINGS FOR APPROVED

STORM SEWER

- MANHOLES SHALL BE 4-FOOT DIAMETER TYPE "C" AS PER CITY OF ALBUQUERQUE STANDARD DRAWING
- SINGLE "D" STORM INLETS SHALL BE AS PER CITY OF ALBUQUERQUE STANDARD DRAWING 2206.
- ALL TRENCH BACKFILL SHALL BE COMPACTED TO A MINIMUM OF 90% OF OPTIMUM, AS PER ASTM D-1557.
- 4. ALL STORM DRAIN SHALL BE PVG (SDR 35).
- 5. RCP IS AN ACCEPTABLE PIPE MATERIAL IN LIEU OF PVC. RCP SHALL BE ASTM CLASS IV (MIN).

EROSION CONTROL MEASURES

- . THE CONTRACTOR SHALL ENSURE THAT NO SOIL ERODES FROM THE SITE INTO PUBLIC RIGHT-OF-WAY OR ONTO PRIVATE PROPERTY. THIS CAN BE ACHIEVED BY CONSTRUCTING TEMPORARY BERMS AT THE PROPERTY LINES AND WETTING THE SOIL TO KEEP IT FROM BLOWING.
- 2. THE CONTRACTOR SHALL PROMPTLY CLEAN UP ANY MATERIAL EXCAVATED WITHIN THE PUBLIC RIGHT-OF-WAY SO THAT THE EXCAVATED MATERIAL IS NOT SUSCEPTIBLE TO BEING WASHED DOWN THE STREET.
- 3. THE CONTRACTOR SHALL SECURE "TOPSOIL DISTURBANCE PERMIT" PRIOR TO BEGINNING CONSTRUCTION.

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GRADING AND DRAINAGE PLAN

RIO BRAVO SHOPPING CENTER

DESIGN BY 11/1988 LPU REVISE GRADES & CALC.5 6 - 1988 DATE C.V. M. SHEET

