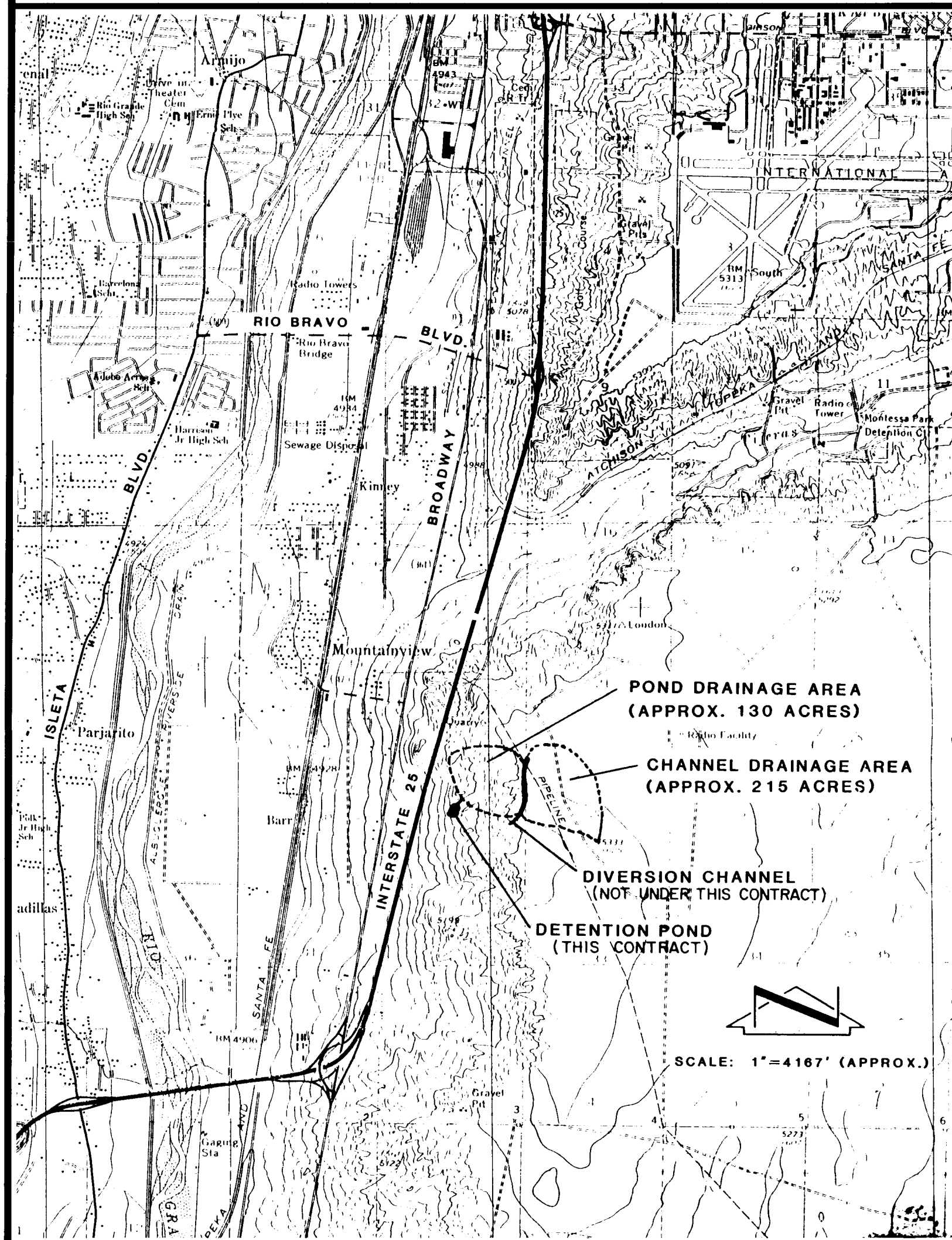


VICINITY MAP



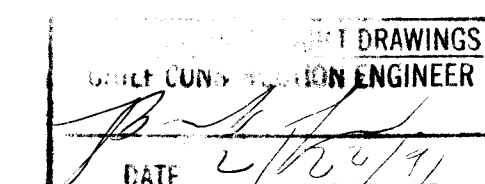
SOUTH BROADWAY LANDFILL (AREA 5)

DETENTION POND ALBUQUERQUE, NEW MEXICO

INDEX OF SHEETS

SHEET NO.	DESCRIPTION
1.	Title Sheet/Location Map/Index of Sheets
2.	Filing Sheet
3.	Legend/General and Structural Notes
4.	Quantities
5.	Detention Pond Plan and Profile 1'-100'
6.	Detention Pond Site Plan 1'-50'
7.	Geologic Section and Foundation Treatment
8.	Embankment Cross Sections
9.	Principal Spillway Profile and Details
10.	Emergency Spillway Plan and Profile
11.	Concrete Chute Half - Plan and Profile
12.	Structural Sections
13.	Structural Details

SOLID WASTE DEPARTMENT
CITY OF ALBUQUERQUE, NEW MEXICO
PROJECT NO. 3935



C of A PWD Maps & Records

1	2	3	4	5	6	7	8	9	10	11	12
24	39	35	01	91							

APPROVED FOR CONSTRUCTION

Richard Smith
1-12-90 CITY ENGINEER

RECORD DRAWING
7/16/90

NOTE: RECORD DRAWINGS DEVELOPED USING
INFORMATION PROVIDED BY CITY OF
ALBUQUERQUE. CONSTRUCTION INSPECTION
PERFORMED BY CITY OF ALBUQUERQUE
PUBLIC WORKS DEPARTMENT.

SET NO. _____

JOB NO. 86102



SCANLON & ASSOCIATES INC.
CONSULTING ENGINEERS

DATE JUNE, 1989

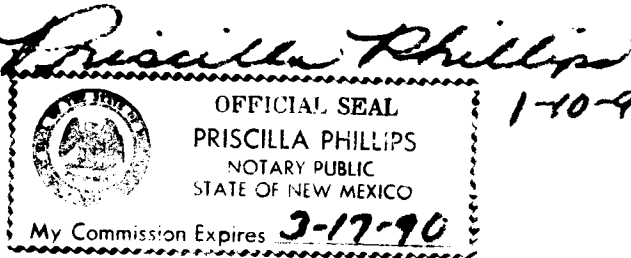
Warning note: Peak flow for 1/2 PMF is 1434 cfs. However the concrete chute can only receive 582 cfs. Consequently, only 582 cfs will flow into the detention pond. Flows exceeding 582 cfs will by-pass the pond to the west in the historic flow path. The concrete chute and wingwalls should not be raised without a complete reanalysis of Hydrology and Hydraulics. The emergency spillway is designed for only 484 cfs according to these construction plans.

DEPTH vs STORAGE		
ELEVATION FEET ABOVE MSL	STAGE ACRE FEET	
5146	0.00	
5148	1.27	
5150	2.74	
5152	4.41	
5154	6.30	
5156	8.42	
5158 EMERGENCY SPILLWAY CREST	10.78	
5160	13.40	
5162	16.30	
5163 TOP OF DAM	17.86	

STATISTICS OF SOUTH BROADWAY LANDFILL DETENTION DAM	
MAXIMUM HEIGHT ABOVE FOUNDATION	23.0
MAXIMUM LENGTH	323.0
MAXIMUM WIDTH AT BASE	190.0
CREST WIDTH	15.0
SLOPE UPSTREAM FACE	3:1
SLOPE DOWNSTREAM FACE	5:1
ELEVATION AT SPILLWAY CREST	5158.0
ELEVATION BOTTOM OUTLET CONDUIT AT UPSTREAM FACE	5146.0
FREEBOARD @ 100-YEAR FREQUENCY FLOOD	1.5
FREEBOARD AT P.M.P.	1.6
WIDTH OF SPILLWAY	25.0
DISCHARGE CAPACITY OF SPILLWAY (CFS AT ELEV)	484 @ 5161.4
OUTLET CONDUIT TYPE AND SIZE	1-24" C.C.P.
OUTLET CONDUIT CAPACITY (CFS AT ELEV)	41 @ 5161.4
1/2 - 6 HOUR P.M.P. EFFECTIVE RAINFALL-INCHES	5.875
P.M.F. DISCHARGE CFS	582.0
EVACUATION TIME - HOURS (FOR P.M.P.)	20.0
DRAINAGE AREA	130 Ac.
W. S. ELEV. AT P.M.P.	5161.4
W.S. ELEV. AT 100-YEAR (includes 2.4 ac ft sediment)	5156.5

Note: The bottom 1-1/2 feet of the pond shall be considered the maximum depth for a working sediment volume. The pond shall be cleaned and maintained when a depth of sediment and debris greater than 1-1/2 feet is achieved.

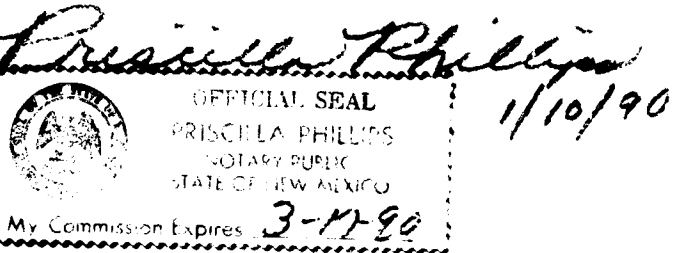
STATE OF NEW MEXICO)
CITY OF ALBUQUERQUE) SS



I hereby certify that the accompanying maps and statements have been duly examined by me and are approved as to form and content, and were duly accepted for filing on the 16th day of June 1990.

State Engineer
City of Albuquerque
State of New Mexico

ENGINEER'S CERTIFICATE
STATE OF NEW MEXICO)
COUNTY OF BERNALILLO) SS



I, Richard Lovato, being first duly sworn upon my oath, state that I am the Registered Professional Engineer who made the plans of the South Broadway Landfill Detention Pond and Diversion Channel, that such plans consisting of 13 sheets were prepared by me or under my direction and that the same are true and correct to the best of my knowledge and belief.

Registered Professional Engineer No. 6823

Scanlon and Associates, Inc.
8008 Pennsylvania Circle, NE
Albuquerque, New Mexico 87110

Richard Lovato

Subscribed and sworn to before me this 10 day of JANUARY 1990.

STATE OF NEW MEXICO)
COUNTY OF BERNALILLO) SS

I hereby certify that the accompanying maps and statements have been duly examined by me and are approved as to form and content, and were duly accepted for filing on the 11 day of Jan, 1990.

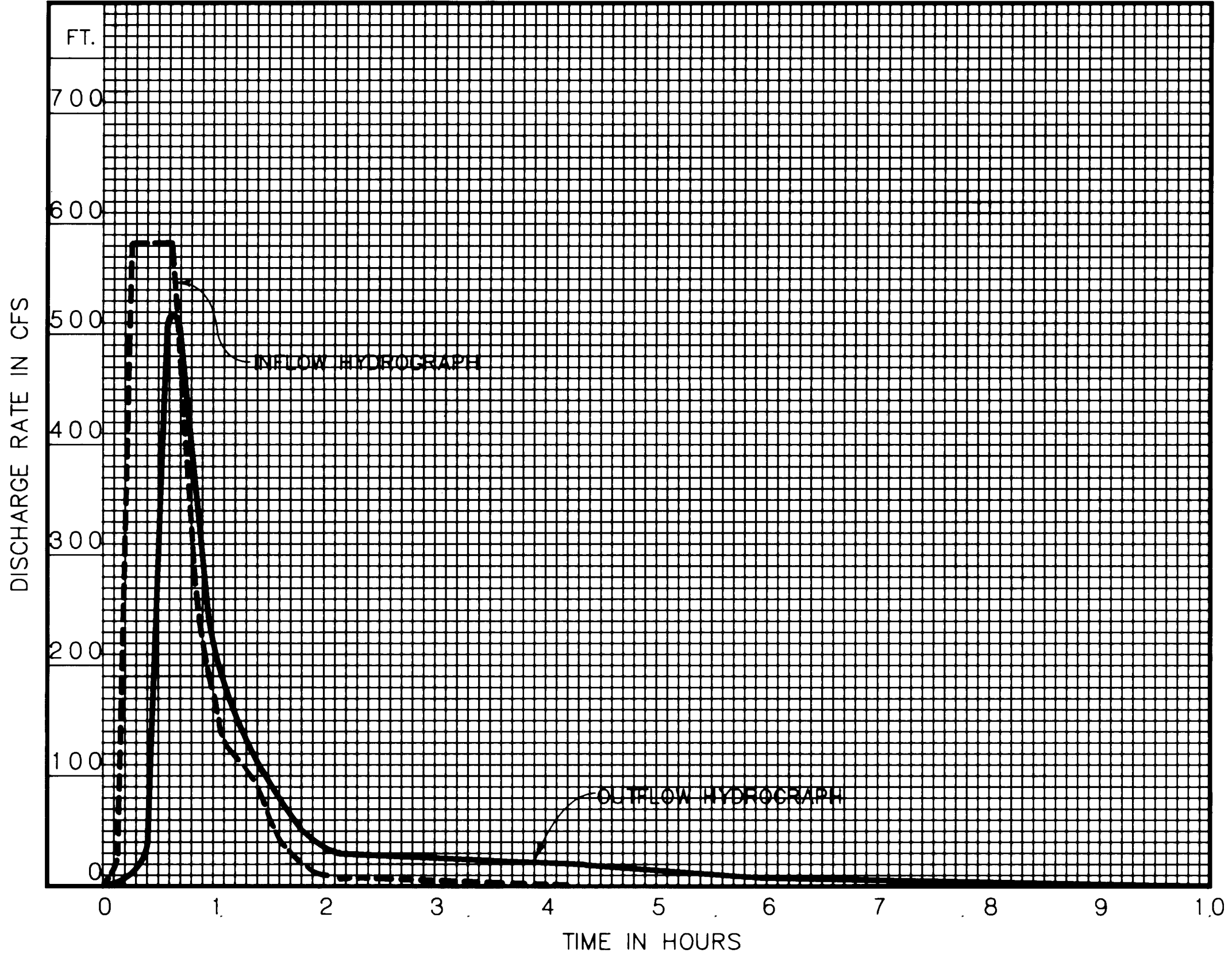
State Engineer
State of New Mexico

C of A Maps & Records
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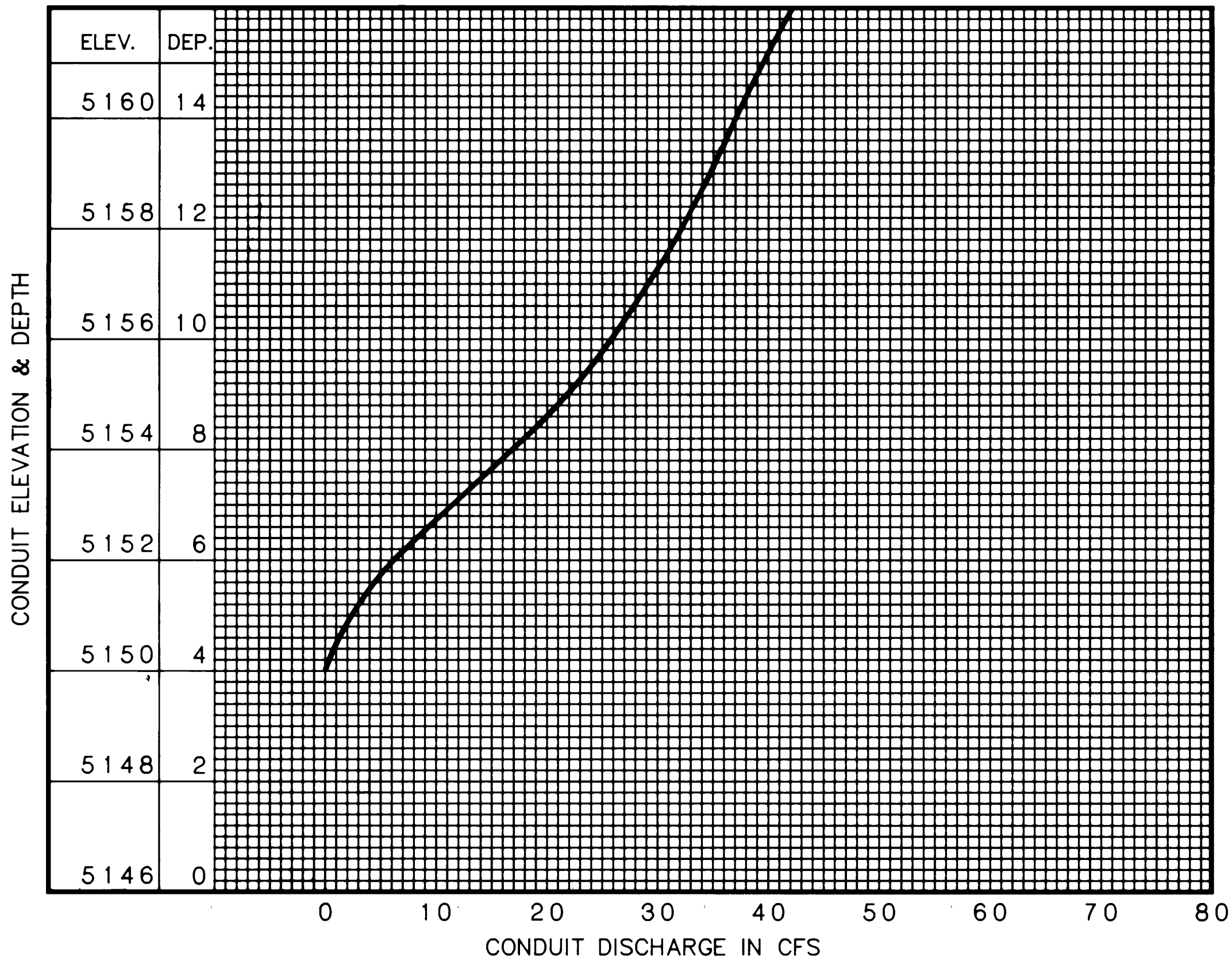
Richard Lovato

RECORD DRAWING
7/16/90

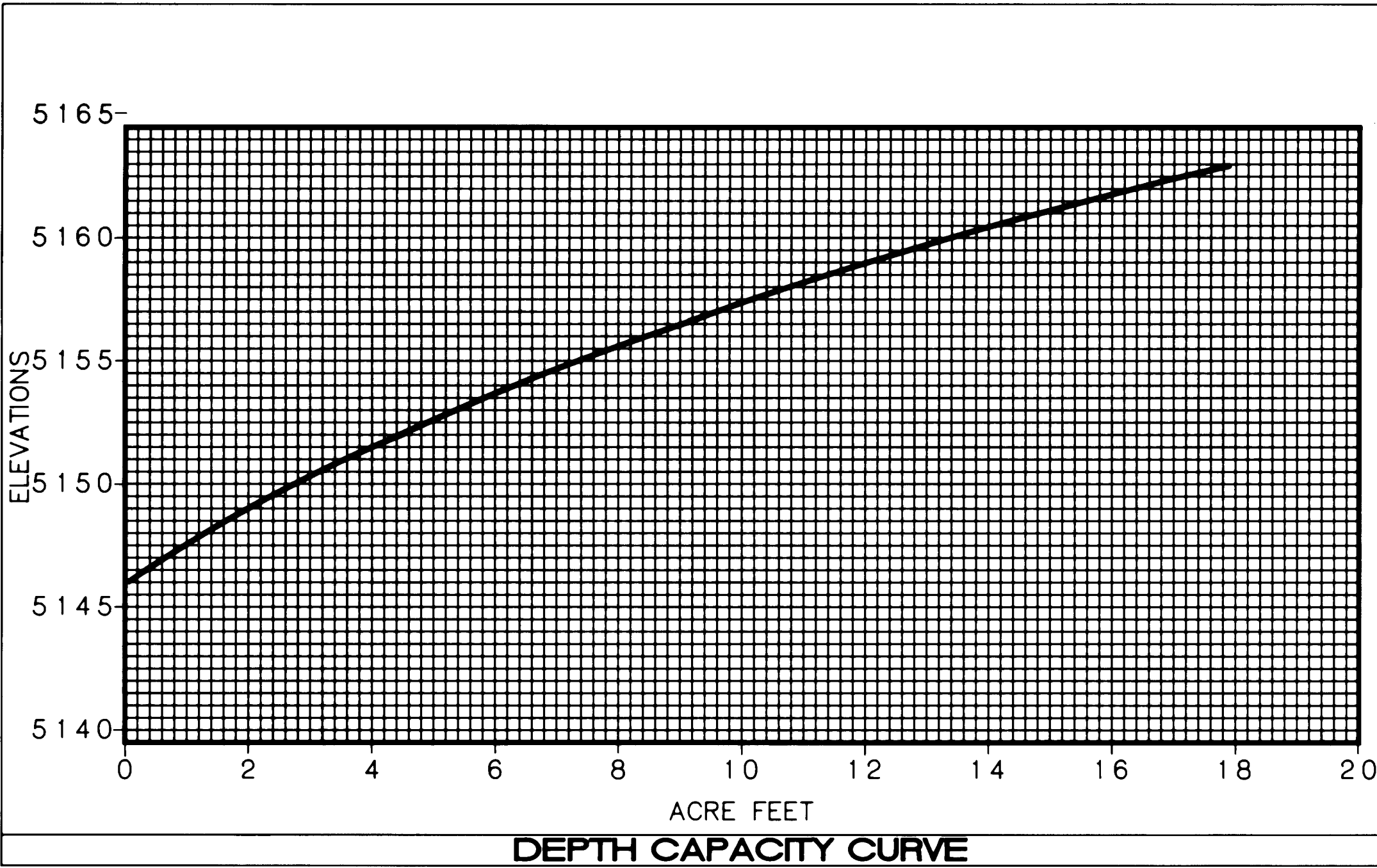
01/90 HYDROLOGY REVISIONS		JEAC
SOUTH BROADWAY LANDFILL DIVERSION CHANNEL & DETENTION DAM		
FILING SHEET		
SOLID WASTE DEPARTMENT, CITY OF ALBUQUERQUE		
DATE 7/21/89	JOB NO. 86102	SCALE AS SHOWN
DRAWN RLH	CHECKED JEAC	SHEET 2 OF 13
SCANLON & ASSOCIATES INC. CONSULTING ENGINEERS		



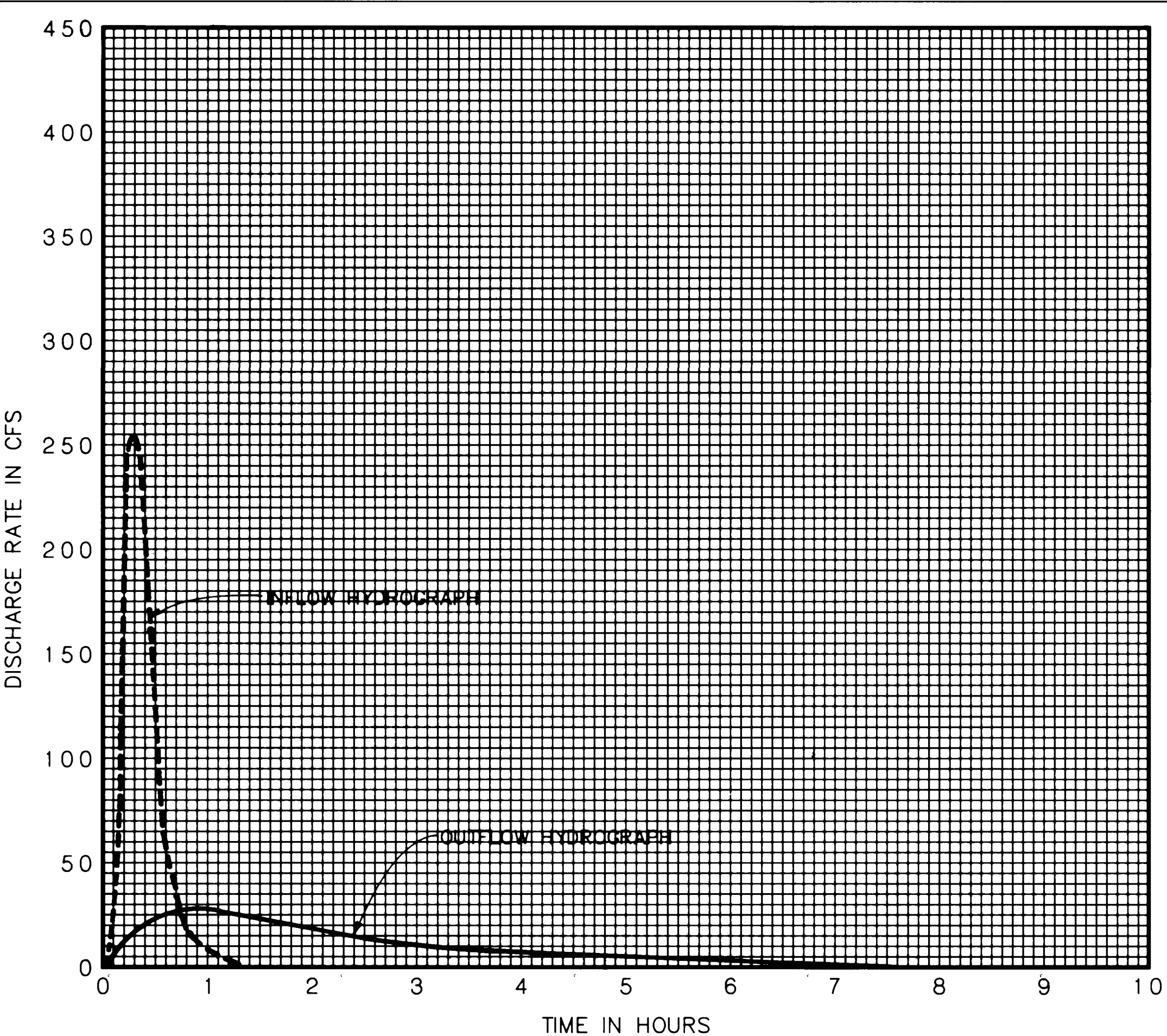
1/2 PMF INFLOW-OUTFLOW HYDROGRAPH



ELEVATION DISCHARGE CURVE - CONDUIT



DEPTH CAPACITY CURVE

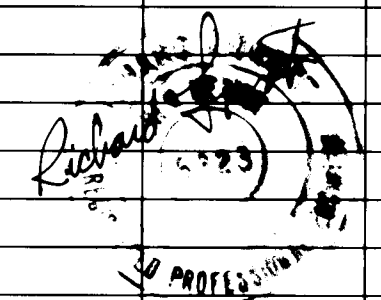


100-yr INFLOW - OUTFLOW HYDROGRAPH

SUMMARY OF QUANTITIES

ITEM NO.	ITEM	UNIT	ESTIMATE	FINAL	ESTIMATE	FINAL	ESTIMATE	FINAL	ESTIMATE	FINAL	ESTIMATE	FINAL	ESTIMATE	FINAL	ESTIMATE	FINAL	ESTIMATE	FINAL	ESTIMATE	FINAL	ESTIMATE	FINAL	ESTIMATE	FINAL
1.	Clearing and Grubbing for Pond	AC	4.5	4.5																				
2.	Subgrade Preparation	SY	21,500 *	21,500																				
3.	Concrete Structures	CY	385	385																				
4.	Earthwork for Pond-Excavation	CY	43,245 *	43,245																				
5.	Earthwork for Pond-Embankment	CY	7,675 *	7,675																				
6.	Riprap	CY	510	515																				
7.	Granular Bedding	CY	335	315																				
8.	Principal Spillway, 24" D. CCP	LF	217	217																				

C of A PND Maps & Records
214 136 1510 4971



RECORD DRAWING
7/16/90

PLAN	SURVEYED.....	BY	DATE
	PLOTTED.....		
	ALIGNMENT CHECKED.....		
	RT. OF WAY CHECKED.....		
NOTE BOOK NO.			

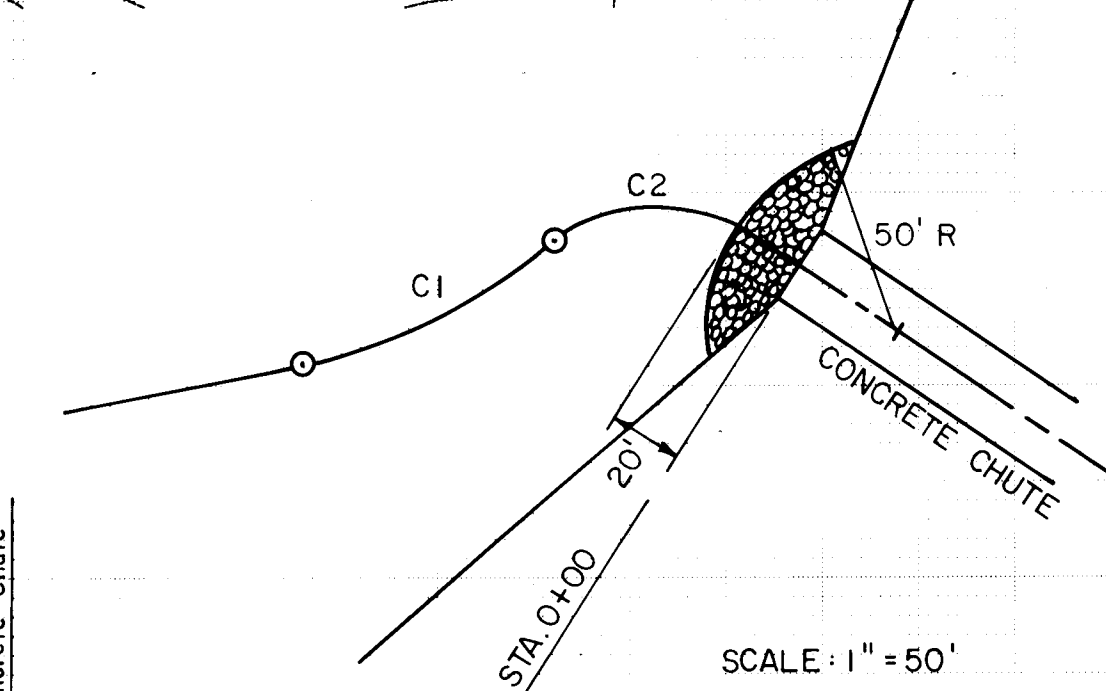
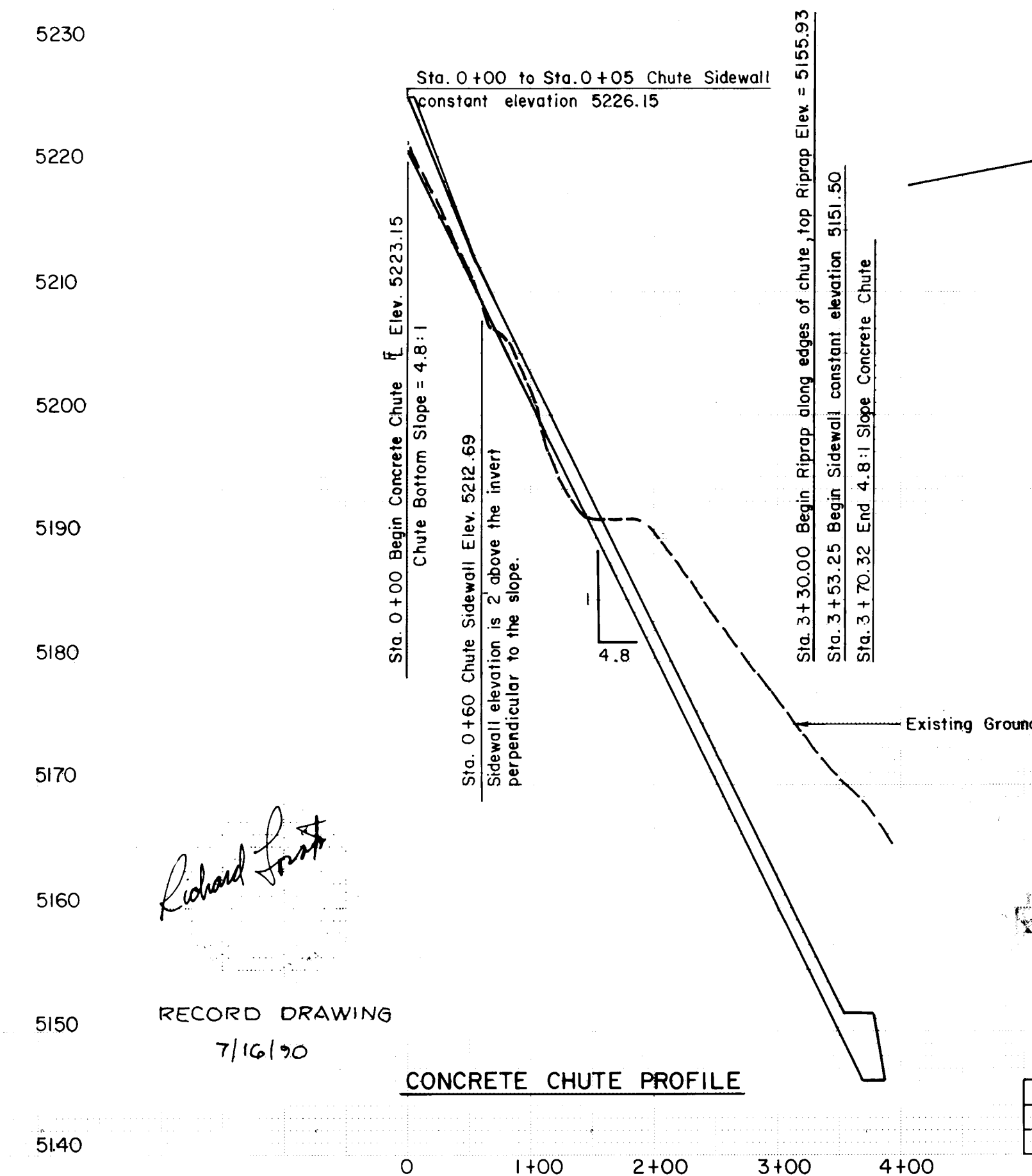
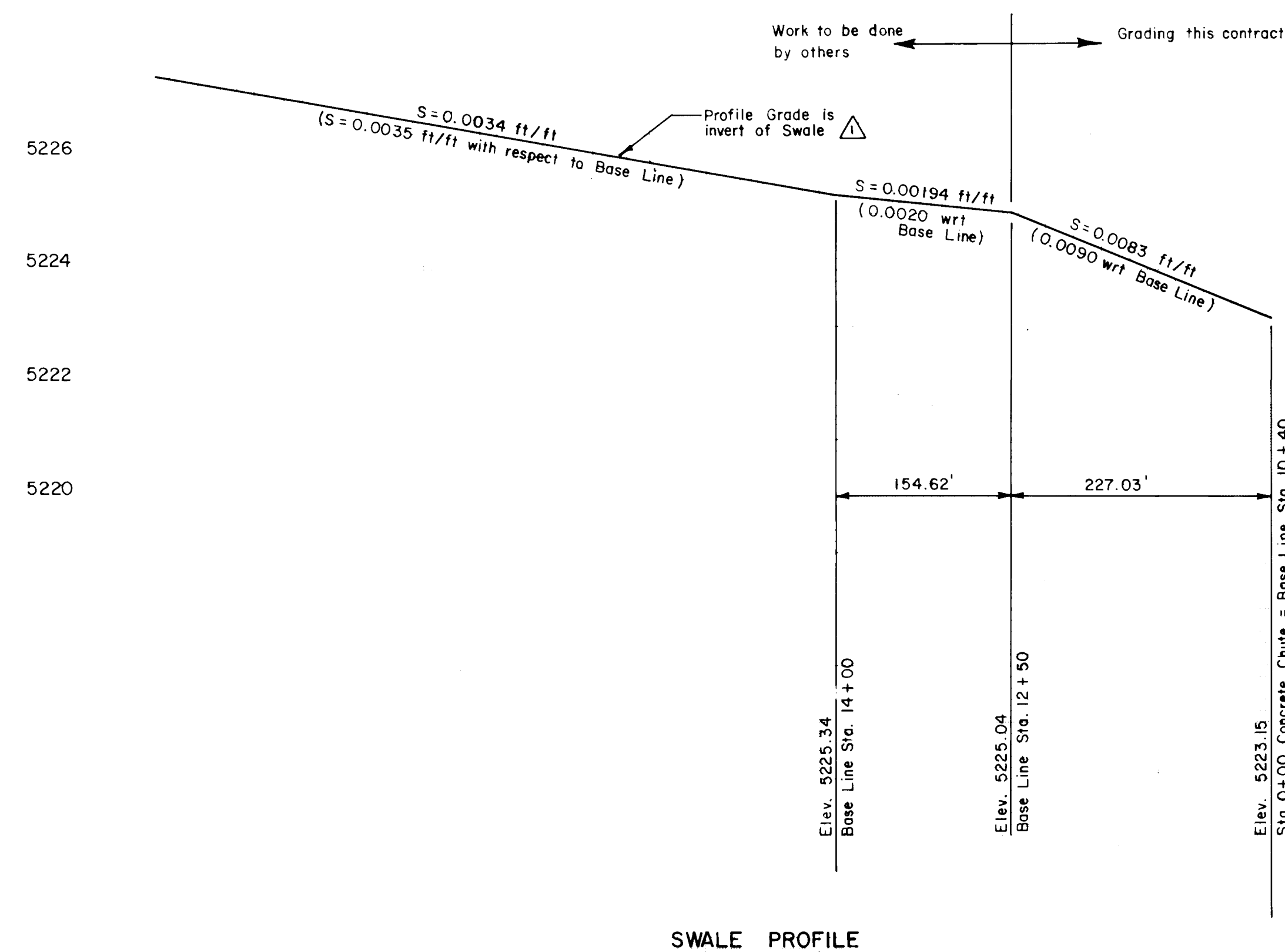
PROFILE NOTE BOOK NO. _____ GRADES CHECKED _____ B. M.'S NOTED _____ STRUCTURE NOTATIONS CHMD _____	SURVEYED _____	BY _____	DATE _____
	PLOTTED _____		

C CURVE TABLE						
CURVE	RADIUS	LENGTH	TANGENT	DELTA	CHORD	DESCRIPTION
C1	150.00'	71.21'	36.29'	27°11'55"	70.54' - S 27° 38' 08" E	SWALE
C2	50.00'	63.47'	36.82'	72°44'07"	59.30' - S 04° 52' 02" E	SWALE
C3	88.48'	103.70'	58.73'	67°06'56"	97.87' - S 21° 30' 36" E	C-DAM
C4	126.04'	100.64'	53.18'	45°44'58"	97.99' - S 34°51' 02" E	C DAM
C5	293.31'	186.08'	96.29'	36°20'57"	182.98' - S 03° 39' 26" E	C ACCESS ROAD
C6	347.51'	180.00'	92.07'	29°40'39"	177.99' - S 15° 22' 28" E	C EMERGENCY SPILLWAY
C7	29.98'	52.17'	35.54'	39°41'22"	45.83' - S 37° 52' 08" W	POND BOTTOM
C8	67.54'	211.99'	47.74743'	179°50'16"	135.08' - N 78° 06' 19" E	POND BOTTOM

NOTES :

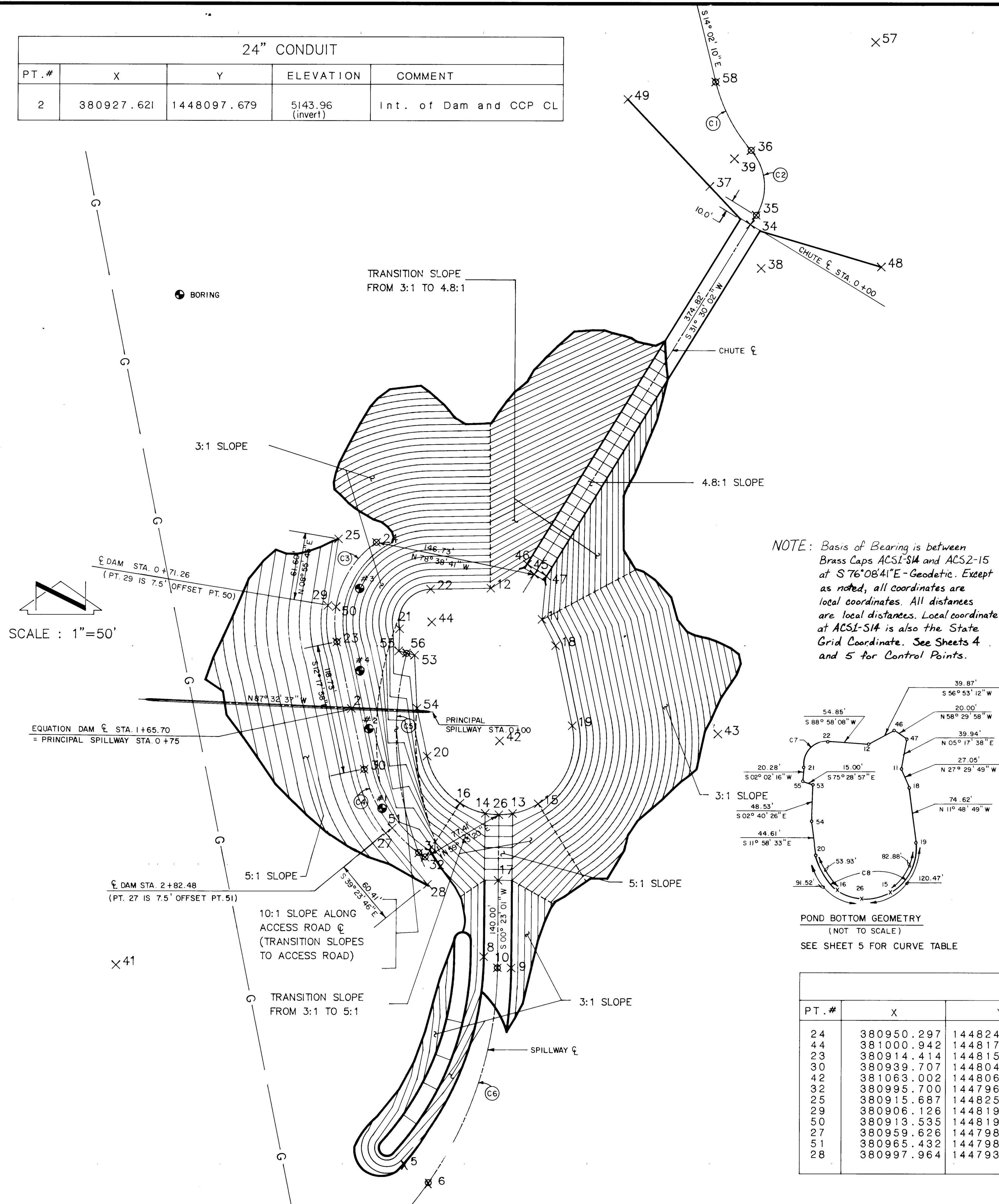
1. The finished contours shown in the area of the drainage swale reflect elevations after the final cover has been placed on the landfill. Grading in this area has been done with respect to the Base Line. The contractor will be responsible for all grading adjacent to the wing-walls and all grading up to elevation 5225. The Contractor shall coordinate with the Landfill.
2. None of the riprap is shown on this plan for the sake of clarity.
3. The swale profile is shown along the swale, not along the Base Line.
4. The chute profile can be seen in detail on Sheet II.

CONTROL POINTS				
HV-4	N 1448448.445	E 381050.567	ELEV. = 5212.93	
HV-6	N 1447389.707	E 381082.292	ELEV. = 5152.19	
HV-4	TO HV-6 IS 1059.213' AT S 01° 42' 58.95" E			



NO.	DATE	REMARKS	BY
①	11/89	NOTE CLARIFICATIONS, ILLUSTRATION OF RIPRAP IN PLAN VIEW	JEAC
②	01/90	SEDIMENT DEPTH POSTS	JEAC

24" CONDUIT				
PT. #	X	Y	ELEVATION	COMMENT
2	380927.621	1448097.679	5143.96 (invert)	Int. of Dam and CCP CL



CONCRETE CHUTE AND UPSTREAM GRADING				
PT. #	X	Y	ELEVATION	COMMENT
58	381257.475	1448670.101	5224.35	P.C.
57	381402.996	1448706.481	-	Radius Point
36	381290.194	1448607.609	5223.76	P.C.C.
37	381252.593	1448574.652	-	Radius Point
35	381295.225	1448548.526	5223.23	P.T.
49	381177.310	1448654.166	5226.15	End of Wingwall (Center)
34	381290.000	1448540.000	5223.15	Chute Sta. 0+00
48	381409.323	1448501.361	5226.15	End of Wingwall (Center)
38	381300.000	1448500.000	-	Original Swale Line
39	381275.000	1448600.000	-	Original Swale Line
45	381094.154	1448220.415	5146.00	Chute Sta. 3+74.82

BOTTOM OF POND				
PT. #	X	Y	ELEVATION	COMMENT
12	381054.583	1448207.695	5146.00	P.I.. Slope Transition
22	380999.746	1448206.708	5146.00	P.C.
44	381000.942	1448176.750	5146.00	Radius Point
21	380971.612	1448170.529	5146.00	P.C.. (not tangent)
55	380970.891	1448150.265	5146.00	Bottom of Road
56	380978.152	1448148.385	5146.00	CL of Road
53	380985.412	1448146.505	5146.00	Bottom of Road
54	380987.676	1448098.028	5146.00	P.I.
20	380996.934	1448054.386	5146.00	P.C.
42	381063.002	1448068.400	5146.00	Radius Point
19	381129.111	1448082.227	5146.00	P.T.
18	381113.834	1448155.267	5146.00	P.I.
11	381101.347	1448179.257	5146.00	P.I.. Slope Transition
47	381105.032	1448219.027	5146.00	P.I.
46	381087.979	1448229.477	5146.00	P.I.
16	381026.938	1448011.296	5146.00	Slope Transition
14	381050.058	1448002.113	5146.00	Slope Transition
13	381075.057	1448001.946	5146.00	Slope Transition
15	381098.298	1448010.818	5146.00	Slope Transition

ACCESS ROAD				
PT. #	X	Y	ELEVATION	COMMENT
56	380978.152	1448148.385	5146.00	Bottom of Road
43	381262.101	1448074.859	-	Radius Point
31	380989.823	1447965.782	5163.00	Int. of Road and Dam CL Sta. 3+15.99

SPILLWAY				
PT. #	X	Y	ELEVATION	COMMENT
26	381062.549	1448000.863	5146.00	CL Spillway Sta 0+00
17	381062.147	1447940.864	5158.00	Spillway Crest, Sta 0+60
10	381061.611	1447860.866	5158.00	Spillway Sta 1+40, P.C.
41	380714.109	1447863.195	-	Radius Point
6	381001.585	1447667.949	~5160.00	Spillway Sta 3+45
5	380978.422	1447683.680	~5148.60	West Side End of Grading

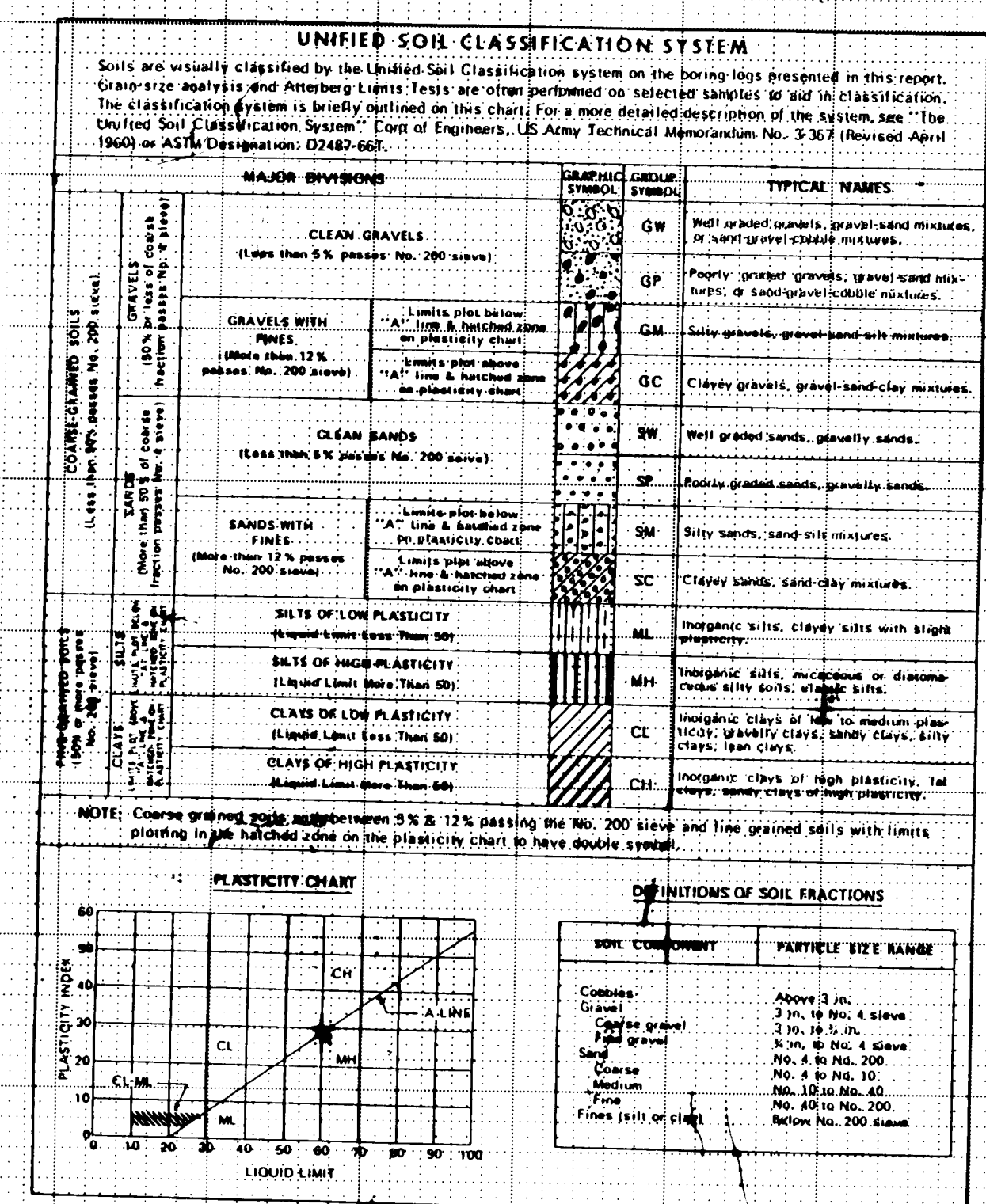
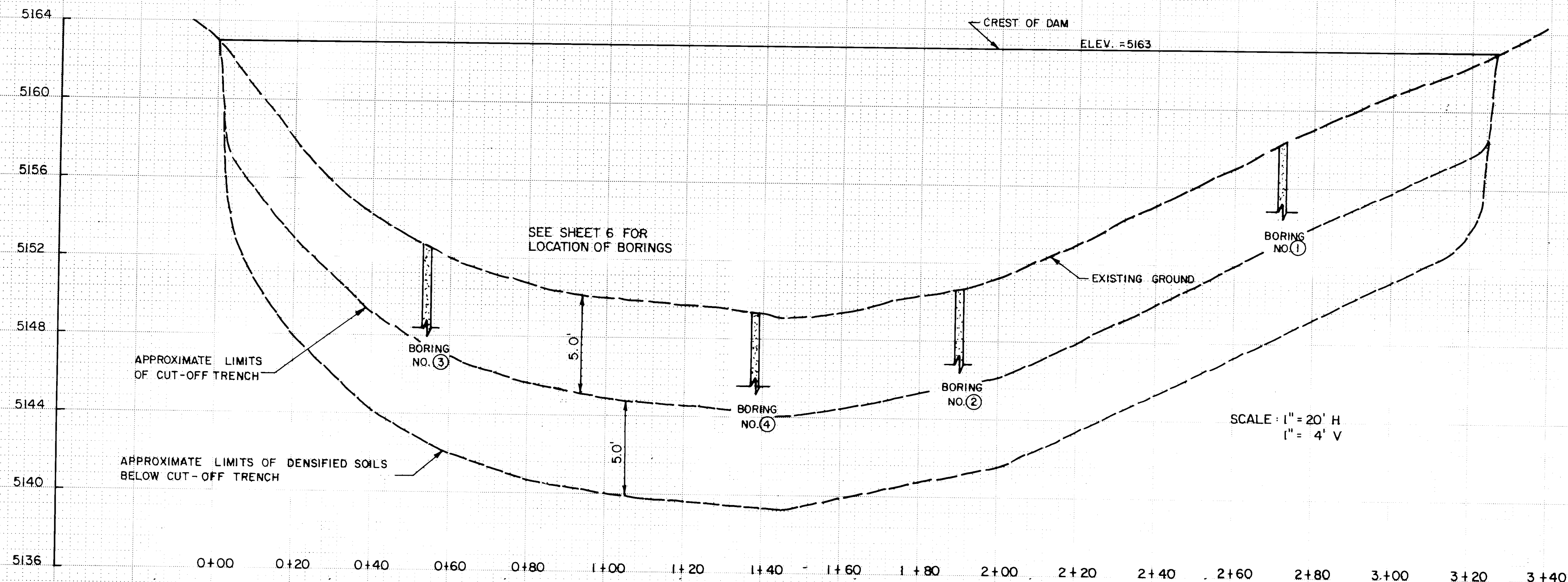
CENTERLINE OF DAM				
PT. #	X	Y	ELEVATION	COMMENT
24	380950.297	1448249.305	5163.00	CL Dam Sta 0+00 (P.C.)
44	381000.942	1448176.750	-	Radius Point
23	380914.414	1448158.256	5163.00	CL Dam Sta 1+03.70 (P.T.)
30	380939.707	1448042.247	5163.00	CL Dam Sta 2+22.43 (P.C.)
42	381063.002	1448068.400	-	Radius Point
32	380995.700	1447961.835	5163.00	CL Dam Sta 3+23.07 (End)
25	380915.687	1448252.519	5163.00	Top Edge Dam
29	380906.126	1448191.668	5163.00	Top Edge Dam
50	380913.535	1448190.502	5163.00	CL Dam Sta 0+71.26
27	380959.626	1447983.866	5163.00	Top Edge Dam
51	380965.432	1447988.614	5163.00	CL Dam Sta 2+82.48
28	380997.964	1447937.187	5163.00	Top Edge Dam

Richard J. Smith
7/16/90

RECORD DRAWING
7/16/90

26 31 10 11 12 13 14 15 16 17 18 19 20 21 22 23 24 25 26 27 28 29 30 31 32 33 34 35 36 37 38 39 40 41 42 43 44 45 46 47 48 49 50 51 52 53 54 55 56 57 58 59 60 61 62 63 64 65 66 67 68 69 70 71 72 73 74 75 76 77 78 79 80 81 82 83 84 85 86 87 88 89 90 91 92 93 94 95 96 97 98 99 100

SOUTH BROADWAY LANDFILL DIVERSION CHANNEL & DETENTION POND				
DETENTION POND SITE PLAN				
SOLID WASTE DEPARTMENT, CITY OF ALBUQUERQUE				
DATE JUNE 1989	JOB NO. 86102	SCALE 1"=50'	DRAWN JGL	SHEET 6 OF 13
SCANLON & ASSOCIATES INC. CONSULTING ENGINEERS				



TERMINOLOGY USED TO DESCRIBE THE RELATIVE DENSITY, CONSISTENCY, OR FIRMNESS OF SOILS																																																					
The terminology used on the boring logs to describe the relative density, consistency, or firmness of soils is relative to the standard penetration (N) resistance in blows per foot (ft) obtained by ASTM D1586 procedure using 2" O.D., 1-3/8" I.D. sampler. 1. Relative Density. Terms for description of relative density of cohesionless, unconsolidated sands and sand-gravel mixtures. <table border="1"> <tr> <th>Relative Density</th> <th>Remarks</th> </tr> <tr> <td>0-4</td> <td>Very loose</td> </tr> <tr> <td>5-10</td> <td>Loose</td> </tr> <tr> <td>11-30</td> <td>Medium dense</td> </tr> <tr> <td>31-50</td> <td>Dense</td> </tr> <tr> <td>51+</td> <td>Very dense</td> </tr> </table> 2. Relative Consistency. Terms for the description of clays which are saturated or near saturation. <table border="1"> <tr> <th>Relative Consistency</th> <th>Remarks</th> </tr> <tr> <td>0-2</td> <td>Very soft</td> </tr> <tr> <td>3-4</td> <td>Soft</td> </tr> <tr> <td>5-8</td> <td>Medium stiff</td> </tr> <tr> <td>9-15</td> <td>Stiff</td> </tr> <tr> <td>16-30</td> <td>Very stiff</td> </tr> <tr> <td>31+</td> <td>Hard</td> </tr> </table> 3. Relative Firmness. Terms for the description of partially saturated and/or cemented soils which commonly occur in the Southwest including clays, cemented granular materials, silts and silty and clayey granular soils: <table border="1"> <tr> <th>Relative Firmness</th> <th>Remarks</th> </tr> <tr> <td>0-4</td> <td>Very soft</td> </tr> <tr> <td>5-8</td> <td>Soft</td> </tr> <tr> <td>9-15</td> <td>Moderately firm</td> </tr> <tr> <td>16-30</td> <td>Firm</td> </tr> <tr> <td>31-50</td> <td>Very firm</td> </tr> <tr> <td>51+</td> <td>Hard</td> </tr> </table>	Relative Density	Remarks	0-4	Very loose	5-10	Loose	11-30	Medium dense	31-50	Dense	51+	Very dense	Relative Consistency	Remarks	0-2	Very soft	3-4	Soft	5-8	Medium stiff	9-15	Stiff	16-30	Very stiff	31+	Hard	Relative Firmness	Remarks	0-4	Very soft	5-8	Soft	9-15	Moderately firm	16-30	Firm	31-50	Very firm	51+	Hard	SCHEMATIC OF SOIL FRACTIONS <table border="1"> <tr> <th>SOIL COMPONENT</th> <th>PARTICLE SIZE RANGE</th> </tr> <tr> <td>Coarse</td> <td>Above 2 in.</td> </tr> <tr> <td>Gravel</td> <td>2 in. to No. 4 sieve</td> </tr> <tr> <td>Sand</td> <td>No. 4 to No. 200 sieve</td> </tr> <tr> <td>Silt</td> <td>No. 200 to No. 425 sieve</td> </tr> <tr> <td>Clay</td> <td>No. 425 to No. 2000 sieve</td> </tr> </table>	SOIL COMPONENT	PARTICLE SIZE RANGE	Coarse	Above 2 in.	Gravel	2 in. to No. 4 sieve	Sand	No. 4 to No. 200 sieve	Silt	No. 200 to No. 425 sieve	Clay	No. 425 to No. 2000 sieve
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- NOTES:
- The area beneath the embankment and extending at least 5.0 feet beyond the embankment shall be scarified to a depth of 12 inches and prewetted to optimum moisture or higher to a depth of about 5 feet, or as deep as practical. Following this deep wetting, the ground surface shall be subjected to at least 20 passes of a 20-ton vibratory roller. The roller shall have a compacted density of at least 95 percent of maximum density as determined in accordance with the ASTM D698 test procedure, at the optimum moisture content.
 - Clearing and grubbing of the foundation area prior to prewetting should include removing all vegetation, debris, and deleterious material.
 - The embankment and cutoff fill materials shall be placed at optimum moisture content or above and compacted to at least 95 percent of the maximum dry density as determined in accordance with the ASTM D698 test procedure. Lifts should have a thickness, when compacted, of no more than 8 inches unless otherwise authorized by the geotechnical engineer.
 - In areas where the finished grade requires a cut greater than 5.0' from the existing ground, the soils shall be scarified and prewetted to a depth of 12 inches. These 12 inches shall have a compacted density of at least 95 percent of maximum density as determined in accordance with ASTM D698.
 - Foundation Treatment shall be paid for on a lump sum basis under Bid Item Subgrade Preparation.
 - Dam Embank to be constructed using existing materials blended to maintain a range of 10 - 55% retained on the # 200 Sieve and Compacted to 95% of maximum density

PROJECT: South Broadway Landfill Detention Dam
 JOB NO. 100-1054 DATE: 5/10/89

LOG OF TEST BORING NO. ③

DEPTH (FEET)	SOIL TYPE	REMARKS	TEST RESULTS
0-10	SAND, predominantly fine, trace silt, nonplastic, tan		
10-15	SAND, predominantly medium, occasional gravel, nonplastic, tan		
15-20	hard		
20-25	SILTY SAND, fine, occasional gravel, nonplastic, tan		
25-30	hard		
30-35	Stopped auger @ 29' 6" Sampler refused @ 29' 11"		

PROJECT: South Broadway Landfill Detention Dam
 JOB NO. 100-1054 DATE: 5/10/89

LOG OF TEST BORING NO. ④

DEPTH (FEET)	SOIL TYPE	REMARKS	TEST RESULTS
0-10	SAND, predominantly fine, trace silt, nonplastic, tan		
10-15	hard		
15-20	SILTY SAND, predominantly fine, occasional gravel, nonplastic, tan		
20-25	hard		
25-30	Stopped auger @ 29' 6" Sampler refused @ 30' 3"		

PROJECT: South Broadway Landfill Detention Dam
 JOB NO. 100-1054 DATE: 5/10/89

LOG OF TEST BORING NO. ②

DEPTH (FEET)	SOIL TYPE	REMARKS	TEST RESULTS
0-10	SAND, predominantly fine to medium, trace silt, nonplastic, tan		
10-15	medium to very dense		
15-20	hard		
20-25	SILTY SAND, predominantly fine, occasional gravel, nonplastic, tan		
25-30	very dense		
30-35	Stopped auger @ 29' 6" Sampler refused @ 30' 4"		

PROJECT: South Broadway Landfill Detention Dam
 JOB NO. 100-1054 DATE: 5/10/89

LOG OF TEST BORING NO. ①

DEPTH (FEET)	SOIL TYPE	REMARKS	TEST RESULTS
0-10	SAND, predominantly fine to medium, trace silt, nonplastic, tan		
10-15	very dense		
15-20	SAND, predominantly medium to coarse, occasional fine gravel, nonplastic, tan		
20-25	very dense		
25-30	hard		
30-35	SILTY SAND, fine, nonplastic, tan		
35-40	Stopped auger @ 29' 6" Sampler refused @ 29' 10"		

GEOLOGIC SECTION AND FOUNDATION TREATMENT

PLATE 3 CROSS SECTION - DOTTED
 CHARLES BRUNING COMPANY
 MADE IN U.S.A.

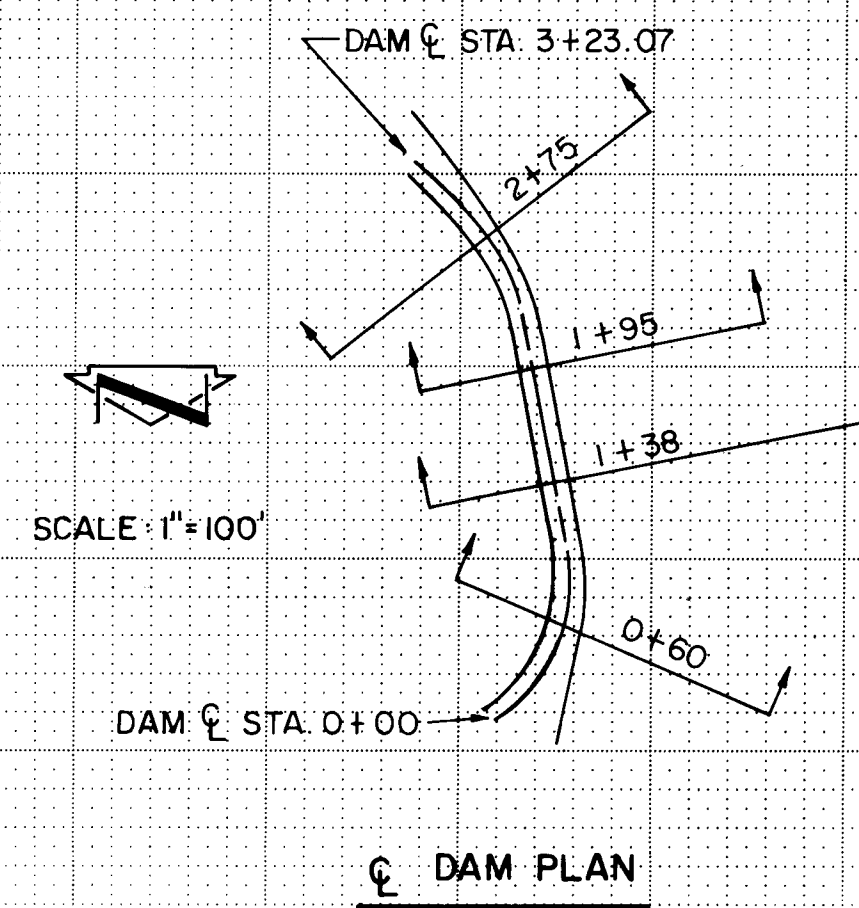
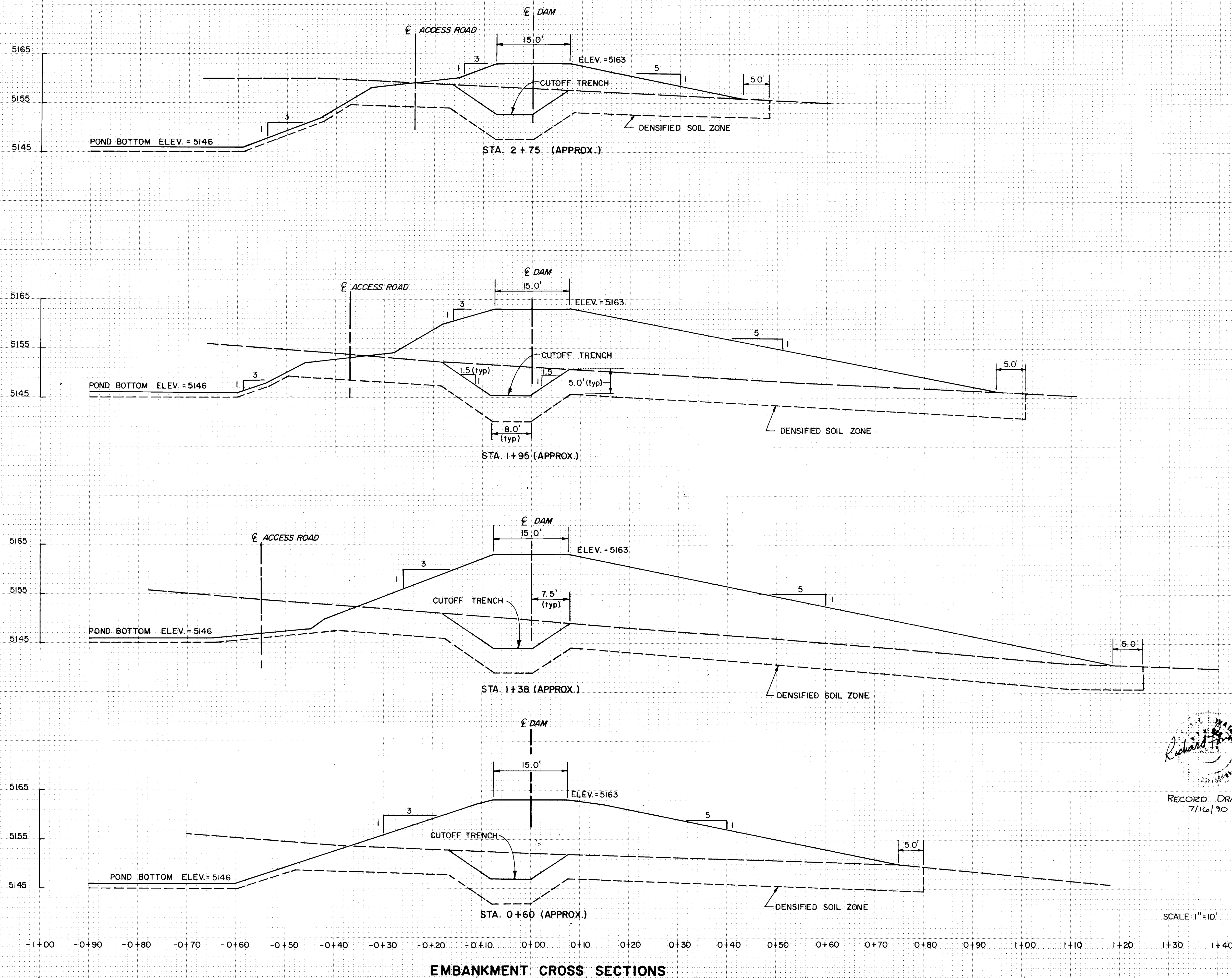
RECORD DRAWING
 7/16/90

C. A. F. & S. Engineers
 161 3rd St. S.W.
 9/1

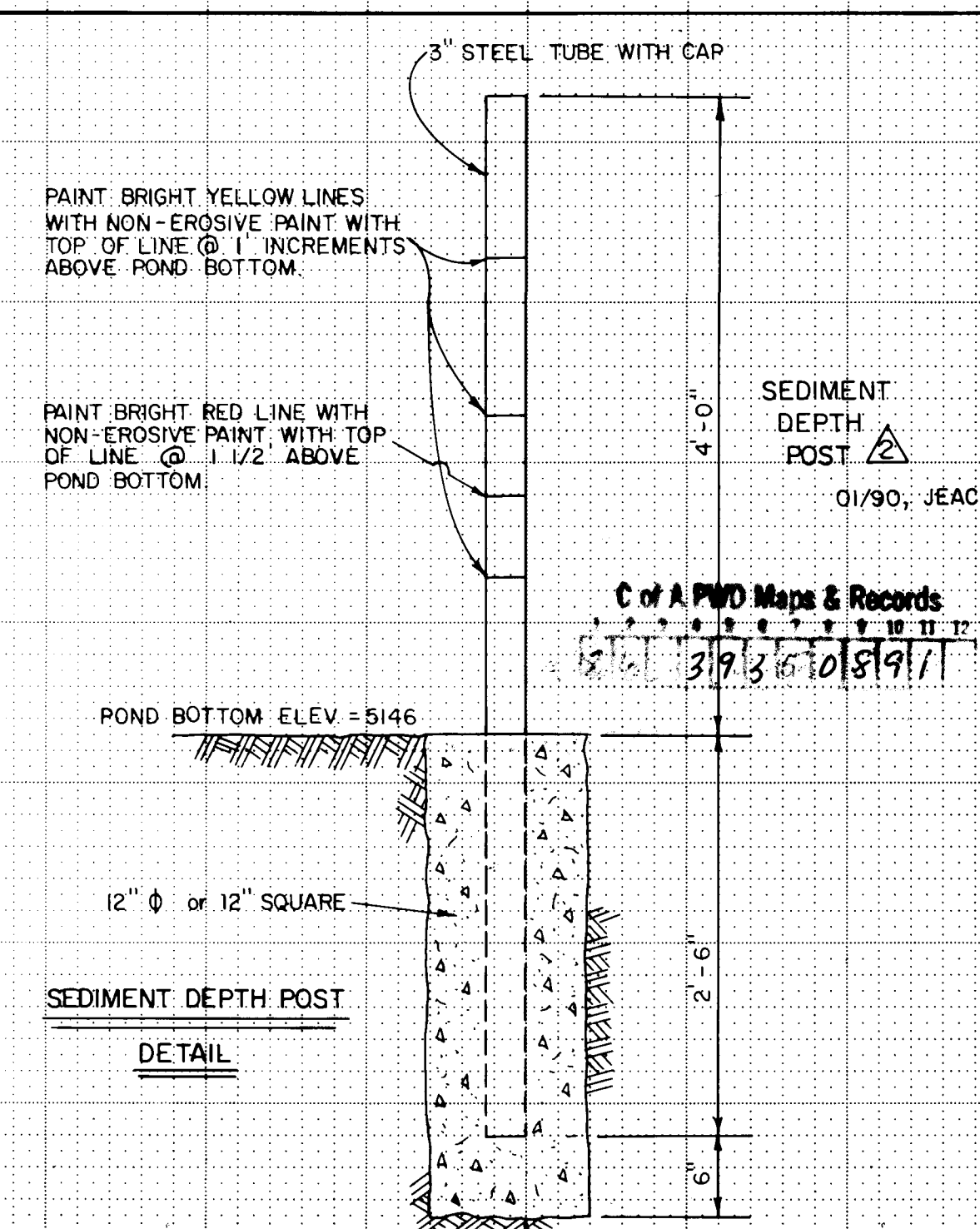
DATE	BY	FINAL SURVEY	NO.
		PLOTTED	
		TEMPLATE	
		NOTE BOOK	
		AREAS CHECKED	

SCANLON AND ASSOCIATES
ENGINEERS PLANNERS

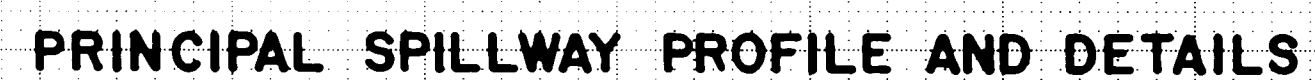
DATE	BY	ORIGINAL SURVEY	NO.
		PLOTTED	
		TEMPLATE	
		NOTE BOOK	
		AREAS CHECKED	



- NOTES:
- All sections are shown perpendicular to the Dam CL.
 - The crest of the Dam is 15' wide except for the ends where the crest is widened for service vehicles to turn.
 - The Access Road is 15' wide perpendicular to the Access Road CL. The road appears wider on these sections because they are skewed with respect to the Road CL.
 - For Foundation Treatment See Sheet 7.
 - All stations are CL Dam.



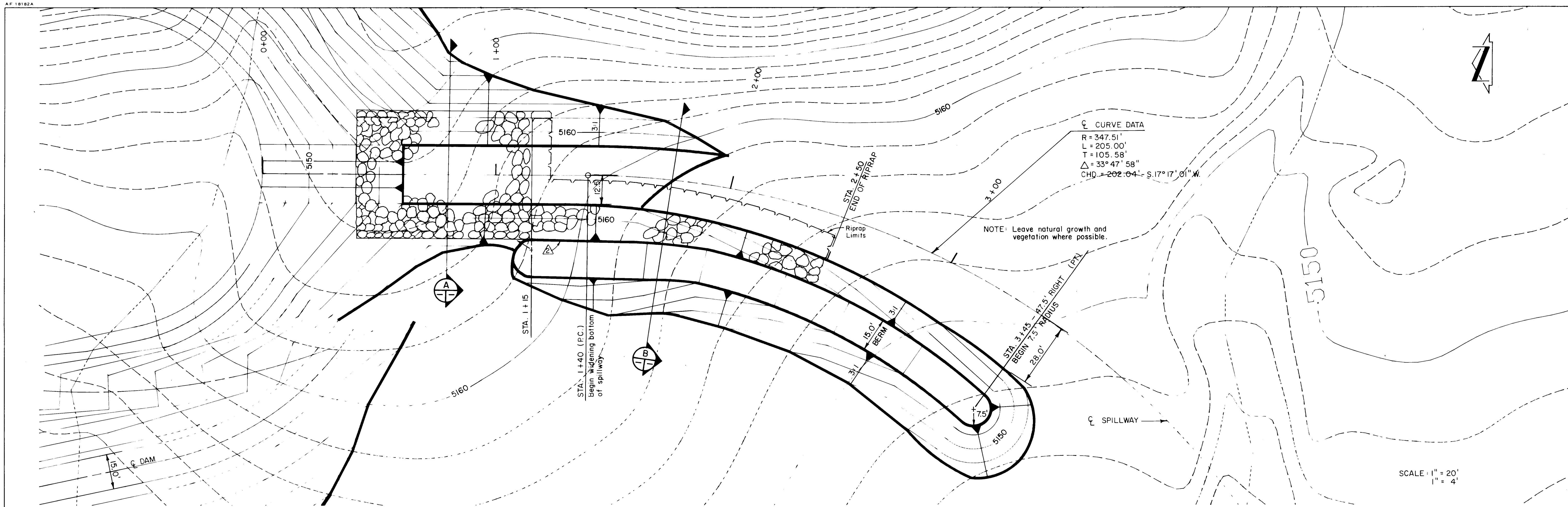
RECORD DRAWING
7/16/90



PLAN	DATE	BY	DATE	BY
SURVEYED				
PLOTTED				
CHECKED				
REVIEWED				
DATE				

SCANLON & ASSOCIATES
CONSULTING ENGINEERS

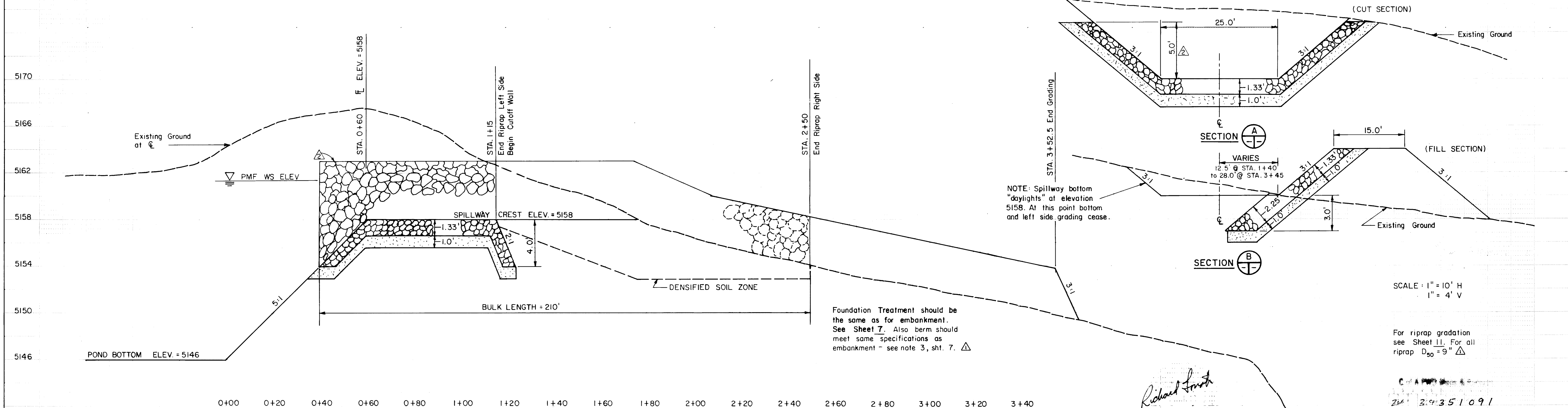
PROFILE	DATE	BY	DATE	BY
SURVEYED				
PLOTTED				
CHECKED				
REVIEWED				
DATE				



℄ CURVE DATA
R = 347.51'
L = 205.00'
T = 105.58'
Δ = 33° 47' 58"
CHD = 202.04' - S.17° 17' 01" W.

NOTE: Leave natural growth and vegetation where possible.

SCALE: 1" = 20'
1" = 4'



NOTE: Spillway bottom "daylights" at elevation 5158. At this point bottom and left side grading cease.

Foundation Treatment should be the same as for embankment. See Sheet 7. Also berm should meet same specifications as embankment - see note 3, sht. 7. Δ

SECTION A

SECTION B

SCALE: 1" = 10' H
1" = 4' V

For riprap gradation see Sheet 11. For all riprap $D_{50} = 9"$ Δ

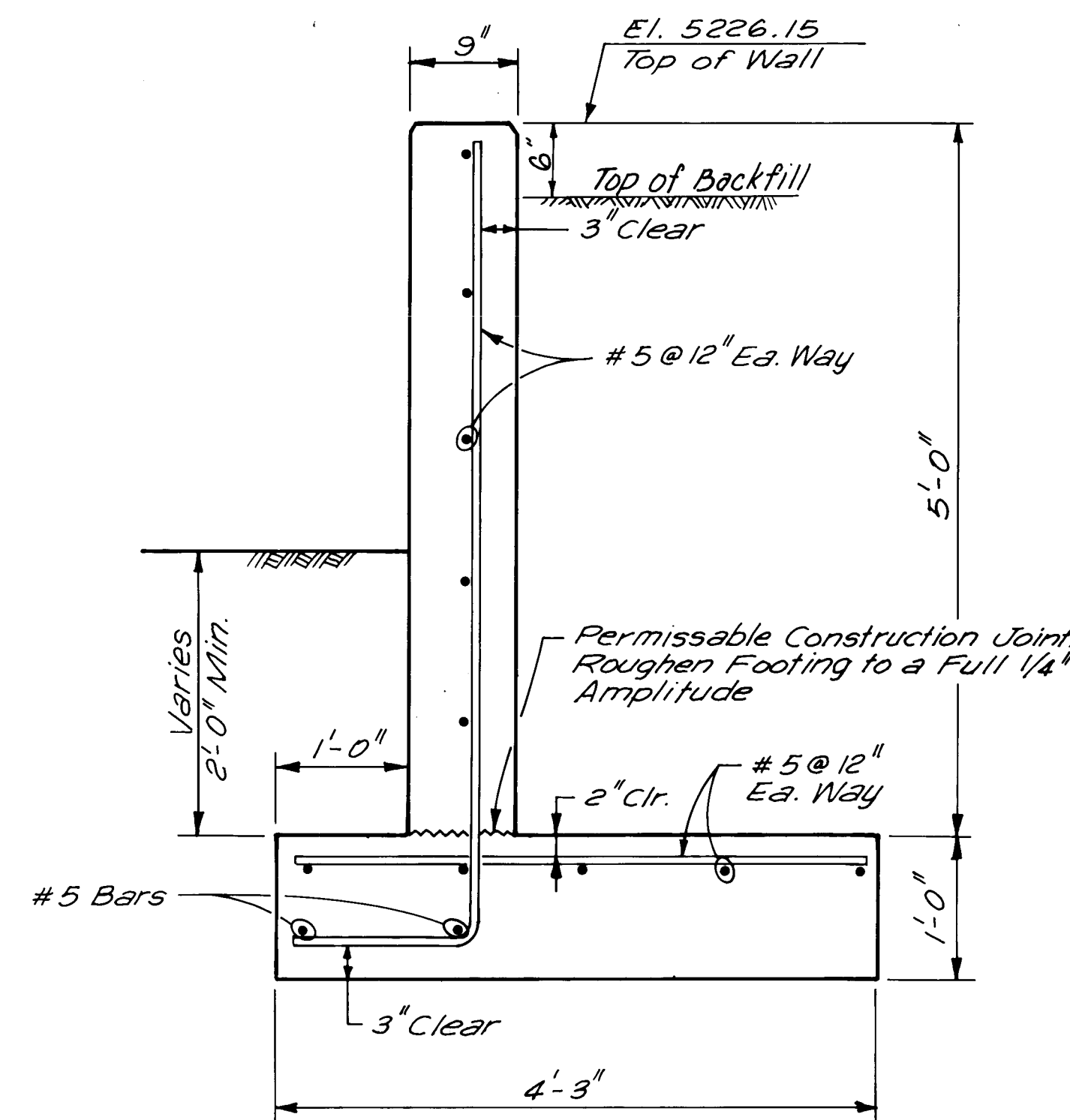
Richard Smith

RECORD DRAWING
7/16/90

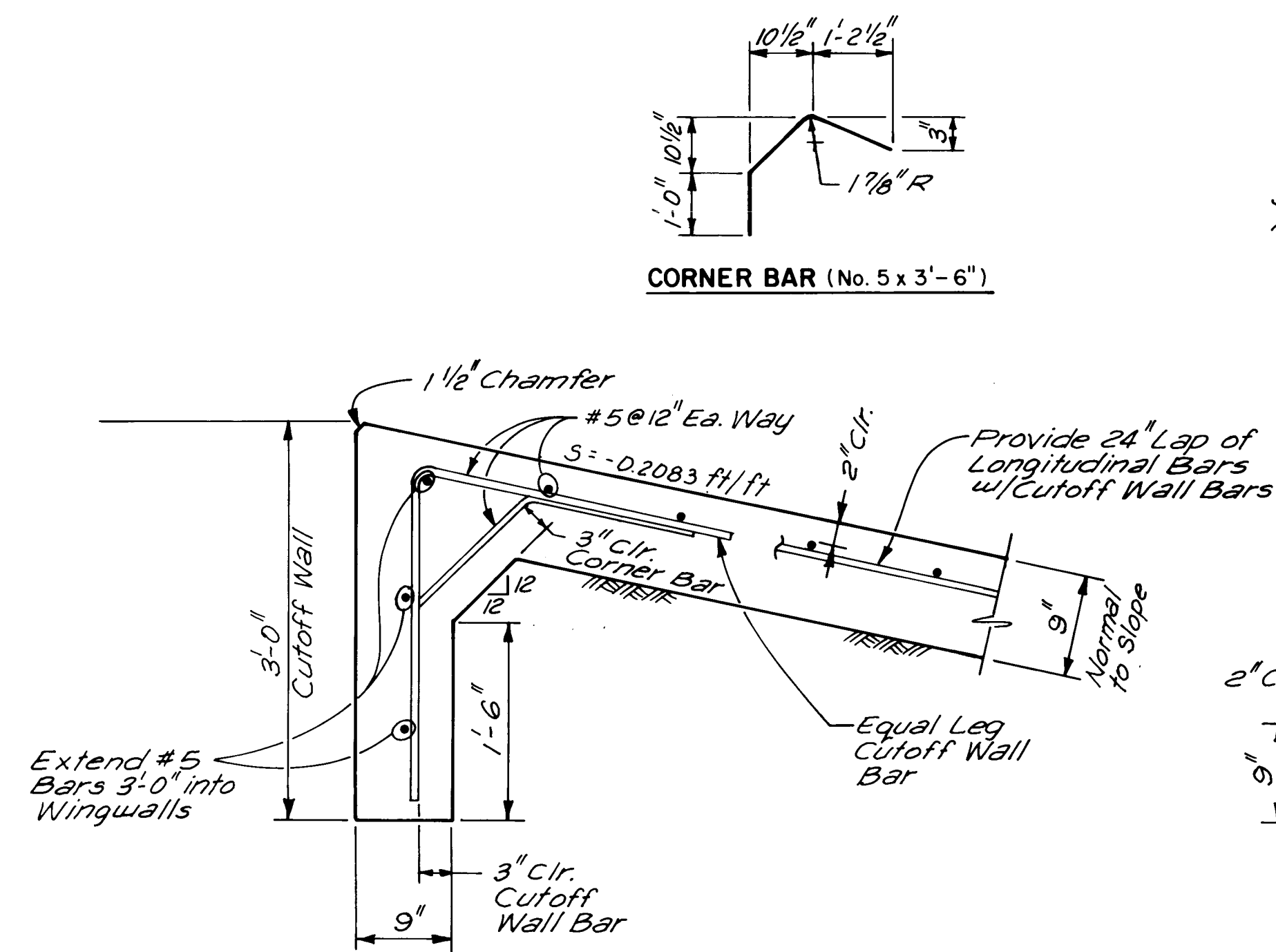
NO.	DATE	REMARKS	BY
1	11/89	NOTE CLARIFICATIONS	JEAC
2	01/90	BERM RAISED	JEAC

EMERGENCY SPILLWAY PLAN AND PROFILE

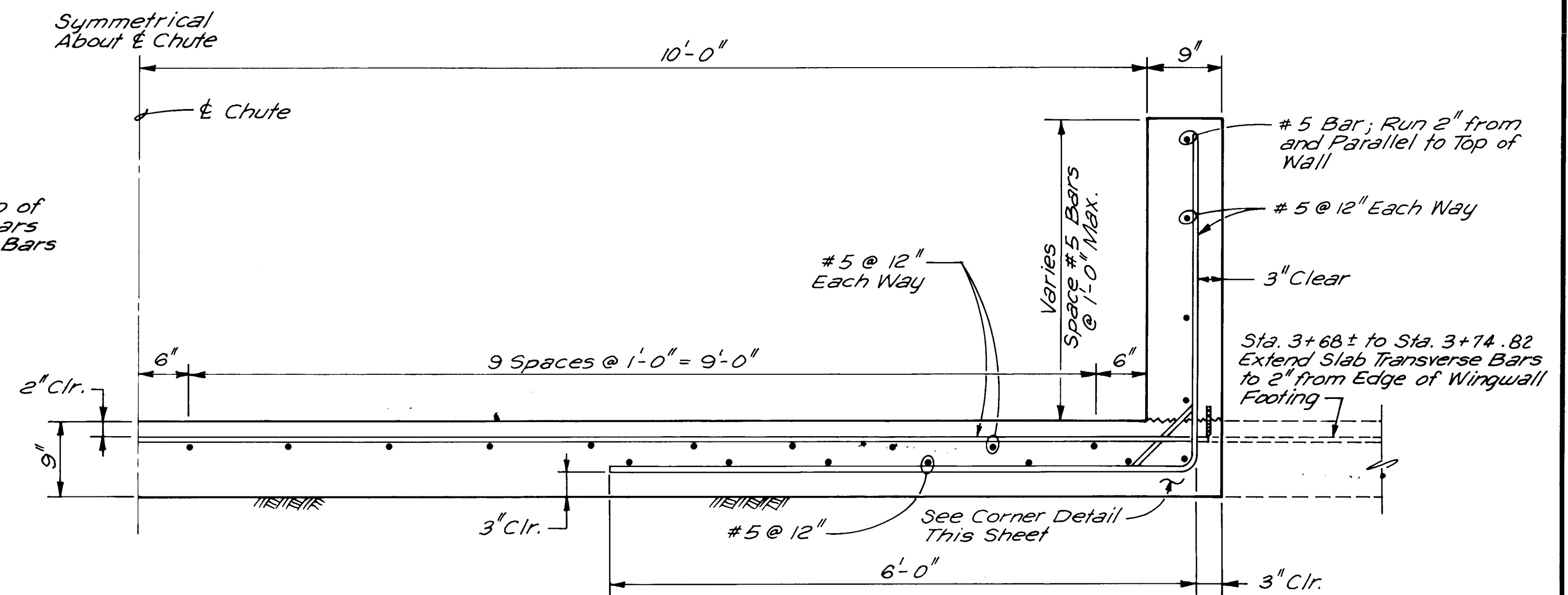




SECTION (BACK OF CHUTE WALL TO END OF WINGWALL) A
1" = 1'-0"

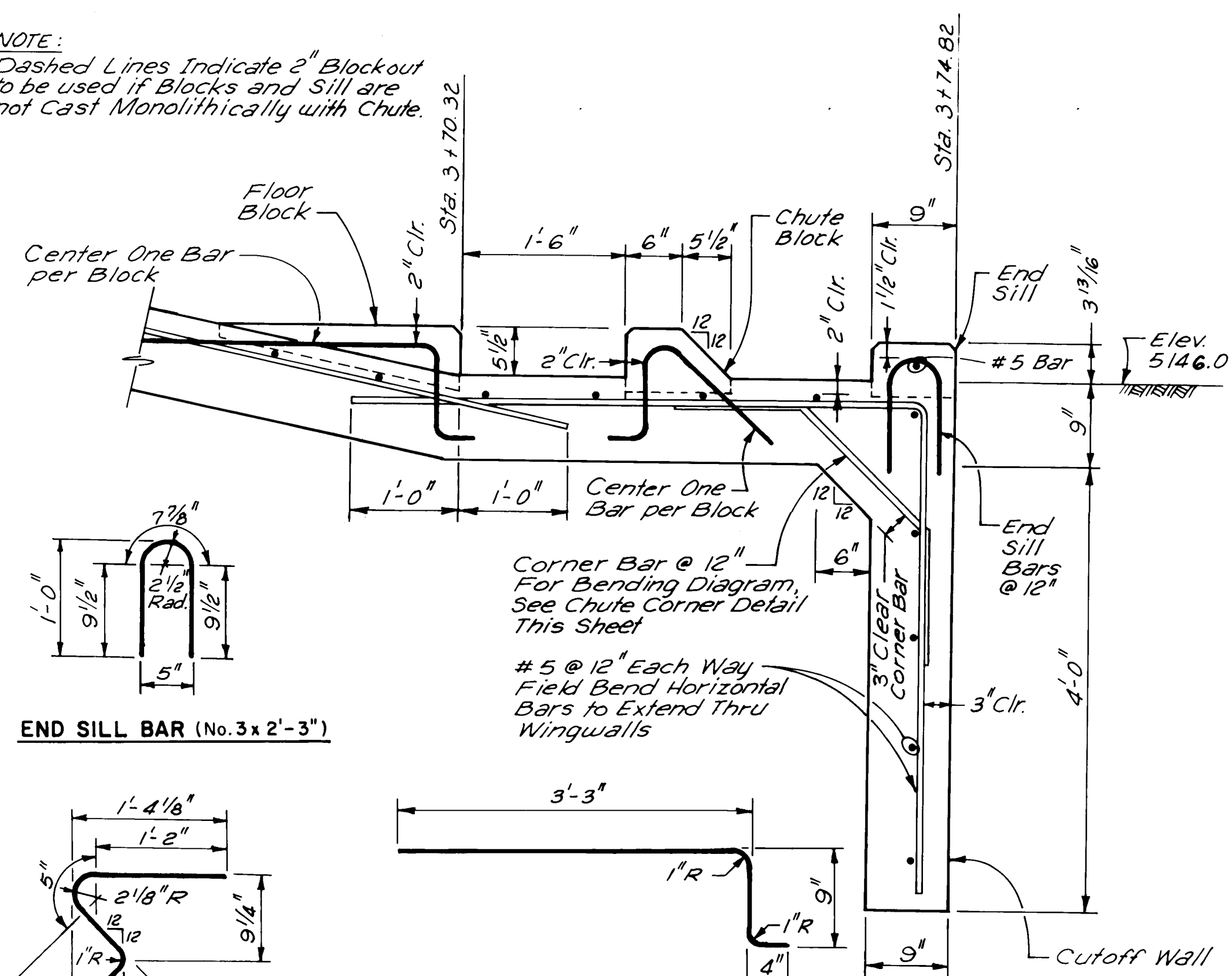


SECTION (BACK TO BACK OF CHUTE WALLS) B
1" = 1'-0"

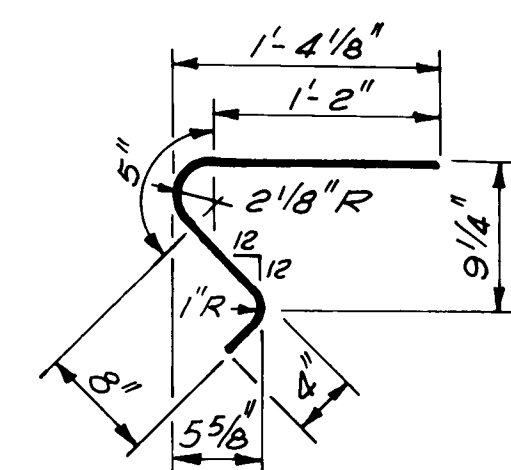


SECTION C
1" = 1'-0"

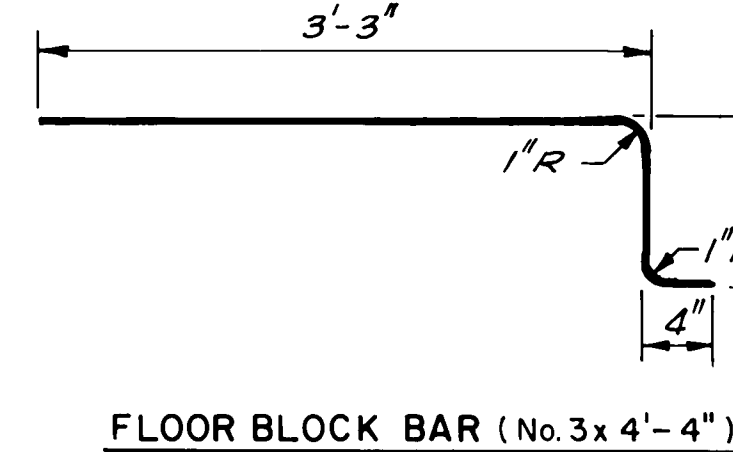
NOTE:
Dashed Lines Indicate 2" Blockout
to be used if Blocks and Sill are
not cast Monolithically with Chute.



END SILL BAR (No. 3 x 2'-3")

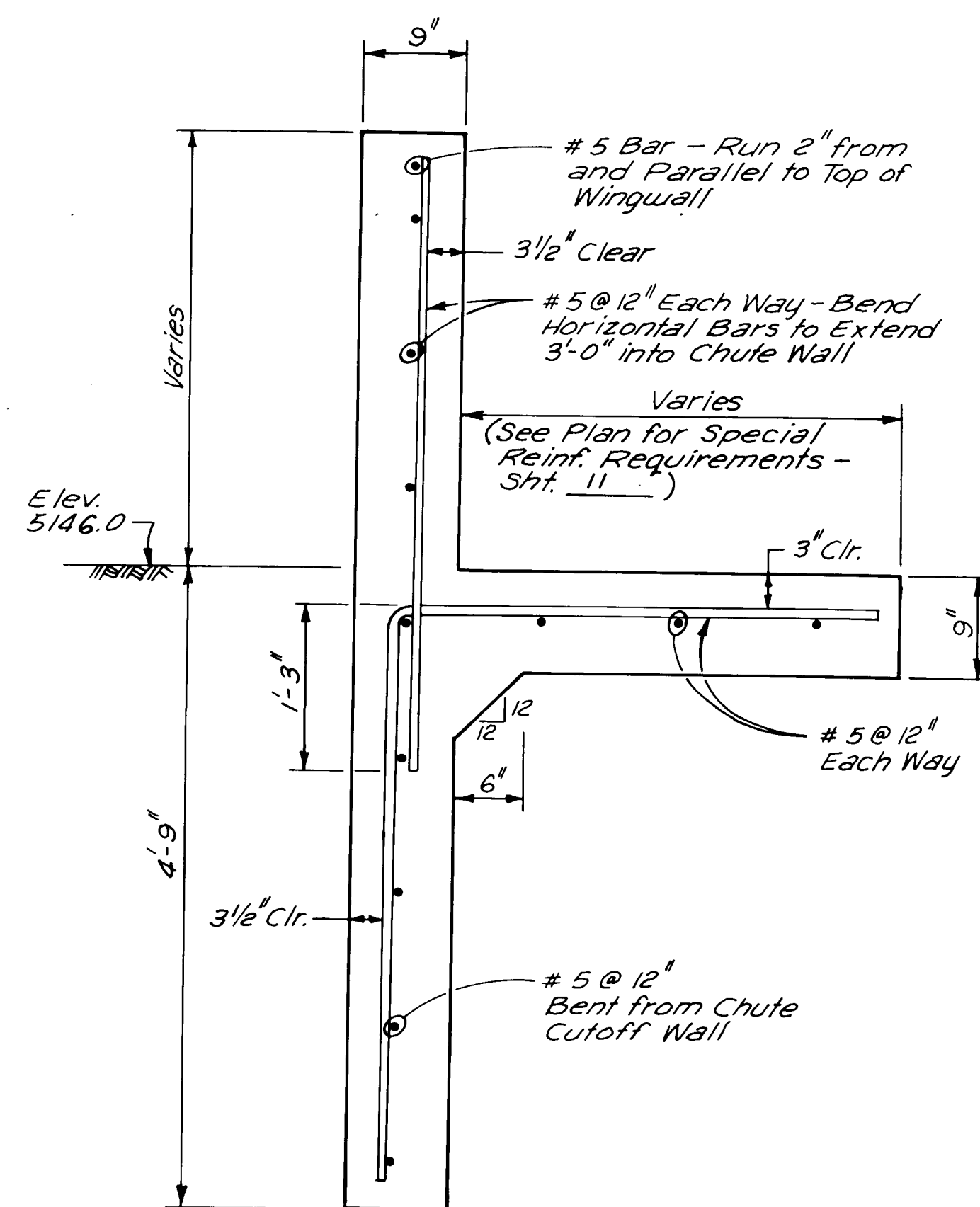


CHUTE BLOCK BAR (No. 3 x 2'-7")

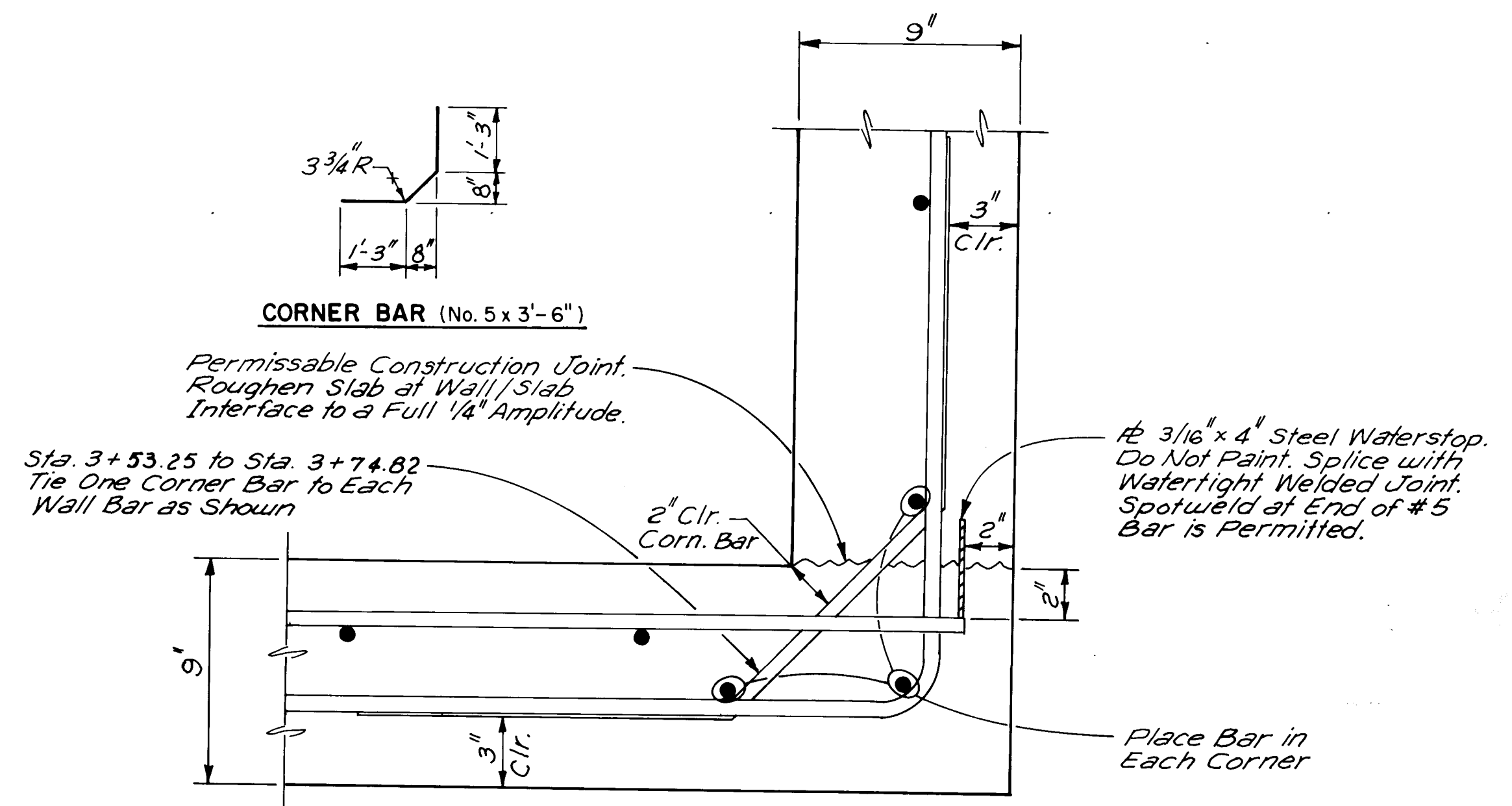


FLOOR BLOCK BAR (No. 3 x 4'-4")

SECTION D
1" = 1'-0"



SECTION E
1" = 1'-0"



CORNER DETAIL (FROM SECTION "C")
1" = 5"

Richard J. Foster
SEAL

RECORD DRAWING
7/16/90

SOUTH BROADWAY LANDFILL
DIVERSION CHANNEL & DETENTION POND

STRUCTURAL SECTIONS

SOLID WASTE DEPARTMENT, CITY OF ALBUQUERQUE

DATE MAY 1989	JOB NO. 86102	SCALE AS SHOWN	DRAWN M.Z.M.	CHECKED J.E.A.C.	SHEET 12 OF 13
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SCANLON & ASSOCIATES
CONSULTING ENGINEERS

