

CONSTRUCTION PLANS

FOR

U.S. NEW MEXICO FEDERAL CREDIT UNION / IRVING BOULEVARD N.W. AND EAGLE RANCH ROAD N.W. STREET IMPROVEMENTS

ALBUQUERQUE, NEW MEXICO

JUNE, 1991

LEGEND

MATERIALS

CONCRETE
RIP-RAP

LINES

SUBDIVISION BOUNDARY
PROPERTY LINE (PLAN)
PROPERTY LINE (SECTION)
CENTERLINE
EASEMENT LINE
MATCH LINE
SECTION CUT LINE

EARTHWORK

CONTOUR LINE
SPOT ELEVATION
PROJECT / PHASE BOUNDARY
SWALE
DIRECTION OF FLOW

MISCELLANEOUS UTILITIES

GAS LINE
UNDERGROUND TELEPHONE
UNDERGROUND ELECTRICAL
STORM DRAIN
STORM DRAIN MANHOLE
STORM DRAIN INLET

SANITARY SEWER

SANITARY SEWER LINE
SANITARY SEWER MANHOLE
SAS SERVICE CONNECTIONS
SAS CAP OR PLUG
ENCASEMENT

WATER

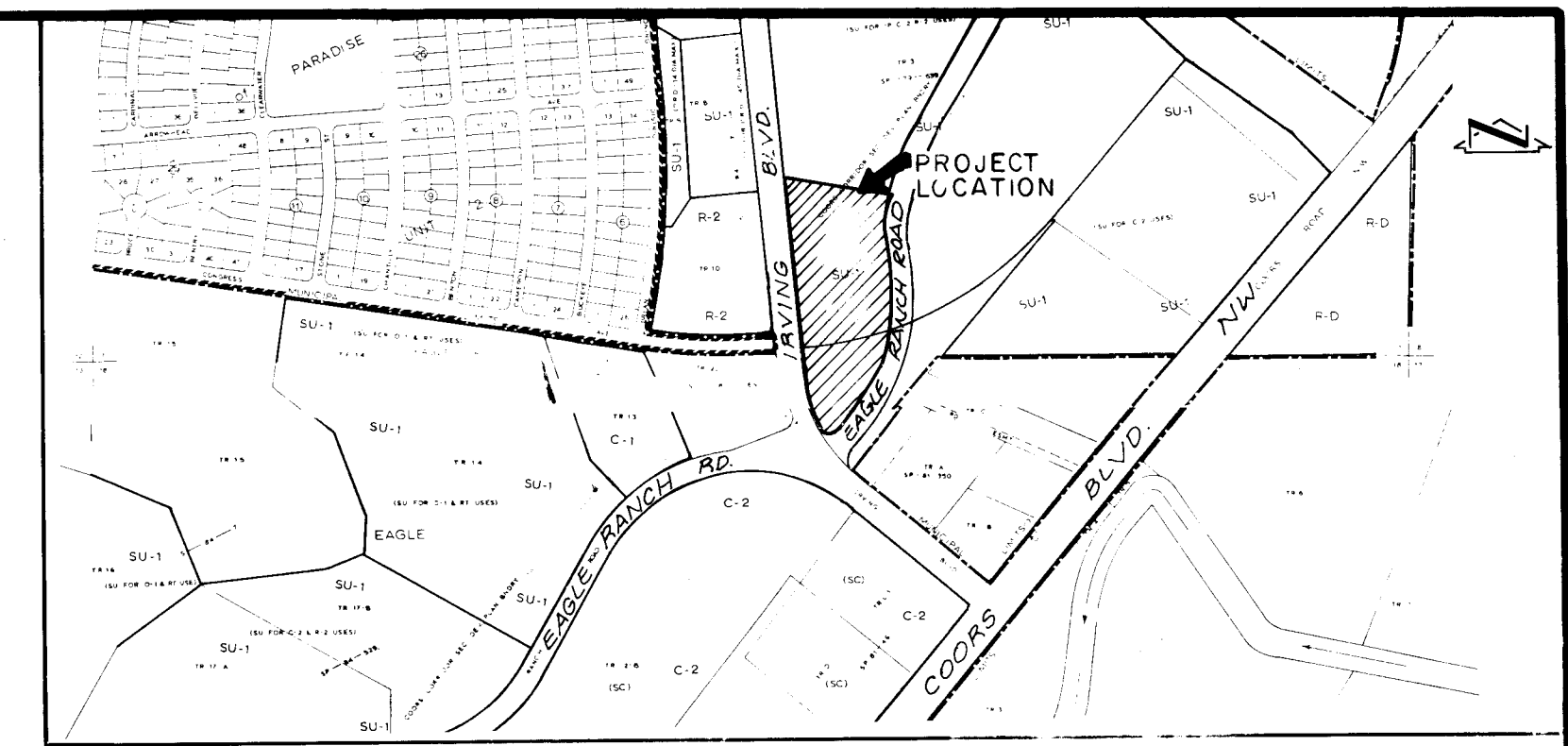
WATER LINE
WATER SERVICE CONNECTIONS
GATE VALVE
FIRE HYDRANT
BUTTERFLY VALVE
REDUCER
WATER PRESSURE ZONE BOUNDARY

WATER FITTINGS

CAPS AND PLUGS
ELBOW
CROSS
TEE

MISCELLANEOUS

CHAINLINK FENCE
FIELD FENCE
COMMON YARD WALL
RETAINING WALL
POWER OR TELEPHONE POLE



VICINITY MAP

B-13 & C-13

SCALE 1" = 500' (APPROX.)

GENERAL NOTES

- ALL WORK DETAILED ON THESE PLANS TO BE PERFORMED UNDER CONTRACT SHALL, EXCEPT AS OTHERWISE STATED OR PROVIDED FOR HEREON, BE CONSTRUCTED IN ACCORDANCE WITH THE CITY OF ALBUQUERQUE STANDARD SPECIFICATIONS - PUBLIC WORKS CONSTRUCTION - 1986. WATER AND SANITARY SEWER IMPROVEMENTS SHALL BE CONSTRUCTED IN ACCORDANCE WITH THE NEW MEXICO UTILITIES, INC. - SPECIFICATIONS FOR WATER AND WASTE WATER FACILITIES MARCH 2, 1990.
- TWO (2) WORKING DAYS PRIOR TO ANY EXCAVATION, CONTRACTOR MUST CONTACT NEW MEXICO ONE CALL SYSTEM, 260-1990 AND NMUI, 898-2661, FOR LOCATION OF EXISTING UTILITIES.
- IF ANY UTILITY LINES, PIPELINES, OR UNDERGROUND UTILITY LINES ARE SHOWN ON THESE DRAWINGS, THEY ARE SHOWN IN AN APPROXIMATE MANNER ONLY, AND SUCH LINES MAY EXIST WHERE NONE ARE SHOWN. IF ANY SUCH EXISTING LINES ARE SHOWN, THE LOCATION IS BASED UPON INFORMATION PROVIDED BY THE OWNER OF SAID UTILITY, AND THE INFORMATION MAY BE INCOMPLETE, OR MAY BE OBSOLETE BY THE TIME CONSTRUCTION COMMENCES. THE ENGINEER HAS CONDUCTED ONLY PRELIMINARY INVESTIGATION OF THE LOCATION, DEPTH, SIZE, OR TYPE OF EXISTING UTILITY LINES, PIPELINES, OR UNDERGROUND UTILITY LINES. THIS INVESTIGATION IS NOT CONCLUSIVE, AND MAY NOT BE COMPLETE, THEREFORE, MAKES NO REPRESENTATION PERTAINING THERETO, AND ASSUMES NO RESPONSIBILITY OR LIABILITY THEREFOR. THE CONTRACTOR SHALL INFORM ITSELF OF THE LOCATION OF ANY UTILITY LINE, PIPELINE, OR UNDERGROUND UTILITY LINE IN OR NEAR THE AREA OF THE WORK IN ADVANCE OF AND DURING EXCAVATION WORK. THE CONTRACTOR IS FULLY RESPONSIBLE FOR ANY AND ALL DAMAGE CAUSED BY ITS FAILURE TO LOCATE, IDENTIFY AND PRESERVE ANY AND ALL EXISTING UTILITIES, PIPELINES, AND UNDERGROUND UTILITY LINES. IN PLANNING AND CONDUCTING EXCAVATION, THE CONTRACTOR SHALL COMPLY WITH STATE STATUTES, MUNICIPAL AND LOCAL ORDINANCES, RULES AND REGULATIONS, IF ANY, PERTAINING TO THE LOCATION OF THESE LINES AND FACILITIES.
- SHOULD A CONFLICT EXIST BETWEEN THESE PLANS AND ACTUAL FIELD CONDITIONS, THE CONTRACTOR SHALL PROMPTLY NOTIFY THE ENGINEER IN WRITING SO THAT THE CONFLICT CAN BE RESOLVED WITH A MINIMUM AMOUNT OF DELAY FOR ALL PARTIES.
- THE CONTRACTOR SHALL MAINTAIN ACCESS TO ADJACENT PROPERTIES DURING CONSTRUCTION.
- ALL WORK ON THIS PROJECT SHALL BE PERFORMED IN ACCORDANCE WITH APPLICABLE FEDERAL, STATE AND LOCAL LAWS, RULES AND REGULATIONS CONCERNING SAFETY AND HEALTH.
- CONTRACTOR SHALL COMPLY WITH SECTION 19 OF THE "STANDARD SPECIFICATIONS".
- ALL UTILITIES AND UTILITY SERVICE LINES SHALL BE INSTALLED PRIOR TO PAVING.
- BACKFILL COMPACTION SHALL BE ACCORDING TO SPECIFIED STREET USE.
- TACK COAT REQUIREMENTS SHALL BE DETERMINED DURING CONSTRUCTION BY THE PROJECT ENGINEER.
- SIDEWALKS AND WHEELCHAIR RAMPS WITHIN THE CURB RETURNS SHALL BE CONSTRUCTED WHEREVER A NEW CURB RETURN IS CONSTRUCTED.
- IF CURB IS DEEPENED FOR A DRIVEPAD OR A HANDICAP RAMP, THE DRIVEPAD OR RAMP SHALL BE CONSTRUCTED PRIOR TO ACCEPTANCE OF THE CURB AND GUTTER.
- ALL STORM DRAINAGE FACILITIES SHALL BE COMPLETED PRIOR TO FINAL ACCEPTANCE.
- CONTRACTOR SHALL COORDINATE WITH NEW MEXICO UTILITIES INC. (898-2661) FOR THE LOCATION OF EXISTING UTILITIES NOT LESS THAN THREE (3) WORKING DAYS IN ADVANCE OF ANY WORK THAT MAY AFFECT THE EXISTING PUBLIC WATER UTILITIES. ONLY WATER SYSTEM DIVISION PERSONNEL SHALL OPERATE EXISTING VALVES. REFER TO SECTION 18 OF THE SPECIFICATION.
- CONTRACTOR SHALL NOTIFY THE ENGINEER NOT LESS THAN SEVEN (7) DAYS PRIOR TO STARTING WORK IN ORDER THAT THE ENGINEER MAY TAKE NECESSARY MEASURES TO INSURE THE PRESERVATION OF SURVEY MONUMENTS. CONTRACTOR SHALL NOT DISTURB PERMANENT SURVEY MONUMENTS WITHOUT THE CONSENT OF THE ENGINEER AND SHALL NOTIFY THE ENGINEER AND BEAR THE EXPENSE OF REPLACING ANY THAT MAY BE DISTURBED WITHOUT PERMISSION. REPLACEMENT SHALL BE DONE ONLY BY THE ENGINEER. WHEN A CHANGE IS MADE IN THE FINISHED ELEVATION OF THE PAVEMENT OF ANY ROADWAY IN WHICH A PERMANENT SURVEY MONUMENT IS LOCATED, CONTRACTOR SHALL, AT HIS OWN EXPENSE, ADJUST THE MONUMENT COVER TO THE NEW GRADE UNLESS OTHERWISE SPECIFIED. REFER TO SECTION 4.4 OF THE SPECIFICATIONS.
- THREE (3) WORKING DAYS PRIOR TO BEGINNING CONSTRUCTION THE CONTRACTOR SHALL SUBMIT TO THE CONSTRUCTION CO-ORDINATION DIVISION A DETAILED CONSTRUCTION SCHEDULE. TWO (2) WORKING DAYS PRIOR TO CONSTRUCTION THE CONTRACTOR SHALL OBTAIN A BARRICADING PERMIT FROM THE CONSTRUCTION CO-ORDINATION DIVISION. CONTRACTOR SHALL NOTIFY BARRICADE ENGINEER (768-2551) PRIOR TO OCCUPYING AN INTERSECTION. CONTRACTOR MUST REFER TO SECTION 19 OF THE STANDARD SPECIFICATION FOR TRAFFIC CONTROL. REFER TO SECTION 19 OF THE SPECIFICATIONS.
- ALL STREET STRIPING ALTERED OR DESTROYED SHALL BE REPLACED IN KIND BY CONTRACTOR TO LOCATION AND IN KIND AS EXISTING OR AS INDICATED BY THIS PLAN SET.

INDEX OF DRAWINGS

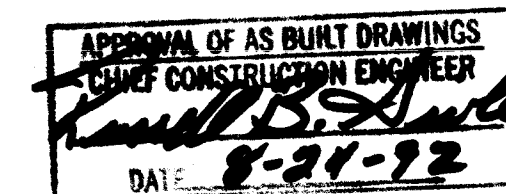
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- HYDROLOGIC AND HYDRAULIC CALCULATIONS
- SITE PLAN
- PAVING AND DRAINAGE IMPROVEMENTS PLAN & PROFILE
- IRVING BOULEVARD N.W.
- EAGLE RANCH DRIVE N.W.
- SECTIONS & DETAILS
- WATER AND SANITARY SEWER IMPROVEMENTS PLAN & PROFILE
- EAGLE RANCH DRIVE N.W.
- TRAFFIC CONTROL DETAILS
- STRIPING AND EROSION CONTROL PLANS

RECORD DRAWING

I, Jeffrey G. Mortensen, Registered Professional Engineer in the State of New Mexico, do hereby certify that this "as-built" information was obtained by me or under my supervision and represents the "as-built" conditions of this project, and is true and correct to the best of my knowledge and belief. All vertical and horizontal dimensions should be field verified prior to use on future projects.



26 4296.90 1900192



REV	SHEETS	CITY ENGINEER	DATE	USER DEPARTMENT	DATE	USER DEPARTMENT	DATE

APPROVAL OF REVISIONS

07-15-91

JEFF MORTENSEN & ASSOCIATES, INC.
800-B MIDWAY PARK BLVD N.E.
ALBUQUERQUE, NEW MEXICO 87105
(505) 345-4296

APPROVED FOR CONSTRUCTION

10-2-91

C.E.

4296.90

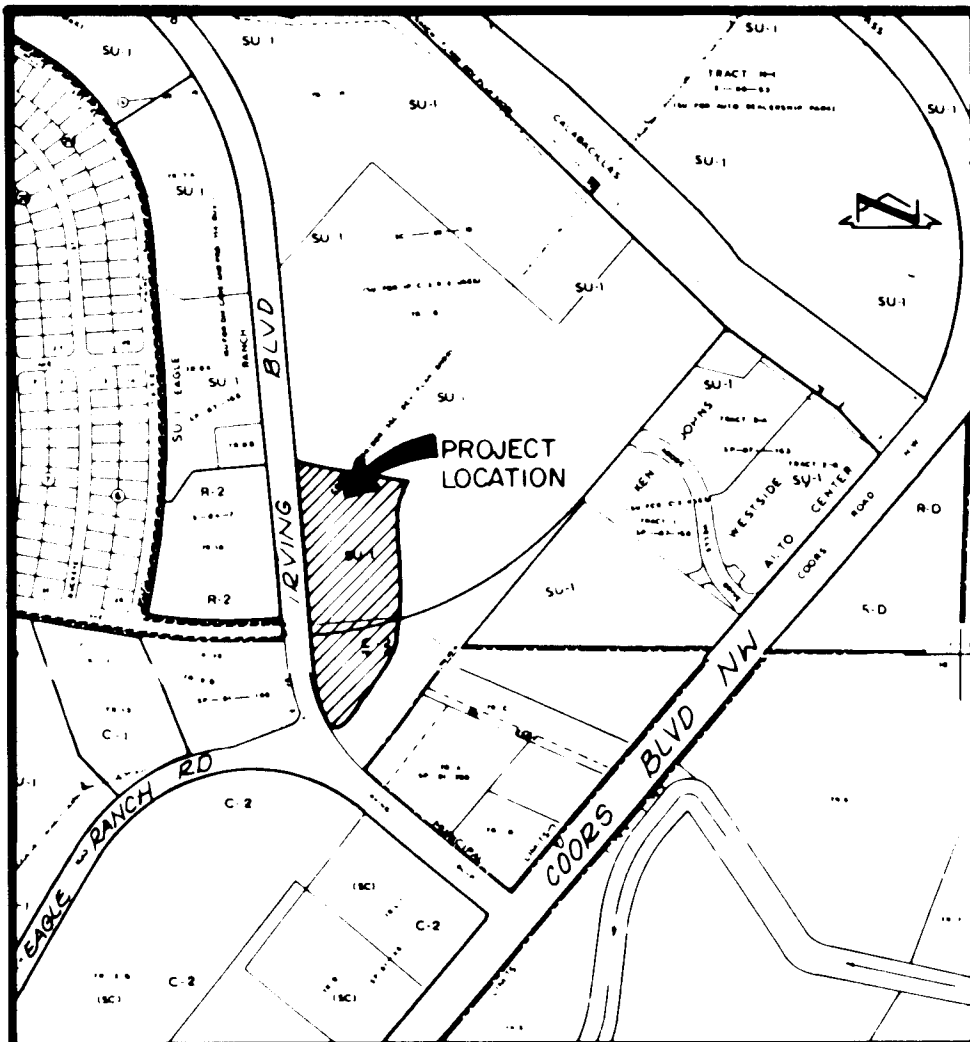
SHEET 1 OF 11

CAUTION:

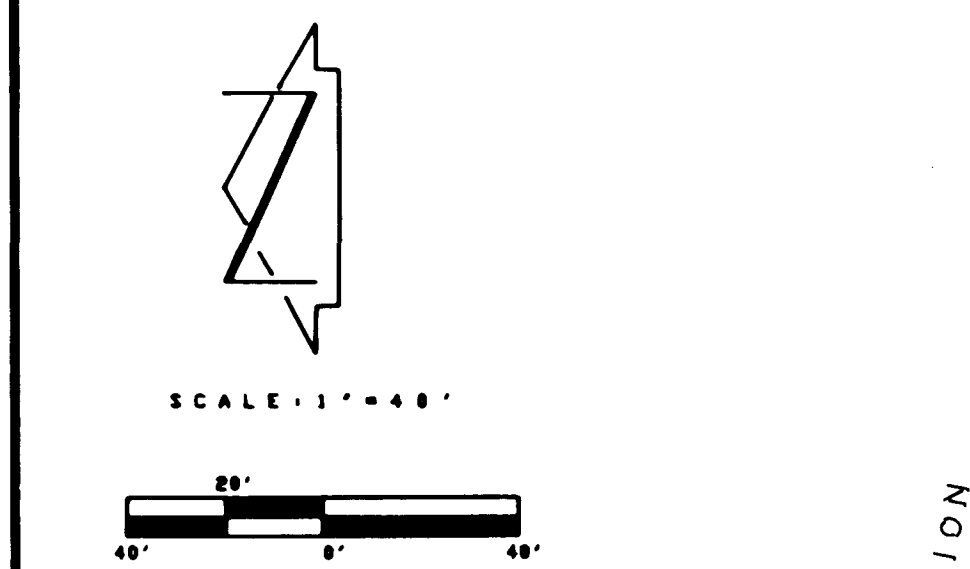
THESE DRAWINGS DO NOT INCLUDE NECESSARY COMPONENTS FOR CONSTRUCTION SAFETY WHICH SHALL REMAIN THE RESPONSIBILITY OF THE CONTRACTOR.

SCANNED BY LASON

JMA 900765



VICINITY MAP
B-138 C-13
NTS

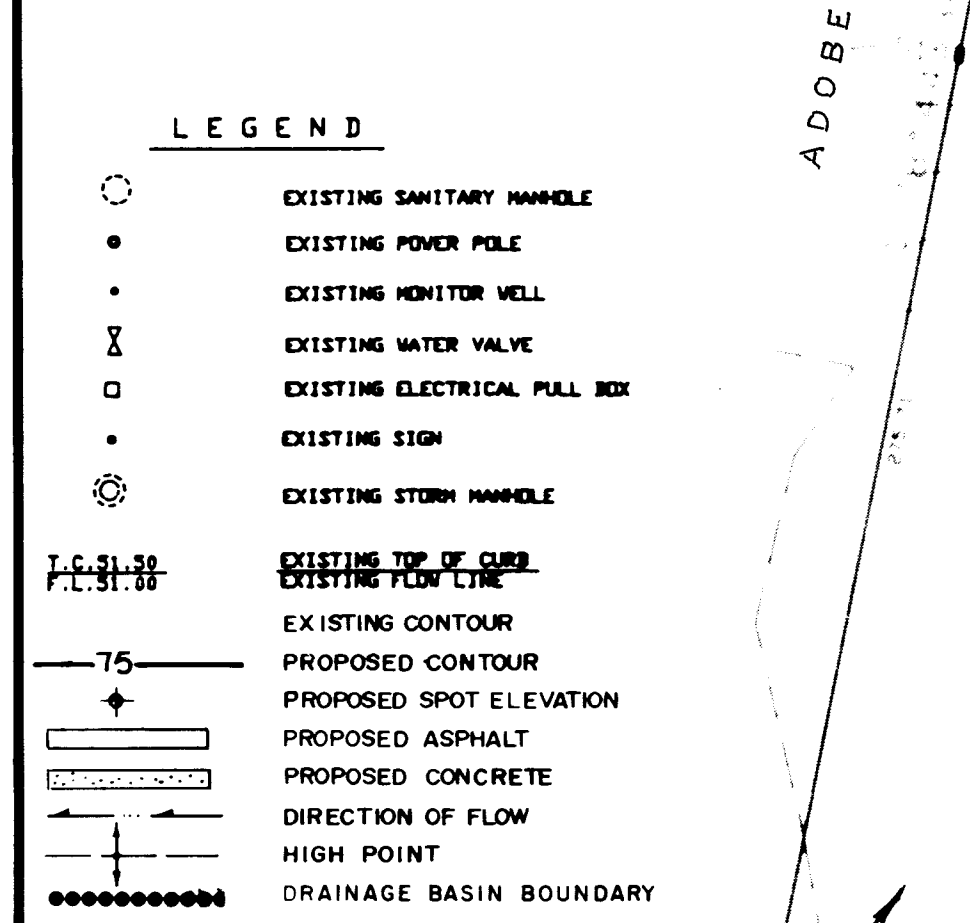


LEGAL DESCRIPTION
TRACT D, ADOBE WELLS SUBDIVISION

PROJECT BENCHMARK
CITY OF ALBUQUERQUE BENCHMARK 5-813, AN ACS 1 3/4" ALUMINUM BISK STAMPED "ACS BM, 5-813", LOCATED ON THE EAST SIDE OF IRVING BLVD. N.W. AT THE P.C. APPROXIMATELY 0.57 OF A MILE N.W. OF CORNER 89, N.W. AND IS ON TOP OF THE CURB.
ELEVATION = 5097.71 FEET (N.S.L.S.)

T.B.M.
TOP OF CURB LOCATED ON THE EAST SIDE OF IRVING BOULEVARD N.W. AT THE NORTHWEST CORNER OF SITE AS SHOWN BELOW.
ELEVATION = 5197.33 FEET (N.S.L.S.)

LEGEND
EXISTING SANITARY MANHOLE
EXISTING POWER POLE
EXISTING MONITOR WELL
EXISTING WATER VALVE
EXISTING ELECTRICAL PULL BOX
EXISTING SIGN
EXISTING STORM MANHOLE
EXISTING TOP OF CURB
EXISTING FLOW LINE
EXISTING CONTOUR
PROPOSED CONTOUR
PROPOSED SPOT ELEVATION
PROPOSED ASPHALT
PROPOSED CONCRETE
DIRECTION OF FLOW
HIGH POINT
DRAINAGE BASIN BOUNDARY



IRVING BOULEVARD N.W.

EAGLE RANCH ROAD N.W.

ADOBE WELLS SUBDIVISION

PROPOSED BUILDING
FF = 5068.00

LANDSCAPING
TREES, SHRUBS, BLUE
GRASS (BUFFALO GRASS)

STREET IMPROVEMENTS BY
CITY WORK ORDER

WINDROW DUNE, TYP.

GRADE WINDROW PER
EROSION CONTROL PLAN

LIMITS OF GRADING

GRADE @ 3:1 MAX.

SEDIMENTATION BASIN

FOR EROSION CONTROL PLAN
SEE CITY PROJECT # 4296.90
SHEET 11 OF 11

FOR HYDROLOGIC AND HYDRAULIC CALCULATIONS
SEE CITY PROJECT # 4296.90
SHEET 4 OF 11

FOR EAGLE RANCH IMPROVEMENTS
SEE CITY PROJECT # 4296.90
SHEET 7 OF 11

RECORD DRAWING

CONSTRUCTION NOTES:

1. TWO (2) WORKING DAYS PRIOR TO ANY EXCAVATION, CONTRACTOR MUST CONTACT NEW MEXICO ONE CALL SERVICE 260-1990 AND NMUI, 898-2661 FOR LOCATION OF EXISTING UTILITIES.

2. PRIOR TO CONSTRUCTION, THE CONTRACTOR SHALL EXCAVATE AND VERIFY THE HORIZONTAL AND VERTICAL LOCATION OF ALL POTENTIAL OBSTRUCTIONS. SHOULD A CONFLICT EXIST, THE CONTRACTOR SHALL NOTIFY THE ENGINEER IN WRITING SO THAT THE CONFLICT CAN BE RESOLVED WITH A MINIMUM AMOUNT OF DELAY.

3. ALL WORK ON THIS PROJECT SHALL BE PERFORMED IN ACCORDANCE WITH APPLICABLE FEDERAL, STATE AND LOCAL LAWS, RULES AND REGULATIONS CONCERNING CONSTRUCTION SAFETY AND HEALTH.

4. ALL CONSTRUCTION WITHIN PUBLIC RIGHT-OF-WAY SHALL BE PERFORMED IN ACCORDANCE WITH APPLICABLE CITY OF ALBUQUERQUE STANDARDS AND PROCEDURES.

5. IF ANY UTILITY LINES, PIPELINES, OR UNDERGROUND UTILITY LINES ARE SHOWN ON THESE DRAWINGS, THEY ARE SHOWN IN AN APPROXIMATE MANNER ONLY, AND SUCH LINES MAY EXIST WHERE NONE ARE SHOWN. IF ANY SUCH EXISTING LINES ARE SHOWN, THE LOCATION IS BASED UPON INFORMATION PROVIDED BY THE OWNER OF SAID UTILITY, AND THE INFORMATION MAY BE INCOMPLETE, OR MAY BE OBSOLETE BY THE TIME CONSTRUCTION COMMENCES. THE ENGINEER HAS UNDERTAKEN NO FIELD VERIFICATION OF THE LOCATION, DEPTH, SIZE, OR TYPE OF EXISTING UTILITY LINES, PIPELINES, OR UNDERGROUND UTILITY LINES. MAKES NO REPRESENTATION PERTAINING THERETO, AND ASSUMES NO RESPONSIBILITY OR LIABILITY THEREFOR. THE CONTRACTOR SHALL INFORM ITSELF OF THE LOCATION OF ANY UTILITY LINE, PIPELINE, OR UNDERGROUND UTILITY LINE IN OR NEAR THE AREA OF THE WORK IN ADVANCE OF AND DURING EXCAVATION WORK. THE CONTRACTOR IS FULLY RESPONSIBLE FOR ANY AND ALL DAMAGE CAUSED BY ITS FAILURE TO LOCATE, IDENTIFY AND PRESERVE ANY AND ALL EXISTING UTILITIES, PIPELINES, AND UNDERGROUND UTILITY LINES. IN PLANNING AND CONDUCTING EXCAVATION, THE CONTRACTOR SHALL COMPLY WITH STATE STATUTES, MUNICIPAL AND LOCAL ORDINANCES, RULES AND REGULATIONS, IF ANY, PERTAINING TO THE LOCATION OF THESE LINES AND FACILITIES.

6. THE DESIGN OF PLANTERS AND LANDSCAPED AREAS IS NOT PART OF THIS PLAN. ALL PLANTERS AND LANDSCAPED AREAS ADJACENT TO THE BUILDING(S) SHALL BE PROVIDED WITH POSITIVE DRAINAGE TO AVOID ANY PONDING ADJACENT TO THE STRUCTURE. FOR CONSTRUCTION DETAILS, REFER TO LANDSCAPING PLAN.

EROSION CONTROL MEASURES

1. THE CONTRACTOR SHALL ENSURE THAT NO SOIL ERODES FROM THE SITE INTO PUBLIC RIGHT-OF-WAY OR ONTO PRIVATE PROPERTY. THIS CAN BE ACHIEVED BY CONSTRUCTING TEMPORARY BERMS AT THE PROPERTY LINES AND WETTING THE SOIL TO KEEP IT FROM BLOWING.

2. THE CONTRACTOR SHALL PROMPTLY CLEAN UP ANY MATERIAL EXCAVATED WITHIN THE PUBLIC RIGHT-OF-WAY SO THAT THE EXCAVATED MATERIAL IS NOT SUSCEPTIBLE TO BEING WASHED DOWN THE STREET.

3. THE CONTRACTOR SHALL SECURE "TOPSOIL DISTURBANCE PERMIT" PRIOR TO BEGINNING CONSTRUCTION.

DESIGNED BY V.S.F./J.P.K.
DRAWN BY S.G.H.
APPROVED BY J.G.M.

NO DATE BY
2/1/91 JPK
3-92 DKM

REVISIONS
ADD SEDIMENTATION BASIN & WINDROWS TO EROSION CONTROL PLAN
AS-BUILT

JOB NO 900754
DATE 04/91
SHEET 3 OF 11

JEFF MORTENSEN & ASSOCIATES, INC.
6010-B MIDWAY PARK BLVD. N.E.
ALBUQUERQUE, NEW MEXICO 87109
ENGINEERS (505) 345-4250

GRADING AND DRAINAGE PLAN
SHEET 3 OF 11
CITY PROJECT # 4296.90
U.S. NEW MEXICO FEDERAL CREDIT UNION

DRAINAGE PLAN

The following items concerning the U.S. New Mexico Federal Credit Union Drainage Plan are contained as part of this submittal:

Description	Sheet No.
1. Vicinity Map	3
2. Grading Plan	3
3. Nomographs and Calculations	4

As shown by the Vicinity Map, the site is located at the northeast corner of the intersection of Irving Boulevard N.W. and Eagle Ranch Road N.W. At present, the site is undeveloped. The existing 60" RCP storm drain through the site was designed and built by C.O.A. Project No. 2204, dated August 27, 1985. Accompanying Project No. 2204 is the "Design Report for Eagle Ranch Storm Drain", dated January 1985 by Easterling & Associates. Eagle Ranch Road paving and drainage was designed and approved for construction in June of 1986 as C.O.A. Project No. 2774. A revised Drainage Management Plan for the Adobe Wells Subdivision prepared by Bohannon Huston Inc., dated 7/30/85 and 9/14/88, has been approved which encompasses the subject site. A Conceptual Grading & Drainage Plan was prepared by Jeff Mortensen and Associates in December of 1990 and updated in April 1991 for the site. The current Plan is consistent with these previously approved Plans.

The Grading Plan shows 1) existing topography indicated by spot elevations and contours at 1'0" intervals, 2) proposed grades indicated by spot elevations, 3) the proposed drainage scheme indicated by flow arrows, 4) the limit and character of the existing improvements, and 5) the limit and character of the proposed improvements. As shown by this plan, the site presently drains in a southeasterly direction to Eagle Ranch Road N.W. An existing 36" CMP collects runoff from a portion of Basin A and discharges it into an existing 60" RCP running through the site. These drainage improvements were constructed by City Project No. 2204.

The proposed grading separates the site into two drainage sub-basins. Basin A-2 contains that portion of the site which will be developed with buildings, pavement, and landscaping. The runoff from A-2 will be collected at a low point in the new parking lot and be discharged into a new storm inlet and storm drain which will connect to the existing 60" storm drain through the site. Basin A-1 contains that portion of the site which will remain primarily undeveloped at this time. The runoff from A-1 will be collected at a low point in the new Eagle Ranch Road N.W. pavement and will discharge into a new storm inlet and storm drain which will discharge into the existing 60" storm drain. A new manhole and 30" RCP will be constructed in place of the existing 36" CMP to collect runoff from Basin A-1 upon future development.

The runoff computed by the present Drainage Plan compares favorably with the design runoff used in the prior plans. The Drainage Management Plan quantifies the 100-year runoff from the site (Tract D) at 20.8 cfs (Analysis Point 10 of the Drainage Management Plan), while the runoff derived by these calculations is 8.4 + 7.5 = 15.9 cfs (Basins A-1 and A-2 combined). The Drainage Management Plan quantifies the 100-year runoff in the street at 6.4 cfs (Analysis Point 9 of the Drainage Management Plan). This represents half-street flows of 3.2 cfs which are much less than the capacity of the new inlet and storm drain in Eagle Ranch Road N.W. As shown by the accompanying nomograph, the gutter flow depth in the new street, carrying this half-street flow, will be less than 0.3 feet. This is less than the curb height.

While the paving and site development will result in an increase in runoff above existing undeveloped values, the new storm drains will accept and convey runoff to the existing storm drain system. This is in accordance with prior approved Plans; thereby the site development will not adversely impact downstream properties due to decreasing the amount of overland runoff leaving the site. The existing 60" storm drain into which the new storm drains will discharge was built as part of City Project No. 2204. According to page three of the Report that accompanied City Project No. 2204, "the storm drain was designed to accommodate the ultimate developed runoff as well as the existing developed runoff." This included the subject tract, which is a portion of Parcel 11 referenced by the Report. As indicated by the accompanying hydraulic grade line calculations, the additional runoff entering the existing 60" RCP will have minimal impact. The collective effect on the hydraulic grade line calculations is only 0.26 feet. The hydraulic grade line in the connector pipes will remain well below the surface elevations of the new inlet structures and will remain well below the ground elevation above the existing 60" RCP.

The Calculations which appear hereon analyze both the existing and developed conditions for the 100-year, 6-hour rainfall event. The Rational Method has been used to quantify the peak rate of discharge while the SCS Method has been used to quantify the volume of runoff generated. Both Methods have been used in accordance with the City of Albuquerque Development Process Manual, Volume II, coupled with the Mayor's Emergency Rule dated January 14, 1986.

HYDROLOGIC CALCULATIONS

Ground Cover Information

From SCS Bernalillo County Soil Survey,
Plate 10: (Bluepoint Loamy fine sand and fine sand)
Hydrologic Soil Group: A
Existing Pervious CN = 68 (DPM Plate 22.2 C-2)
Pasture or Range Land: poor condition)
Developed Pervious CN = 49 (DPM Plate 22.2 C-2)
Open space: fair condition)

Time of Concentration/Time to Peak

$T_C = 0.0078 L^{0.77}/S^{0.385}$ (Kirpich Equation)

$T_P = T_C = 10$ min.

Point Rainfall

$P_6 = 2.2$ in. (DPM Plate 22.2 D-1)

Rational Method

Discharge: $Q = C i A$

where C varies

$i = P_6 (6.84) T_C^{-0.51} = 4.65$ in/hr

$P_6 = 2.2$ in (DPM Plate 22.2D-1)

$T_C = 10$ min (minimum)

A = area, acres

SCS Method

Volume: $V = 3630$ (DRO) A

Where DRO = Direct runoff in inches
A = area, acres

Existing Condition

Basin A

$A_{total} = 280,950$ sf = 6.45 Ac
Undeveloped area = 196,200 sf (1.00)
 $C = 0.40$ (Weighted average per Emergency Rule, 1/14/86)
 $Q_{100} = C i A = 0.40(4.65)(6.45) = 12.0$ cfs
% impervious = -0-%
CN = 68 (DPM Plate 22.2 C-3)
DRO = 0.4 in (DPM Plate 22.2 C-4)
 $V_{100} = 3630$ (DRO)A = 9,370 cf

Developed Condition

1. Basin A-1

(Primarily undeveloped, use Pervious CN = 68)
 $A_{total} = 196,200$ sf = 4.50 Ac
Roof area = 810 sf (0.01)
Paved area = 5400 sf (0.03)
Landscaped area = 24,430 sf (0.12)
Undeveloped area = 165,560 sf (0.84)
 $C = 0.40$ (Weighted average per Emergency Rule, 1/14/86)
 $Q_{100} = C i A = 0.40(4.65)(4.50) = 8.4$ cfs
% impervious = 4%
Composite CN = 69 (DPM Plate 22.2 C-3)
DRO = 0.4 in (DPM Plate 22.2 C-4)
 $V_{100} = 3630$ (DRO)A = 6,530 cf

2. Basin A-2

$A_{total} = 84,750$ sf = 1.94 Ac
Roof area = 7,700 sf (0.09)
Paved area = 62,850 sf (0.74)
Landscaped area = 14,200 sf (0.17)
 $C = 0.83$ (Weighted average per Emergency Rule, 1/14/86)
 $Q_{100} = C i A = 0.83(4.65)(1.94) = 7.5$ cfs
% impervious = 83%
Composite CN = 88 (DPM Plate 22.2 C-3)
DRO = 1.4 in (DPM Plate 22.2 C-4)
 $V_{100} = 3630$ (DRO)A = 9,860 cf

Comparison

$\Delta Q_{100} = (8.4 + 7.5) - 12.0 = 3.9$ cfs (increase)
 $\Delta V_{100} = (6,530 + 9,860) - 9,370 = 7,020$ cf (increase)

STORM DRAIN-HYDRAULICS

INLET CAPACITIES (SUMP CONDITION)

Use Orifice Equation with $C = 0.60$:
 $Q = C A (2gh)^{1/2}$

Assume effective grate area = 0.50 total grate area

TYPE DOUBLE 'D' INLET CAPACITY (BASIN A-2)

Effective Area = $0.50(2' \times 3' \times 2) = 6$ SF

$h = 0.5'$
 $Q = 0.60(6)[2(32.2)(0.5)]^{1/2} = 20.4$ cfs > $Q_{100} = 7.5$ cfs

TYPE DOUBLE 'A' INLET CAPACITY (BASIN A-1)

Effective Area = $0.50(2' \times 3' \times 2) = 6$ sf

$h = 0.6'$
 $Q = 0.60(6)[2(32.2)(0.6)]^{1/2} = 22.4$ cfs > $Q_{100} = 8.4$ cfs

RCP PIPE CAPACITIES (FLOWING FULL)

Use Manning's Equation with $n = 0.013$:

$Q = (1.486/n)AR^{2/3}S^{1/2}$

Use Field's Hydraulic Calculator to calculate friction slopes,
sf

24" RCP PRIVATE STORM DRAIN (BASIN A-2)

$Q = (1.486/0.013)3.14(0.50)^{2/3}(0.0836)^{1/2} =$
65.4 cfs >> $Q_{100} = 7.5$ cfs
 $sf = 0.0011$ << $S = 0.0836$

30" RCP PRIVATE STORM DRAIN (FUTURE BASIN A-1)

$Q = (1.486/0.013)4.91(0.625)^{2/3}(0.0280)^{1/2} =$
68.7 cfs >> $Q_{100} = 8.4$ cfs
 $sf = 0.0004$ << $S = 0.0280$

24" RCP CONNECTOR PIPE (BASIN A-1)

$Q = (1.486/0.013)3.14(0.50)^{2/3}(0.0188)^{1/2} =$
31.0 cfs >> $Q_{100} = 8.4$ cfs
 $sf = 0.0014$ << $S = 0.0188$

HYDRAULIC GRADE LINE CALCULATIONS

Existing 60" RCP

Junction Loss:

$\Delta y = (Q_2 V_2 - Q_1 V_1 - Q_3 V_3 \cos \theta) / [g(A_1 + A_2) / 2]$

$+ 1/2(S_{f1} + S_{f2})L$

$h_j = y + h_{v1} - h_{v2}$

Velocity head = $V^2 / (2g)$

Junction at New Manhole

(Station 26 + 64, Eagle Ranch Road Storm Drain)

$Q_1 = 214$ cfs*	$Q_2 = 221.5$ cfs	$Q_3 = 7.5$ cfs
$D_1 = 5.0'$	$D_2 = 5.0'$	$D_3 = 2.0'$
$A_1 = 19.64$ sf	$A_2 = 19.64$ sf	$A_3 = 3.14$ sf
$V_1 = 10.90$ fps	$V_2 = 11.29$ fps	$V_3 = 2.39$ fps
$S_{f1} = 0.0066$	$S_{f2} = 0.0071$	$\theta = 45^\circ$
$h_{v1} = 1.84'$	$h_{v2} = 1.98'$	$L = 0'$

$\Delta y = [221.5(11.29) - 214(10.90) - 7.5(2.39)\cos 45] /$
 $[32.2(19.64 + 19.64) / 2]$
 $= 0.25'$

$h_j = 0.25' + 1.84' - 1.98' = 0.11'$

*Per Design Report for Eagle Ranch Storm Drain

Junction at Existing Manhole

(Sta 25 + 83, M.H. No. 5 Eagle Ranch Road Storm Drain)
(Basin A-1 discharges into manhole)

$Q_1 = 221.5$ cfs	$Q_2 = 229.9$ cfs	$Q_3 = 8.4$ cfs
$D_1 = 5.0'$	$D_2 = 5.0'$	$D_3 = 2.0'$
$A_1 = 19.64$ sf	$A_2 = 19.64$ sf	$A_3 = 3.14$ sf
$V_1 = 11.29$ fps	$V_2 = 11.71$ fps	$V_3 = 2.67$ fps
$S_{f1} = 0.0071$	$S_{f2} = 0.0076$	$\theta = 90^\circ$
$h_{v1} = 1.98'$	$h_{v2} = 2.13'$	$L = 0'$

$\Delta y = [229.9(11.71) - 221.5(11.29) - 8.4(2.67)\cos 90] /$
 $[32.2(19.64 + 19.64) / 2]$
 $= 0.30'$

$h_j = 0.30' + 1.98' - 2.13' = 0.15'$

Existing 60 x 30 Wye

(Station 25 + 89, Eagle Ranch Road Storm Drain)
Future Runoff (Basin A-1 discharges into wye)

$Q_1 = 221.5$ cfs	$Q_2 = 229.9$ cfs	$Q_3 = 8.4$ cfs
$D_1 = 5.0'$	$D_2 = 5.0'$	$D_3 = 2.5'$
$A_1 = 19.64$ sf	$A_2 = 19.64$ sf	$A_3 = 4.91$ sf
$V_1 = 11.29$ fps	$V_2 = 11.71$ fps	$V_3 = 1.71$ fps
$S_{f1} = 0.0071$	$S_{f2} = 0.0076$	$\theta = 45^\circ$
$h_{v1} = 1.98'$	$h_{v2} = 2.13'$	$L = 0'$

$\Delta y = [229.9(11.71) - 221.5(11.29) - 8.4(1.71)\cos 45] /$
 $[32.2(19.64 + 19.64) / 2]$
 $= 0.29'$

$h_j = 0.29' + 1.98' - 2.13' = 0.14'$

Junction at Existing Manhole

(Sta 25 + 83, MH No. 5, Eagle Ranch Road Storm Drain)
Future Runoff (only street discharges into manhole)

$Q_1 = 229.9$ cfs	$Q_2 = 236.3$ cfs	$Q_3 = 6.4$ cfs*
$D_1 = 5.0'$	$D_2 = 5.0'$	$D_3 = 2.0'$
$A_1 = 19.64$ sf	$A_2 = 19.64$ sf	$A_3 = 3.14$ sf
$V_1 = 11.71$ fps	$V_2 = 12.03$ fps	$V_3 = 2.04$ fps
$S_{f1} = 0.0076$	$S_{f2} = 0.0080$	$\theta = 90^\circ$
$h_{v1} = 2.13'$	$h_{v2} = 2.25'$	$L = 0'$

$\Delta y = [23.63(12.03) - 229.9(11.71) - 6.4(2.04)\cos 90] /$
 $[32.2(19.64 + 19.64) / 2]$
 $= 0.24'$

$h_j = 0.24' + 2.13' - 2.25' = 0.12'$

*Per Drainage Management Plan: Ultimate Street Runoff at
Analysis Point 9

Hydraulic Grade Lines Summary

Total change in head after present phase of site development:

$\Delta h = 0.11' + 0.15' = +0.26'$

Total change in head after ultimate site development:

$\Delta h = 0.14' + 0.12' = +0.26'$

24" RCP Private Storm Drain (Basin A-2)

H.G.L. elevation @ 60" RCP (Station 26+64, Eagle Ranch Road
Storm Drain Plans): 5049.31

Change in head due to junctions (above) = 0.26'

Head due to friction slope = $h_f \times L = 0.0011 \times 106' = 0.12'$

Thus H.G.L. @ new double "D" inlet = 5049.69

Grate elevation = 5057.45 > 5049.69

24" RCP Connector Pipe (Basin A-1)

H.G.L. elevation @ 60 RCP (M.H. No. 5, Station 25+83,
Eagle Ranch Road Storm Drain Plans): 5048.40

Change in head due to junctions (above) = 0.26'

Head due to friction slope = $0.0014 \times 110' = 0.15'$

Thus H.G.L. @ new double "A" inlet = 5048.81

Grate elevation = 5053.75 > 5048.81

30" RCP Private Storm Drain (Future Basin A-1)

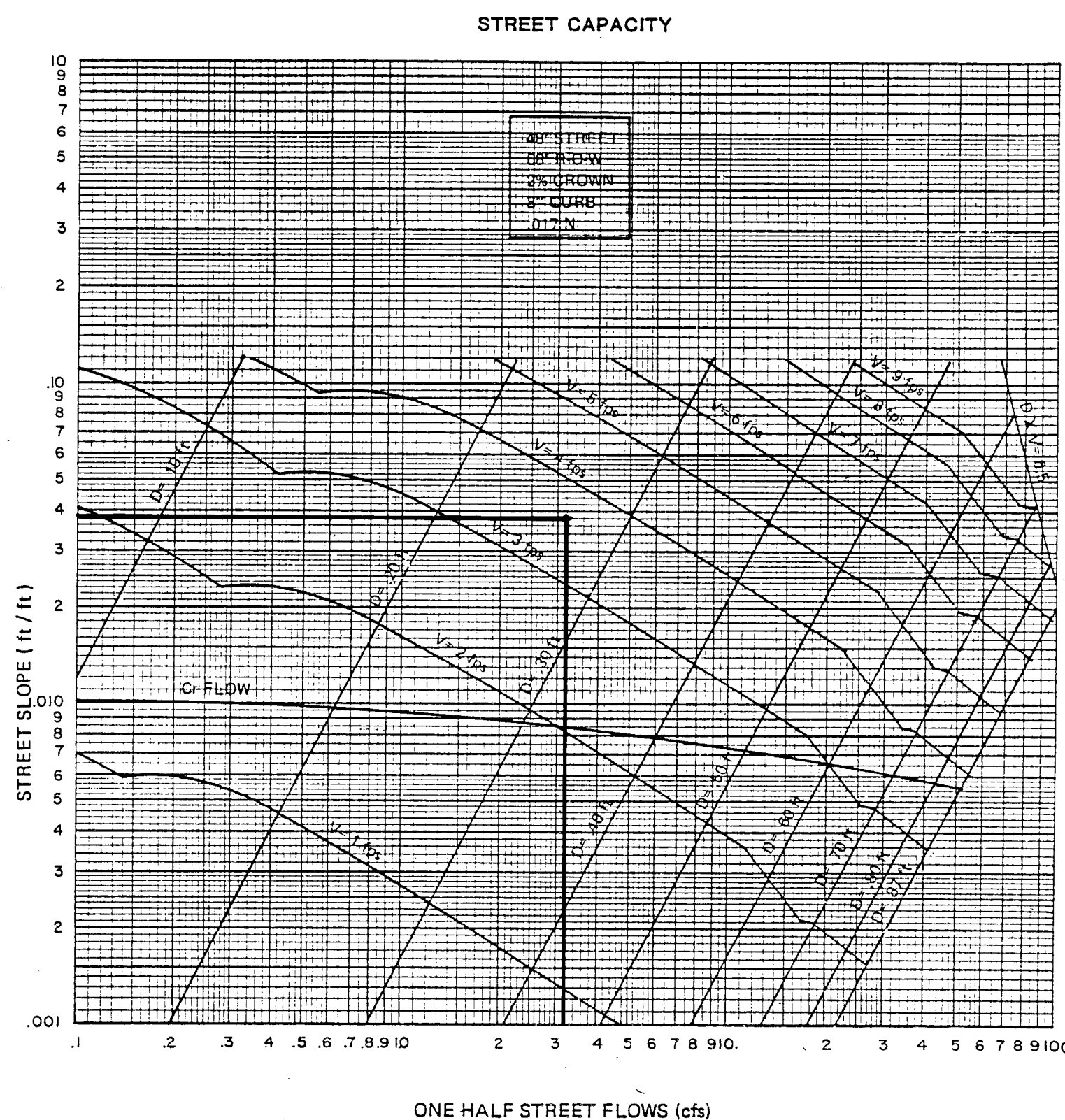
H.G.L. @ 60" RCP (60" x 30" Wye, Station 25 + 89, Eagle
Ranch Road Storm Drain Plans): 5048.80

Change in head due to junctions (above) = 0.26'

Head due to friction slope = $0.0004 \times 95' = 0.04'$

Thus H.G.L. @ new private SDMH = 5049.10

Rim Elevation = 5058.8 > 5049.10



RECORD DRAWING

I, Jeffrey G. Mortensen, Registered Professional Engineer in the State of New Mexico, do hereby certify that this "as-built" information was obtained by me or under my supervision and represents the "as-built" conditions of this project, and is true and correct to the best of my knowledge and belief. All vertical and horizontal dimensions should be field verified prior to use on future projects.

Jeffrey G. Mortensen
N.M.P.E. NO. 8547
Date 7-15-91



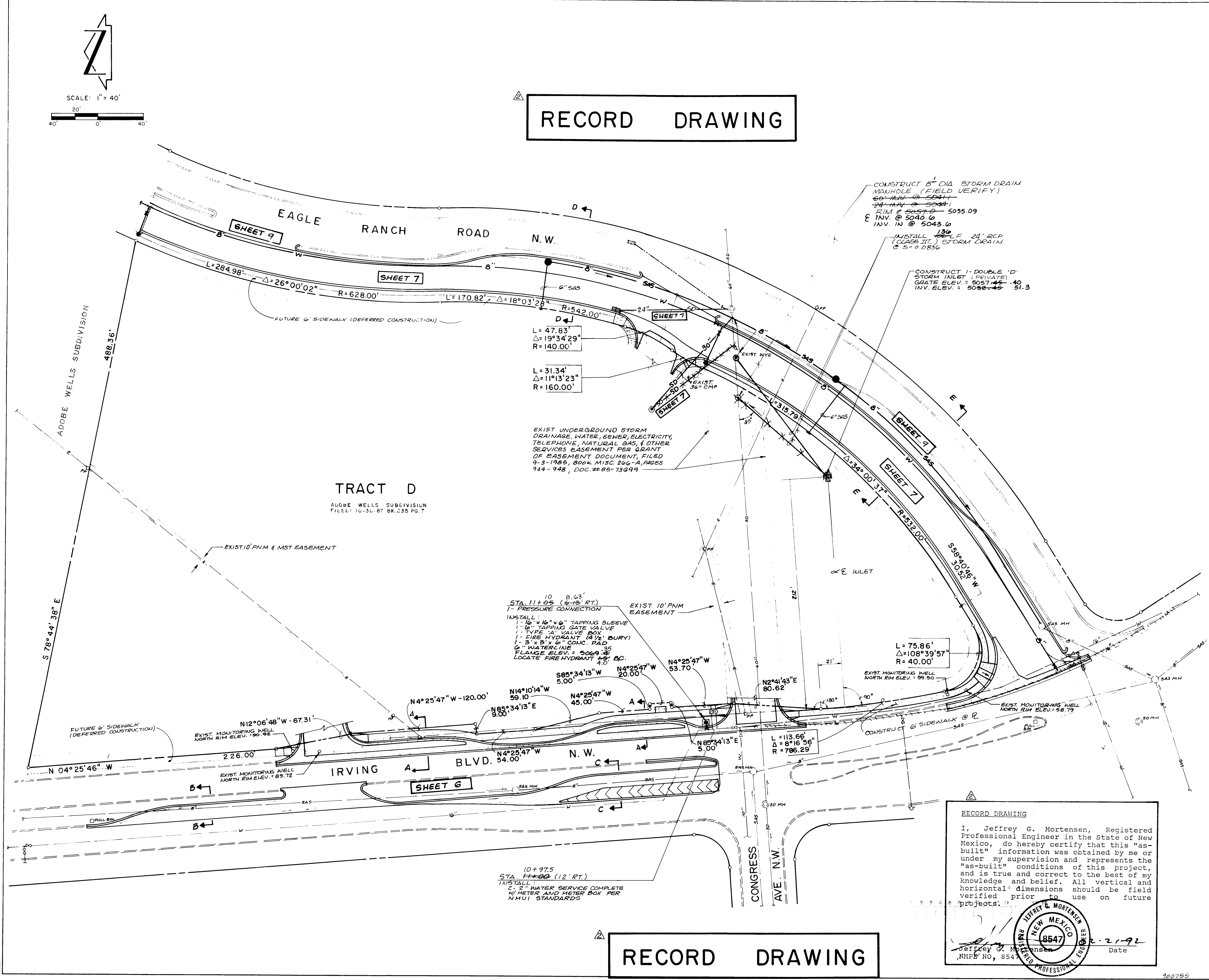
AS BUILT INFORMATION		BENCH MARKS		SURVEY INFORMATION		ENGINEER'S SEAL	
CONTRACTOR	SUNDANCE MECHANICAL	A STANDARD ACS ALUMINUM TABLET STAMPED	"ACS BM 5-813" ON TOP OF CURB. LOCATED ON THE EAST SIDE OF IRVING BLVD. NW AT THE PC	NO.	DATE	NO.	DATE
DESIGNED BY	JMA	INSPECTOR'S ACCEPTANCE BY	DKM	BY	DATE	BY	DATE
DATE	3-92	APPROXIMATELY 0.57 OF A MILE NNW OF COORS RD. NNW.	DATE	3-92	DATE	DATE	DATE
DATE	3-92	VERIFICATION BY	DKM	REMARKS	DESIGN	REVISIONS	DESIGN
DATE	3-92	CORRECTED BY	DKM	NO.	DATE	NO.	DATE
DATE	3-92	RECORDED BY	JGM	DESIGNED BY	JPK	DRAWN BY	JMA
DATE	3-92	NO.		CHECKED BY	JGM	DATE	7-91
DATE	3-92	NO.		DATE	7-91	DATE	7-91



1 2 3 4 5 6 7 8 9 10 11 12
26 1296.1900492

SCANNED BY
BY LASON

CITY OF ALBUQUERQUE PUBLIC WORKS DEPARTMENT ENGINEERING GROUP					
TITLE: U.S. NEW MEXICO FEDERAL CREDIT UNION / IRVING BLVD. N.W. AND EAGLE RANCH RD. N.W. STREET IMPROVEMENTS HYDROLOGIC AND HYDRAULIC CALCULATIONS					
APPROVALS	ENGINEER	DATE	APPROVALS	ENGINEER	DATE
DRC CHAIRMAN	Jeffrey G. Mortensen	7-31-91	WATER	ALL	7-22-91
TRANSPORTATION	Jeffrey G. Mortensen	7-25-91	WASTE WATER	ALL	"
HYDROLOGY	Jeffrey G. Mortensen	7-30-91	NMUI	Jeffrey G. Mortensen	7-19-91
PROJECT NO.	4296.90	MAP NO.	B-13 C-13	SHEET	OF 11



RECORD DRAWING

RECORD DRAWING

RECORD DRAWING

I, Jeffrey G. Mortensen, Registered Professional Engineer in the State of New Mexico, do hereby certify that this "as-built" information was obtained by me or under my supervision and represents the "as-built" conditions of this project, and is true and correct to the best of my knowledge and belief. All vertical and horizontal dimensions should be field verified prior to use on future projects.

Jeffrey G. Mortensen
NMPE NO. 8547

DATE 7-21-92

- WATER EXTENSION NOTES:
1. WATERLINE STATIONING IS BASED UPON STREET CENTERLINE WHERE THE WATERLINE PARALLELS SAID CENTERLINE, UNLESS INDICATED OTHERWISE.
 2. ALL WATERLINE SHALL BE PVC (C-900) UNLESS OTHERWISE APPROVED BY NEW MEXICO UTILITIES, INC.
 3. WHERE WATER AND SANITARY SEWER LINES CROSS, THE SANITARY SEWER LINE SHALL BE CONCRETE ENCASED FOR A DISTANCE OF 10 FEET EACH SIDE OF THE CROSSING IF 18 INCHES OF VERTICAL SEPARATION DOES NOT EXIST.
 4. ELECTRONIC MARKER DISKS (EMD'S) SHALL BE PLACED ABOVE ALL FITTINGS IN ACCORDANCE WITH SECTION 170 OF THE "STANDARD SPECIFICATIONS".
 5. IN ACCORDANCE WITH SECTION 801 OF THE "STANDARD SPECIFICATIONS", METALIZED DETECTABLE WARNING TAPE SHALL BE INSTALLED 18 INCHES ABOVE ALL PVC PIPE INSTALLED ON THIS PROJECT.
 6. REFER TO NEW MEXICO UTILITIES, INC. STANDARD DRAWINGS. NEW MEXICO UTILITIES INC STANDARDS SHALL SUPERCEDE THE CITY OF ALBUQUERQUE STANDARD SPECIFICATIONS FOR PUBLIC WORKS CONSTRUCTION WHERE THE REQUIREMENTS ARE FOUND TO DIFFER.
 7. THE EXISTING 16" WATERLINE MATERIAL TYPE IS NOT KNOWN. IN THE EVENT THAT THE LINE IS AC (ASBESTOS CEMENT), THEN THE TAPPING SLEEVE FURNISHED AND INSTALLED MUST HAVE A MINIMUM 36" WIDTH.

AS BUILT INFORMATION		BENCH MARKS		SURVEY INFORMATION		ENGINEER'S SEAL		REVISIONS		DESIGNED BY		DRAWN BY		CHECKED BY	
NO.	DATE	NO.	DATE	NO.	DATE	NO.	DATE	NO.	DATE	NO.	DATE	NO.	DATE	NO.	DATE
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11	9/90	11	9/90	11	9/90	11	9/90	11	9/90	11	9/90	11	9/90	11	9/90
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14	9/90	14	9/90	14	9/90	14	9/90	14	9/90	14	9/90	14	9/90	14	9/90
15	9/90	15	9/90	15	9/90	15	9/90	15	9/90	15	9/90	15	9/90	15	9/90

SCANNED BY BY LASON

1 2 3 4 5 6 7 8 9 10 11 12 13 14 15

26 7296.190592

CITY OF ALBUQUERQUE
PUBLIC WORKS DEPARTMENT
ENGINEERING GROUP

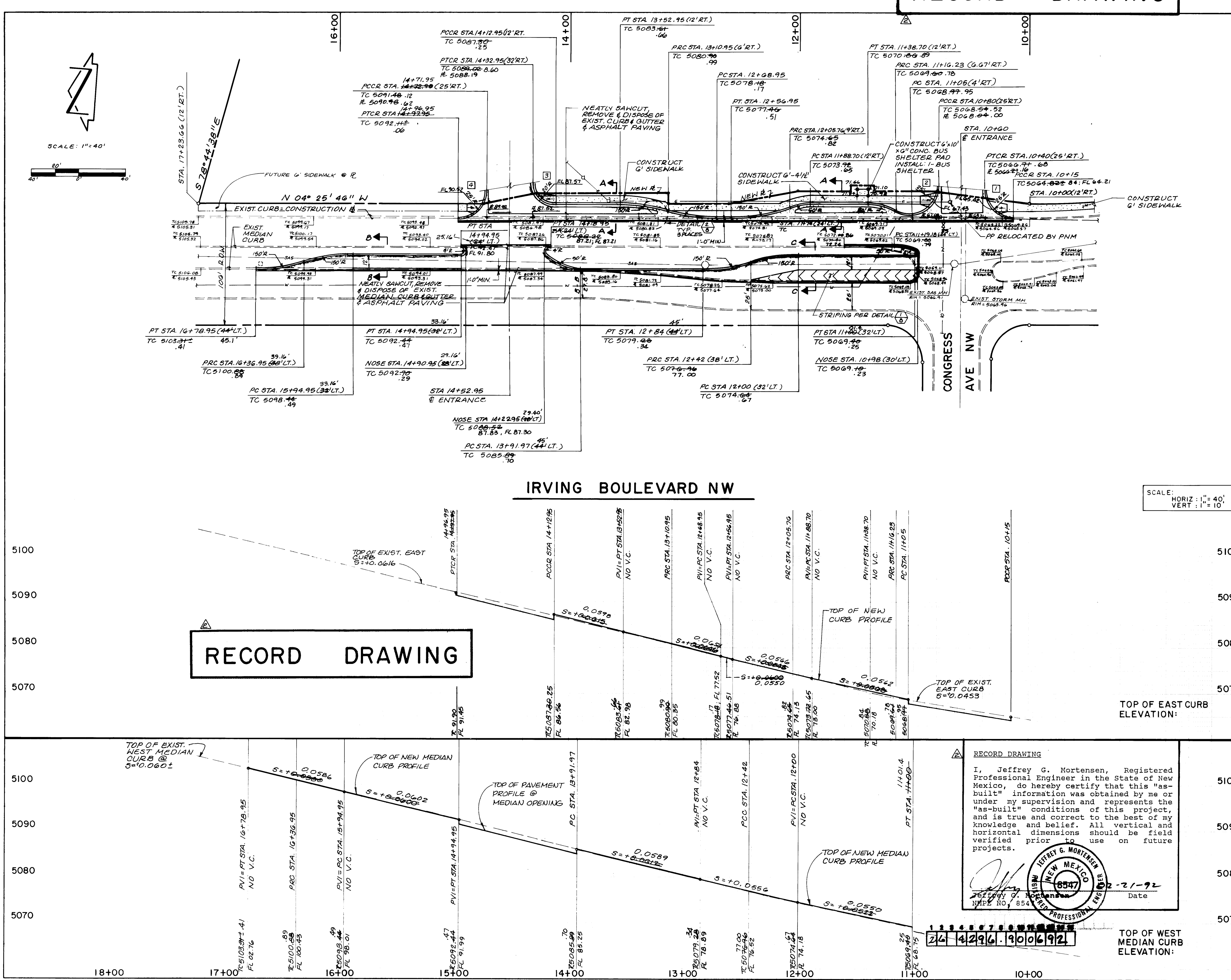
TITLE U.S. NEW MEXICO FEDERAL CREDIT UNION/IRVING BLVD. N.W. AND EAGLE RANCH RD. N.W. STREET IMPROVEMENTS

SITE PLAN

APPROVALS	ENGINEER	DATE	APPROVALS	ENGINEER	DATE
CHIEF CHAIRMAN			WATER		
TRANSPORTATION			WASTE WATER		
HYDROLOGY			NMUI		

PROJECT NO. 4296.90 MAP B-13 / SHEET 5 OF 11

RECORD DRAWING



STREET PAVING NOTES:

- ALL SLOPES AS SHOWN ON PROFILES ARE BASED ON STREET CENTERLINE.
- ALL DIMENSIONS OF CURB AND CURB RETURNS ARE SHOWN TO FACE OF CURB, UNLESS OTHERWISE NOTED.
- WHERE REMOVAL OF EXISTING CURB AND GUTTER, SIDEWALK OR PAVEMENT IS REQUIRED, THE CONTRACTOR SHALL SAWCUT AND/OR REMOVE TO THE NEAREST JOINT. CURB AND GUTTER SHOWN AS EXISTING AND NOT TO BE REMOVED UNDER THIS CONTRACT WHICH IS DAMAGED OR DISPLACED BY THE CONTRACTOR SHALL BE REMOVED AND REPLACED BY THE CONTRACTOR AT THE CONTRACTOR'S EXPENSE.
- CONTRACTOR SHALL ADJUST TO GRADE ALL VALVE BOXES AND MANHOLES ENCOUNTERED WITHIN THE CONSTRUCTION LIMITS.
- BACKFILL COMPACTION SHALL BE ACCORDING TO SPECIFIED STREET USE.
- ROADWAY GRADING IS INCIDENTAL TO SUBGRADE PREPARATION, THEREFORE NO SEPARATE PAYMENT WILL BE MADE.

AS BUILT INFORMATION

CONTRACTOR: SUNDANCE MECHANICAL

DATE: 10-91

INSPECTOR: JMA

DATE: 3-92

FIELD: JMA

DATE: 3-92

VERIFICATION: JMA

DATE: 3-92

CORRECTED BY: JMA

DATE: 3-92

RECORDED BY: JMA

DATE: 3-92

ENGINEER'S SEAL

JEFFREY G. MORTENSEN

NEW MEXICO

REGISTERED PROFESSIONAL ENGINEER

07-15-91

06-17-91

CURB RETURN ELEVATIONS

TC	RE
1 65.45	64.78
2 65.40	67.28
3 66.30	65.80

TC	RE
1 87.77	87.10
2 87.57	90.58
3 88.54	88.04

TC	RE
1 87.77	87.10
2 87.57	90.58
3 88.54	88.04

TC	RE
1 87.77	87.10
2 87.57	90.58
3 88.54	88.04

REVISIONS

NO.	DATE	REMARKS
1	05-1991	DESIGNED BY JCM
2	05-1991	DRAWN BY MJT
3	05-1991	CHECKED BY JCM

CITY OF ALBUQUERQUE

PUBLIC WORKS DEPARTMENT

ENGINEERING GROUP

TITLE: U.S. NEW MEXICO FEDERAL CREDIT UNION/IRVING BLVD. N.W. AND EAGLE RANCH RD. N.W. STREET IMPROVEMENTS

IRVING BOULEVARD N.W. STA. 11+00 TO STA. 16+78.95

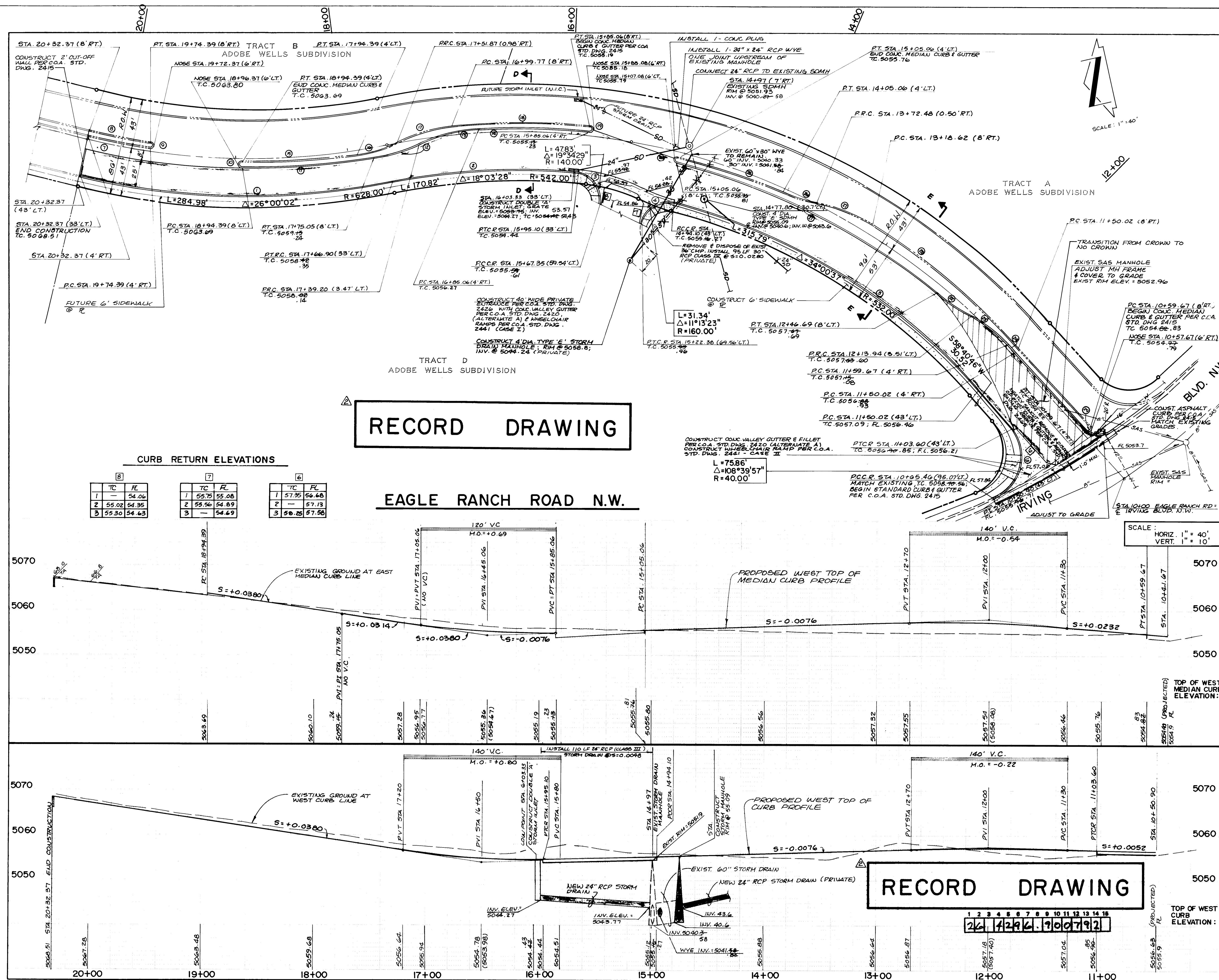
PAVING AND DRAINAGE IMPROVEMENTS PLAN & PROFILE

APPROVALS	ENGINEER	DATE	APPROVALS	ENGINEER	DATE
DRC CHAIRMAN	[Signature]	7-31-91	WATER	[Signature]	7-22-91
TRANSPORTATION	[Signature]	7-25-91	WASTE WATER	[Signature]	7-22-91
HYDROLOGY	[Signature]	7-30-91	N.M.U.I.	[Signature]	7-22-91

PROJECT NO. 4296.90

MAP B-13/ NO. C-13

SHEET 6 OF 11



STREET PAVING NOTES:

1. ALL SLOPES AS SHOWN ON PROFILES ARE BASED ON STREET CENTERLINE.
 2. ALL DIMENSIONS OF CURB AND CURB RETURNS ARE SHOWN TO FACE OF CURB.
 3. WHERE REMOVAL OF EXISTING CURB AND GUTTER, SIDEWALK OR PAVEMENT IS REQUIRED, THE CONTRACTOR SHALL SAWCUT AND/OR REMOVE TO THE NEAREST JOINT. CURB AND GUTTER TO BE SHOWN AS EXISTING AND NOT TO BE REMOVED UNDER THIS CONTRACT WHICH IS DAMAGED OR DISPLACED BY THE CONTRACTOR SHALL BE REMOVED AND REPLACED BY THE CONTRACTOR AT THE CONTRACTOR'S EXPENSE.
 4. CONTRACTOR SHALL ADJUST TO GRADE ALL VALVE BOXES AND MANHOLES ENCOUNTERED WITHIN THE CONSTRUCTION LIMITS.
 5. BACKFILL COMPACTION SHALL BE ACCORDING TO SPECIFIED STREET USE.
 6. ROADWAY GRADING IS INCIDENTAL TO SUBGRADE PREPARATION, THEREFORE NO SEPARATE PAYMENT WILL BE MADE.
- STORM DRAINAGE NOTES:**
7. STATIONS AS SHOWN HEREON ARE BASED UPON CONSTRUCTION BASELINE (CENTERLINE OF RIGHT-OF-WAY).
 8. SLOPES ARE CALCULATED BASED ON TRUE DISTANCES; LENGTH OF PIPE IS MEASURED FROM CENTER OF STRUCTURE TO CENTER OF STRUCTURE.

RECORD DRAWING $\triangle 2$

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 21-9
 Jefferson G. Rosen
 NMID NO. 654

CURB CURVE DATA

CURVE	DELTA	R	L
①	26°00'00"	618.00'	280.44'
2	16°49'37"	552.00'	162.12'
3	94°53'54"	25.00'	41.41'
4	94°59'36"	25.00'	41.45'
5	33°42'03"	542.00'	318.79'
6	107°34'48"	40.00'	75.11'
7	5°40'43"	581.00'	57.58'
8	5°40'43"	577.00'	57.19'
9	180°00'00"	2.00'	6.29'
10	180°00'00"	2.00'	6.28'
11	11°41'19"	593.00'	120.97'
12	13°36'40"	150.00'	36.07'
13	20°59'41"	150.00'	54.96'
14	9°47'39"	589.00'	100.68'
15	9°47'39"	589.00'	100.68'
16	16°26'59"	150.00'	43.07'
17	20°19'54"	150.00'	53.23'
18	11°14'06"	593.00'	116.28'
19	180°00'00"	2.00'	6.28'
20	180°00'00"	2.00'	6.29'
21	25°18'22"	577.00'	254.85'
22	12°31'32"	150.00'	32.79'
23	21°02'56"	150.00'	55.11'
24	0°56'43"	589.00'	9.72'
25	9°47'39"	581.00'	99.32'
26	12°33'26"	150.00'	32.87'
27	21°01'25"	150.00'	55.04'
28	16°30'49"	593.00'	170.91'
②	180°00'00"	2.00'	6.28'

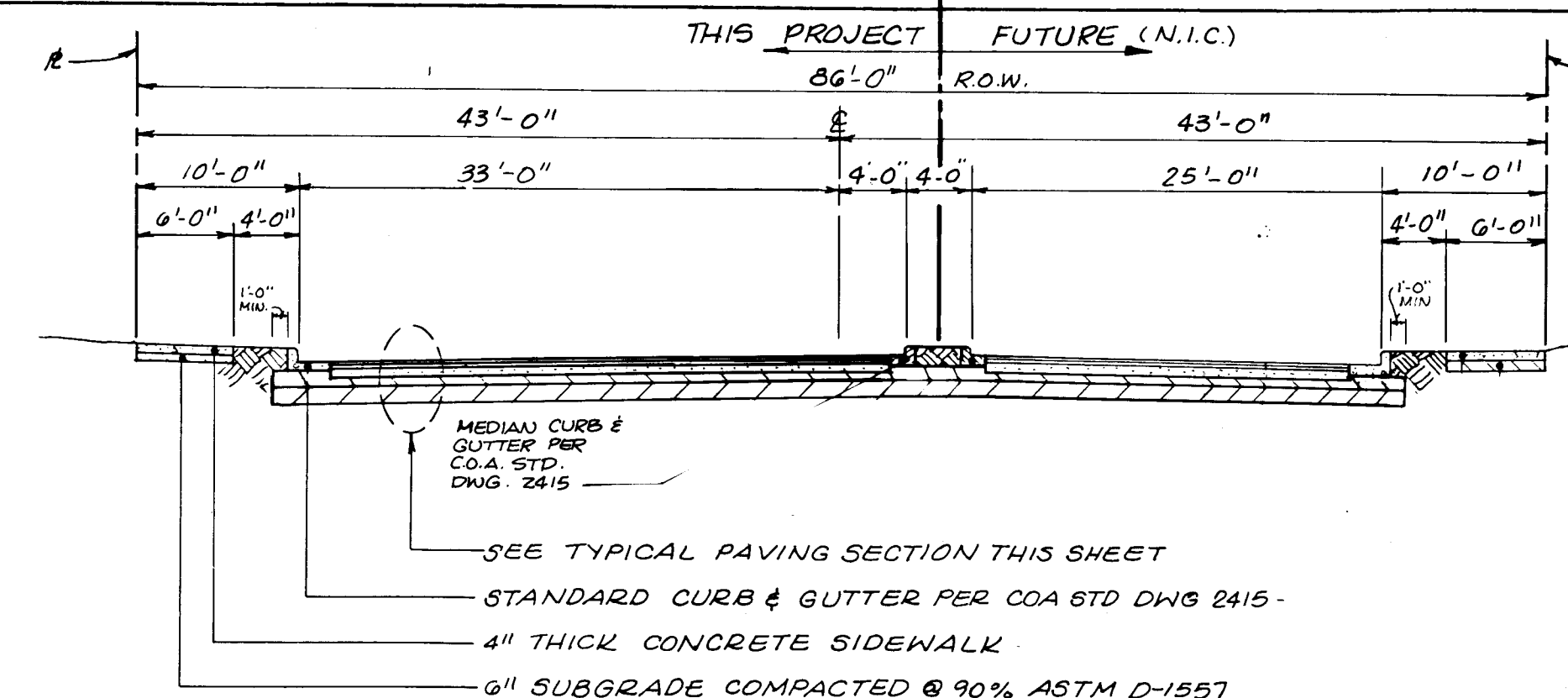
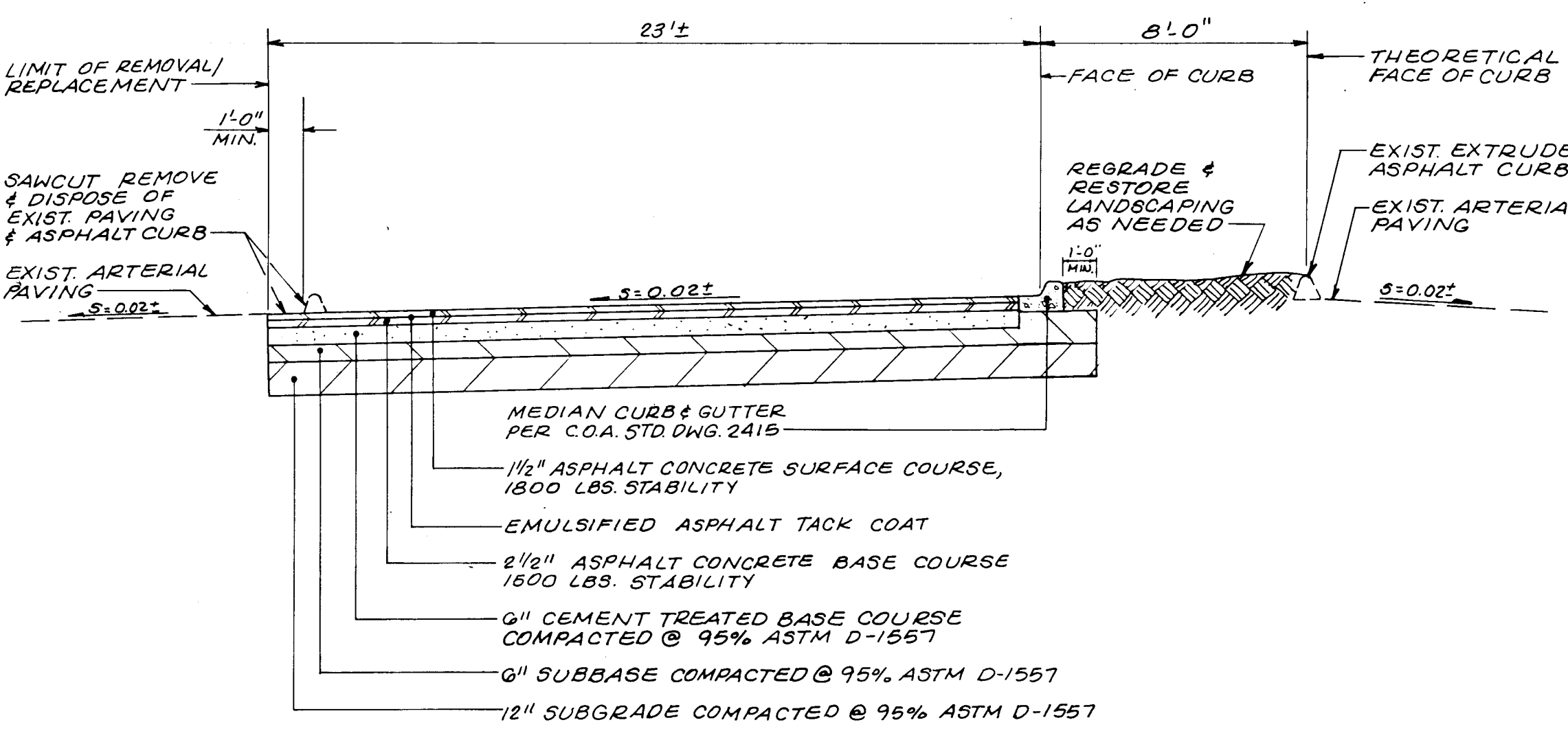
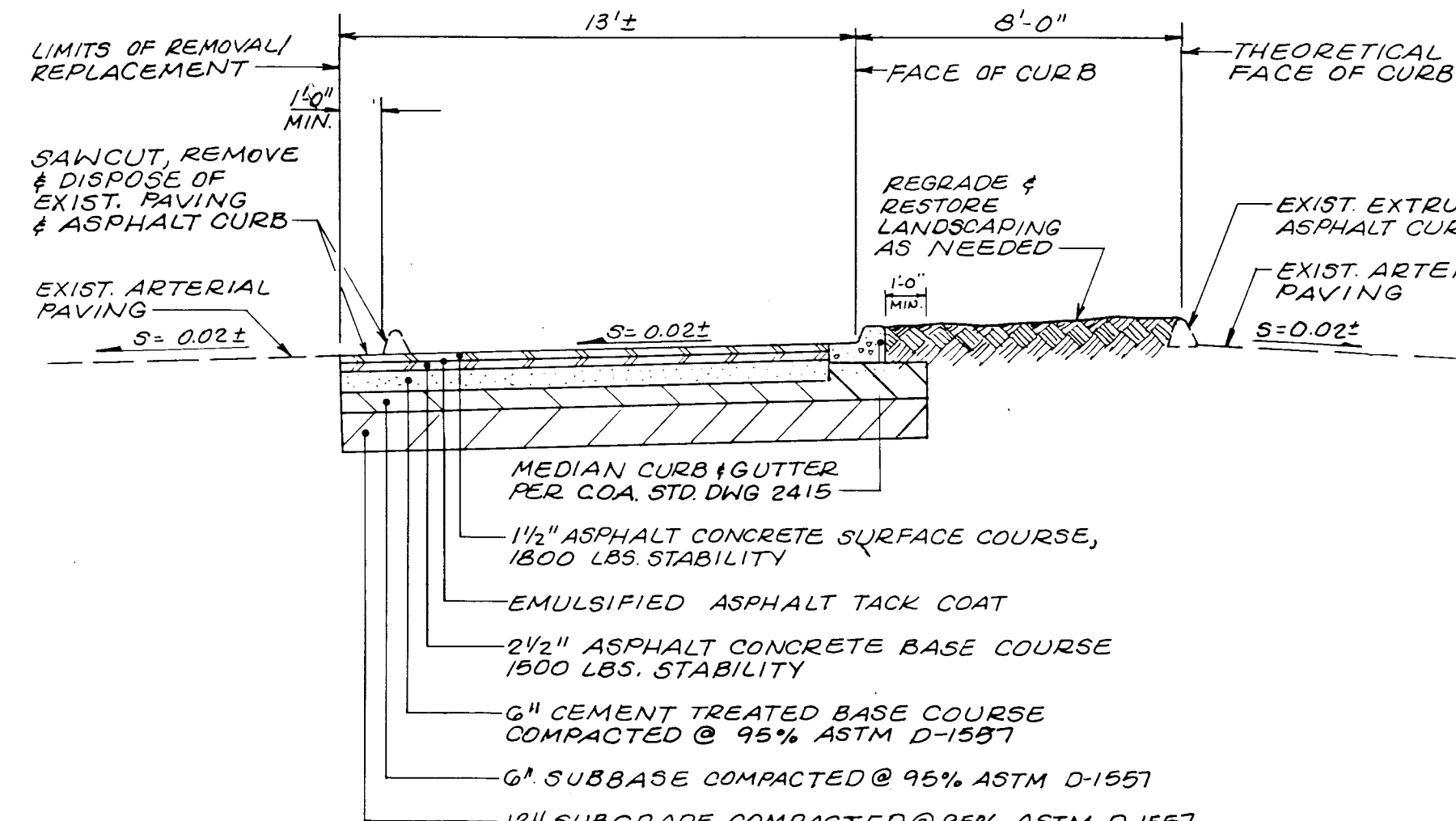
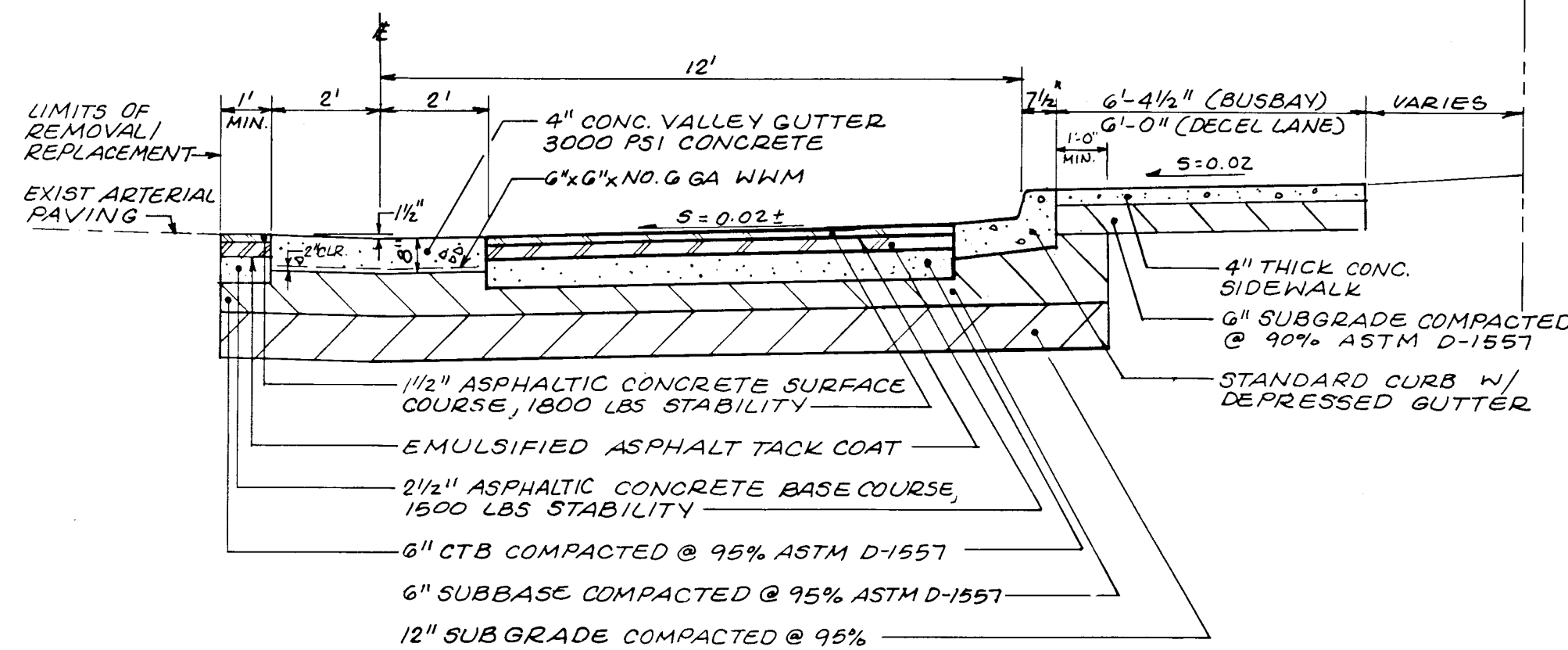
CITY OF ALBUQUERQUE
PUBLIC WORKS DEPARTMENT
ENGINEERING GROUP

TITLE: U.S. NEW MEXICO FEDERAL CREDIT UNION / IRVING BLVD. N.W.
AND EAGLE RANCH RD. N.W. STREET IMPROVEMENTS
EAGLE RANCH ROAD N.W. STA. 10+00 TO STA. 20+32.37
PAVING AND DRAINAGE IMPROVEMENTS PLAN & PROFILE

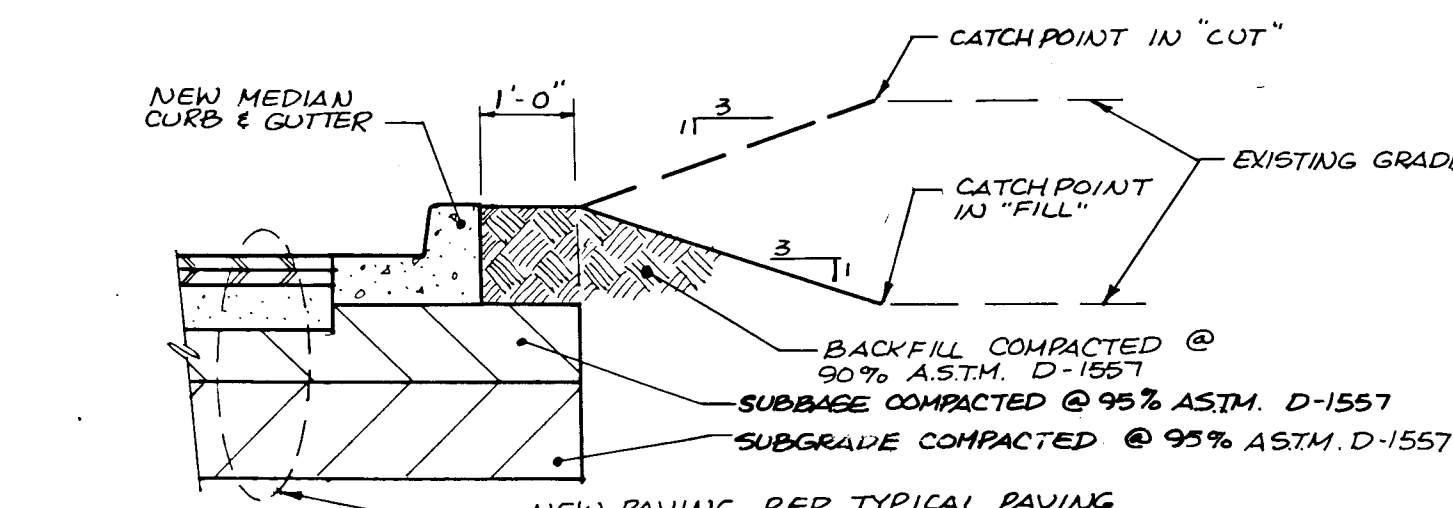
APPROVALS	ENGINEER	DATE	APPROVALS	ENGINEER	DATE
DRC CHAIRMAN	<i>Randy Dora</i>	7-31-91	WATER	<i>HCC</i>	7-22-91
TRANSPORTATION	<i>T. Dora</i>	7-25-91	WASTE WATER	<i>HRC</i>	"
HYDROLOGY	<i>A. Olney</i>	7-30-91	NMUI	<i>H. Olney</i>	7-19-91

PROJECT NO.	4296.90	MAP B-13/ NO. C-13	SHEET 7 OF 11
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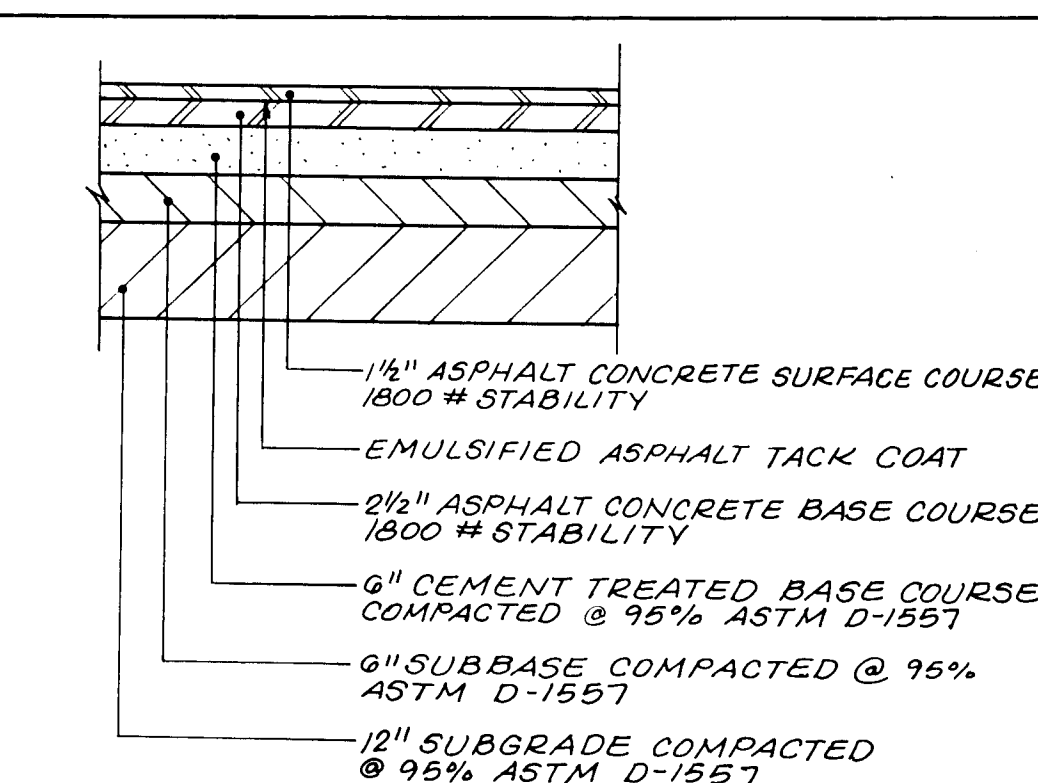
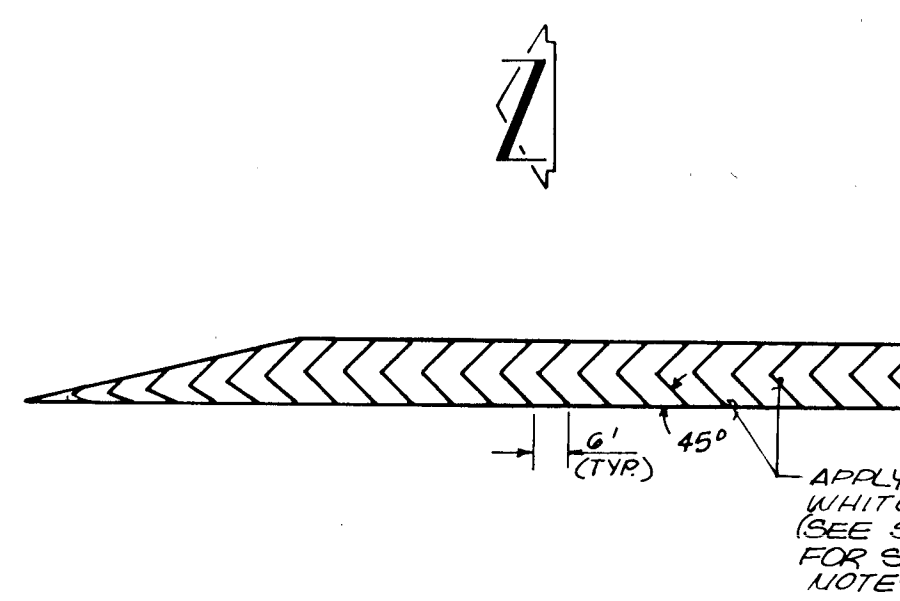
SCANNED BY
BY LASON



RECORD DRAWING



TYPICAL MEDIAN CURB SECTION
(EAGLE RANCH ROAD)



TYPICAL PAVING SECTION

RECORD DRAWING

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JEFFREY G. MORTENSEN
NEW MEXICO
REGISTERED PROFESSIONAL ENGINEER
8547
DATE: 7-21-92
NMPE NO. 8547

CONSTRUCT STANDARD CURB & GUTTER PER C.O.A. STD. DWG. 2415

TRANSITION FROM STANDARD CURB & GUTTER TO VALLEY GUTTER PER C.O.A. STD. DWG. 2421 (SECTION B-B)

CONSTRUCT 4' CONC. VALLEY GUTTER PER C.O.A. STD. DWG. 2421 (SECTION A-A)

CONSTRUCT STANDARD CURB & GUTTER PER C.O.A. STD. DWG. 2415

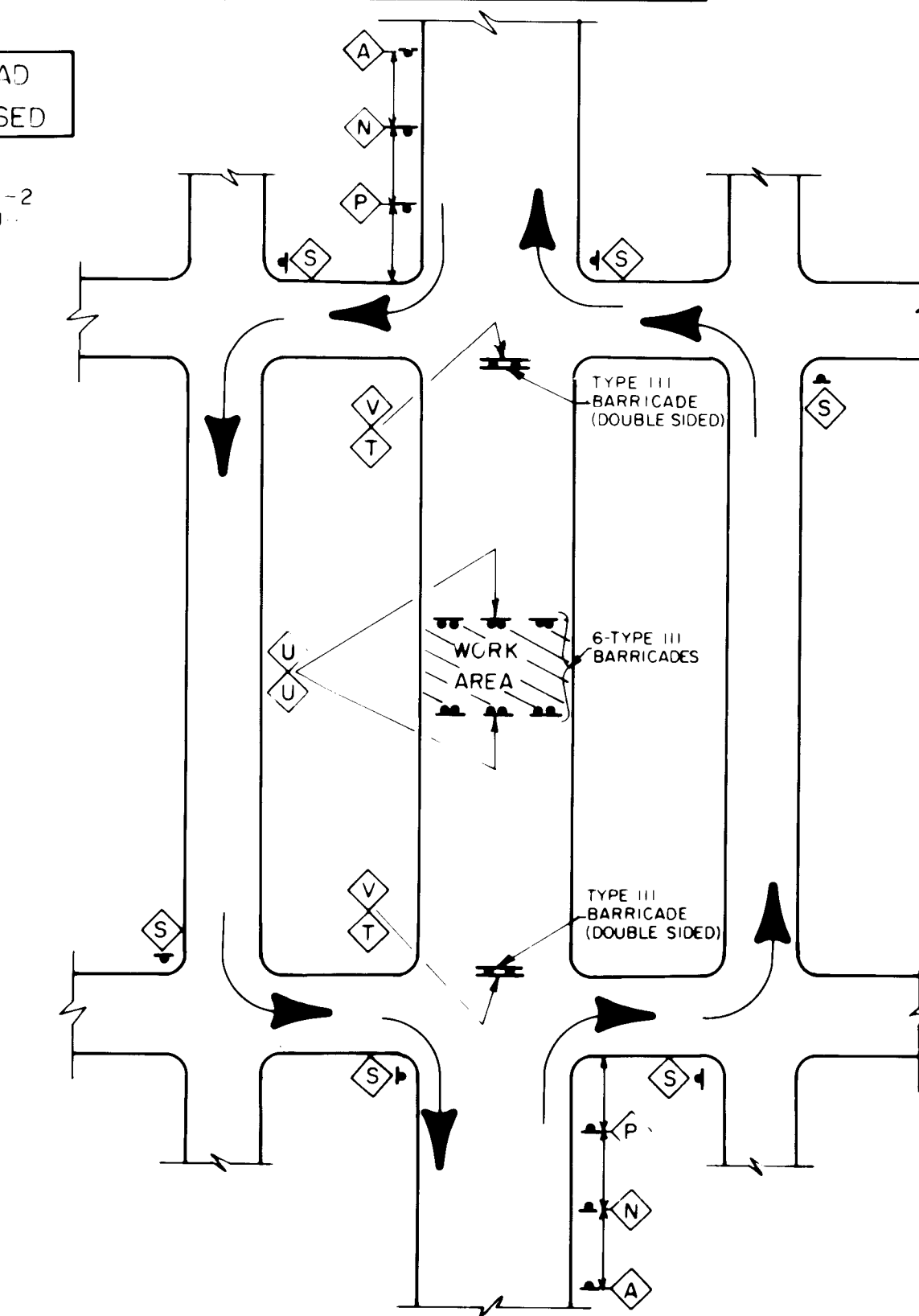
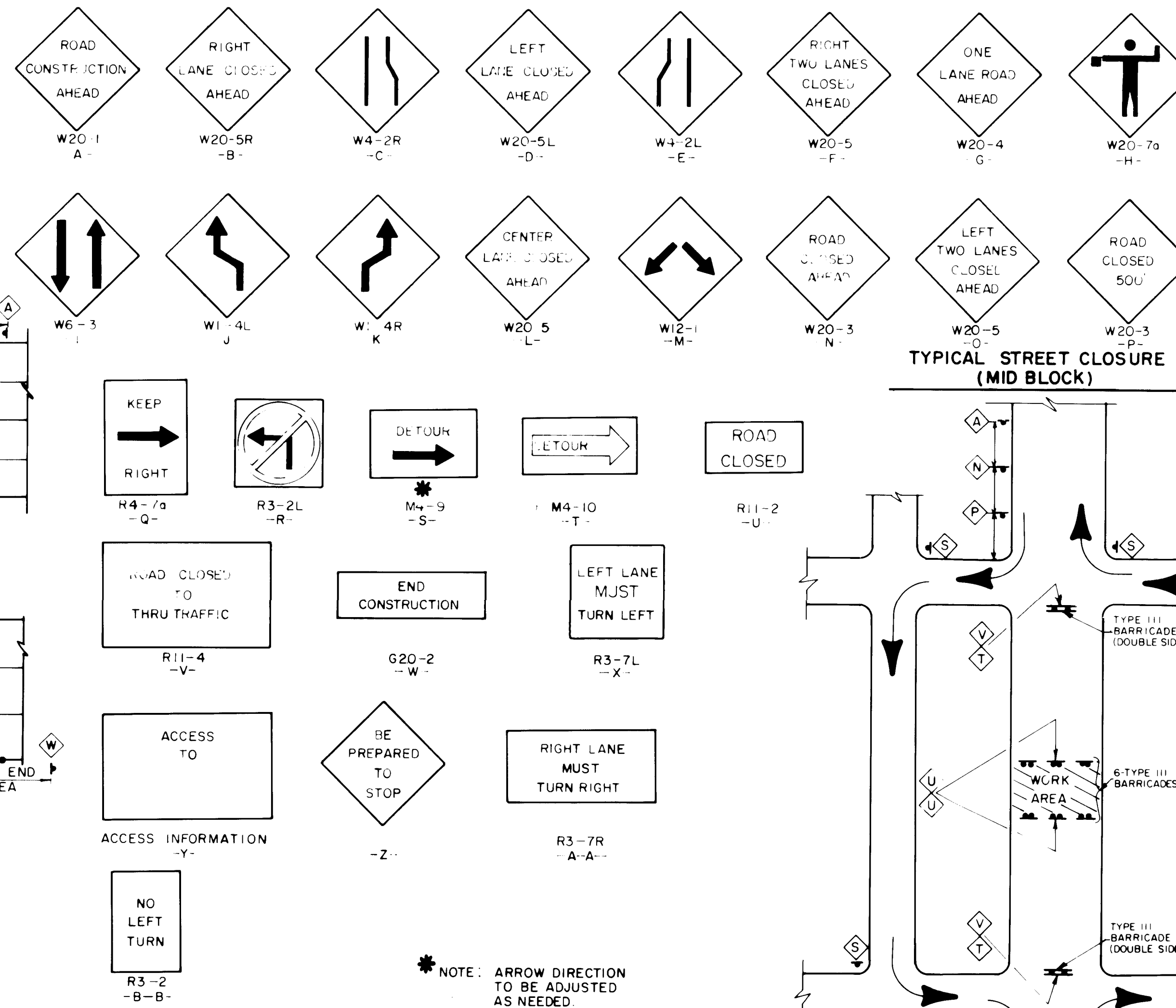
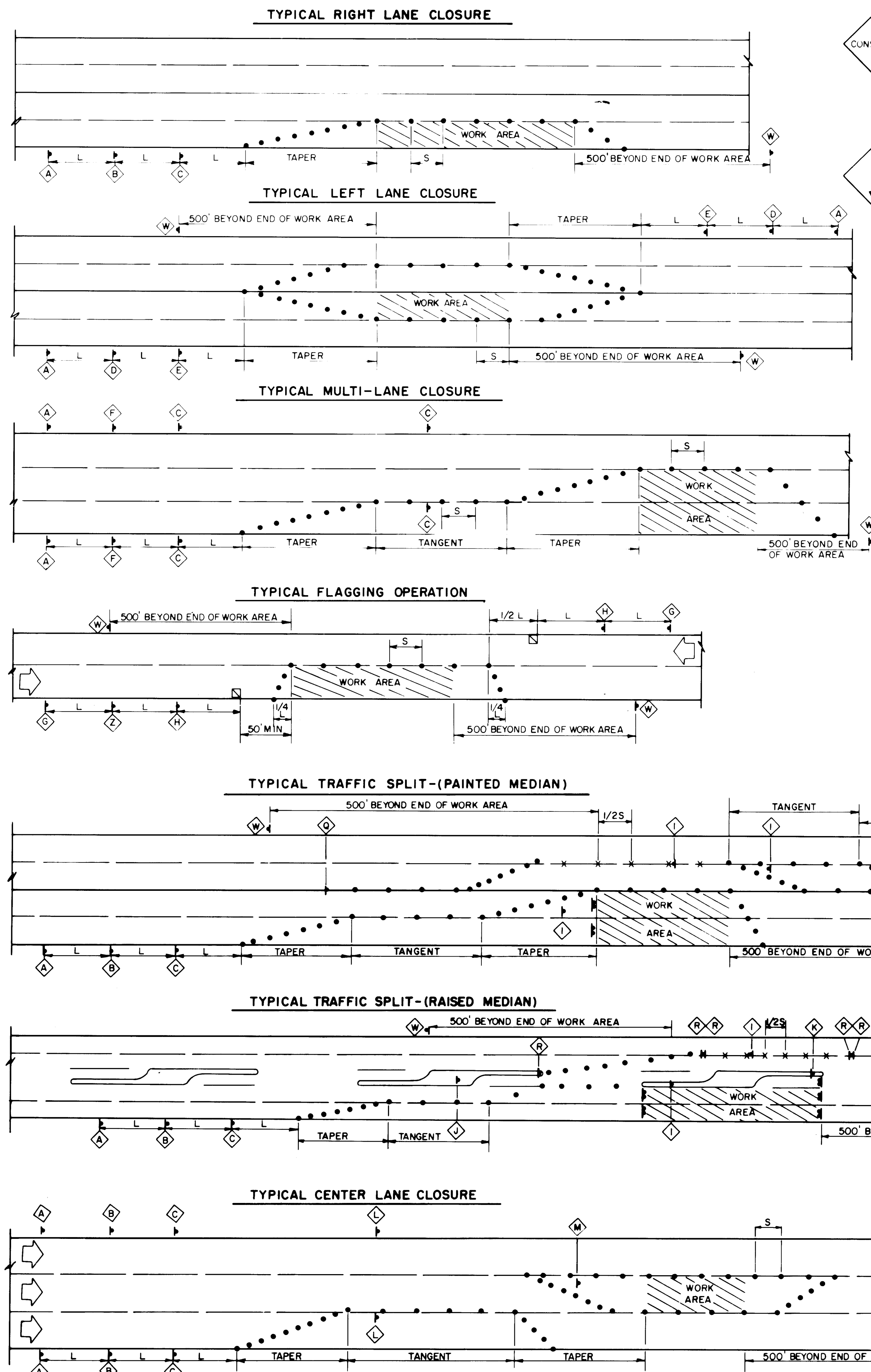
NOTE: REVERSE DETAIL @ NORTH END OF BUSBAY

DETAIL

SCALE: 1" = 6'

BENCH MARKS		SURVEY INFORMATION		ENGINEER'S SEAL		REVISIONS		CITY OF ALBUQUERQUE PUBLIC WORKS DEPARTMENT ENGINEERING GROUP	
NO.	DATE	NO.	DATE	NO.	DATE	NO.	DATE	NO.	DATE
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19	9/90	19	9/90	19	9/90	19	9/90	19	9/90
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98	9/90	98	9/90	98	9/90	98	9/90	98	9/90
99	9/90	99	9/90	99	9/90	99	9/90	99	9/90
100	9/90	100	9/90	100	9/90	100	9/90	100	9/90

PROJECT NO. 4296.90 MAPB-13/ NO. C-13 SHEET 8 OF 11



RECORD DRAWING

TAPER REQUIREMENTS

SPEED LIMIT (MPH)	TAPER LENGTH (FEET)			MAXIMUM DISTANCE BETWEEN DEVICES (FEET)	MINIMUM NUMBER OF DEVICES REQUIRED
	10' LANE	11' LANE	12' LANE		
25	104	115	125	25	6
30	150	165	185	30	7
35	204	225	245	35	8
40	267	293	320	40	9
45	450	495	540	45	13
50	500	550	600	50	13
55	550	605	660	55	13

SCANNED BY BY IASON

26 4294.901092

CITY OF ALBUQUERQUE PUBLIC WORKS DEPARTMENT ENGINEERING GROUP

TITLE: U.S. NEW MEXICO FEDERAL CREDIT UNION/IRVING BLVD. NW AND EAGLE RANCH RD. N.W. STREET IMPROVEMENTS TRAFFIC CONTROL DETAILS

APPROVALS	ENGINEER	DATE	APPROVALS	ENGINEER	DATE
D.R.C. CHAIRMAN	Rogel Br...	7-31-91	WATER	NCO	7-22-91
TRANSPORTATION	R. Br...	7-25-91	WASTE WATER	NCO	
HYDROLOGY	R. Br...	7-30-91	NMUI	J. Br...	7-19-91

PROJECT NO. 4296.90 MAP NO. B-13 / C-13 SHEET 10 OF 11

LEGEND

NOTES

AS BUILT INFORMATION

ENGINEER'S SEAL

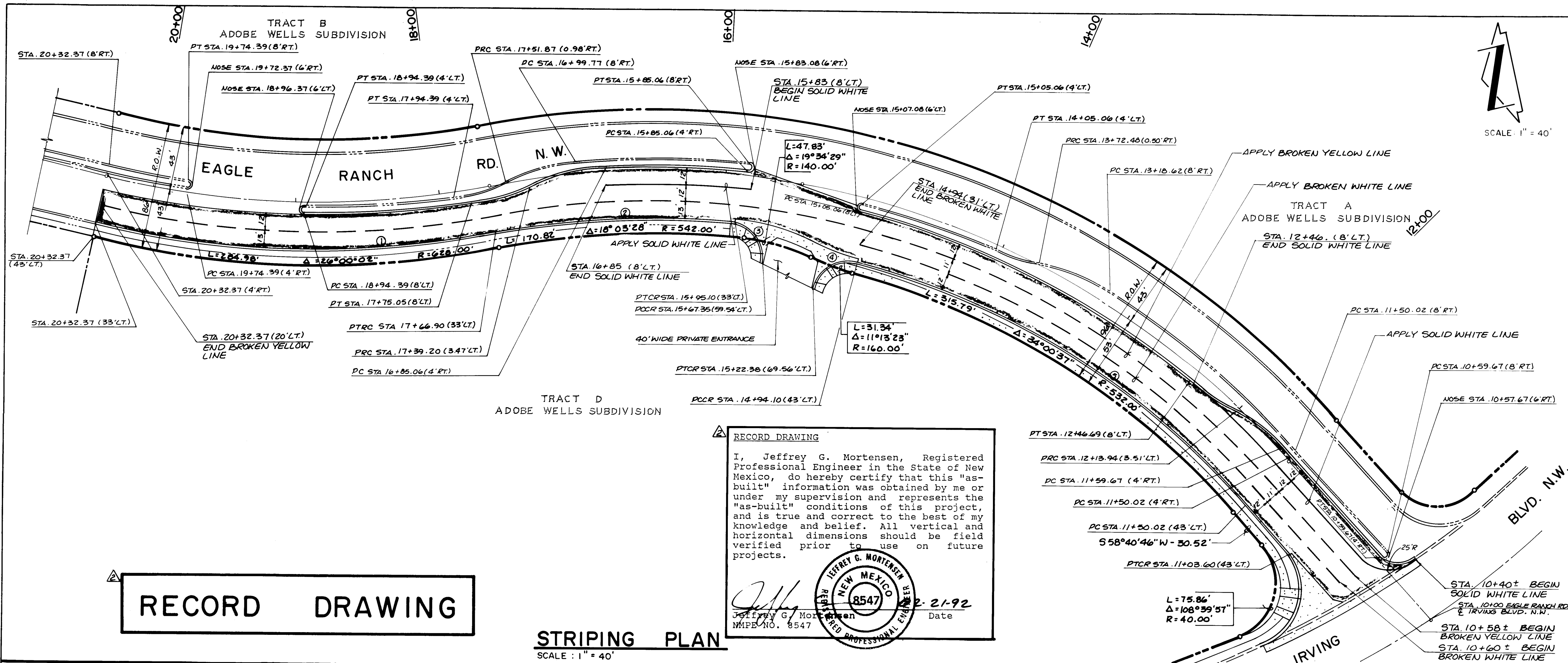
REVISIONS

DESIGN

DATE

DATE

DATE



RECORD DRAWING

STRIPING PLAN
SCALE: 1" = 40'

RECORD DRAWING
I, Jeffrey G. Mortensen, Registered Professional Engineer in the State of New Mexico, do hereby certify that this "as-built" information was obtained by me or under my supervision and represents the "as-built" conditions of this project, and is true and correct to the best of my knowledge and belief. All vertical and horizontal dimensions should be field verified prior to use on future projects.
Jeffrey G. Mortensen
NMPE NO. 8547
Date 2-21-92

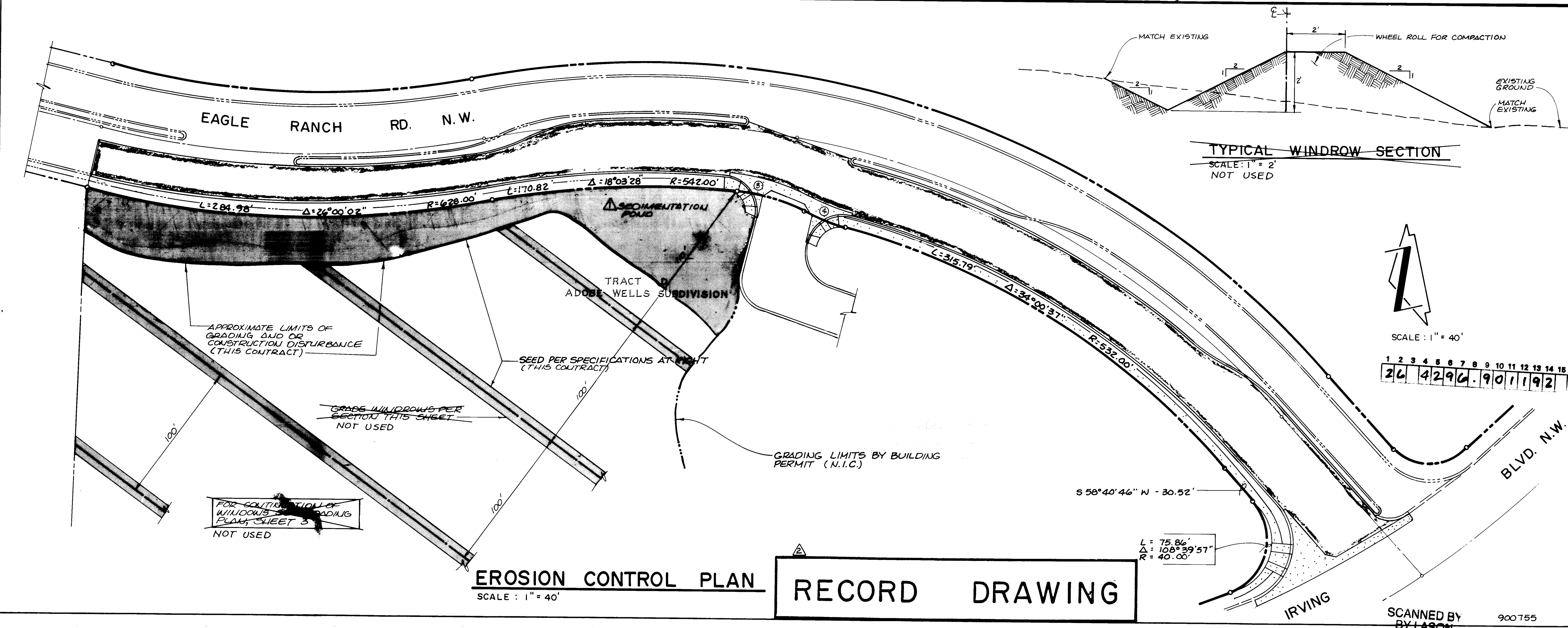
STRIPING NOTES:

- CURB AND STRIPING STATIONING BASED UPON STREET CENTERLINE STATIONS.
- PAVEMENT MARKINGS SHALL BE PLASTIC AND MAY EITHER BE THERMAL OR COLD-APPLIED.

CURB CURVE DATA

CURVE	DELTA	R	L
①	26°00'00"	618.00'	280.44'
②	16°49'37"	552.00'	162.12'
③	94°53'54"	25.00'	41.41'
④	94°59'36"	25.00'	41.45'
⑤	33°42'03"	542.00'	318.79'

AS BUILT INFORMATION		BENCH MARKS		SURVEY INFORMATION		FIELD NOTES		ENGINEER'S SEAL	
CONTRACTOR	SUNDANCE MECHANICAL	STANDARD	AS ALUMINUM TABLET STAMPED	DATE	9/90	NO.	N/A		JPK DKM BY REMARKS DESIGN NO. DATE DESIGNED BY JGM DRAWN BY SGH CHECKED BY JGM
DATE	10-91	INSPECTOR'S	LOCATED ON	DATE	3-92	DATE	07-1991		
FIELD	PLANS BY	APPROXIMATELY	0.57 OF A MILE N.W. OF COORS	DATE	3-92	DATE	07-1991		
DATE	3-92	APPROXIMATELY	0.57 OF A MILE N.W. OF COORS	DATE	3-92	DATE	07-1991		
DATE	3-92	APPROXIMATELY	0.57 OF A MILE N.W. OF COORS	DATE	3-92	DATE	07-1991		
DATE	3-92	APPROXIMATELY	0.57 OF A MILE N.W. OF COORS	DATE	3-92	DATE	07-1991		



EROSION CONTROL PLAN
SCALE: 1" = 40'

RECORD DRAWING

VEGETATION FOR EROSION CONTROL

All areas disturbed by grading and/or other construction activity shall be seeded for erosion control immediately following the completion of rough grading and subsequent disturbance. The following specifications shall apply.

- SEED RATE:

Species	Pure Live Seed (lbs/acre)*
Annual Rye Grass	8.0
Giant Dropseed	2.0
Sand Dropseed	1.0
Indian Ricegrass	4.0
Sideoats Grama	6.0
TOTAL:	21.0

* Rates apply to drilled seed. Double rates listed if seed is broadcast.

- SEED APPLICATION:

- Flat areas - cultivate area to produce an acceptable, friable seed bed, then drill seed to a depth of 1/4 to 1/2 inches.
- Slopes 3:1 or greater - hand broadcast and cultivate into top 1/4 to 1/2 inch of soil.

- FERTILIZER:

16-20-0 @ 200 lbs. per acre applied simultaneously with seed.

- MULCH:

5,000 lbs. hay mulch per acre. Hay mulch shall be crimped into the soil so as not to exceed 2 inches in depth.

- WATERING SCHEDULE:

CONTRACTOR shall maintain a wet seed bed for a period of at least 6 weeks following seeding.

CITY OF ALBUQUERQUE PUBLIC WORKS DEPARTMENT ENGINEERING GROUP					
TITLE: U.S. NEW MEXICO FEDERAL CREDIT UNION/IRVING BLVD. N.W. AND EAGLE RANCH RD. N.W. STREET IMPROVEMENTS EAGLE RANCH ROAD N.W. STA. 10+00 TO STA. 20+32.37 STRIPING AND EROSION CONTROL PLANS					
APPROVALS	ENGINEER	DATE	APPROVALS	ENGINEER	DATE
DRC CHAIRMAN	Roger J. Blum	7-31-91	WATER	ML	7-22-91
TRANSPORTATION	Blum	7-25-91	WASTE WATER	ML	
HYDROLOGY	J. Blum	7-30-91	NMUI	Blum	7-19-91
PROJECT NO.	4296.90	MAP NO.	B13/C13	SHEET	OF 11

SCANNED BY BY LASON 900755