

PINON CREEK VILLAGE  
SUBDIVISION

GENERAL NOTES

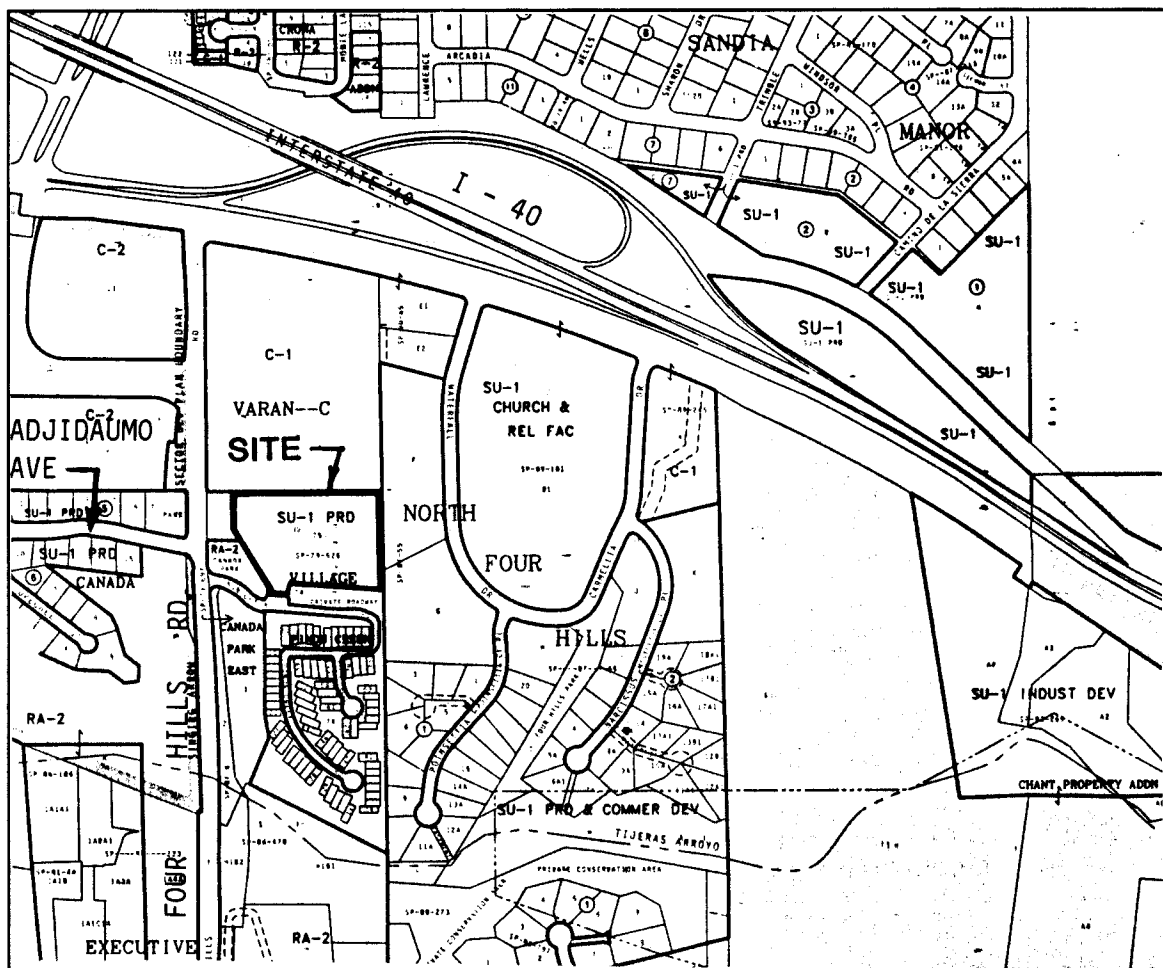
- All work detailed on these plans to be performed under contract shall, except as otherwise stated or provided for hereon, be constructed in accordance with the City of Albuquerque Standard Specifications for Public Works Construction 1986 Edition as amended with Update No. 6.
- Prior to construction, the Contractor shall excavate & verify the horizontal and vertical locations of all obstructions. Should a conflict exist, the Contractor shall notify the Engineer so that the conflict can be resolved with minimum delay.
- Two (2) working days prior to any excavation, Contractor must contact Line Locating Service @ 260-1990 for location of existing utilities.
- Contractor shall support all existing, underground utility lines which become exposed during construction. Payment for supporting work shall be incidental to water-line and/or sewerline costs.
- All utility line stationing within the subdivision refers to street centerline stationing.
- Contractor shall conduct his operations in a manner which will minimize interference with local traffic. Contractor shall comply with all applicable laws, ordinances, rules, regulations, and orders of any public body having jurisdiction for the safety of persons or property, and to protect them from damage, injury, or loss. Contractor shall erect and maintain, as required by the conditions and progress of the work, all necessary safeguards for safety continuously and not limited to normal working hours, throughout the duration of the project. Contractor shall adhere to Section 19 of the General Conditions of the City of Albuquerque Standard Specifications for Public Works Construction, 1986, as amended with Update No. 6.
- The Contractor agrees that he shall assume the sole and complete responsibility for the job site conditions during the course of construction of this project, including safety of all persons and property; that this requirement shall apply continuously and not be limited to normal working hours; and that the Contractor shall defend, indemnify, and hold harmless the Owner & Engineer from any and all liability real or alleged, in connection with the performance of work on this project, excepting liability arising from the sole negligence of the Owner or Engineer.
- Proposed waterline materials shall be either PVC pipe meeting AWWA C900 requirements or ductile iron pipe, thickness class 50.
- TRAFFIC CONTROL: Three (3) working days prior to beginning construction the Contractor shall submit to the Construction Coordination Division a detailed construction schedule. Two (2) working days prior to construction, the Contractor shall obtain a barricading permit from the Construction Coordination Division. Contractor shall notify Barricade Engineer (768-2551) prior to occupying an intersection. See section 19 of the specifications. All street striping altered or destroyed shall be replaced by Contractor to location and in kind as existing and as an incidental cost of street paving.
- The Contractor shall promptly clean up any material excavated within the public right-of-way so that the excavated material is not susceptible to being washed down the street or into any public drainage facility.
- Contractor shall assist the Engineer/Inspector in the recording of data on all utility lines and accessories as required by the City of Albuquerque for the preparation of record drawings. Contractor shall not cover utility lines and accessories until all data has been recorded.
- Existing curb and gutter not to be removed under the contract which is damaged or displaced by the Contractor shall be removed and replaced by the Contractor at his expense.
- All work on this project shall be performed in accordance with applicable federal, state, and local laws, rules, and regulations concerning construction safety and health.
- All fittings on waterlines shall have restrained joints.
- The Contractor is responsible for protecting and maintaining all existing monumentation controls. In the event of inadvertent destruction or alteration, the Contractor must immediately notify the City Chief Surveyor.
- PNM will provide at no cost to the City or the Contractor the required personnel for inspection or observation deemed necessary by PNM while the Contractor is exposing PNM's cables. However, the Contractor shall be charged the total cost associated with repairs to any damaged cables or for any cost associated with supporting or relocating the poles and cables during construction.
- WARNING--EXISTING UTILITY LINE LOCATIONS are shown in an approximate manner only, and such lines may exist where none are shown. The location of any such existing lines is based upon information provided by the utility company, the Owner, or by others, and the information may be incomplete or may be obsolete by the time construction commences.

The Engineer has undertaken no field verification of the location, depth, size, or type of existing underground utility lines, makes no representation pertaining thereto, and assumes no responsibility or liability therefor. The Contractor shall inform itself of the location of any utility line in or near the area of the work in advance of and during excavation work. The Contractor is fully responsible for any and all damage caused by its failure to locate, identify, and preserve any and all existing utilities. The Contractor shall comply with State statutes, municipal and local ordinances, rules and regulations pertaining to the location of these lines and facilities, in planning and conducting excavation, whether by calling or notifying the utilities, complying with "Blue Stakes" procedures, or otherwise.

- Contractor is responsible for contacting Water Systems (857-8200) for operation of water valves. Water distribution system shut-offs and turn-ons shall be per Section 18.3 of the General Conditions. The Contractor shall contact Water Systems five (5) working days prior to making a connection.
- New water mains shall be Class 150 PVC. New water service lines shall be Type K Copper.
- New sewer mains shall be SDR 35 PVC.
- The Contractor shall call the Engineer (268-8828) to witness all rock excavation. No rock excavation payment will be allowed unless witnessed by the Engineer.
- Orange Barrel Policy (#FW-021). Certain portion of the work covered by these drawings may be affected by the City Orange Barrel Policy for CIP projects as determined by the Public Works Department during the work order phase and in conjunction with securing a barricading permit for the work.

INDEX TO DRAWINGS

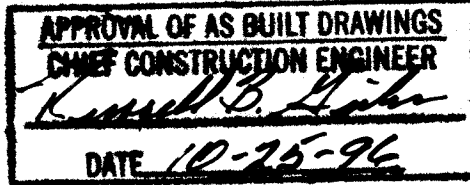
SHEET	TITLE
1	VICINITY MAP & GENERAL NOTES
2	SUBDIVISION PLAT
3	GRADING PLAN
4	RETAINING WALL ELEVATIONS & DETAILS
5	EARTHWORK SPECIFICATIONS & TEST PIT LOTS
6	PAVING PLAN ~ PINON CREEK TRAIL
7	SANITARY SEWER ~ FOUR HILLS ROAD
8	WATER & SANITARY SEWER ~ PINON CREEK TRAIL



VICINITY MAP

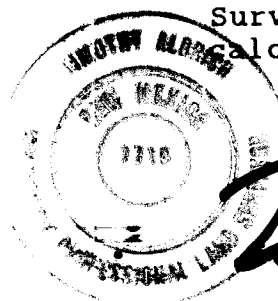
L-23

26-5-113.70 01/96



SURVEYOR'S CERTIFICATION

"I, Timothy Aldrich, a duly qualified Registered Professional Land Surveyor under the Laws of the State of New Mexico, do hereby certify that the "as-built" information shown on these drawings was obtained from field construction and "as-built" surveys performed by me, or under my supervision, that the "as-built" information shown on these drawings was added by me, or under my supervision and that this "as-built" information is true and correct to the best of my knowledge and belief." Aldrich Land Surveying, Inc., is not responsible for any of the design concepts, calculations, engineering and/or intent of the record drawings.



Timothy Aldrich 09-10-96

SCANNED BY  
BY LASON

ISAACSON & ARFMAN, P.A.  
Consulting Engineering Associates  
128 Monroe Street N.E.  
Albuquerque New Mexico  
733CVR.DWGanw 8/8/95

REV	SHEETS	CITY	ENGINEER	DATE	USER	DEPARTMENT	DATE	USER	DEPARTMENT	DATE
ENGINEERS STAMP & SIGNATURE		APPROVALS		ENGINEER	DATE	*****				
		DRC Chairman		Sally J. Beckley	10-31-95	APPROVED FOR CONSTRUCTION				
		Transportation		R. W. Kone	9-01-95					
		Water/Wastewater		R. W. Kone	8-30-95					
		Hydrology		S. Calogian	10-16-95					
		Parks								
City Project No.		5113.90		Sheet		1		Of		
								8		

NOTES

1. EASEMENT WITHIN LOT 74, PINON CREEK, FOR INGRESS, EGRESS, DRAINAGE AND UTILITIES TO LOT 75, PINON CREEK.
2. FIELD BEARINGS AND DISTANCES MATCH THOSE OF RECORD.
3. BASIS OF BEARINGS: THE NORTHERLY LINE OF LOT 75, PINON CREEK, FILED 8/20/79 IN VOLUME D-9, FOLIO 172. ROTATE PLAT BEARING 00°28'54" CLOCKWISE TO OBTAIN GRID BEARINGS.
4. DISTANCES ARE GROUND DISTANCES.
5. FIELD SURVEY PERFORMED SEPTEMBER, 1992.
6. ALL STREET CENTERLINE POINTS SHOWN THUS  $\Delta$  WILL BE MARKED BY A 4" ALUMINUM DISK STAMPED CENTERLINE MONUMENT, WITH SURVEYOR'S REGISTRATION NUMBER.
7. MANHOLES WILL BE OFFSET AT ALL POINTS OF CURVATURE AND OTHER ANGLE POINTS TO ALLOW USE OF CENTERLINE MONUMENTATION.

CURVE TABLE

STATION	CHORD BEARING	CHORD DISTANCE	ARC BEARING	ARC DISTANCE
1	N 89° 27' 00" E	77.00'	N 89° 27' 00" E	77.00'
2	N 89° 27' 00" E	65.00'	N 89° 27' 00" E	65.00'
3	N 89° 27' 00" E	65.00'	N 89° 27' 00" E	65.00'
4	N 89° 27' 00" E	65.00'	N 89° 27' 00" E	65.00'
5	N 89° 27' 00" E	65.00'	N 89° 27' 00" E	65.00'
6	N 89° 27' 00" E	65.00'	N 89° 27' 00" E	65.00'
7	N 89° 27' 00" E	65.00'	N 89° 27' 00" E	65.00'
8	N 89° 27' 00" E	65.00'	N 89° 27' 00" E	65.00'
9	N 89° 27' 00" E	65.00'	N 89° 27' 00" E	65.00'
10	N 89° 27' 00" E	65.00'	N 89° 27' 00" E	65.00'
11	N 89° 27' 00" E	65.00'	N 89° 27' 00" E	65.00'
12	N 89° 27' 00" E	65.00'	N 89° 27' 00" E	65.00'
13	N 89° 27' 00" E	65.00'	N 89° 27' 00" E	65.00'
14	N 89° 27' 00" E	65.00'	N 89° 27' 00" E	65.00'
15	N 89° 27' 00" E	65.00'	N 89° 27' 00" E	65.00'
16	N 89° 27' 00" E	65.00'	N 89° 27' 00" E	65.00'

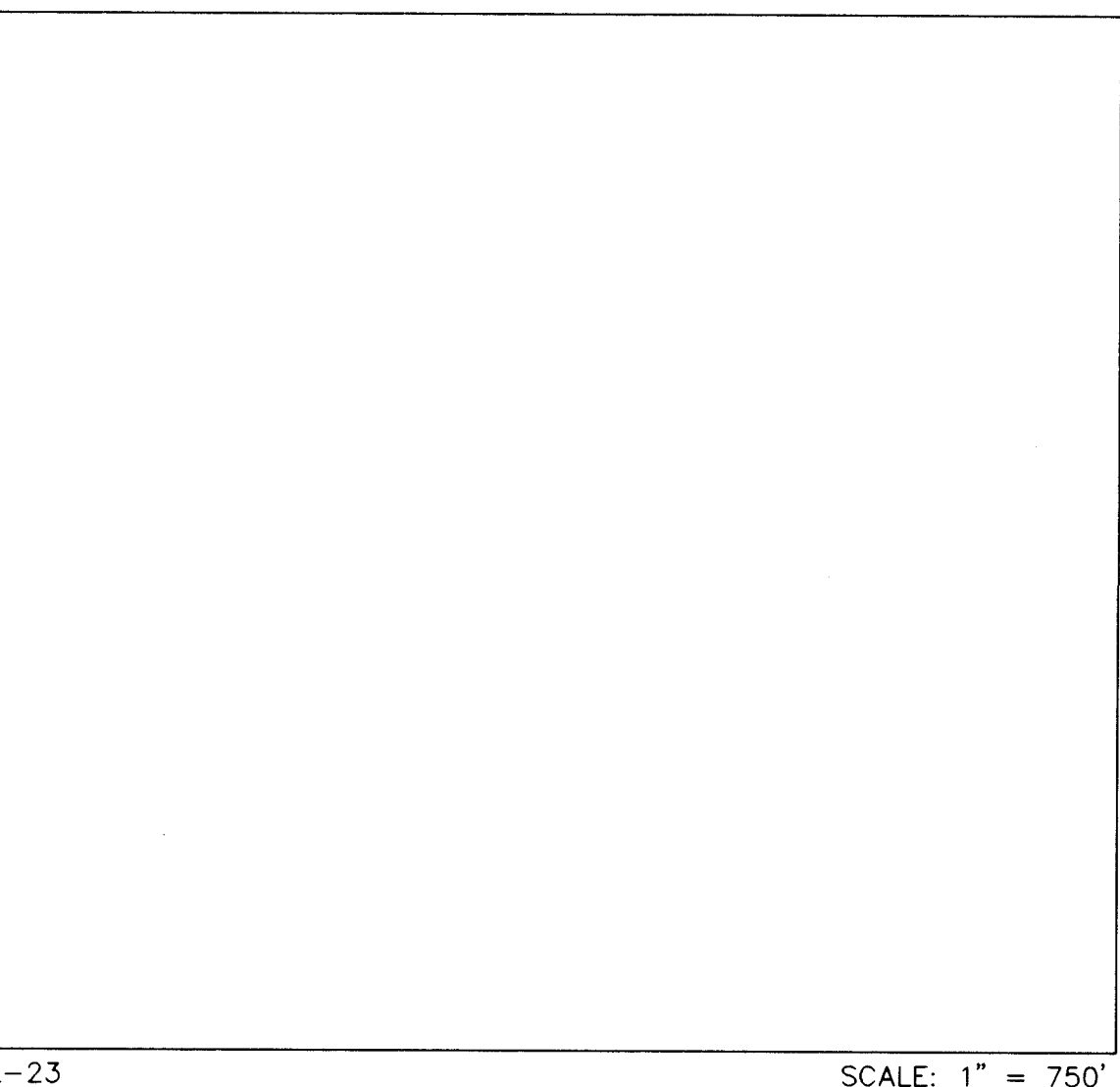
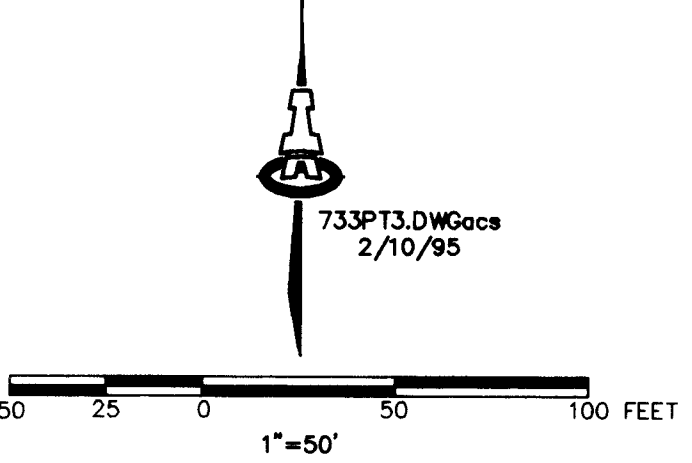
LEGEND

- 61.00' DISTANCE ALONG EASEMENT LINE
- N/R NON RADIAL LINE

PRELIMINARY PLAT OF  
PINON CREEK  
VILLAGE

BEING A REPLAT OF LOT 75, PINON CREEK  
CITY OF ALBUQUERQUE

BERNALILLO COUNTY, NEW MEXICO  
JUNE, 1994



VICINITY MAP

LEGAL DESCRIPTION

A TRACT OF LAND CONSISTING OF LOT 75 OF THE PINON CREEK SUBDIVISION, CITY OF ALBUQUERQUE, BERNALILLO COUNTY, NEW MEXICO, AS FILED IN THE OFFICE OF THE BERNALILLO COUNTY CLERK ON AUGUST 20, 1979, IN VOLUME D-9, FOLIO 172, AND MORE PARTICULARLY DESCRIBED AS FOLLOWS:

BEGINNING AT THE NORTHEAST CORNER OF THE HEREIN DESCRIBED TRACT, FROM WHENCE ACS SURVEY MONUMENT "TOM" (X = 427,964.15; Y = 1,478,915.87) BEARS N 85°40'01" E, (GRID) A DISTANCE OF 531.47 FEET; THENCE FROM BEGINNING POINT S 00°38'29" E, 390.00 FEET; THENCE N 84°51'49" W, 365.00 FEET; THENCE S 05°30'24" W, 57.10 FEET; THENCE N 84°29'36" W, 87.55 FEET; THENCE N 06°10'10" E, 17.70 FEET; THENCE N 32°37'22" W, 279.76 FEET; THENCE N 01°40'20" W, 146.77 FEET; THENCE N 89°27'00" E, 605.02 FEET TO THE POINT OF BEGINNING AND CONTAINING 4.8534 ACRES MORE OR LESS.

ACS BENCHMARK

ACS CONTROL STATION "TOM" LOCATED AS SHOWN FROM NORTHEAST SUBDIVISION CORNER, ELEVATION: 5711.67

APPROVED FOR MONUMENTATION AND  
STREET NAMES

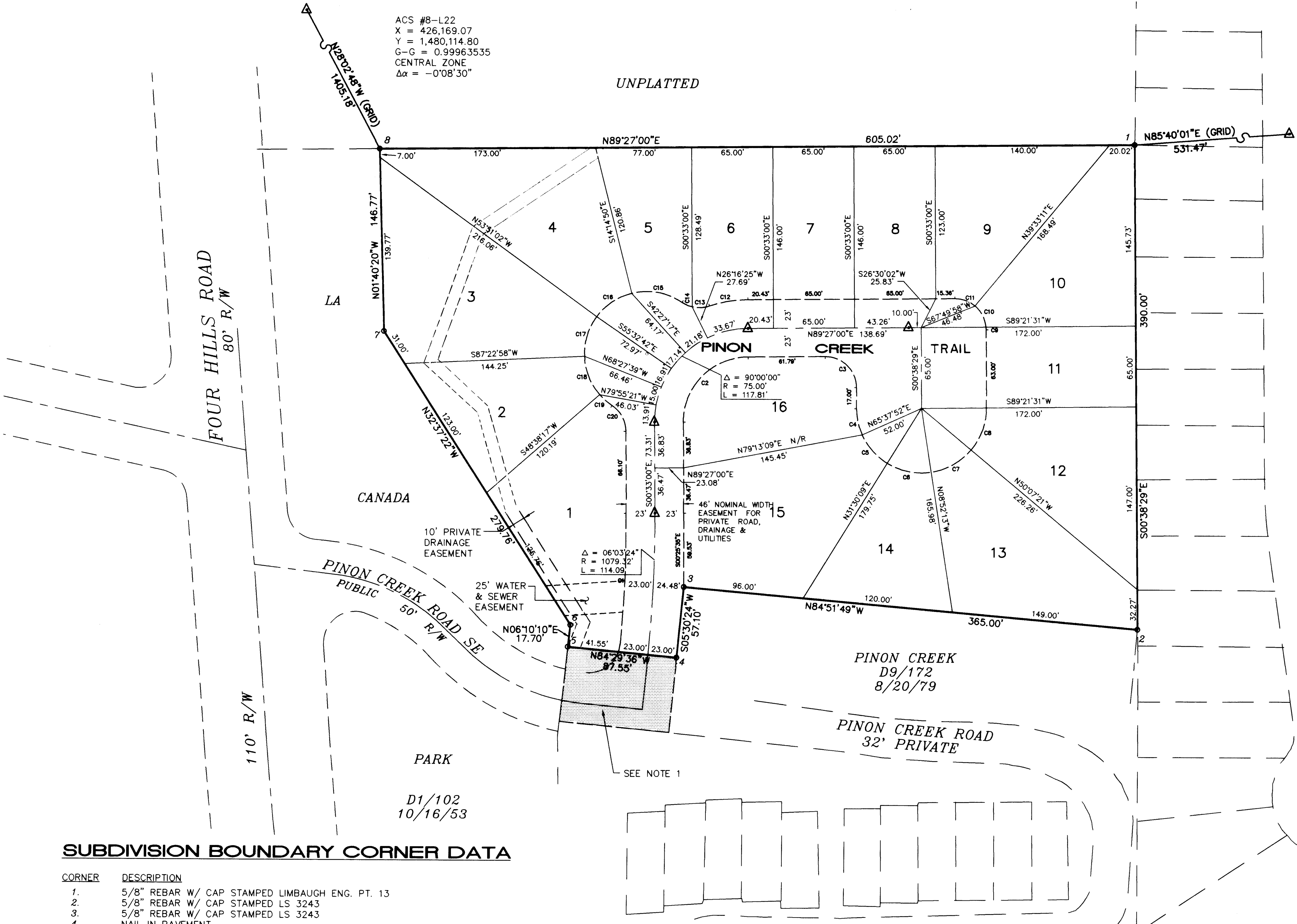
CITY SURVEYOR \_\_\_\_\_ DATE \_\_\_\_\_

OWNERSHIP

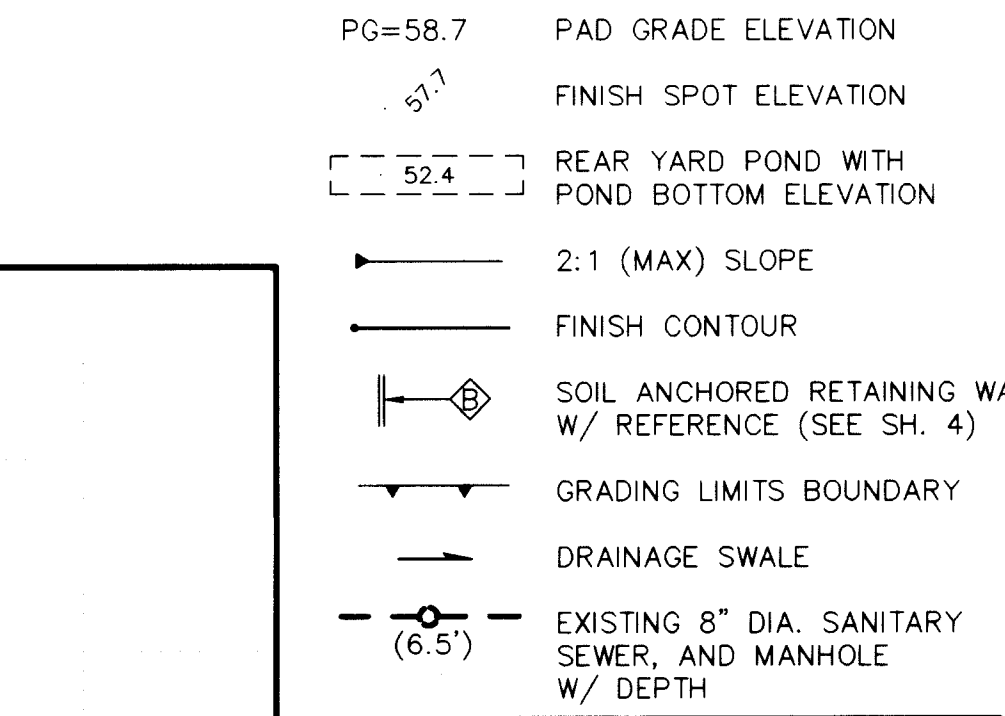
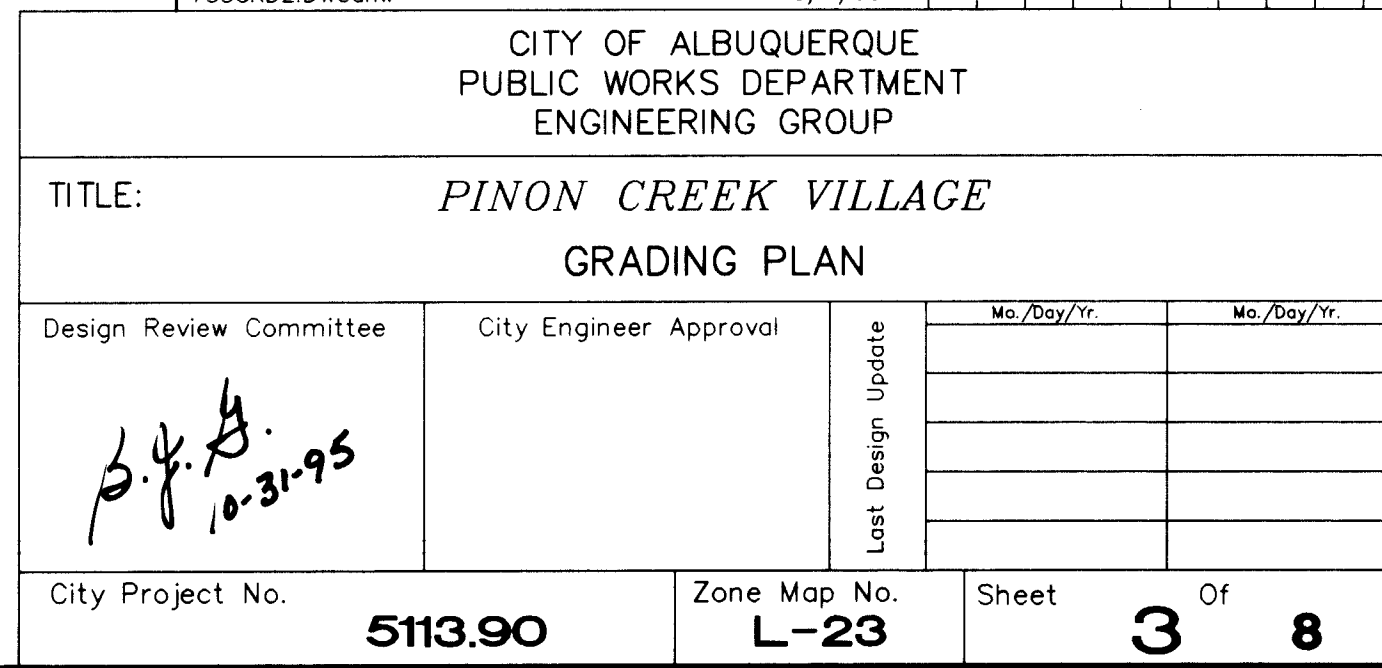
DOWNUM'S UBC, INC. \_\_\_\_\_ DATE \_\_\_\_\_  
FREDRICK E. DOWNUM, JR., PRESIDENT

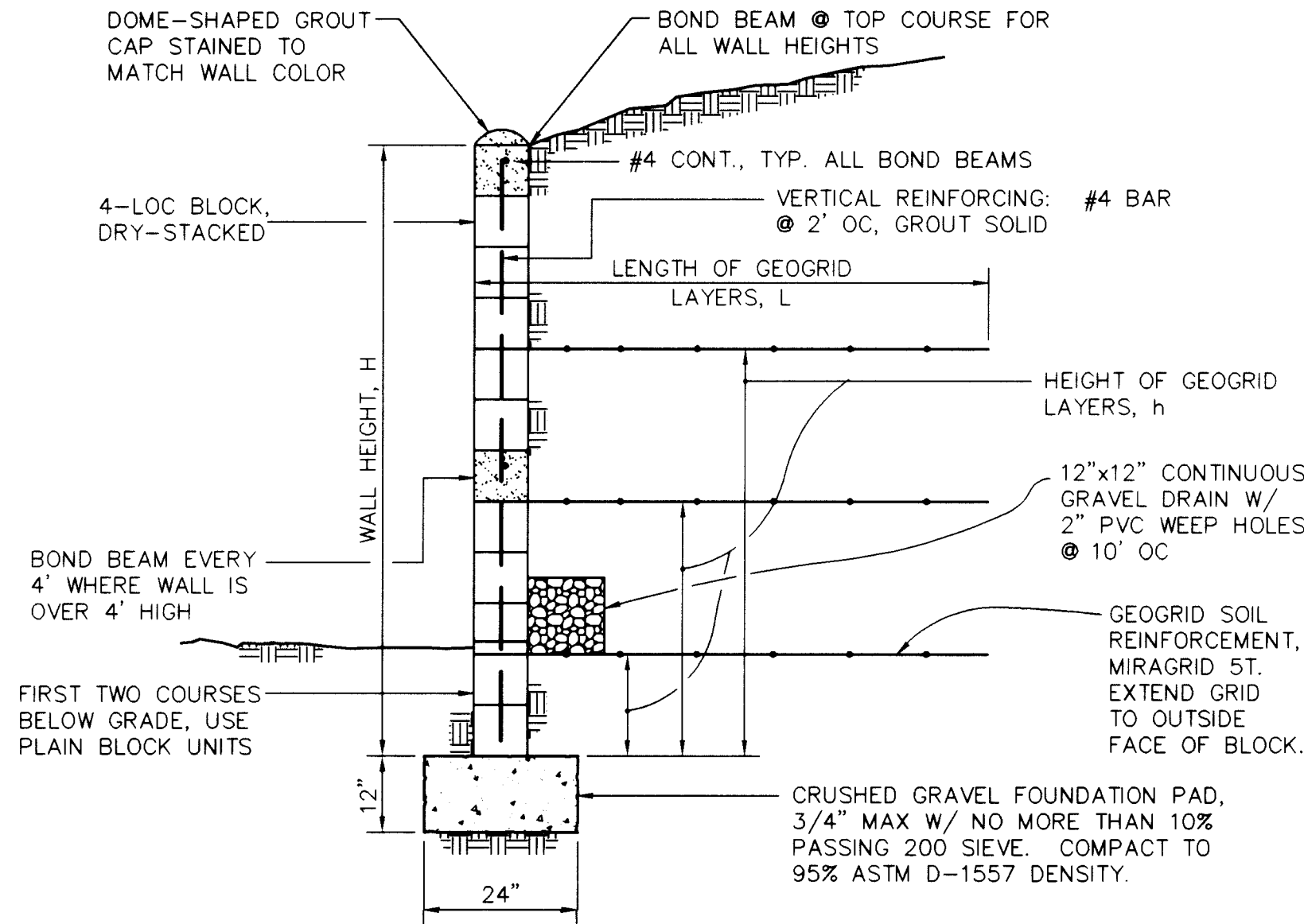
SUBDIVISION BOUNDARY CORNER DATA

CORNER	DESCRIPTION
1.	5/8" REBAR W/ CAP STAMPED LIMBAUGH ENG. PT. 13
2.	5/8" REBAR W/ CAP STAMPED LS 3243
3.	5/8" REBAR W/ CAP STAMPED LS 3243
4.	NAIL IN PAVEMENT
5.	5/8" REBAR W/ CAP STAMPED LS 3243
6.	5/8" REBAR W/ CAP STAMPED LS 1010
7.	5/8" REBAR W/ CAP STAMPED LIMBAUGH ENG. PT. F
8.	5/8" REBAR W/ CAP STAMPED LIMBAUGH ENG. PT. G





[illegible]

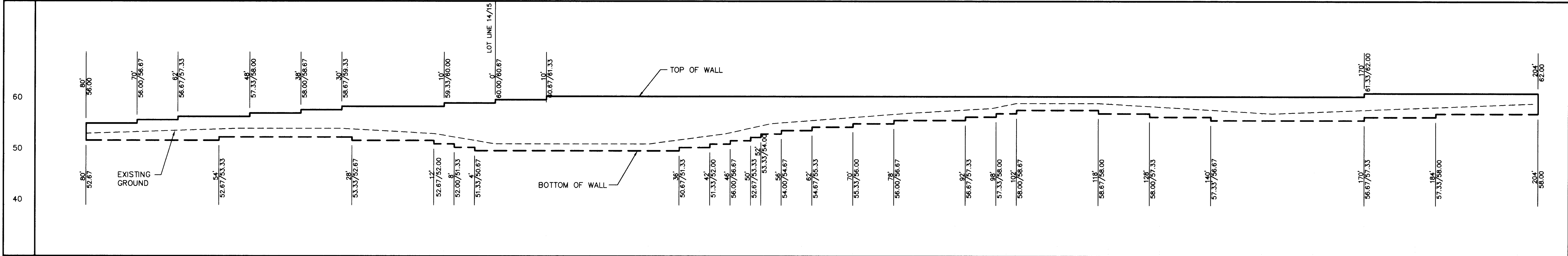


SOIL ANCHORED RETAINING WALL

SCALE: 1/2"=1'-0"

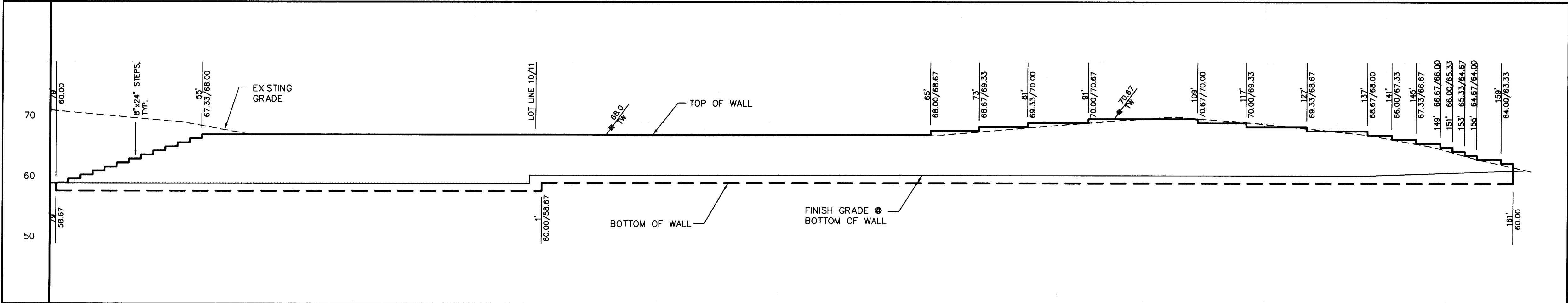
WALL HEIGHT H (FT)	REINFORCEMENT HEIGHTS h (FT)	REINFORCEMENT LENGHT L (FT)
2-3.3	1.3	4.1
4.0	1.3, 2.7	"
4.7	1.3, 3.3	"
5.3	1.3, 4.0	"
6.0	1.3, 3.3, 4.7	6.3
6.7	1.3, 3.3, 5.3	"
7.3	1.3, 3.3, 6.0	"
8.0	1.3, 3.3, 6.7	"
8.7	1.3, 3.3, 7.3	"
9.3	0.7, 2.7, 4.7, 7.3	8.0
10.0	0.7, 2.7, 4.7, 7.3	"
8.7	"	4.1
10.7	0.7, 2.7, 4.7, 7.3	8.0
9.3	"	4.1
11.3	0.7, 2.7, 4.7, 7.3	8.0
	10.0	4.1

- NOTES:
- ALL BOND BEAMS SHALL BE CONTINUOUS AND SHALL BE REINFORCED WITH 1-#4 BAR.
  - ALL GROUT SHALL BE MADE WITH 3/8" MAXIMUM SIZE AGGREGATE AND HAVE A 28 DAY COMPRESSIVE STRENGTH OF 2500 PSI. A HIGH SLUMP CONCRETE MUST BE USED. A SUPER PLASTICIZER MAY BE USED TO ACHIEVE THE HIGH SLUMP.
  - IF WALL IS UNDER 4' HIGH, FILL ALL CELLS (EXCEPT THOSE WITH VERTICAL REINFORCEMENT) WITH FILL MATERIAL OR PEA GRAVEL.
  - IF WALL IS OVER 4' HIGH, GROUT ENTIRE WALL SOLID WITH 2500 PSI CONCRETE GROUT.
  - GEOGRID SHALL BE PLACED AT HEIGHTS SHOWN IN TABLE AND THEN ROLLED UP AND TIED BEHIND WALL. WHEN BACKFILL REACHES GEOGRID LEVEL, IT SHALL BE ROLLED OUT AND STAKED TIGHT. BACKFILLING SHALL BE DONE IN 8" LIFTS AND COMPACTED TO 95% ASTM D-1557 DENSITY.



RETAINING WALL 'A' ~ Looking North

SCALE: 1"=10'



RETAINING WALL 'B' ~ Looking East

SCALE: 1"=10'

SCANNED BY LASON

AS-BUILT INFORMATION		BENCH MARKS		SURVEY INFORMATION		ENGINEER'S SEAL	
CONTRACTOR	HYDRO SYSTEMS	COA BRASS CAP "TOM"	DATE	NO.	BY		REVISIONS
MARKED BY	ALS, INC	ELEV = 5711.67					
STAKED BY		SEE VICINITY MAP, SHEET 3, FOR LOCATION.					
ACCEPTANCE BY							
FIELD DRAWING BY	ALS, INC						
CORRECTED BY	ALS, INC					DESIGNED BY	TOI
MICRO-FILM INFORMATION						DRAWN BY	ANW
						CHECKED BY	TOI
						DATE	11/94
						DATE	11/94
						DATE	11/94

26-5113.90 04996	
CITY OF ALBUQUERQUE PUBLIC WORKS DEPARTMENT ENGINEERING GROUP	
TITLE: PINON CREEK VILLAGE RETAINING WALL ELEVATIONS & DETAILS	
Design Review Committee	City Engineer Approval
Mo./Day/Yr.	Mo./Day/Yr.
City Project No. 5113.90	Zone Map No. L-23
Sheet 4	Of 8



V &amp; A

Appendix  
EARTHWORK PROCEDURES

## General

The Geotechnical Engineer shall be the Owner's representative to observe and evaluate the earthwork operations. The Contractor shall cooperate with the Geotechnical Engineer in the performance of the Engineer's duties.

## Clearing and Grubbing

Prior to placing structural fill all borrow areas and areas to receive structural fill shall be stripped of vegetation and deleterious materials. Strippings shall be hauled offsite or stockpiled for subsequent use in landscaped areas or non structural fill areas as designated by the Owner or his representative and approved by the Geotechnical Engineer.

## Site Preparation - Fill Areas

Prior to placing structural fill the areas to be filled shall be scarified to a depth of eight inches and moisture conditioned as described below. The area to be filled shall then be compacted to a minimum of 95 percent of maximum density as determined by ASTM D-1557 with a minimum of twenty passes (of a minimum 20 ton static compactor) of a vibratory compactor. The vibratory compactor shall exert a minimum dynamic force of twenty tons. If vibratory compaction techniques pose a threat to the structural integrity of near by facilities a static compactor shall be used. Any soft or "spongy" areas shall be removed as directed by the Geotechnical Engineer and replaced with structural fill as described herein.

## Site Preparation - Cut Areas

Following excavation to rough grade all building and pavement areas shall be scarified to a depth of eight inches and moisture conditioned as described below. All building and paved areas shall be compacted to a minimum of 95 percent of maximum density as determined by ASTM D-1557 with a minimum of twenty passes (of a minimum 20 ton static compactor) of a vibratory compactor. The vibratory compactor shall exert a minimum dynamic force of twenty tons. If vibratory compaction techniques pose a threat to the structural integrity of near by facilities a static compactor shall be used. Any soft or "spongy" areas shall be removed as directed by the Geotechnical Engineer and replaced with structural fill as described herein.

## Foundation, Slab and Pavement Subgrade Preparation

Prior to placing reinforcement, footings, slabs, or pavement the supporting soils shall be prepared, moisture conditioned and compacted as described herein.

## Fill Material

Fill material shall be non expansive soil which may be gravel, sand, silt or clay or a combination thereof.

Sieve Size	Percent Passing By Weight
2"	100
No. 4	70 - 100
No. 200	5 - 40

Fill material shall exhibit a plasticity index of five or less. No organic or decomposable material shall be utilized. In addition to the above criteria all fill material within eighteen inches of pavement subgrade elevation shall exhibit an R-value of 50 or greater.

## Fill Placement

Fill material shall be blended as necessary to produce a homogeneous material. Fill material shall be spread in horizontal lifts no greater than eight inches in uncompacted thickness but in no case thicker than can be properly compacted with the equipment to be utilized. If fill is to be placed on slopes steeper than 5:1 (horizontal:vertical) the natural ground shall be benched with minimum three foot wide benches at maximum two foot vertical intervals.

## Moisture Conditioning

Fill material shall be dried or moistened as necessary, prior to compacting, to within  $\pm$  three percent of optimum moisture content as determined by ASTM D-1557. Moisture shall be distributed uniformly throughout each lift.

## Compaction

Structural fill shall be mechanically compacted to the following:

	Minimum Compaction ASTM D-1557
Foundation Support	95%
Slab Support	90%
Below Slab Utility Trenches	90%
General Site Grading	90%
Pavement Support	95%
Upper 12" of Subgrade	90%
All other fill below pavement	90%

Aggregate Base Course shall be compacted to a minimum of 95% of maximum density as determined by ASTM D-1557.

Asphaltic concrete shall be compacted to within 93% to 97% of maximum theoretical density as determined by ASTM D-2041.

Compaction by flooding and jetting is specifically prohibited unless authorized in advance by the Owner or his representative and the Geotechnical Engineer.

## Observation and Testing

The Geotechnical Engineer or his representative shall perform field density tests with a frequency and at the locations he feels appropriate. The Geotechnical Engineer or his representative will perform Proctor tests on representative samples of all fill material. To minimize delays the Earthwork Contractor is encouraged to submit soil samples prior to use for proctor testing.

LOG OF TEST PIT NO. 1									
Project: Pinon Creek Village					Project No. 94-1-120				
Elevation - Top of Test Hole: 5629					Date Drilled: 8/5/94				
Depth to Groundwater: not encountered					Drilling Method: 6" HSA				
Depth, feet	Blow/foot	Sample Type	Dry Density pcf	Water Content, %	Additional Testing	Unified Classification	Material Description		
5	8			2.1	1, 2	SM	SAND, very silty, fine to coarse grained, very gravelly, some cobbles, poorly graded, slightly moist, brown		
10							Weathered granite, dense		
							Bottom of hole at 10'		

LOG OF TEST PIT NO. 2									
Project: Pinon Creek Village					Project No. 94-1-120				
Elevation - Top of Test Hole: 5636					Date Drilled: 8/5/94				
Depth to Groundwater: not encountered					Drilling Method: 6" HSA				
Depth, feet	Blow/foot	Sample Type	Dry Density pcf	Water Content, %	Additional Testing	Unified Classification	Material Description		
5						SM	SAND, very silty, fine to coarse grained, very gravelly, some cobbles and boulders, poorly graded, slightly moist, brown		
							Weathered granite		
							Bottom of hole at 4'		

LOG OF TEST PIT NO. 3									
Project: Pinon Creek Village					Project No. 94-1-120				
Elevation - Top of Test Hole: 5644					Date Drilled: 8/5/94				
Depth to Groundwater: not encountered					Drilling Method: 6" HSA				
Depth, feet	Blow/foot	Sample Type	Dry Density pcf	Water Content, %	Additional Testing	Unified Classification	Material Description		
5						SM	SAND, silty, fine to coarse grained, very gravelly, poorly graded, slightly moist, brown		
							Weathered granite, dense		
							Bottom of hole at 5'		

LOG OF TEST PIT NO. 4									
Project: Pinon Creek Village					Project No. 94-1-120				
Elevation - Top of Test Hole: 5657					Date Drilled: 8/5/94				
Depth to Groundwater: not encountered					Drilling Method: 6" HSA				
Depth, feet	Blow/foot	Sample Type	Dry Density pcf	Water Content, %	Additional Testing	Unified Classification	Material Description		
5						SM	SAND, silty, fine to coarse grained, very gravelly, some cobbles, poorly graded, slightly moist, brown		
							Boulders		
							Bottom of hole at 8'		

LOG OF TEST PIT NO. 5									
Project: Pinon Creek Village					Project No. 94-1-120				
Elevation - Top of Test Hole: 5651					Date Drilled: 8/5/94				
Depth to Groundwater: not encountered					Drilling Method: 6" HSA				
Depth, feet	Blow/foot	Sample Type	Dry Density pcf	Water Content, %	Additional Testing	Unified Classification	Material Description		
5						SM	SAND, slightly silty, fine to coarse grained, gravelly, poorly graded, slightly moist to moist, brown		
10							Bottom of hole at 11'		

LOG OF TEST PIT NO. 6									
Project: Pinon Creek Village					Project No. 94-1-120				
Elevation - Top of Test Hole: 5658					Date Drilled: 8/5/94				
Depth to Groundwater: not encountered					Drilling Method: 6" HSA				
Depth, feet	Blow/foot	Sample Type	Dry Density pcf	Water Content, %	Additional Testing	Unified Classification	Material Description		
5	8			11.8	1, 2	SM	SAND, very silty to silty, fine to coarse grained, poorly graded, moist, brown		
							Clayey lens		
							Silty to slightly silty lens		
10	8			2.4	1, 2		Weathered granite		
							Bottom of hole at 11'		

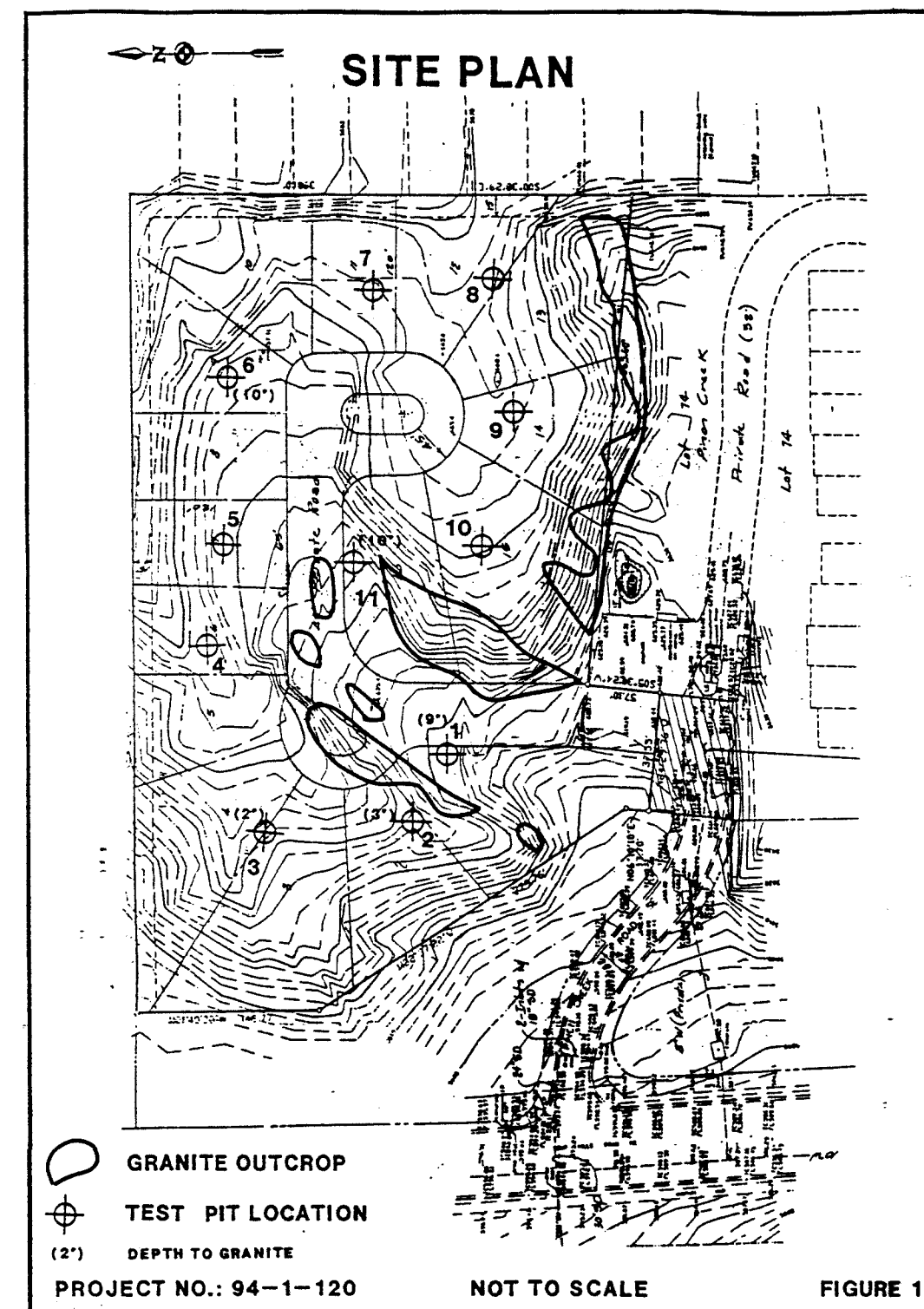
LOG OF TEST PIT NO. 7									
Project: Pinon Creek Village					Project No. 94-1-120				
Elevation - Top of Test Hole: 5664					Date Drilled: 8/5/94				
Depth to Groundwater: not encountered					Drilling Method: 6" HSA				
Depth, feet	Blow/foot	Sample Type	Dry Density pcf	Water Content, %	Additional Testing	Unified Classification	Material Description		
5	8			1.9	1, 2	SM	SAND, silty, fine to coarse grained, very gravelly, poorly graded, slightly moist, brown to light brown		
							Silty to slightly silty, medium moist, brown		
10							Bottom of hole at 9 1/2'		

LOG OF TEST PIT NO. 8									
Project: Pinon Creek Village					Project No. 94-1-120				
Elevation - Top of Test Hole: 5666					Date Drilled: 8/5/94				
Depth to Groundwater: not encountered					Drilling Method: 6" HSA				
Depth, feet	Blow/foot	Sample Type	Dry Density pcf	Water Content, %	Additional Testing	Unified Classification	Material Description		
5						SM	SAND, silty, fine to coarse grained, very gravelly, some cobbles, poorly graded, slightly moist, brown		
10						GM	GRAVEL, silty, fine to coarse, some cobbles, very sandy, fine to coarse grained, poorly graded, slightly moist, light brown		
							Bottom of hole at 9 1/2'		

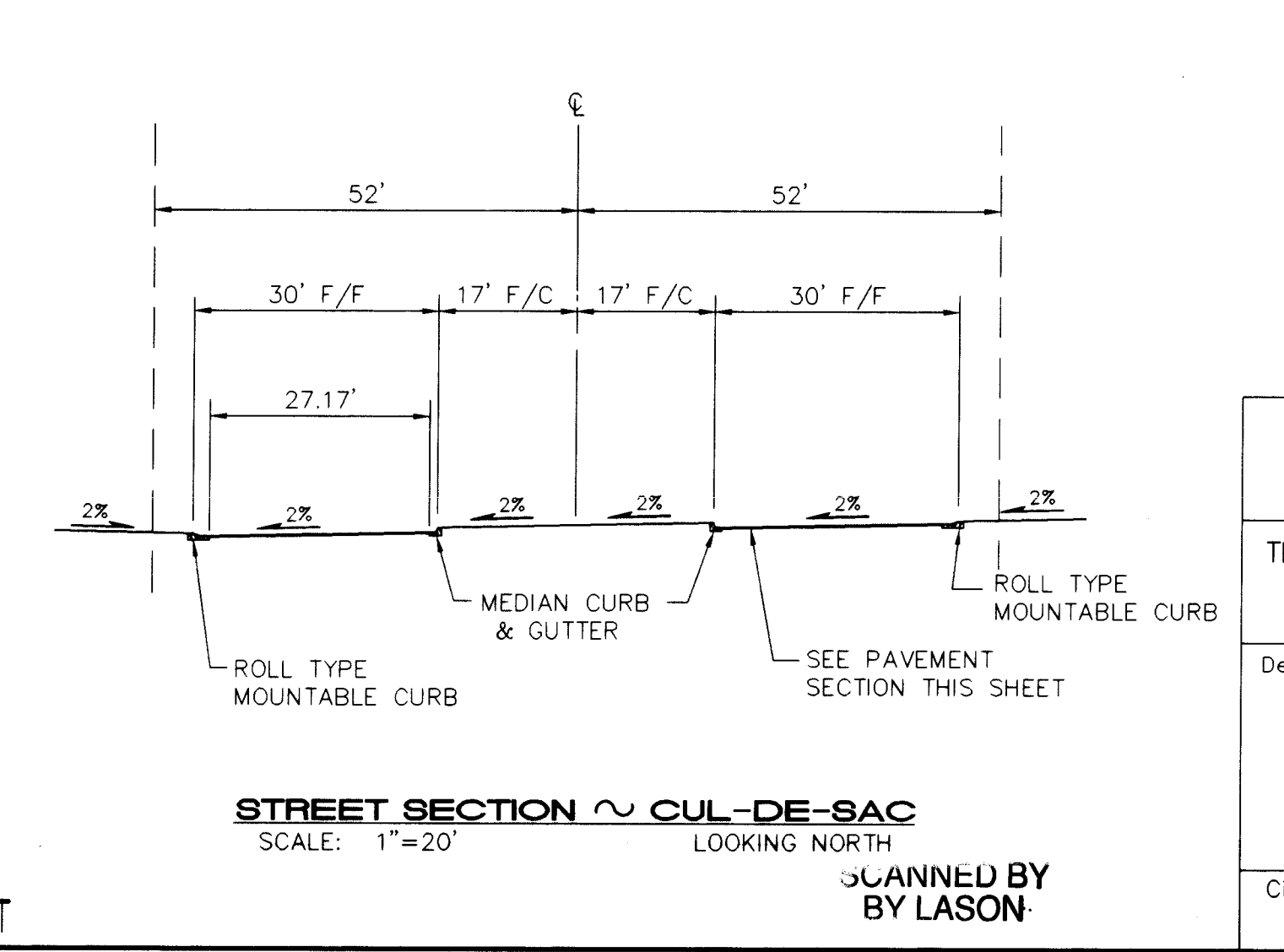
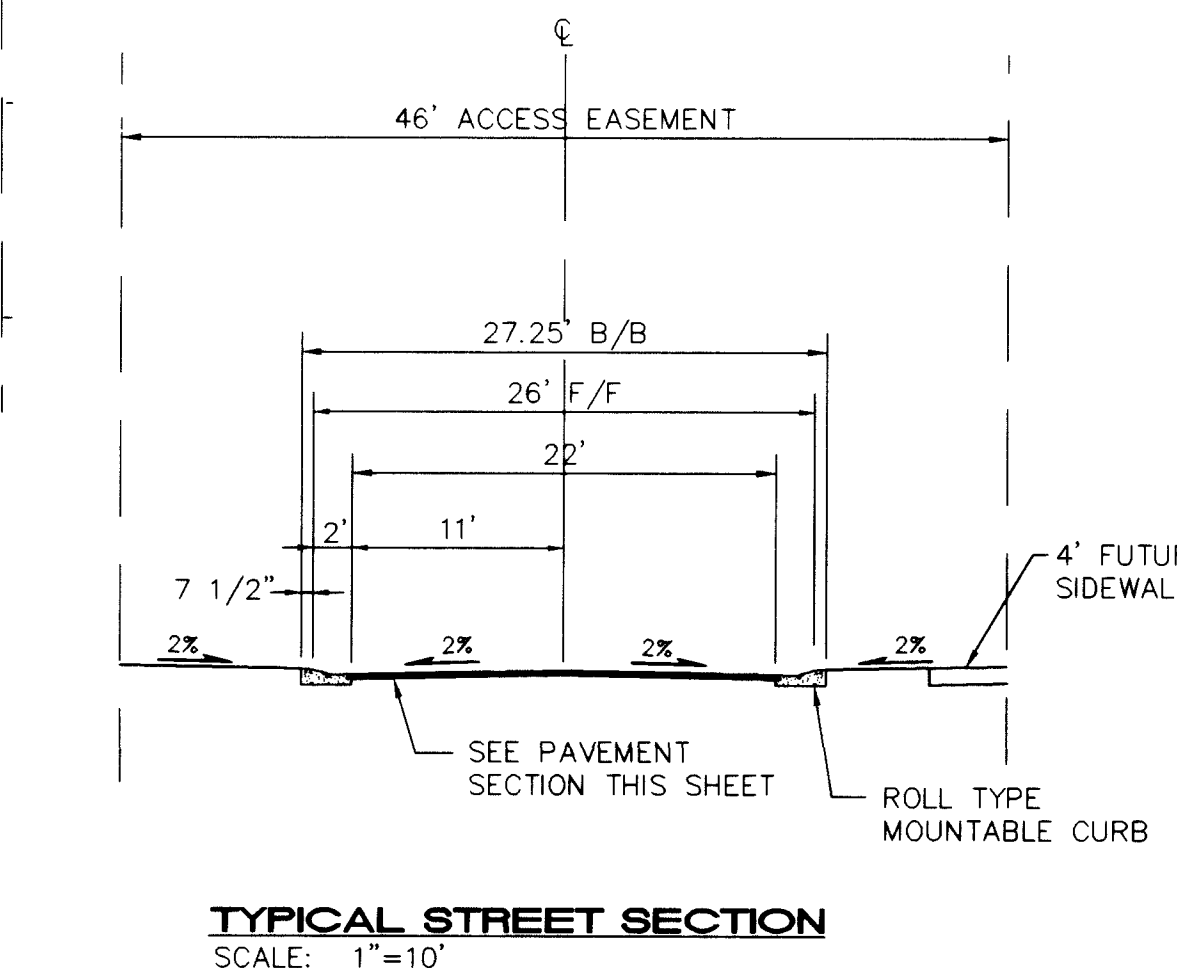
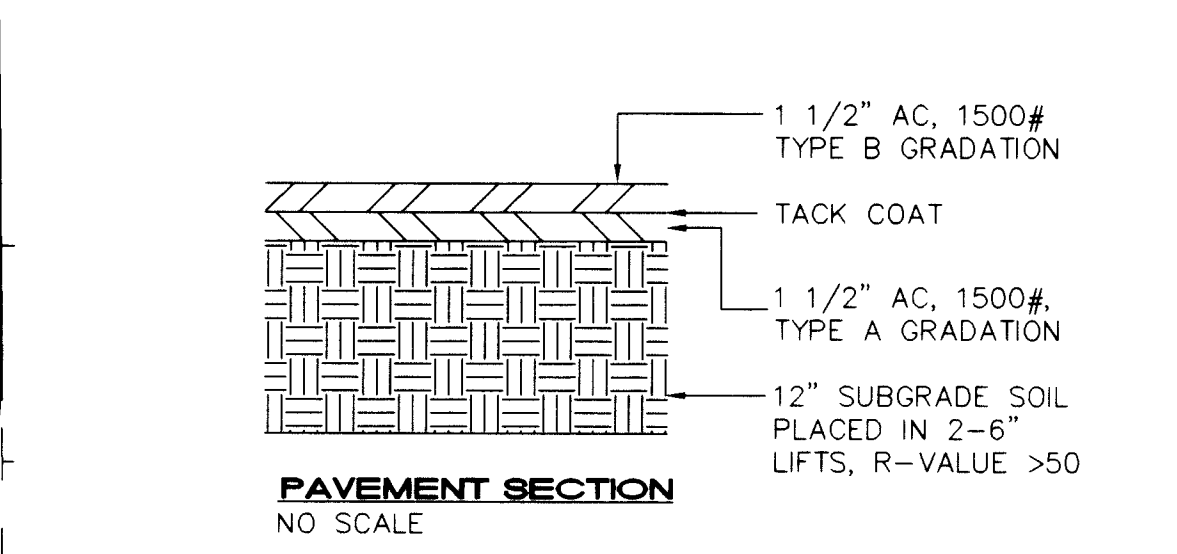
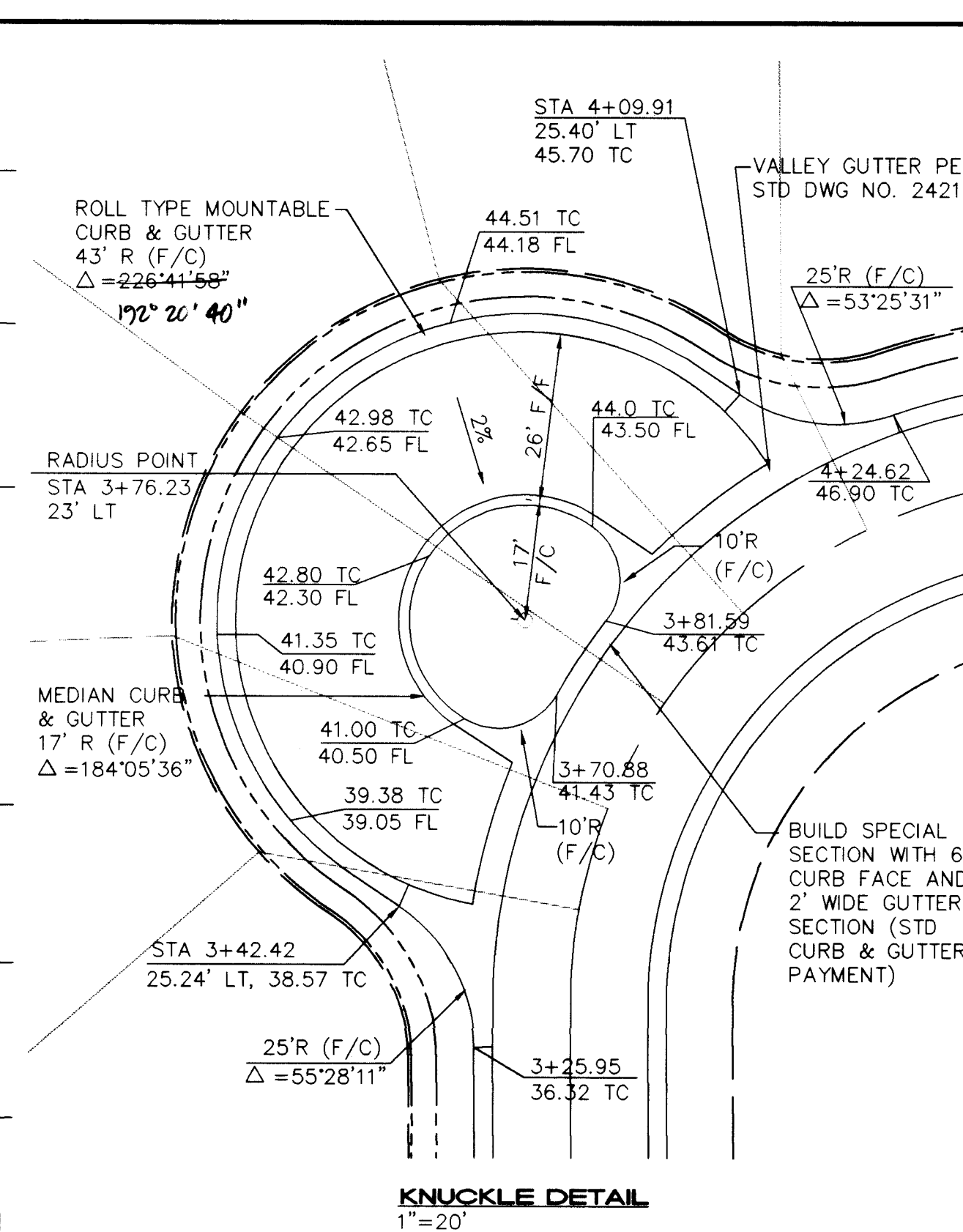
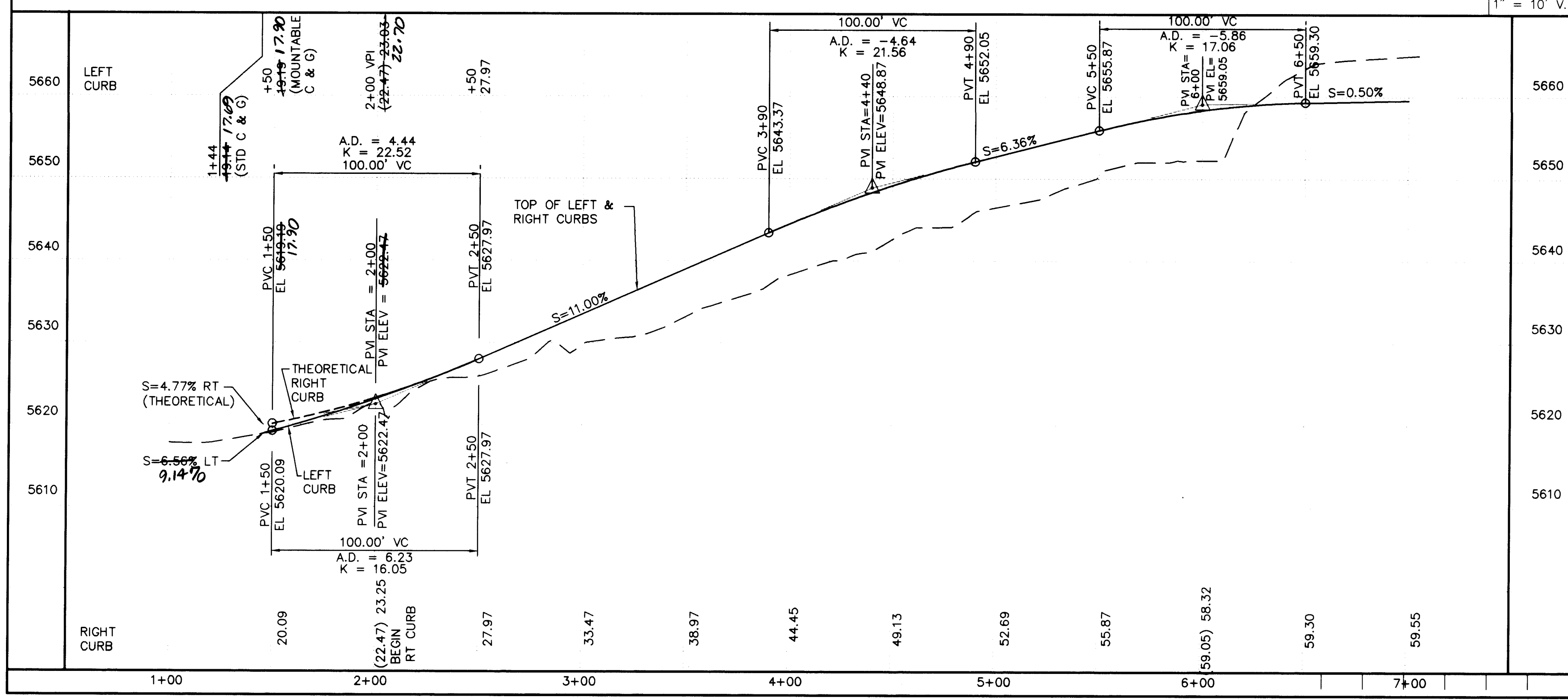
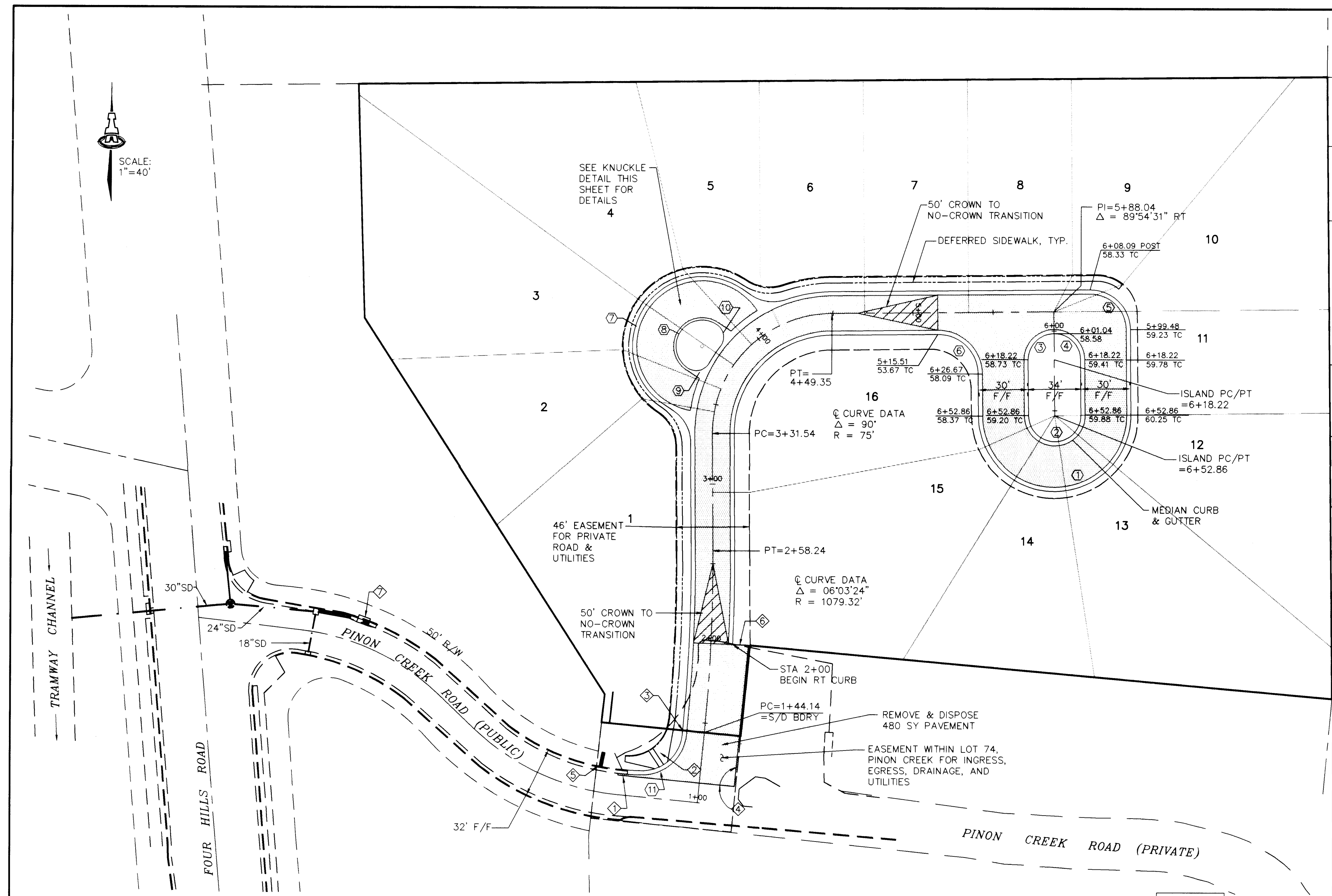
LOG OF TEST PIT NO. 9									
Project: Pinon Creek Village					Project No. 94-1-120				
Elevation - Top of Test Hole: 5669					Date Drilled: 8/5/94				
Depth to Groundwater: not encountered					Drilling Method: 6" HSA				
Depth, feet	Blow/foot	Sample Type	Dry Density pcf	Water Content, %	Additional Testing	Unified Classification	Material Description		
5	8			0.9	1	SM	SAND, very silty, fine to coarse grained, gravelly, brown		
						GM	GRAVEL, fine to coarse, some cobbles and boulders, silty, very sandy, fine to coarse grained, poorly graded, slightly moist, light brown, slightly calcareous at 2'		
10							Bottom of hole at 8'		

LOG OF TEST PIT NO. 10									
Project: Pinon Creek Village					Project No. 94-1-120				
Elevation - Top of Test Hole: 5660					Date Drilled: 8/5/94				
Depth to Groundwater: not encountered					Drilling Method: 6" HSA				
Depth, feet	Blow/foot	Sample Type	Dry Density pcf	Water Content, %	Additional Testing	Unified Classification	Material Description		
5						SM	SAND, very silty, fine to coarse grained, gravelly, fine to coarse, poorly graded, medium moist, light brown		
10						GM	GRAVEL, fine to coarse, some cobbles and boulders, silty to very silty, very sandy, fine to coarse grained, poorly graded, slightly moist, light brown		
							Bottom of hole at 8'		

LOG OF TEST PIT NO. 11									
Project: Pinon Creek Village					Project No. 94-1-120				
Elevation - Top of Test Hole: 5640					Date Drilled: 8/5/94				
Depth to Groundwater: not encountered					Drilling Method: 6" HSA				
Depth, feet	Blow/foot	Sample Type	Dry Density pcf	Water Content, %	Additional Testing	Unified Classification	Material Description		
5						SM	SAND, silty, fine to coarse grained, gravelly, fine to coarse, poorly graded, medium moist, brown		
10							Weathered granite, dense		
							Bottom of hole at 11'		



AS-BUILT INFORMATION		CONTRACTOR		HYDRO SYSTEMS		DATE		NO.	
DRAWN BY		ALC, INC		DATE		8/5/94		NO.	
CHECKED BY		ALC, INC		DATE		8/5/94		NO.	
RECORDED BY		ALC, INC		DATE		8/5/94		NO.	
MICRO-FILM INFORMATION		DATE		NO.		8/5/94		NO.	
SURVEY INFORMATION		FIELD NOTES		DATE		NO.		BY	
NO.		BY		DATE		NO.		BY	
ENGINEER'S SEAL		REMARKS		DESIGN		DATE		11/94	
NO.									



**KEYED CONSTRUCTION NOTES**

- REMOVE 6 LF EXISTING CURB AND GUTTER (TRANSITION SECTION) AND REPLACE WITH STD. CURB & GUTTER.
- BUILD STD. CURB & GUTTER & WHEEL CHAIR RAMP. RAMP TO BE PER STD DWG 2441, CASE II.
- STA 1+44.14 TO 1+50.14--TRANSITION CURB FACE FROM STD. TO MOUNTABLE CURB AND GUTTER SECTION.
- LIMITS OF ASPHALT REMOVAL & REPLACEMENT.
- BUILD 10 LF SIDEWALK CULVERT, 24" CHANNEL WIDTH, AND 2.00% SLOPE. REMOVE & REPLACE 6 LF STD CURB & GUTTER.
- REMOVE & DISPOSE EXISTING FENCE FROM EXISTING WEST END TO CLOSEST LINE POST EAST OF SUBDIVISION BOUNDARY. (INCIDENTAL WORK.)
- BUILD NEW TYPE 'A' INLET, TC=96.81, TG=95.91, INV=92.81. INSTALL 20 LF 18" DIA CLASS III RCP, INV IN=92.81, INV OUT=91.40, S=7.00%. REMOVE & REPLACE 30 LF STD CURB & GUTTER AND 13 SY RESIDENTIAL PAVEMENT PER STD DWG 2465.

**CURB & GUTTER 1/4 POINT ELEVATIONS**

MARK	POINT	TC	FL	R
①	CR	60.25	59.92	47'
	1	59.78	59.45	
	2	59.31	58.98	
②	CR	58.84	58.51	17'
	1	58.37	58.04	
	2	57.91	57.58	
③	CR	58.88	58.35	17'
	1	58.73	58.23	
	2	58.69	58.19	
④	CR	58.66	58.16	17'
	1	58.62	58.12	
	2	58.58	58.08	
⑤	CR	58.58	58.08	25'
	1	58.54	58.04	
	2	58.50	58.00	
⑥	CR	58.46	58.00	25'
	1	58.42	58.00	
	2	58.38	58.00	
⑦	CR	58.34	58.00	43'
	1	58.30	58.00	
	2	58.26	58.00	
⑧	CR	58.22	58.00	17'
	1	58.18	58.00	
	2	58.14	58.00	
⑨	CR	58.12	58.00	10'
	1	58.08	58.00	
	2	58.04	58.00	
⑩	CR	58.02	58.00	10'
	1	57.98	58.00	
	2	57.94	58.00	
⑪	CR	57.92	58.00	25'
	1	57.88	58.00	
	2	57.84	58.00	

**ENGINEER'S SEAL**

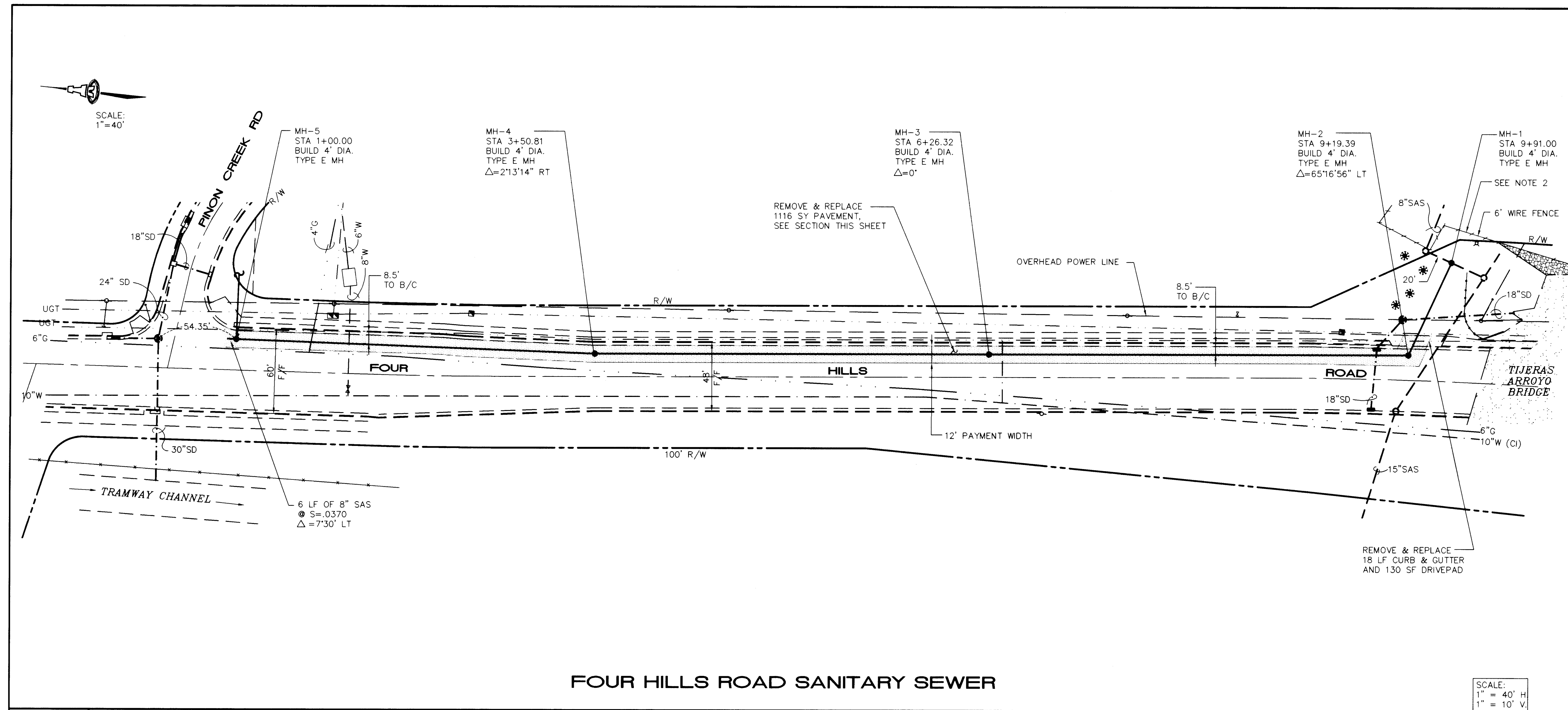
ISAACSON & ARFMAN, P.A.  
Consulting Engineering Associates  
128 Monroe Street N.E.  
Albuquerque, New Mexico  
87102  
8/7/95

**REVISIONS**

NO.	DATE	REMARKS
1	11/94	DESIGN
2	11/94	DESIGN
3	11/94	DESIGN
4	11/94	DESIGN
5	11/94	DESIGN
6	11/94	DESIGN
7	11/94	DESIGN
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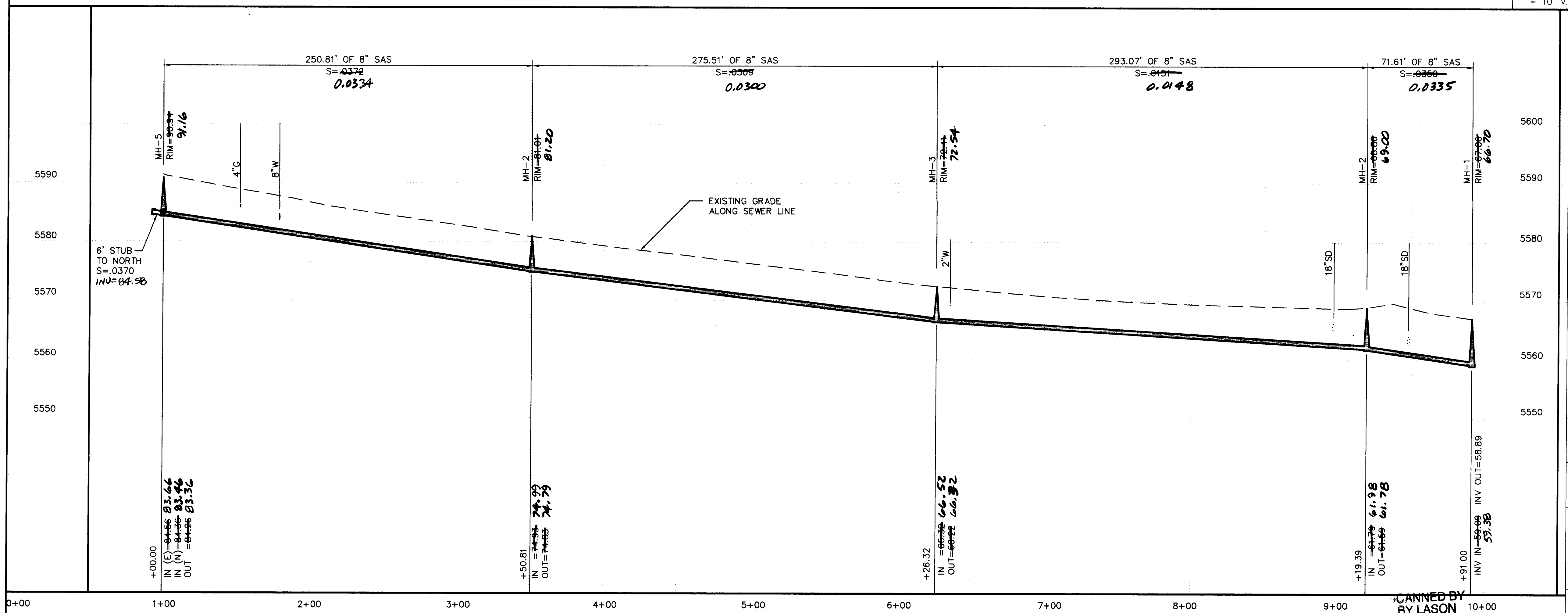
**SCANNED BY LASON**





FOUR HILLS ROAD SANITARY SEWER

SCALE:  
1" = 40' H  
1" = 10' V



NOTES

1. FIELD VERIFY ALL MANHOLE RIM ELEVATIONS DURING CONSTRUCTION LAYOUT SURVEYS. MODIFY PLAN RIM ELEVATIONS TO MATCH FIELD GRADES IF NECESSARY.
2. CONTRACTOR TO COORDINATE WORK WITH STABLE OWNER TO ASSURE ACCESS TO STABLE AS NEEDED.

LEGEND

- ✕ MONITORING WELL
- \* 6" COTTONWOOD TREES
- ◇ POWER POLE
- GUY
- TELEPHONE PEDESTAL BOX
- WATER METER
- B/C BACK OF CURB
- 8" RIPRAP SLOPE
- GUARD RAIL

- ① TACK COAT
- ② 6" CTB
- 12" SCARIFIED & COMPACTED SUBGRADE

- ① 1 1/2" ASPHALT COURSE, 1800# STABILITY
- ② 2 1/2" ASPHALT BASE COURSE, 1500# STABILITY

PAVEMENT REPLACEMENT SECTION

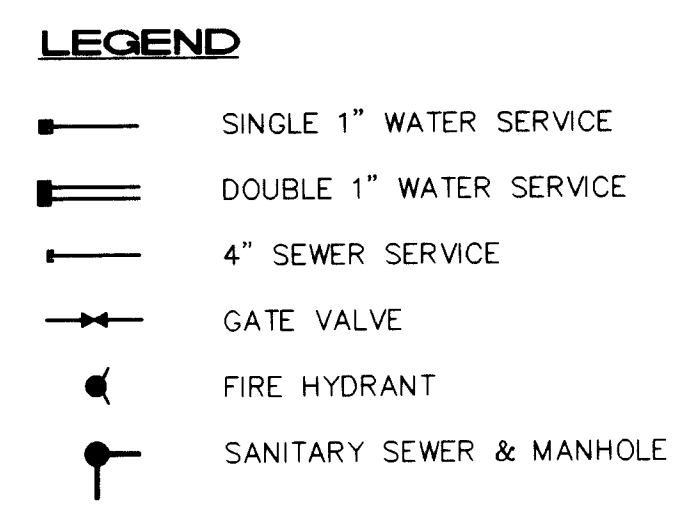
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24-5117-90 0796

ISAACSON & ARFMAN, P.A.  
Consulting Engineering Associates  
128 Monroe Street N.E.  
Albuquerque New Mexico  
7330UTL1.DWGonw VIEW: 1 8/8/95

CITY OF ALBUQUERQUE  
PUBLIC WORKS DEPARTMENT  
ENGINEERING GROUP

TITLE: PINON CREEK VILLAGE  
FOUR HILLS ROAD  
SANITARY SEWER

Design Review Committee	City Engineer Approval	Mo./Day/Yr.	Mo./Day/Yr.
B.g.A. 10.31.95			
Sheet	7	Of	8



SCALE:  
1" = 40' H.  
1" = 10' V.



LOTS 1 THRU 12 RINCON DEL RIO ADDITION  
ALBUQUERQUE, NEW MEXICO

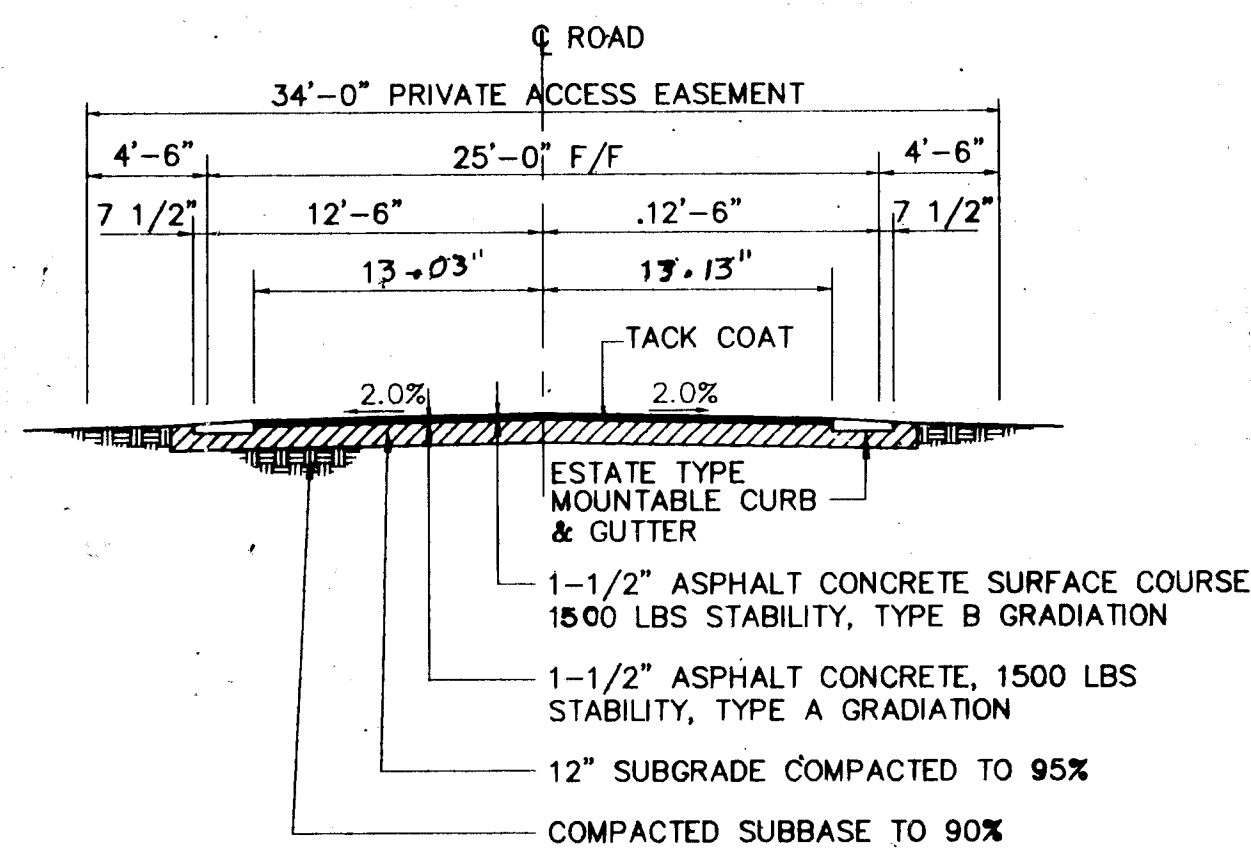
STA 3+04.06, 10 LT  
INSTALL 1-4" CAP  
W/BLOCKING  
350 LF 4" WL

LINE	BEARING	DISTANCE
L2	S 9°10'13" W	16.38
L3	N 9°10'13" E	33.95

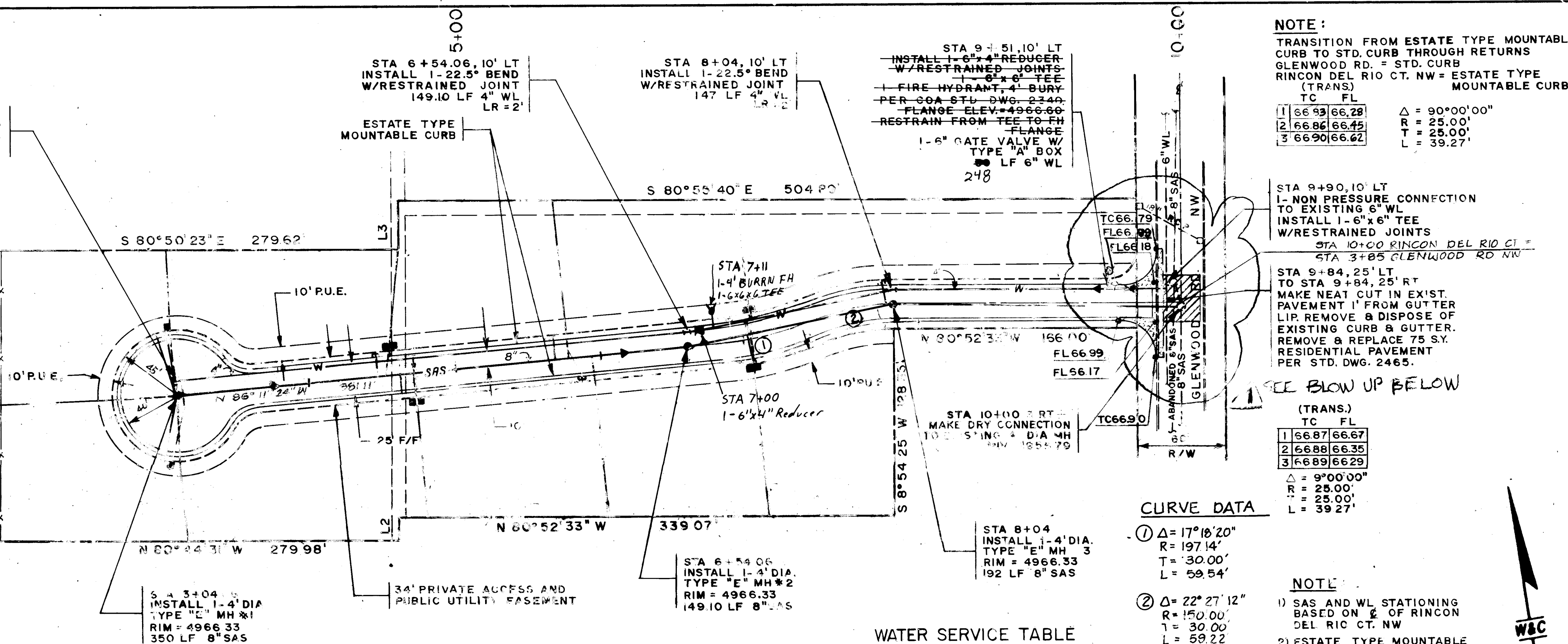
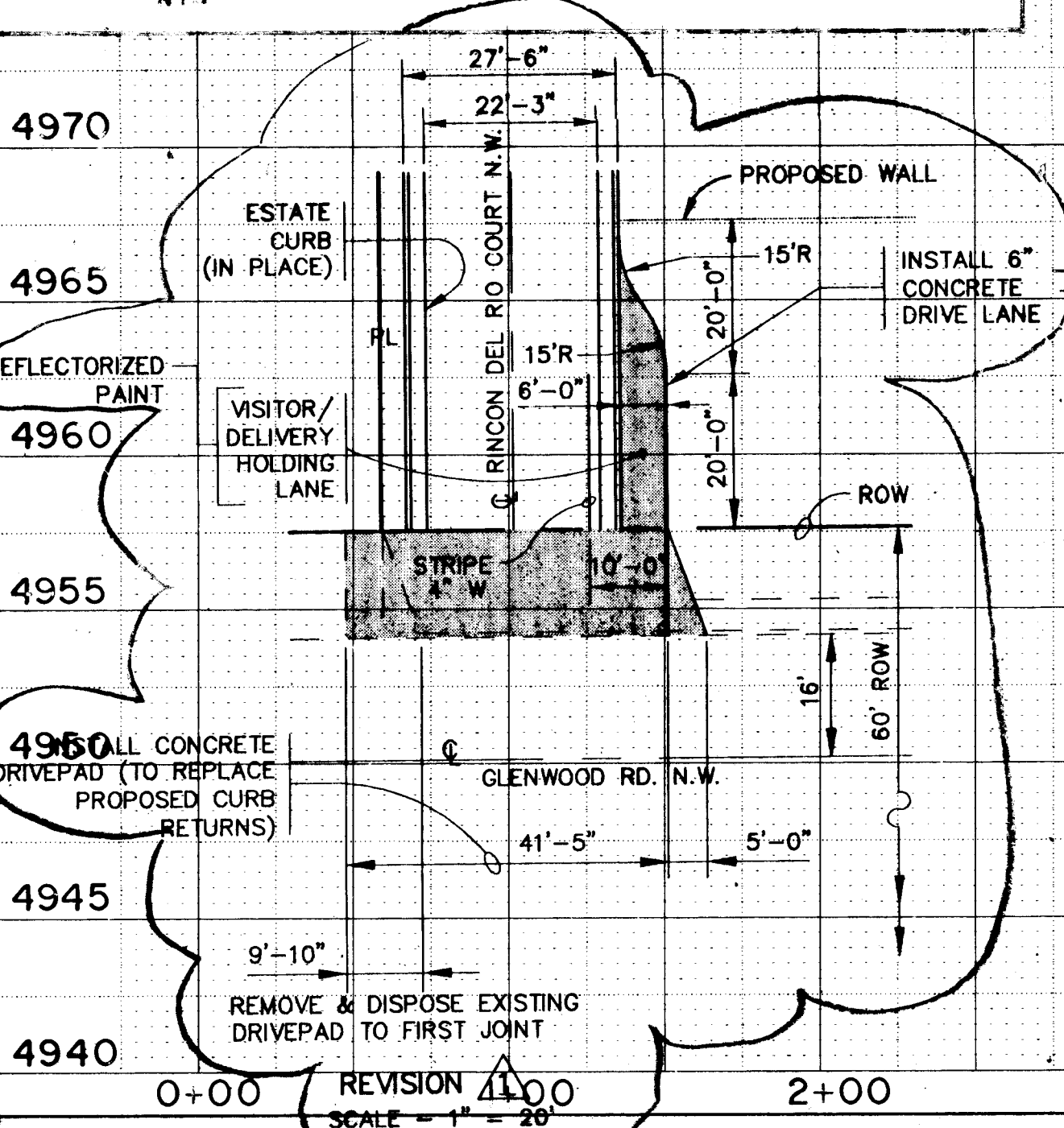
SANITARY SEWER SERVICE TABLE  
4" LINE PER STD DWG 2125

STATION	LINE	PER	ST	OW	FEET	REMARKS
		LENGTH	IN			
		LF	E.O.L	#		
3 + 06.04		44	62.0			RADIAL FROM MH
3 + 06.04		44	62.0			
4 + 46.59		17	62.0			
4 + 52.25		17	52.0			
5 + 68.43		17	62.0			
5 + 85.34		17	62.0			
6 + 30.04, 350.1T		44	52.0			
6 + 92.61, 951T		21	62.0			
7 + 92.97, 191RT		18	62.0			
8 + 02.66, 058RT		15	62.0			

\* EOL - EDGE OF LOT



### TYPICAL ROADWAY SECTION



RINCON DEL RIO COURT N.W.

## PLAN

# PROFILE

STATION	LENGTH L.F.	TYPE	REMARKS
3 + 07.23	10' LT	45 SINGLE	
3 + 10.23	0' LT	35 SINGLE	
4 + 58.16	10' LT	35 DOUBLE	
4 + 72.25	10' LT	27 DOUBLE	
7 + 02.83	14' RT	31 DOUBLE	
7 + 06.52	14' RT	31 DOUBLE	



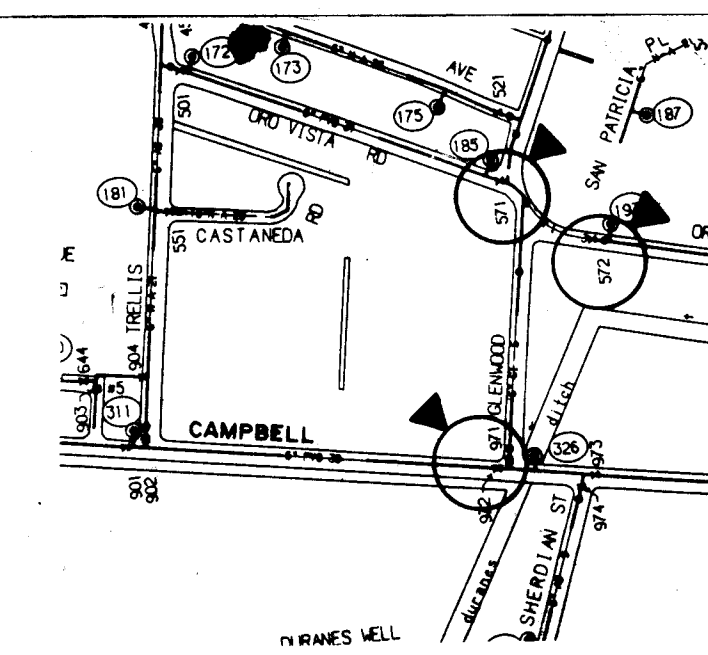
①  $\Delta = 17^{\circ} 18' 20''$   
 $R = 197.14'$   
 $T = 30.00'$   
 $L = 59.54'$

②  $\Delta = 22^{\circ} 27' 12''$   
 $R = 150.00'$   
 $T = 30.00'$   
 $L = 59.22'$

NOTE

- 1) SAS AND WL STATIONING  
BASED ON 2 OF RINCON  
DEL RIO CT. NW
- 2) ESTATE TYPE MOUNTABLE  
CURB & PAVEMENT ON RINCON  
DEL RIO CT. BY OTHERS

SCALE  
HORIZ: 1" = 50'  
VERT: 1" = 5'



### WATER VALVE SHUT-OFF PLAN

- Close Valves 971, 572, 571

NOTE

THE CONTRACTOR WILL BE RESPONSIBLE FOR COORDINATING THE EXECUTION OF THE WATER VALVE SHUT-OFF PLAN WITH THE WATER SYSTEMS DIVISION (857-8200) THREE (3) WORKING DAYS IN ADVANCE OF ANY WORK THAT MAY AFFECT EXISTING PUBLIC WATER UTILITIES.

APPROVAL OF AS BUILT DRAWING  
CHIEF CONSTRUCTION ENGINEER  
*Russell B. Hunt*  
DATE *4-29-86*

WCEA. \*94540  
SEPT. 1994

### LOCATION MAP

ZONE ATLAS MAP NO. G-12

## NOTES

ALL WORK DETAILED ON THESE PLANS TO BE PERFORMED UNDER CONTRACT SHALL, EXCEPT AS OTHERWISE STATED OR PROVIDED FOR HEREON, BE CONSTRUCTED IN ACCORDANCE WITH THE CITY OF ALBUQUERQUE STANDARD SPECIFICATIONS FOR PUBLIC WORKS CONSTRUCTION, 1986 EDITION, AS AMENDED THROUGH UPDATE 8.

TWO (2) WORKING DAYS PRIOR TO ANY EXCAVATION, THE CONTRACTOR MUST CONTACT NEW MEXICO ONE CALLS SYSTEM (260-1990) FOR LOCATION OF EXISTING UTILITIES.

PRIOR TO CONSTRUCTION, THE CONTRACTOR SHALL EXCAVATE AND VERIFY THE HORIZONTAL AND VERTICAL LOCATIONS OF ALL OBSTRUCTIONS. SHOULD A CONFLICT EXIST, THE CONTRACTOR SHALL NOTIFY THE ENGINEER OR SURVEYOR IMMEDIATELY SO THAT THE CONFLICT CAN BE RESOLVED WITH A MINIMUM AMOUNT OF DELAY.

THREE (3) WORKING DAYS PRIOR TO BEGINNING CONSTRUCTION, THE CONTRACTOR SHALL SUBMIT TO CONSTRUCTION COORDINATION DIVISION A DETAILED CONSTRUCTION SCHEDULE. TWO (2) WORKING DAYS PRIOR TO CONSTRUCTION, THE CONTRACTOR SHALL OBTAIN A BARRICADE PERMIT FROM THE CONST. COORDINATION DIVISION. CONTRACTOR SHALL NOTIFY BARRICADE ENGINEER (768-2551) PRIOR TO OCCUPANCY AND INTERSECTION. REFER TO SECTION 19 OF THE GENERAL CONDITIONS OF THE STANDARD SPECIFICATIONS.

ALL STREET STRIPING ALTERED OR DESTROYED SHALL BE REPLACED IN KIND BY CONTRACTOR TO LOCATION AND IN KIND AS EXISTING OR AS INDICATED BY THIS PLAN SET.

CONTRACTOR SHALL NOTIFY THE ENGINEER NOT LESS THAN SEVEN (7) DAYS PRIOR TO STARTING WORK IN ORDER THAT THE ENGINEER MAY TAKE NECESSARY MEASURES TO INSURE THE PRESERVATION OF SURVEY MONUMENTS. CONTRACTOR SHALL NOT DISTURB PERMANENT SURVEY MONUMENTS WITHOUT THE CONSENT OF THE ENGINEER AND SHALL NOTIFY THE ENGINEER AND BEAR THE EXPENSE OF REPLACING ANY THAT MAY BE DISTURBED. ANY REPLACEMENT REQUIREMENT SHALL BE DONE ONLY BY THE CITY SURVEYOR. WHEN A CHANGE IS MADE IN THE FINISHED ELEVATIONS OF THE PAVEMENT OF ANY ROADWAY IN WHICH A PERMANENT SURVEY MONUMENT IS LOCATED, CONTRACTOR SHALL, AT HIS OWN EXPENSE, ADJUST THE MONUMENT COVER TO THE NEW GRADE UNLESS OTHERWISE SPECIFIED. REFER TO SECTION 44.4 OF THE GENERAL CONDITIONS OF THE STANDARD SPECIFICATIONS.

THE FOLLOWING NOTES ALSO APPLY WHEN CHECKED:

- ALL UTILITIES AND UTILITY SERVICE LINES SHALL BE INSTALLED PRIOR TO PAVING.
- BACKFILL COMPACTION SHALL BE ACCORDING TO SPECIFIED STREET USE.
- TACK COAT REQUIREMENTS SHALL BE DETERMINED BY THE CITY ENGINEER.
- SIDEWALKS AND WHEELCHAIR RAMPS - WHEELCHAIR RAMPS WITHIN THE CURB RETURN SHALL BE CONSTRUCTED WHEREVER A NEW CURB RETURN IS CONSTRUCTED.
- IF CURB IS DEEPENED FOR A DRIVEPAD, THE DRIVEPAD SHALL BE CONSTRUCTED PRIOR TO ACCEPTANCE OF CURB AND GUTTER.
- ALL STORM DRAINAGE FACILITIES SHALL BE COMPLETED PRIOR TO FINAL ACCEPTANCE.

APPROVED FOR  
CONSTRUCTION  
*[Signature]*  
CITY ENGINEER

CITY OF ALBUQUERQUE  
PUBLIC WORKS DEPARTMENT  
ENGINEERING GROUP

TITLE: RINCON DEL RIO  
PAVING AND UTILITY IMPROVEMENTS  
PLAN AND PROFILE

APPROVALS	ENGINEER	DATE	APPROVALS	ENGINEER	DATE
DRC CHAIRMAN	B.G. Harding	12-15-95	WATER	R.W. Kone	12-7-94
TRANSPORTATION	R. Bunk	11-08-99	WASTE WATER	R.W. Kone	12-7-94
HYDROLOGY	Huntin	12-22-94			

PROJECT NO.	5090 90	MAP NO.	G-12	SHEET	1	OF	1
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