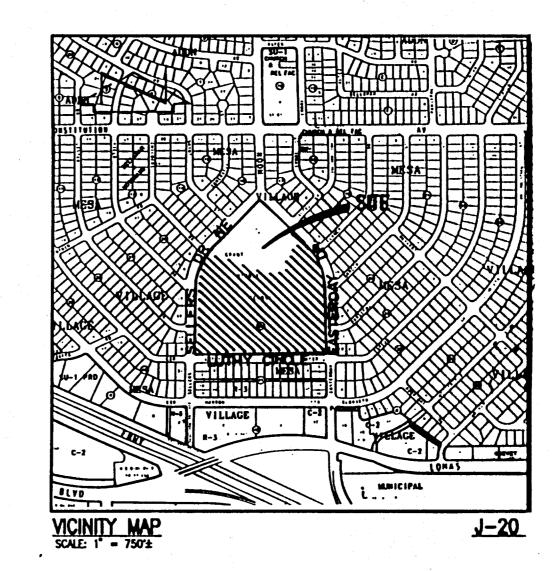
CONSTRUCTION DOCUMENTS

GRANT MIDDLE SCHOOL NEW PARK DEVELOPMENT- PHASE I

CITY PROJECT NO. 5396

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SITE / LANDSCAPE DETAILS	10 OF 10



THE FOLLOWING ALSO APPLY WHEN CHECKED

- All utilities and utility service lines shall be installed prior to paving.
- \mathbf{X} Backfill compaction shall be according to specified street use.
- Tack coat requirements shall be determined by the city engineer.
- Sidewalks and wheelchair ramps within the curb returns shall be constructed wherever a new curb return is constructed.
- If curb is depressed for a drivepad or ramp, it shall be constructed prior to acceptance of the curb and gutter.

May 30, 1996

All storm drainage facilities shall be completed prior to final acceptance.

GENERAL NOTES

- ALL WORK TO BE IN ACCORDANCE WITH THE CITY OF ALBUQUERQUE STANDARD SPECIFICATIONS, 1986 EDITION (AS UPDATED WITH REVISION #6), OR PER ATTACHED SPECIFICATIONS.
- CONTRACTOR SHALL COORDINATE WITH THE WATER SYSTEM DIVISION FOR THE EXECUTION OF THE VALVE SHUT-OFF PLAN, NOT LESS THAN FIVE (5) WORKING DAYS IN ADVANCE OF ANY WORK THAT MAY AFFECT THE EXISTING PUBLIC WATER UTILITIES. ONLY WATER SYSTEM DIVISION PERSONNEL SHALL OPERATE EXISTING VALVES DEED TO SECTION 18 OF THE SPECIFICATIONS
- TWO (2) WORKING DAYS PRIOR TO ANY EXCAVATION, CONTRACTOR MUST CONTACT NEW MEXICO ONE CALL SYSTEM, 260-1990, FOR LOCATION OF EXISTING UTILITIES.
- UTILITY LINES AND COVERS ARE SHOWN ON THESE DRAWINGS IN AN APPROXIMATE MANNER ONLY. LINES MAY EXIST WHERE NONE ARE SHOWN. THE CONTRACTOR IS FULLY RESPONSIBLE FOR ANY AND ALL DAMAGE CAUSED BY FAILURE TO LOCATE, IDENTIFY AND PRESERVE ANY AND ALL EXISTING UTILITIES, PIPELINES AND UNDERGROUND UTILITY LINES. IN PLANNING AND CONDUCTING EXCAVATION, THE CONTRACTOR SHALL COMPLY WITH STATE STATUTES, MUNICIPAL AND LOCAL ORDINANCES, RULES AND REGULATIONS, IF ANY, PERTAINING TO THE LOCATION OF THESE LINES AND FACILITIES.
- SHOULD A CONFLICT EXIST BETWEEN THE PLANS AND THE ACTUAL FIELD CONDITIONS, THE CONTRACTOR SHALL PROMPTLY NOTIFY THE LANDSCAPE ARCHITECT IN WRITING SO THAT THE CONFLICT CAN BE RESOLVED WITH THE MINIMUM AMOUNT OF DELAY FOR ALL PARTIES.
- ALL EXISTING IMPROVEMENTS ARE TO REMAIN UNLESS SPECIFICALLY NOTED TO BE REMOVED. CONTRACTOR SHALL REPAIR ANY CONTRACTOR CAUSED DAMAGE TO THE SATISFACTION OF THE OWNER AT THE CONTRACTOR'S EXPENSE. ALL REPAIRS WITHIN THE CITY RIGHT-OF-WAY SHALL BE IN ACCORDANCE WITH CITY OF ALBUQUERQUE STANDARDS.
- PRIOR TO CONSTRUCTION, THE CONTRACTOR SHALL CHECK ALL DIMENSIONS, BOTH HORIZONTAL AND VERTICAL, AND SHALL EXCAVATE AND VERIFY THE HORIZONTAL AND VERTICAL DIMENSIONS OF ALL OBSTRUCTIONS.
- THE CONTRACTOR SHALL MAINTAIN ACCESS TO ADJACENT PROPERTIES DURING CONSTRUCTION.
- ALL STORM DRAINAGE FACILITIES SHALL BE COMPLETED PRIOR TO FINAL ACCEPTANCE.
- CONTRACTOR SHALL NOTIFY CITY SURVEYING NOT LESS THAN SEVEN (7) DAYS PRIOR TO STARTING WORK IN ORDER THAT CITY SURVEYING MAY TAKE NECESSARY MEASURES TO INSURE THE PRESERVATION OF SURVEY MONUMENTS. CONTRACTOR SHALL NOT DISTURB PERMANENT SURVEY MONUMENTS WITHOUT THE CONSENT OF CITY SURVEYING AND SHALL NOTIFY CITY SURVEYING AND BEAR THE EXPENSE OF REPLACING ANY THAT MAY BE DISTURBED WITHOUT PERMISSION. REPLACEMENT SHALL BE DONE ONLY BY CITY SURVEYING. WHEN A CHANGE IS MADE IN THE FINISHED ELEVATION OF THE PAVEMENT OF ANY ROADWAY IN WHICH A PERMANENT SURVEY MONUMENT IS LOCATED, CONTRACTOR SHALL AT HIS OWN EXPENSE, ADJUST THE MONUMENT COVER TO THE NEW GRADE UNLESS OTHERWISE SPECIFIED. REFER TO SECTION 4.4 OF THE STANDARD SPECIFICATIONS.
- ANY WORK AFFECTING AN ARTERIAL ROADWAY REQUIRES TWENTY FOUR HOUR CONSTRUCTION.
- THREE (3) WORKING DAYS PRIOR TO BEGINNING CONSTRUCTION THE CONTRACTOR SHALL SUBMIT TO THE CONSTRUCTION CO-ORDINATION DIVISION A DETAILED CONSTRUCTION SCHEDULE. TWO (2) WORKING DAYS PRIOR TO CONSTRUCTION THE CONTRACTOR SHALL OBTAIN A BARRICADING PERMIT FROM THE CONSTRUCTION CO-ORDINATION DIVISION. CONTRACTOR SHALL NOTIFY BARRICADE ENGINEER (768-2551) PRIOR TO OCCUPYING AN INTERSECTION.

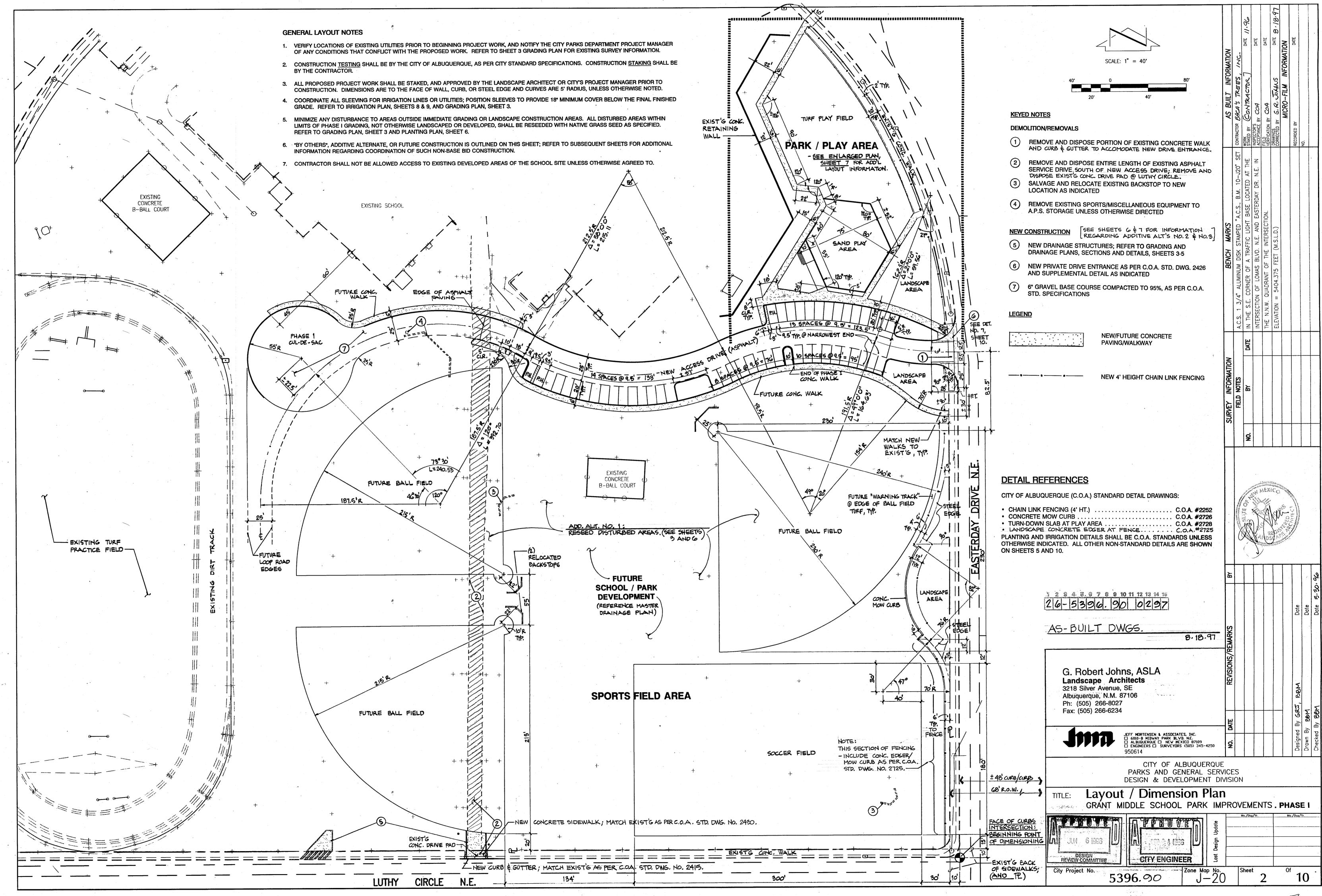
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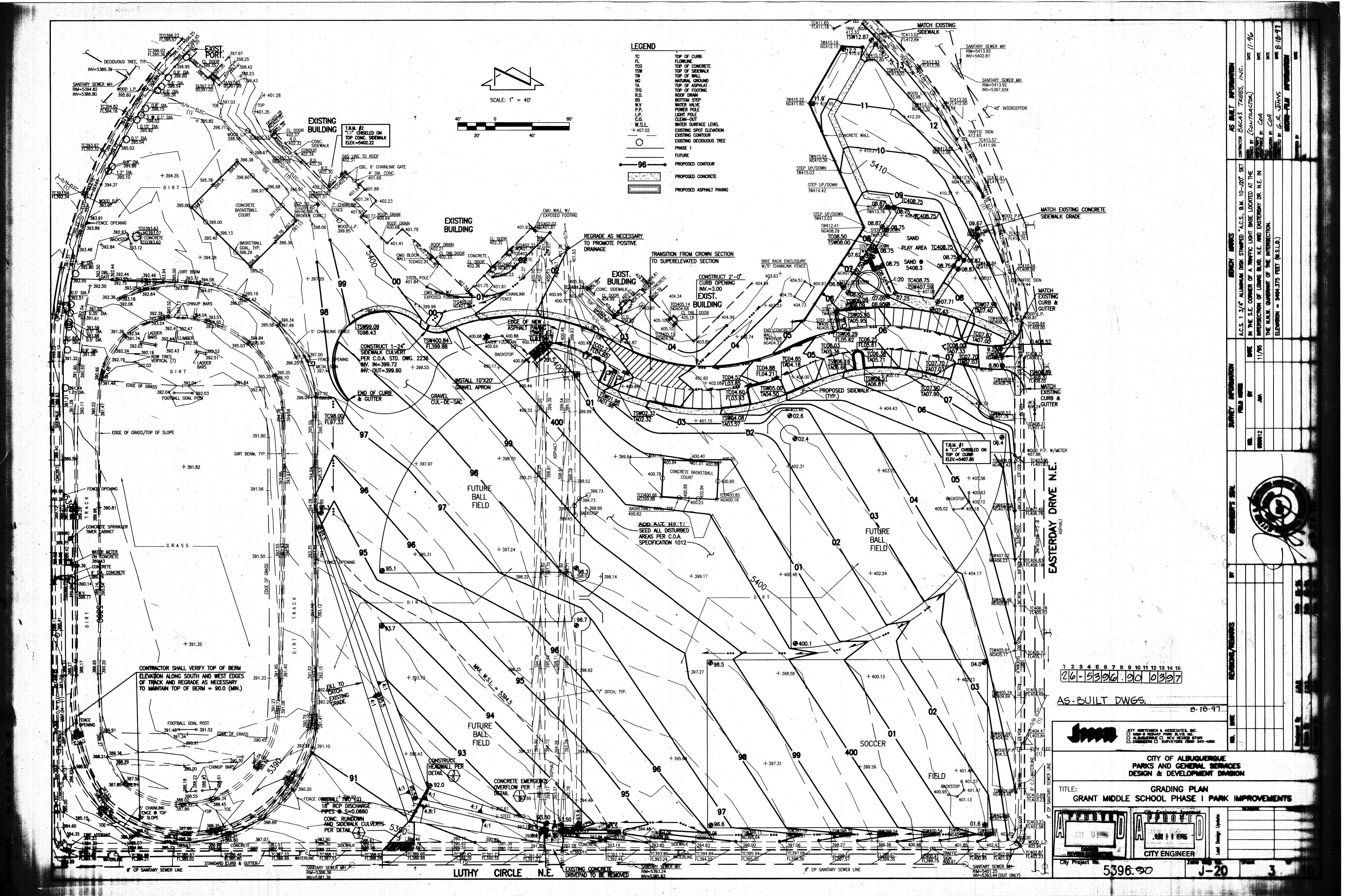
PPROVAL OF AS BUILT DRAWINGS
CHIEF CONSTRUCTION ENGINEER

DATE 8-28-97

City Project No.

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C. Sub-Basin 'E'
                                                                                                                                      E^{\mathbf{M}} = (E^{\mathbf{A}} \mathbf{A}^{\mathbf{A}} + E^{\mathbf{B}} \mathbf{A}^{\mathbf{B}} + E^{\mathbf{C}} \mathbf{A}^{\mathbf{C}} + E^{\mathbf{D}} \mathbf{A}^{\mathbf{D}}) / \mathbf{A}^{\mathbf{L}}
Site Characteristics
                                                                                                                                      E_{W} = (0.92(0.93) + 1.29(7.96) + 2.36(3.46))/12.35 = 1.56 in
   Precipitation Zone =
                                                                                                                                      V_{100} = (E_W/12)A_T
                                                                                                                                     V_{100} = (1.56/12)12.35 = 1.6055 ac.ft. = 69,940 cf
3. Total Area (A_{\tau}) = 980,350 \text{ sf}/22.51 \text{ ac}
                                                                                                                                      II. Peak Discharge
4. Existing Land Treatment
                                                                                                                                      Q_p = Q_{PA}A_A + Q_{PB}A_B + Q_{PC}A_C + Q_{PD}A_D
             Basin 'B' - A_{TB} = 247,690 \text{ sf}/5.69 \text{ ac}
                                                                                                                                     Q_{D} = Q_{100} = 2.60(0.93) + 3.45(7.96) + 5.02(3.46) = 47.2 \text{ cfs}
                                                                                 27.8
58.7
13.5
                                                                                                                   Hydrograph Calculations — Sub-Basin 'E
                                               145,680/3.34
                                                                                                                       Phase I Development - Worst Case Scenario
                                                33,400/0.77
             Basin 'C' - A_{TC} = 139,720 \text{ sf}/3.21 \text{ ac}
                                                                                                                              t_{R} = 2.107E(A_{T}/Q_{D}) - 0.25 (A_{D}/A_{T})
              Basin 'E' - A_{TF} = 417,440 \text{ sf}/9.58 \text{ ac}
                                                                                                                                      E = 1.56 in
                                                                                   90.7
                                                                                                                                      A_{\rm T} = 12.35 \, {\rm ac}
                                                                                   9.3
                                                  38,900/0.89
                                                                                                                                     Q_D = 47.2 \text{ cfs}
     Developed Land Treatmen
                                                                                                                                     A_D = 3.46 ac
             Basin 'B' - A_{TR} = 244,600 \text{ sf}/5.62 \text{ ac}
                                                                                                                              t_D = 0.79 \text{ hr} = 47.4 \text{ mir}
                                                                                   28.1
58.2
13.7
                                                 142,590/3.27
                                                                                                                      B. Time to Peak
                                                  33,400/0.77
                                                                                                                                 _{\rm D} = 0.7t_{\rm C} + (1.6 - A_{\rm D}/A_{\rm T})/12
             Basin 'C' .- A<sub>TC</sub> = 14,220 sf/0.38 ac
                                                                                                                                       t_{c} = 0.2 \text{ hr}
                                                                                                                                     A_D = 3.46 ac
      C. Basin 'E' -A_{TF} = 537,960 \text{ sf}/12.35 ac
                                                                                                                                      A_T = 12.35 ac
                                                Area (sf/ac)
40,240/0.93
                                                                                   64.4
                                                346,820/7.96
                                                                                                                                 = 0.25 \text{ hr} = 15.0 \text{ min}
                                                 150,900/3.46
                                                                                                                      C. Time of Peak
      Existing Condition
                                                                                                                               t_{\rm nk} = 0.25(A_{\rm D}/A_{\rm T})
       A. Basin 'B'
                                                                                                                                     A_D = 3.46 \text{ ac}
                                                                                                                                      A_{T} = 12.35 \text{ ac}
               E_{\mathbf{W}} = (E_{\mathbf{A}}A_{\mathbf{A}} + E_{\mathbf{B}}A_{\mathbf{B}} + E_{\mathbf{C}}A_{\mathbf{C}} + E_{\mathbf{D}}A_{\mathbf{D}})/A_{\mathbf{T}}
               E_{W} = (0.92(1.58) + 1.29(3.34) + 2.36(0.77))/5.69 = 1.33 in
                                                                                                                               t_{nk} = 0.07 \text{ hr} = 4.2 \text{ min}
               V_{100} = (E_W/12)A_T
                                                                                                               9. Pond Discharge Rate
               V_{100} = (1.33/12)5.69 = 0.6306 ac.ft. = 27,470 cf
               Q_{p} = Q_{pA}A + Q_{pB}A_{B} + Q_{pC}A_{C} + Q_{pD}A_{D}
                                                                                                                                       g = 32.2 \text{ ft/s}
               Q_p = Q_{100} = 2.60(1.58) + 3.45(3.34) + 5.02(0.77) = 19.5 \text{ cfs}
                                                                                                                                       h = 94.5 - 92.0 - 0.75 = 1.75 \text{ ft}
                                                                                                                                      A = 1.77 \text{ sf/pipe } (2 \text{ pipes}) = 3.54 \text{ sf } (2 - 18^{\circ} \text{ pipes})
                                                                                                                               Q = 22.5 \text{ cfs}
                E_W = (E_A^A + E_B^A + E_C^A + E_D^A)/A_T
                                                                                                                        B. Gravity Flow Condition
                                                                                                                                Using Feild's Calculator for Gravity Flow in Pipes
                E_{W} = (1.29(3.21))/3.21 = 1.29 \text{ in}
                V_{100} = (E_W/12)A_T
                                                                                                                                Let: D = 18^{n}
                                                                                                                                      S = 0.0520
               V_{100} = (1.29/12)3.21 = 0.3451 ac.ft. = 15,030 cf
                                                                                                                                      n = 0.013
                                                                                                                               Therefore: Q = 24.0 cfs/pipe = 48.0 cfs total capacity
                II. Peak Discharge
                Q_p = Q_{PA}A_A + Q_{PB}A_B + Q_{PC}A_C + Q_{PD}A_D
                                                                                                                       C. Pressure Condition Governs Discharge Rate: Q = 22.5 cfs
                                                                                                                10. Comparison
               Q_p = Q_{100} = 3.45(3.21) = 11.1 \text{ cfs}
                                                                                                                        Basins C & E
         C. Besin 'E'
                                                                                                                        A. Phase
                                                                                                                       \Delta V_{100} = 1,550 + 69,940 - 48,340 - 15,030 = 8,120 cf (increase)
                E_{\mathbf{W}} = (E_{\mathbf{A}}A_{\mathbf{A}} + E_{\mathbf{B}}A_{\mathbf{B}} + E_{\mathbf{C}}A_{\mathbf{C}} + E_{\mathbf{D}}A_{\mathbf{D}})/A_{\mathbf{T}}
                                                                                                                       \Delta Q_{100} = 11.1 + 34.4 - 1.1 - 22.5 = 21.9 cfs (decrease)
               E_{W} = (1.29(8.69) + 2.36(0.89))/9.58 = 1.39 in
                V_{100} = (E_W/12)A_T
                                                                                                                       B. Fully Developed
               V_{100} = (1.39/12)9.58 = 1.1097 \text{ ac.ft.} = 48,340 \text{ cf}
                                                                                                                       \Delta V_{100} = 1,550 + 1,630 + 64,560 - 48,340 - 15,030 = 4,370 cf (increase)
                                                                                                                       \Delta Q_{100} = 11.1 + 34.4 - 1.1 - 1.0 - 22.5 = 20.9 cfs (decrease)
                II. Peak Discharge
                Q_p = Q_{PA}A_A + Q_{PB}A_B + Q_{PC}A_C + Q_{PD}A_D
                                                                                                                11. Emergency Overflow Capacity
                                                                                                                       Q = CLH^{1.5}
               Q_p = Q_{100} = 3.45(8.69) + 5.02(0.89) = 34.4 \text{ cfs}
                                                                                                                               C = 2.70
       Developed Condition
                                                                                                                               L = 24.0 \text{ ft}
                                                                                                                                H = 1.0 \text{ ft}
                                                                                                                        Q_{\text{overflow}} = 64.8 \text{ cfs} \gg Q_{E-2} = 47.2 \text{ cfs}
                E^{\mathbf{M}} = (E^{\mathbf{A}}\mathbf{A}^{\mathbf{A}} + E^{\mathbf{B}}\mathbf{A}^{\mathbf{B}} + E^{\mathbf{C}}\mathbf{A}^{\mathbf{C}} + E^{\mathbf{D}}\mathbf{A}^{\mathbf{D}})/\mathbf{A}^{\mathbf{L}}
                                                                                                                12. Sidewalk Culvert Requirements
                E_W = (0.92(1.58) + 1.29(3.27) + 2.36(0.77))/5.62 = 1.33 in
                                                                                                                        A. 24" Sidewalk Culvert Entrance Condition
               V_{100} = (E_W/12)A_T
                                                                                                                             Q = CLH^{1.5}
               V_{100} = (1.33/12)5.62 = 0.6229 ac.ft. = 27,130 cf
                                                                                                                                       L = 2.83 ft (24" culvert ● 45 degree skew)
                II. Peak Discharge
                                                                                                                                        H = 0.67 \text{ ft}
                Q_{\mathbf{p}} = Q_{\mathbf{p}\mathbf{A}}^{\mathbf{A}} + Q_{\mathbf{p}\mathbf{B}}^{\mathbf{A}} + Q_{\mathbf{p}\mathbf{C}}^{\mathbf{A}} + Q_{\mathbf{p}\mathbf{D}}^{\mathbf{A}}
                                                                                                                               Q<sub>culvert</sub> = 4.0 cfs
                Q_p = Q_{100} = 2.60(1.58) + 3.45(3.27) + 5.02(0.77) = 19.3 cfs
                                                                                                                        B. Number of Sidewalk Culverts Required
               Basin 'C'
                                                                                                                                Q<sub>release</sub>/Q<sub>culvert</sub> = # REQUIRED
                I. Volume
                                                                                                                                        22.5 \text{ cfs/4.0 cfs} = 5.625 \text{ culverts}
                E_{\mathbf{W}} = (E_{\mathbf{A}} A_{\mathbf{A}} + E_{\mathbf{B}} A_{\mathbf{B}} + E_{\mathbf{C}} A_{\mathbf{C}} + E_{\mathbf{D}} A_{\mathbf{D}}) / A_{\mathbf{T}}
               E_{W} = (1.29(0.33))/0.33 = 1.29 \text{ in}
                                                                                                                                 Therefore: Five 24" culverts and one 15" culvert are required
                 V_{100} = (E_W/12)A_T
                                                                                                                                                                                                  Σ Volume (cf)
                V_{100} = (1.29/12)0.33 = 0.0355 ac.ft. = 1,550 cf
                 II. Peak Discharge
                                                                                                                                                                                                          2,625
                                                                                                                                                                          2,625
                                                                                                                                                 5,250
                Q_{p} = Q_{pA}A + Q_{pB}A + Q_{pC}AC + Q_{pD}AD
                                                                                                                                                                                                          15,363
                                                                                                                                                                         12,738
                Q_p = Q_{100} = 3.45(0.33) = 1.1 \text{ cfs}
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					IRA TAHAWIRA	ITAMO	_

The following items concerning the Grant Middle School Park and

DRAINAGE PLAN

Vicinity Map Grading Plan Calculations

Circle N.E. It is bounded on the north, east, and west sides by Easterday Drive N.E. and Sellers Drive N.E. All of these roadways are fully developed residential public streets with curb and gutter, sidewalks and asphalt concrete paving.

As shown by the Vicinity Map, the site is located north of Luthy

As shown by Panel 30 of 50 of the Flood Insurance Program Flood Insurance Rate Maps published by F.E.M.A. for the City of Albuquerque, New Mexico dated October 14, 1983, the extreme southwest corner of the site lies within a Flood Hazard Zone AO (Depth 1). Portions of this site contribute to this flood hazard

The Grading Plan shows: 1) existing and proposed grades indicated by spot elevations and contours at 1'0" intervals, 2) the limit and character of the existing improvements, 3) the limit and character of the proposed improvements, and 4) continuity between existing and proposed grades. The development shown hereon, proposed for Grant Middle School, consist of the Phase I improvements for the City of Albuquerque Park and Playground Improvements. As part o these Phase I improvements, a playground area, some associated landscaping, a portion of the access road, a gravel cul-de-sac and associated parking, and mass grading for future improvements at the site are proposed. The grading shown on the plan is consistent with the grading scenario indicated by the Master Drainage Plan submitted by this office for the site. The basin boundaries shown are the same as shown on the Master Drainage Plan.

As outlined in the Master Drainage Plan, improvements will be limited to Basin E, which will reduce the size of Basins C and B. The runoff from Basin E will flow in a southwesterly direction to a new onsite detention pond. The detention facility will have a limited discharge rate of 22.5 cfs, which is less than the existing runoff discharged by Basins C and E which totals 45.5 cfs. The runoff will be conveyed via two 18" RCP storm drain pipes to a new concrete rundown which will then discharge into Luthy Circle N.E. in historic patterns, via six sidewalk culverts. An existing drivepad to the east of the proposed rundown and sidewalk culverts will be utilized as an emergency overflow spillway. This emergency overflow spillway has more than enough capacity to handle the anticipated 100-year runoff event.

Basins B and C will continue in their historic runoff patterns. Basin C will continue to discharge by sheetflow into Luthy Circle N.E. Basin B will continue to drain southwesterly into the existing dirt running track to the southwestern corner, where it is contained by the one foot berm surrounding the track. As identified by the Master Drainage Plan, no additional facilities are proposed for Basin B. The impact of development on Basin B is very minor, and the amount of runoff discharged by Basin B is reduced by this development, therefore, thereby not negatively

With the reduction of peak discharge rate being released from this site, the effects of the runoff on the downstream flood hazard zone are reduced. This flood hazard zone will be eliminated in the future with the development of the storm drain system, 325-01B, as shown on AMDS Plate J-20 which consists of a 48" storm drain from Luthy Circle to 170 feet south of Luthy Circle to connect with Manhole S802. In the interim, onsite detention ponding will serve to mitigate the effects that this site has on downstream flooding This site lies within an infill area and the watershed is fully developed, limiting significant future increases in runoff. Because this project is reducing the amount of runoff bein discharged from the site, the site lies within an infill area with a fully developed watershed and future plans are in place for the installation of a storm drain system that will effectively eliminate the downstream flood hazard zone, it is felt that this drainage scenario is appropriate for this project.

The Calculations which appear hereon analyze both the existing and developed conditions for the 100-year, 6-hour rainfall event. Procedure for 40-acre and Smaller Basins, as set forth in the Revision of Section 22.2, Hydrology of the Development Process Manual, Volume 2, Design Criteria, dated January, 1993, has been used to quantify the peak rate of discharge and volume of runoff generated. As shown by these calculations, an increase of 8,120 ca in volume of runoff is anticipated with this development. Because of the detention facility, a decrease of 21.9 cfs in the peak discharge rate is anticipated. The Weir Equation has been used to quantify the emergency overflow capacity and the sidewalk culvert entrance condition. The Average End Area Method has been used to quantify the pond capacity. At a discharge rate of 22.5 cfs, the detained runoff will drain from the pond in approximately 17.0

27,232

11,869

Pond Volume = 27,232 cf > Pond Required = 22,980 c

Elev	Area (sf)	Vol (cf)	Σ Vol (cf)
87.5	0	1,125	1,125
88.0	4,500	9,375	10,500
89.0	14,250	18,375	28,875
90.0	22,500	10,070	20,070

Volume at $90.0 = 28,875 \text{ cf} > V_{reg'd} = 27,470 \text{ cf}$

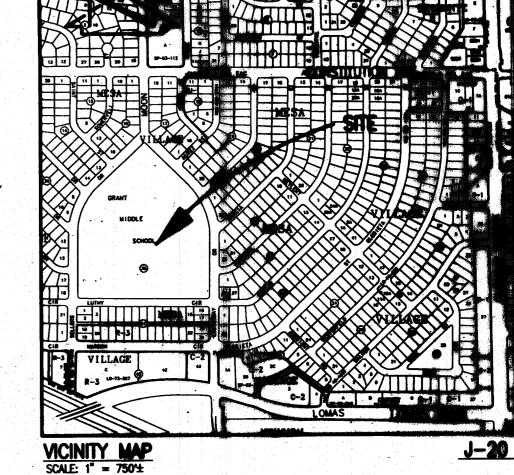
15. 2-YEAR AND 5-YEAR EXISTING PEAK DISCHARGE

FROM TABLE A-3, DPM SECTION 22.2 $Q_2 = Q_{100}(0.434) = (11.1 + 34.4)(0.434) = 19.7CFS$ $= Q_{100}^{100}(0.567) = (11.1 + 34.4)(0.567) = 25.8 \text{ CFS}$ =22.5 CFS = POND RELEASE RATE THEREFORE : $Q_2 = 19.7$ CFS $< Q_{RFI} = 22.5$ CFS $< Q_5 = 25.8$ CFS THE PEAK DISCHARGE RATE FROM THE POND IS BETWEEN THE ESTIMATED PEAK DISCHARGE RATES FOR THE 2 AND 5 YEAR STORM EVENTS.

RATE COMPARISON FROM BASINS 'C' AND 'E' FROM

GENERAL NOTES:

- ALL WORK DETAILED ON THESE PLANS TO BE PERFORMED UNDER CONTRACT SHALL, EXCEPT AS OTHERWISE STATED OR PROVIDED FOR HEREON, BE CONSTRUCTED IN ACCORDANCE WITH THE CITY OF ALBUQUERQUE SPECIFICATIONS FOR PUBLIC WORKS CONSTRUCTION, REVISION G. AMERICAN PUBLIC WORKS ASSOCIATION.
- TWO (2) WORKING DAYS PRIOR TO ANY EXCAVATION, CONTRACTOR MUST CONTACT NEW MEXICO ONE CALL SYSTEM, 260-1990 (ALBUQUERQUE AREA), 1-800-321-ALERT(2537) (STATEWIDE), FOR LOCATION OF
- 3. IF ANY UTILITY LINES, PIPELINES, OR UNDERGROUND UTILITY LINES ARE SHOWN ON THESE DRAWINGS, THEY ARE SHOWN IN AN APPROXIMATE MANNER ONLY, AND SUCH LINES MAY EXIST WHERE NONE ARE SHOWN. IF ANY SUCH EXISTING LINES ARE SHOWN, THE LOCATION IS BASED UPON INFORMATION PROVIDED BY THE OWNER OF SAID UTILITY, AND THE INFORMATION MAY BE INCOMPLETE, OR MAY BE OBSOLETE BY THE TIME CONSTRUCTION COMMENCES. THE ENGINEER HAS CONDUCTED ONLY PRELIMINARY INVESTIGATION OF THE LOCATION, DEPTH, SIZE. OR TYPE OF EXISTING UTILITY LINES, PIPELINES, OR UNDERGROUND UTILITY LINES. THIS INVESTIGATION IS NOT CONCLUSIVE, AND MAY NOT BE COMPLETE, THEREFORE, MAKES NO REPRESENTATION PERTAINING THERETO, AND ASSUMES NO RESPONSIBILITY OR LIABILITY THEREFORE. THE CONTRACTOR SHALL INFORM ITSELF OF THE LOCATION OF ANY UTILITY LINE, PIPELINE, OR UNDERGROUND UTILITY LINE IN OR NEAR THE AREA OF THE WORK IN ADVANCE OF AND DURING EXCAVATION WORK. THE CONTRACTOR IS FULLY RESPONSIBLE FOR ANY AND ALL DAMAGE CAUSED BY ITS FAILURE TO LOCATE, IDENTIFY AND PRESERVE ANY AND ALL EXISTING UTILITIES, PIPELINES, AND UNDERGROUND UTILITY LINES. IN PLANNING AND CONDUCTING EXCAVATION, THE CONTRACTOR SHALL COMPLY WITH STATE STATUTES, MUNICIPAL AND LOCAL ORDINANCES, RULES AND REGULATIONS, IF ANY, PERTAINING TO THE LOCATION OF THESE LINES AND FACILITIES.
- SHOULD A CONFLICT EXIST BETWEEN THESE PLANS AND ACTUAL FIELD CONDITIONS, THE CONTRACTOR SHALL PROMPTLY NOTIFY THE ENGINEER IN WRITING SO THAT THE CONFLICT CAN BE RESOLVED WITH A MINIMUM AMOUNT OF DELAY FOR ALL PARTIES. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ALL INTERPRETATIONS IT MAKES WITHOUT FIRST CONTACTING THE ENGINEER AS REQUIRED ABOVE.
- THE CONTRACTOR SHALL MAINTAIN ACCESS TO SCHOOL FACILITIES DURING CONSTRUCTION
- ALL WORK ON THIS PROJECT SHALL BE PERFORMED IN ACCORDANCE WITH APPLICABLE FEDERAL, STATE AND LOCAL LAWS, RULES AND REGULATIONS CONCERNING SAFETY AND HEALTH.
- THE CONTRACTOR SHALL ENSURE THAT NO SOIL ERODES FROM THE SITE INTO PUBLIC RIGHT-OF-WAY OR ONTO PRIVATE PROPERTY. THIS CAN BE ACHIEVED BY CONSTRUCTING AND MAINTAINING TEMPORARY BERMS AT THE PROPERTY LINES AND WETTING THE SOIL TO KEEP IT FROM BLOWING.
- 8. THE CONTRACTOR SHALL PROMPTLY CLEAN UP ANY MATERIAL EXCAVATED WITHIN THE PUBLIC RIGHT-OF-WAY SO THAT THE EXCAVATED MATERIAL IS NOT SUSCEPTIBLE TO BEING WASHED DOWN THE STREET.
- 9. CONTRACTOR SHALL SECURE "TOPSOIL DISTURBANCE PERMIT" PRIOR TO BEGINNING CONSTRUCTION
- 10. CONTRACTOR SHALL NOTIFY THE ENGINEER NOT LESS THAN SEVEN (7) DAYS PRIOR TO STARTING WORK IN ORDER THAT THE ENGINEER MAY TAKE NECESSARY MEASURES TO ENSURE THE PRESERVATION OF SURVEY MONUMENTS. CONTRACTOR SHALL NOT DISTURB PERMANENT SURVEY MONUMENTS WITHOUT THE CONSENT OF THE ENGINEER AND SHALL NOTIFY THE ENGINEER AND BEAR THE EXPENSE OF REPLACING ANY THAT MAY BE DISTURBED WITHOUT PERMISSION. REPLACEMENT SHALL BE DONE ONLY BY THE ENGINEER.
- 11. A DISPOSAL SITE FOR ALL EXCESS EXCAVATION MATERIAL (CONTAMINATED OR OTHERWISE), ASPHALTIC PAVING, CONCRETE PAVING, ETC. SHALL BE OBTAINED BY THE CONTRACTOR IN COMPLIANCE WITH APPLICABLE REGULATIONS. ALL COSTS INCURRED IN OBTAINING A DISPOSAL SITE AND IN HAUL THERETO SHALL BE CONSIDERED INCIDENTAL TO CONSTRUCTION, THEREFORE, NO SEPARATE PAYMENT SHALL BE MADE.
- 12. A BORROW SITE FOR IMPORT MATERIAL SHALL BE OBTAINED BY THE CONTRACTOR IN COMPLIANCE WITH APPLICABLE REGULATIONS. ALL COSTS INCURRED IN OBTAINING A BORROW SITE AND IN HAUL THERETO SHALL BE CONSIDERED INCIDENTAL TO CONSTRUCTION, THEREFORE, NO SEPARATE PAYMENT SHALL BE MADE.
- 13. THE CONTRACTOR SHALL BE RESPONSIBLE FOR SAFELY OBTAINING THE REQUIRED COMPACTION. THE CONTRACTOR SHALL SELECT AND USE METHODS WHICH SHALL NOT BE INJURIOUS OR DAMAGING TO THE EXISTING FACILITIES AND STRUCTURES WHICH SURROUND THE WORK AREAS.
- 14. THE CONTRACTOR SHALL CONFINE HIS WORK WITHIN THE CONSTRUCTION LIMITS IN ORDER TO PRESERVE THE EXISTING IMPROVEMENTS AND SO AS NOT TO INTERFERE WITH THE OPERATIONS OF THE EXISTING FACILITIES.
- 15. CAUTION: THESE DRAWINGS DO NOT INCLUDE NECESSARY COMPONENTS FOR CONSTRUCTION SAFETY WHICH SHALL REMAIN THE RESPONSIBILITY OF THE CONTRACTOR.



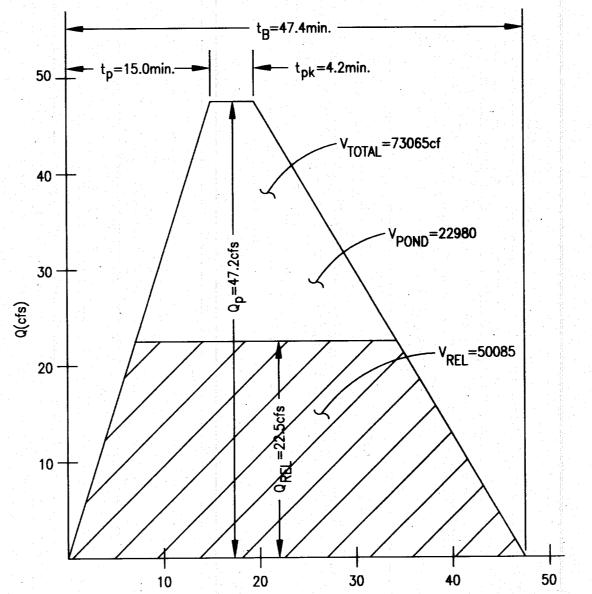
T.B.M. \$1; A TO CHISELED ON THE TOP OF CURB AS SHOWN ON THE DRAWING. (SMEET 3 OF 3) ELEVATION = 5407.85 FEET

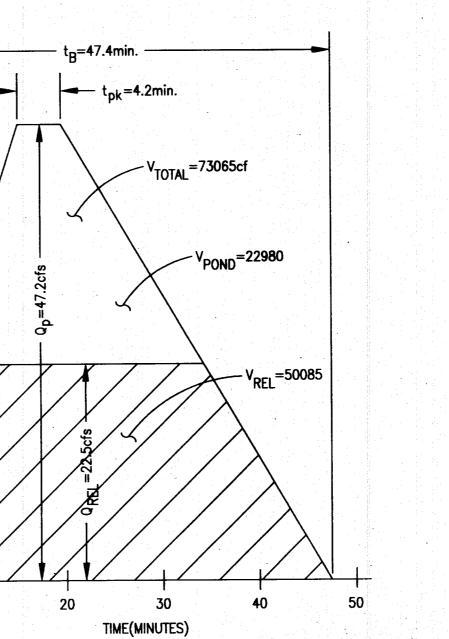
LEGAL DESCRIPTION BLOCK 35, MESA VILLAGE

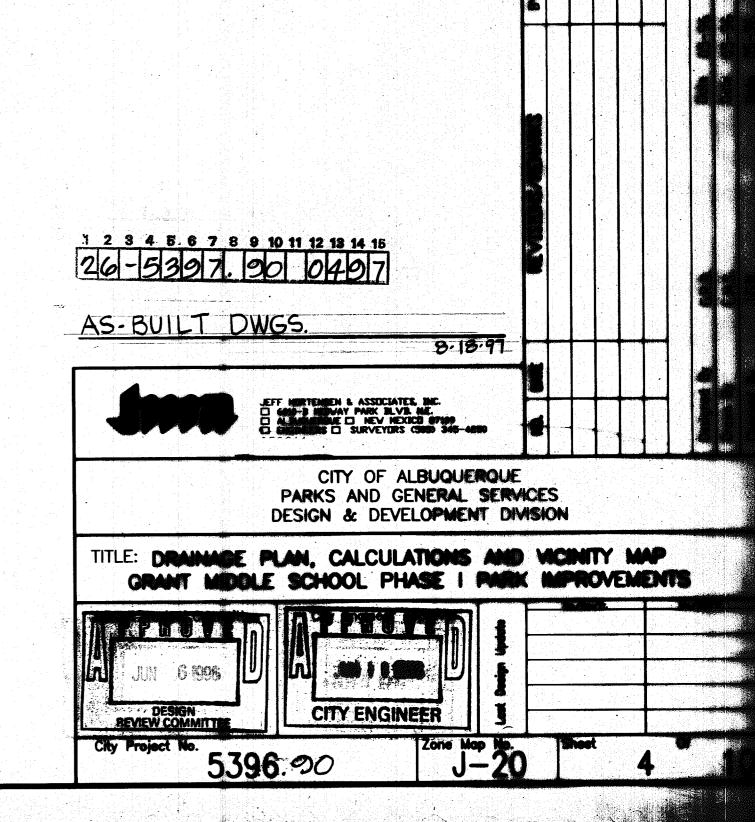
Erosion Control Measures:

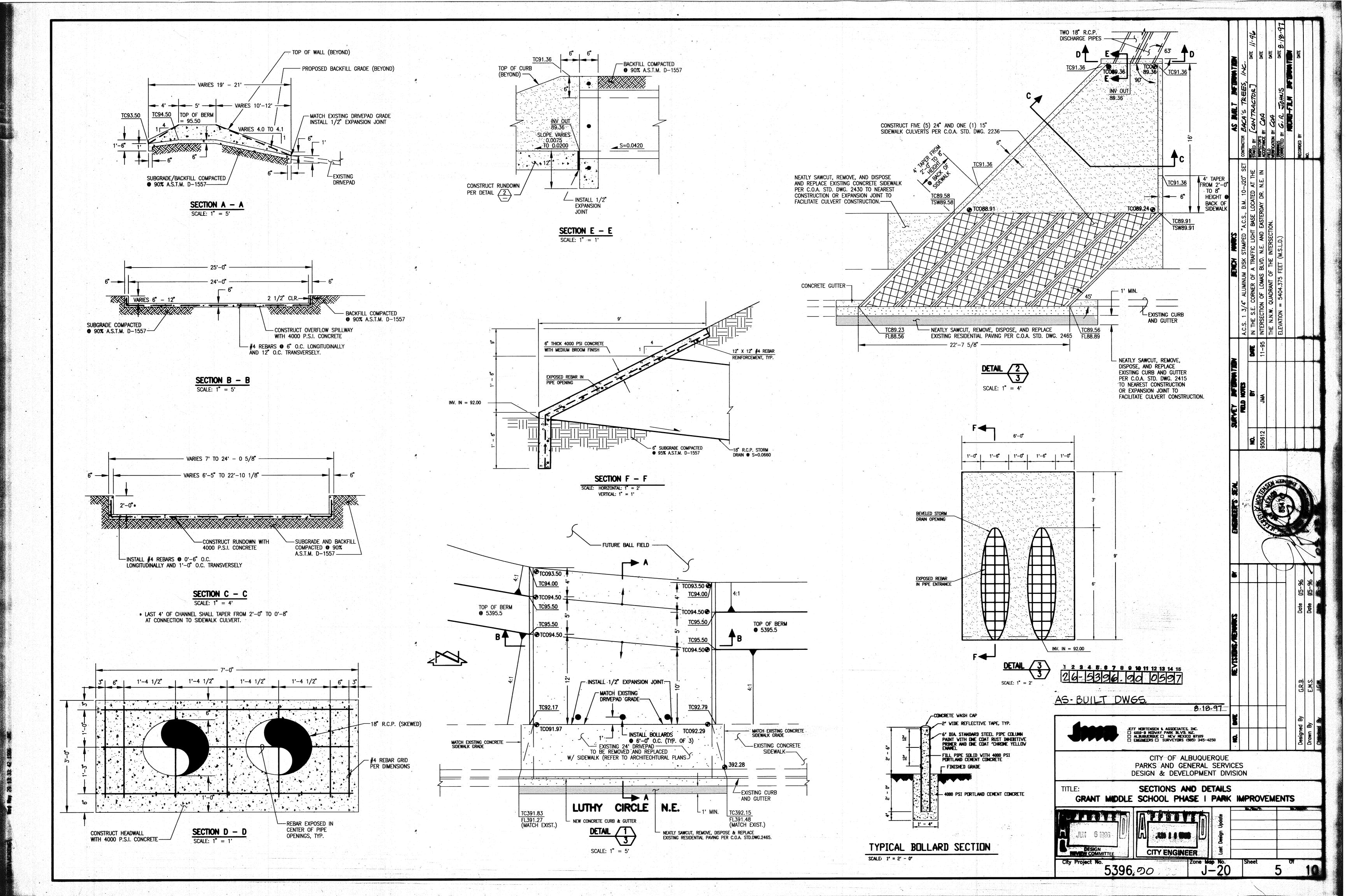
T.B.M. \$2; A TO CHISELED ON TOP OF CONCRETE SIDEWALK AS SHOWN ON THE DRAWING. (SHEET 3 OF 3) ELEVATION = 5402.22 FEET

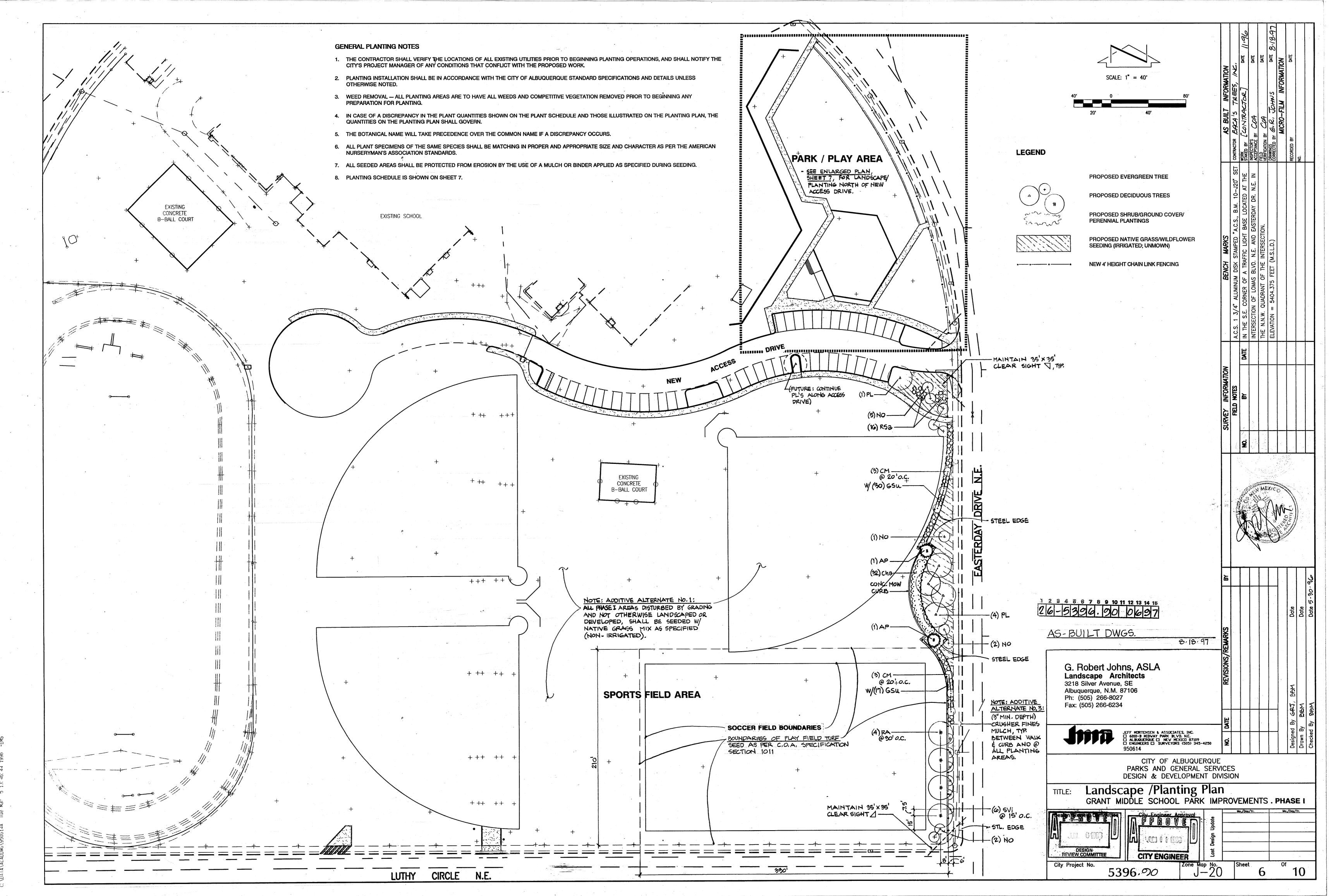
- The contractor shall ensure that no soil erodes from the site into public rightof-way or onto private property.
- 2. The contractor shall promptly clean up any material excavated within the public right-of-way so that the excavated material is not susceptible to being washed down the street.
- The contractor shall secure "Topsoil Disturbance Permit prior to beginning construction.

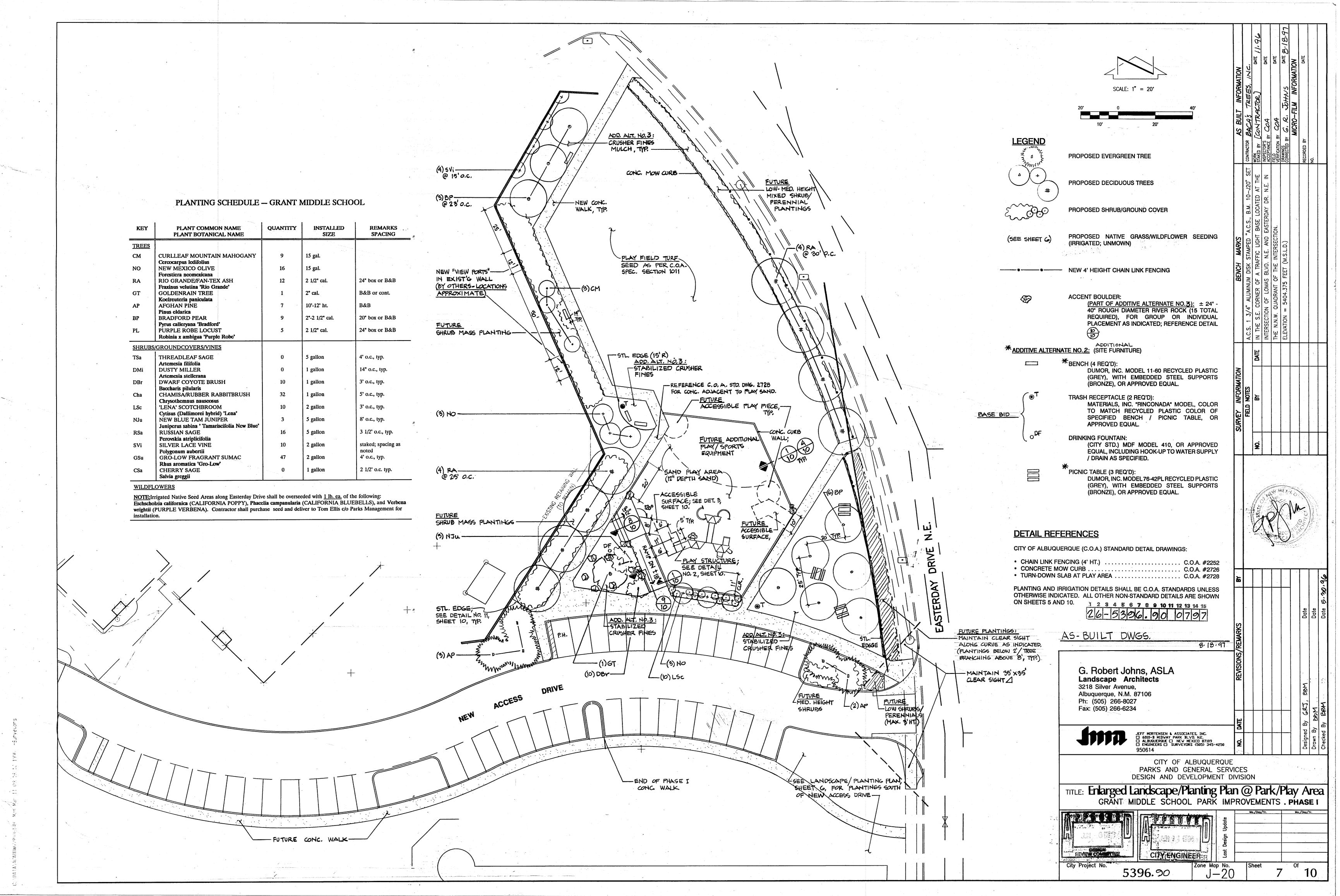


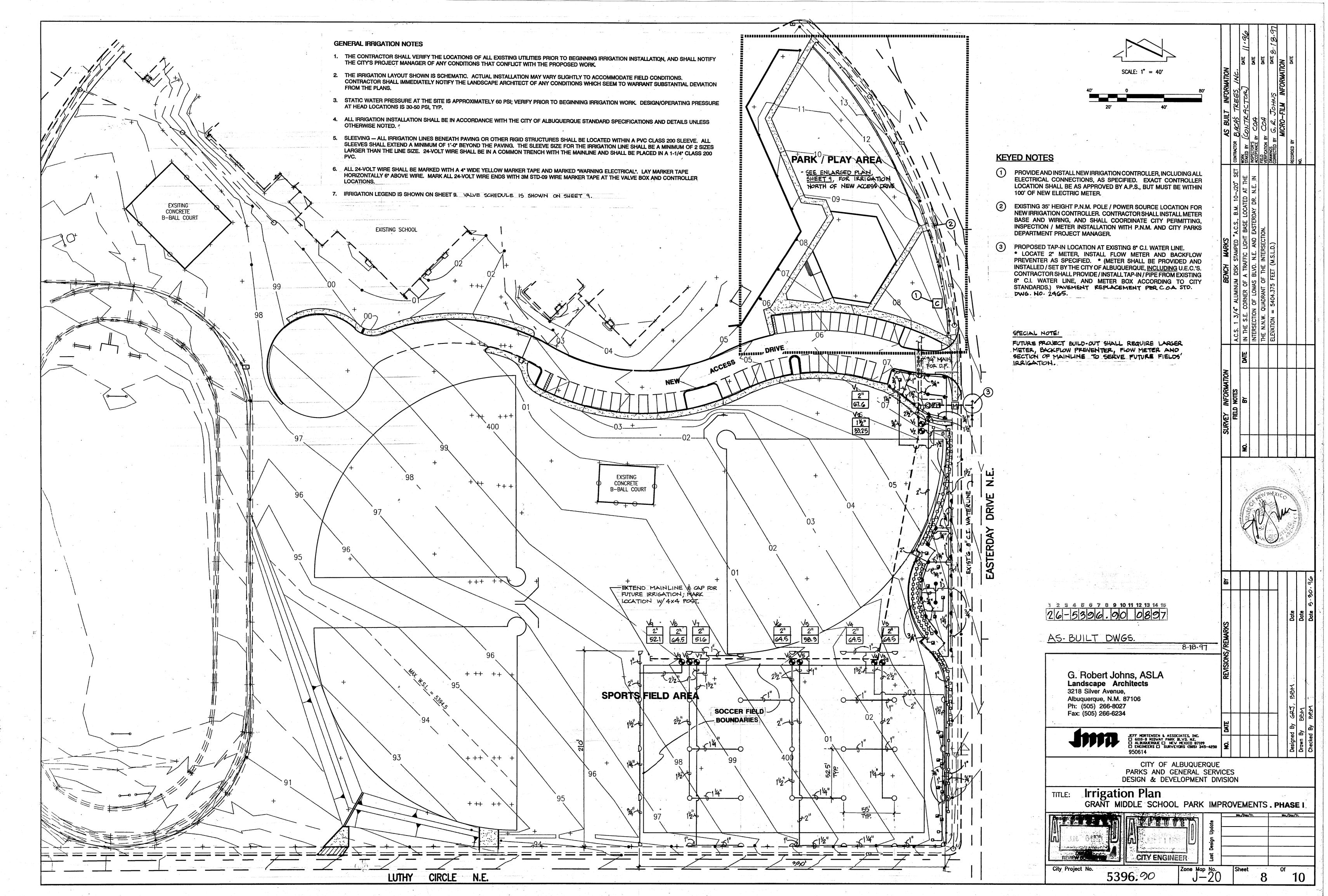




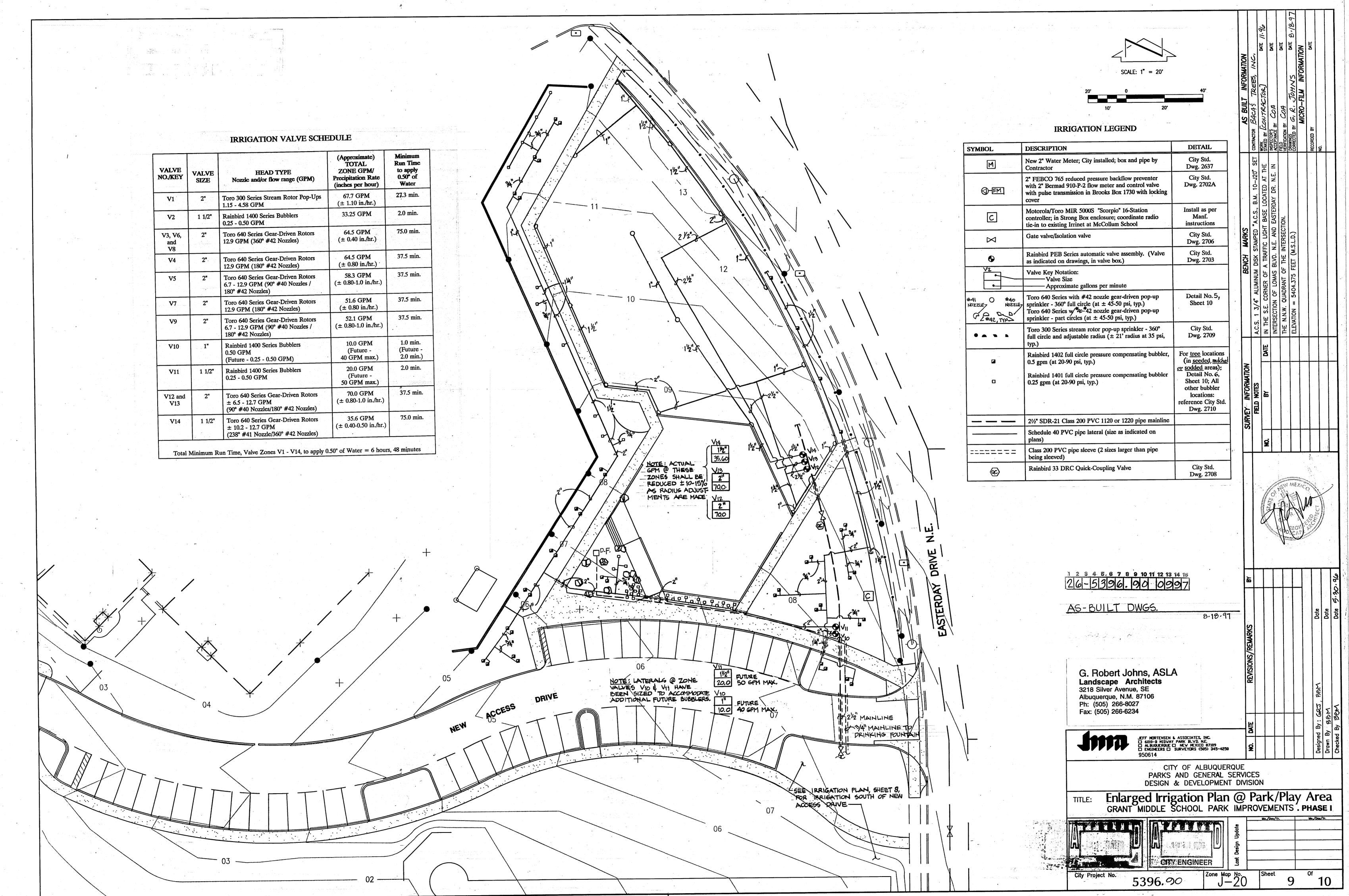








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