



- REINFORCING STEEL SHOP DRAWINGS PER ACI 315.
- CONCRETE MIX DESIGNS SHOWING TYPE AND QUALITY OF MATERIALS, AND STRENGTH.
- CATALOG DATA FOR GROUT.
- WELDER CERTIFICATIONS IN ACCORDANCE WITH AWS D1.1.

1.2 DESIGN CRITERIA

- A. CODES
 - 1. UNIFORM BUILDING CODE, 1991.
 - 2. AISC MANUAL OF STEEL CONSTRUCTION, 9TH EDITION.
 - 3. ACI 318-89, BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE

1.3 INSPECTION

- A. INSPECTION AND TESTING FOR SOILS, CONCRETE, HIGH STRENGTH BOLTS, AND WELDING WILL BE BY OWNER'S INSPECTION AND TESTING FIRM.
- RETESTS FOR NONCONFORMING MATERIALS SHALL BE AT CONTRACTOR'S EXPENSE.

EARTHWORK

- 2.1 SCOPE
 - A: INCLUDES ALL CLEARING AND GRUBBING, REMOVAL OF OBSTRUCTIONS, GENERAL EXCAVATING, GRADING AND FILLING, AND ANY RELATED ITEMS NECESSARY TO COMPLETE THE GRADING FOR THE ENTIRE PROJECT IN ACCORDANCE WITH THESE SPECIFICATIONS
- 2.2 SUBSURFACE SOIL DATA
 - A. SUBSURFACE SOIL INVESTIGATIONS HAVE BEEN MADE, AND THE RESULTS ARE AVAILABLE FOR EXAMINATION BY THE CONTRACTOR. THE CONTRACTOR IS EXPECTED TO EXAMINE THE SITE AND DETERMINE FOR HIMSELF THE CHARACTER OF MATERIALS TO BE ENCOUNTERED.
 - NO ADDITIONAL ALLOWANCE WILL BE MADE FOR ROCK REMOVAL, SITE CLEARING AND GRADING, FILLING, COMPACTION, DISPOSAL, OR REMOVAL OF ANY UNCLASSIFIED MATERIALS.

2.3 CLEARING AND GRUBBING

- A. GENERAL: CLEARING AND GRUBBING WILL BE REQUIRED FOR ALL AREAS SHOWN ON THE PLANS TO BE EXCAVATED OR ON WHICH FILL IS TO BE CONSTRUCTED.
- B. CLEARING: CLEARING SHALL CONSIST OF REMOVAL AND DISPOSAL OF TREES AND OTHER VEGETATION AS WELL AS DOWN TIMBER, SNAGS, BRUSH, RUBBISH, AND EXISTING PAVEMENTS AND STRUCTURES WITHIN THE AREAS TO BE CLEARED. INDIVIDUAL TREES, GROUPS OF TREES, OR OTHER VEGETATION NOT REQUIRED TO BE REMOVED SHALL BE PROTECTED, INSOFAR AS PRACTICAL, AND LEFT STANDING.
- C. GRUBBING: STUMPS, MATTED ROOTS, AND ROOTS LARGER THAN 2 INCHES IN DIAMETER, SHALL BE REMOVED FROM WITHIN 6 INCHES OF THE SURFACE OF AREAS ON WHICH FILLS ARE TO BE CONSTRUCTED EXCEPT IN ROADWAYS. MATERIALS AS DESCRIBED ABOVE WITHIN 18 INCHES OF FINISHED SUBGRADE OF ROADWAYS, IN EITHER OUT OR FILL SECTIONS, SHALL BE REMOVED. AREAS DISTURBED BY GRUBBING WILL BE FILLED AS SPECIFIED HEREIN FOR EMBANKMENT.
- D. GRASS AND TOPSOIL: GRASS, GRASS ROOTS, AND INCIDENTAL TOPSOIL SHALL NOT BE LEFT BENEATH A FILL AREA, NOR SHALL THIS MATERIAL BE USED AS FILL MATERIAL. GRASS, GRASS ROOTS AND TOPSOIL MAY BE STOCKPILED AND LATER USED IN THE TOP 6 INCHES OF LANDSCAPE.

2.4 EARTH EXCAVATION

- A. EARTH EXCAVATION SHALL CONSIST OF THE EXCAVATION AND REMOVAL OF SUITABLE SOILS FOR USE AS EMBANKMENT, AS WELL AS THE SATISFACTORY DISPOSAL OF ALL VEGETATION, EXISTING MAN-MADE FILL, DEBRIS, AND DELETERIOUS MATERIALS ENCOUNTERED WITHIN THE AREA TO BE GRADED AND/OR IN A BORROW AREA.
- B. EXCAVATED AREAS SHALL BE CONTINUOUSLY MAINTAINED SUCH THAT THE SURFACE SHALL BE SMOOTH AND HAVE SUFFICIENT SLOPE TO ALLOW WATER TO DRAIN FROM THE SURFACE.

2.5 EMBANKMENT

A. GENERAL: EMBANKMENT SHALL CONSIST OF A CONTROLLED FILL CONSTRUCTED IN AREAS INDICATED ON THE GRADING PLANS.

B. MATERIALS:

1. PHYSICAL CHARACTERISTICS: EMBANKMENT FILL MATERIAL SHALL CONSIST OF SOILS THAT CONFORM TO THE FOLLOWING PHYSICAL CHARACTERISTICS:

> SIEVE SIZE PERCENT PASSING (SQUARE OPENINGS) BY WEIGHT 3 INCH 100 NO. 4 50-100 NO. 200 10-60

THE PLASTICITY INDEX OF THE MATERIAL. AS DETERMINED IN ACCORDANCE WITH ASTM D4318, SHALL NOT EXCEED 12.

RESULTS OF OUR INVESTIGATION INDICATE THAT SOME OF THE ON-SITE SOILS WILL MEET THESE REQUIREMENTS, HOWEVER SOME BLENDING & IMPORTED FILL MAY BE REQUIRED.

THE FILL MATERIAL SHALL BE FREE FROM ROOTS, GRASS, OTHER VEGETABLE MATTER, CLAY LUMPS, ROCKS LARGER THAN 6 INCHES, OR OTHER DELETERIOUS MATERIALS.

2. BORROW: WHEN THE QUANTITY OF SUITABLE MATERIAL REQUIRED FOR EMBANKMENTS IS NOT AVAILABLE WITHIN THE LIMITS OF THE JOBSITE, THE CONTRACTOR SHALL PROVIDE SUFFICIENT MATERIALS TO CONSTRUCT THE EMBANKMENTS TO THE LINES, ELEVATIONS, AND CROSS-SECTIONS SHOWN ON THE DRAWINGS FROM BORROW AREAS. THE CONTRACTOR SHALL OBTAIN FROM OWNERS OF SAID BORROW AREAS THE RIGHT TO EXCAVATE MATERIAL, SHALL PAY ALL EXPENSES IN DEVELOPING THE SOURCE INCLUDING THE COST OF RIGHT-OF-WAY REQUIRED FOR HAULING THE MATERIAL, SHALL PAY ALL ROYALTIES AND OTHER CHARGES INVOLVED.

C. CONSTRUCTION:

1. FOUNDATION AREA TREATMENT: THE BRIDGE FOUNDATION SHALL BE OVEREXCAVATED SO AS TO REMOVE THE EXISTING FILL SOILS IN THEIR ENTIRETY. THE OVEREXCAVATION SHOULD EXTEND LATERALLY AWAY FROM THE FOUNDATION A DISTANCE EQUAL TO THE PROPOSED DEPTH OF STRUCTURAL FILL BENEATH THE FOOTING.

PRIOR TO PLACEMENT OF FILL, THE FOUNDATION AREAS SHALL BE INSPECTED AND APPROVED BY A REPRESENTATIVE OF THE GEOTECHNICAL ENGINEER TO INSURE SATISFACTORY REMOVAL OF NATIVE SOILS AND THE REMOVAL OF -ANY EXISTING MAN MADE FILL.

THE EXPOSED CUT SURFACE, AS WELL AS SURFACES TO RECEIVE FILL, SHALL BE SCARIFIED TO A MINIMUM DEPTH OF 8 INCHES AND WATERED AS NECESSARY TO BRING THE UPPER 12 INCHES AS CLOSE AS PRACTICABLE TO OPTIMUM MOISTURE CONTENT OR ABOVE. THE UPPER 8 INCHES OF THE NATIVE SOILS SHALL THEN BE COMPACTED TO A MINIMUM OF 95 PERCENT OF MAXIMUM DRY DENSITY AS DETERMINED IN ACCORDANCE WITH ASTM

WHERE VIBRATORY COMPACTION EQUIPMENT IS USED. IT SHALL BE THE CONTRACTOR'S RESPONSIBILITY TO INSURE THAT THE VIBRATIONS DO NOT DAMAGE NEARBY BUILDING OR OTHER ADJACENT PROPERTY.

2. COMPACTION: THE FILL SHALL BE SPREAD IN LAYERS NOT EXCEEDING 8 INCHES, WATERED AS NECESSARY, AND COMPACTED. MOISTURE CONTENT AT THE TIME OF COMPACTION SHALL BE 2 PERCENT BELOW OPTIMUM MOISTURE OR HIGHER. A DENSITY OF NOT LESS THAN 95 PERCENT OF MAXIMUM DRY DENSITY WITHIN THE BRIDGE FOUNDATION EXCAVATION SHALL BE OBTAINED FOR THE STRUCTURAL FILL. STRUCTURAL FILL, AS WELL AS THE NATIVE SOILS, OUTSIDE THE ZONE OF INFLUENCE OF BRIDGE FOUNDATION LOADINGS SHALL BE COMPACTED TO 90 PERCENT OF MAXIMUM DRY DENSITY.

OPTIMUM MOISTURE CONTENT AND MAXIMUM DRY DENSITY FOR EACH SOIL TYPE USED SHALL BE DETERMINED IN ACCORDANCE WITH ASTM D1557.

- WEATHER LIMITATIONS: CONTROLLED FILL SHALL NOT BE CONSTRUCTED WHEN THE ATMOSPHERIC TEMPERATURE IS BELOW 35 DEGREES F. WHEN THE TEMPERATURE FALLS BELOW 35 DEGREES, IT SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR TO PROTECT ALL AREAS OF COMPLETED WORK AGAINST ANY DETRIMENTAL EFFECTS OF GROUND FREEZING BY METHODS APPROVED BY THE GEOTECHNICAL ENGINEER. ANY AREAS THAT ARE DAMAGED BY FREEZING SHALL BE RECONDITIONED, RESHAPED, AND COMPACTED BY THE CONTRACTOR IN CONFORMANCE WITH THE REQUIREMENTS OF THIS SPECIFICATION WITHOUT ADDITIONAL COST TO THE OWNER.
- D. SLOPE PROTECTION AND DRAINAGE: THE EDGES OF THE CONTROLLED FILL EMBANKMENTS SHALL BE GRADED TO THE CONTOURS SHOWN ON THE DRAWINGS AND COMPACTED TO THE DENSITY REQUIRED IN PARAGRAPH 2.5.C(2).

2.6 INSPECTION AND TESTS

A. FIELD INSPECTION AND TESTING: THE OWNER SHALL EMPLOY THE SERVICES OF A REGISTERED, LICENSED GEOTECHNICAL ENGINEER TO OBSERVE AND TEST ALL CONTROLLED EARTHWORK. THE GEOTECHNICAL ENGINEER SHALL PROVIDE CONTINUOUS ON-SITE OBSERVATION BY EXPERIENCED PERSONNEL DURING CONSTRUCTION OF CONTROLLED EARTHWORK. THE CONTRACTOR SHALL NOTIFY THE ENGINEER AT LEAST TWO WORKING DAYS IN ADVANCE OF ANY FIELD OPERATIONS OF CONTROLLED EARTHWORK OR OF ANY RESUMPTION OF OPERATIONS AFTER STOPPAGES. TESTS OF FILL MATERIALS AND EMBANKMENTS WILL BE MADE AT THE FOLLOWING SUGGESTED MINIMUM

- 1. ONE FIELD DENSITY TEST FOR EACH 25 SQUARE YARDS OF ORIGINAL GROUND SURFACE PRIOR TO PLACING FILL OR CONSTRUCTING FLOOR SLABS.
- 2. ONE FIELD DENSITY TEST FOR EACH 125 CUBIC YARDS OF FILL PLACED OR EACH LAYER OF FILL FOR EACH WORK AREA, WHICHEVER IS THE GREATER NUMBER OF TESTS.
- 3. ONE MOISTURE DENSITY CURVE FOR EACH TYPE OF MATERIAL USED, AS INDICATED BY SIEVE ANALYSIS AND PLASTICITY
- B. REPORT OF FIELD DENSITY TESTS: THE GEOTECHNICAL ENGINEER SHALL SUBMIT, DAILY, THE RESULTS OF FIELD DENSITY TESTS REQUIRED BY THESE SPECIFICATIONS.
- C. COSTS OF TESTS AND INSPECTIONS: THE COSTS OF TESTS, INSPECTION, AND ENGINEERING, AS SPECIFIED IN THIS SECTION OF THE SPECIFICATIONS, SHALL BE BORNE BY THE OWNER.

	AS BUILT INFOR	CONTRACTOR	WORK. Stated By	INSPECTOR'S ACCEPTANCE BY	FIELD VERIFICATION BY	DRAWINGS CORRECTED BY	MICRO-FILM INFC		RECORDED BY	NO.
	BENCH MARKS	BENCHMARK "3-F19AB",	E BRASS CAP SET IN TOP OF CONCRETE POST	ABOUT 0.2 FT. BELOW TOP OF CONCRETE	CURB NORTHWEST CORNER OF WYOMING BLVD.	AND OSUNA RD.	ATION = 5440.56			
	SURVEY INFORMATION	FIELD NOTES	NO. BY DATE							
	ENGINEER'S SEAL		PIA	OCO NEVO	1/2	/ -			1.4.9	
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CITY OF ALBUQUERQUE CIP DESIGN AND DEVELOPMENT GROUP

MOON ST./BEAR TRIBUTARY ARROYO BRIDGE

BRIDGE DESIGN CRITERIA										
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CONCRETE

- A. WORK SHALL CONFORM TO ACI 301 AND ACI 318 EXCEPT AS
 - MATERIALS

3.1 CAST-IN-PLACE CONCRETE

NOTED BELOW.

- 1. REINFORCING: ASTM A615, GRADE 60.
- PORTLAND CEMENT: ASTM C150, TYPE I.
- AGGREGATE: ASTM C33.
- 4. WATER: DRINKABLE.
- 5. ADMIXTURES: AS APPROVED BY ENGINEER. DO NOT USE CALCIUM CHLORIDE OR ADMIXTURES CONTAINING MORE THAN 0.1 PERCENT CHLORIDE IONS.
- C. MIX DESIGN
 - 1. COMPRESSIVE STRENGTH 4.000 PSI MINIMUM AT 28 DAYS.
 - 2. SLUMP: 1 TO 4 INCHES.
 - 3. CONCRETE SHALL BE AIR ENTRAINED.
- READY-MIX CONCRETE SHALL CONFORM TO ASTM C94.
- E. ACCURATELY POSITION, SUPPORT, AND SECURE REINFORCEMENT.
- F. DETAILS OF REINFORCEMENT, INCLUDING COVER AND SPLICES, SHALL CONFORM TO ACI 318 UNLESS NOTED OTHERWISE.
- CURE CONCRETE PER ACI 301.
- CONCRETE IN HOT OR COLD WEATHER: CONFORM TO ACI 305 AND ACI 306.
- CONCRETE TESTING WILL BE BY OWNER'S TESTING LABORATORY. ADDITIONAL TESTING AND REMEDIAL WORK FOR NONCONFORMING CONCRETE SHALL BE AT CONTRACTOR'S EXPENSE. TESTS WILL BE MADE AS FOLLOWS:
- 1. SLUMP: ASTM C 143; ONE TEST AT POINT OF DISCHARGE FOR EACH DAY'S POUR OF EACH TYPE OF CONCRETE; ADDITIONAL TESTS WHEN CONCRETE CONSISTENCY SEEMS TO HAVE CHANGED.
- AIR CONTENT: ASTM C 173, VOLUMETRIC METHOD FOR LIGHTWEIGHT OR NORMAL WEIGHT CONCRETE: ASTM C 231 PRESSURE METHOD FOR NORMAL WEIGHT CONCRETE; ONE FOR EACH DAY'S POUR OF EACH TYPE OF AIR-ENTRAINED CONCRETE.
- COMPRESSION TEST SPECIMEN: ASTM C 31: ONE SET OF 4 STANDARD CYLINDERS FOR EACH COMPRESSIVE STRENGTH TEST.
- COMPRESSIVE STRENGTH TESTS: ASTM C 39: ONE SET FOR EACH DAY'S POUR OF EACH TYPE MIX; ONE SPECIMEN TESTED AT 7 DAYS, TWO SPECIMENS TESTED AT 28 DAYS, AND ONE SPECIMEN RETAINED IN RESERVE FOR LATER TESTING IF REQUIRED.
- CHAMFER EXPOSED EDGES OF CONCRETE 3/4" UNLESS NOTED OTHERWISE.

3.2 GROUT

- A. CEMENTITIOUS GROUT: PRE-MIXED, NONMETALLIC, CONFORMING TO CORPS OF ENGINEERS CRD-C 621.
- B. MIX AND INSTALL GROUT IN ACCORDANCE WITH MANUFACTURER'S INSTRUCTIONS.
- 4. PEDESTRIAN BRIDGE SPECIFICATIONS:
 - 4.1 GENERAL

THESE SPECIFICATIONS ARE FOR A FULLY ENGINEERED CLEAR SPAN BRIDGE OF STEEL CONSTRUCTION AND SHALL BE REGARDED AS MINIMUM STANDARDS FOR DESIGN AND CONSTRUCTION.

- 4.2 SPAN AND WIDTH
 - A. CLEAR SPAN LENGTH (STRAIGHT LINE DIMENSION) SHALL BE 65 FT. 0 INCHES.
 - B. BRIDGE INSIDE DECK WIDTH SHALL BE 10 FT. O INCHES.
- 4.3 ENGINEERING
 - A. UNIFORM LIVE LOAD:
 - 1. STANDARD BRIDGES OVER 50 FEET IN LENGTH SHALL BE DESIGNED FOR A LIVE LOAD OF 60 POUNDS PER SQUARE FOOT.
 - B. VEHICLE LOAD:

1. BRIDGES WITH AN INSIDE CLEAR WIDTH OF 8 FEET OR MORE SHALL BE DESIGNED FOR A 10,000 POUND VEHICLE LOAD. PLUS 30% FOR IMPACT. THIS LOAD SHALL BE DISTRIBUTED ASSUMING A FULL-SIZE PICKUP TRUCK DRIVING DOWN THE CENTER OF THE BRIDGE WITH 5,200 POUNDS ON EACH REAR WHEEL AND 1,300 POUNDS ON EACH FRONT WHEEL.

C. WIND LOAD

ALL BRIDGES SHALL BE DESIGNED FOR A WIND LOAD OF 30 POUNDS PER SQUARE FOOT OF PROJECTED SIDE AREA. AS IF

D. DESIGN CRITERIA

- 1. BRIDGE MEMBERS SHALL BE DESIGNED IN ACCORDANCE WITH THE "MANUAL OF STEEL CONSTRUCTION; ALLOWABLE STRESS DESIGN," AS ADOPTED BY THE AMERICAN INSTITUTE OF STEEL CONSTRUCTION (AISC)—LATEST EDITION.
- WELDED TUBULAR CONNECTION DESIGN SHALL BE IN ACCORDANCE WITH THE STRUCTURAL WELDING CODE (ANSI/AWS D1.1-LATEST EDITION)
- F. SHOP DRAWINGS:

MANUFACTURER'S STANDARD SCHEMATIC DRAWINGS AND DIAGRAMS:

- 1. SHOP DRAWINGS SHALL BE UNIQUE DRAWINGS, PREPARED TO ILLUSTRATE THE SPECIFIC PORTION OF THE WORK TO BE DONE.
 - 2. ALL RELATIVE DESIGN INFORMATION SUCH AS MEMBER SIZES, BRIDGE REACTIONS, GENERAL NOTES SHALL BE CLEARLY SPECIFIED ON THE DRAWINGS. COMPLETE STRUCTURAL CALCULATIONS, SHOWING THE GOVERNING STRESSES IN ALL MEMBERS AND CONNECTIONS, BEARING THE SEAL OF A REGISTERED PROFESSIONAL ENGINEER SHALL BE SUBMITTED TO THE OWNER'S REPRESENTATIVE FOR APPROVAL PRIOR TO BEGINNING FABRICATION.
 - 3. SHOP DRAWINGS SHALL BE ACCURATELY PREPARED BY SKILLED DRAFTSMEN TO BE COMPLETE IN EVERY RESPECT. DRAWINGS SHALL HAVE CROSS REFERENCED DETAILS AND SHEET NUMBERS. ALL DRAWINGS SHALL BE SIGNED AND SEALED BY A PROFESSIONAL ENGINEER REGISTERED IN THE STATE WHERE PROJECT IS LOCATED. ALL SHOP DRAWINGS SHALL BE SUBMITTED TO THE OWNER'S REPRESENTATIVE FOR APPROVAL PRIOR TO BEGINNING FABRICATION. CHECK ITEMS REQUIRED
 - 4.4 GEOMETRY
 - A. HALF-THROUGH H-SECTION TRUSS DESIGN
 - DIAGONALS:
 - 2. ONE DIAGONAL PER PANEL
 - 4.5 MATERIALS
 - A. UNPAINTED SELF-WEATHERING BRIDGES SHALL BE FABRICATED FROM HIGH STRENGTH, SELF-WEATHERING, LOW ALLOY, ATMOSPHERIC CORROSION RESISTANT ASTM A847 COLDFORMED WELDED SQUARE AND RECTANGULAR TUBING AND ASTM A588 OR ASTM A242 PLATE AND STRUCTURAL STEEL SHAPES (Fy=50,000 PSI).
 - FIELD SPLICES SHALL BE FULLY BOLTED WITH ASTM A325 TYPE 3 HIGH STRENGTH BOLTS IN ACCORDANCE WITH "SPECIFICATIONS FOR STRUCTURAL JOINTS USING ASTM A325 OR A490 BOLTS."
 - C. CONCRETE DECKING:
 - 1. GALVANIZED FORM DECK, SHOP ATTACHED, FOR OWNER'S USE IN POURING 120 PCF LIGHT WEIGHT REINFORCED CONCRETE DECK. (MINIMUM fc=4,000 PSI, 28 DAY STRENGTH)
 - D. STEEL ATTACHMENTS:
 - 1. LIFE SAFETY RAILS (MAXIMUM) CLEAR OPENINGS OF 4 INCHES) TO THE MINIMUM RAILING HEIGHT SPECIFIED IN THE UNIFORM BUILDING CODE-LATEST EDITION.
 - 2. CONTINUOUS 6" X 1/4" TOE PLATE, 2" ABOVE DECK LEVEL.
 - WELDING ELECTRODES: WELDING SHALL BE WITH E70XX OR E80XX ELECTRODES WHICH PRODUCE WELDS HAVING THE SAME CHARACTERISTICS AS THE STEEL BASE METAL.
 - F. : HAND RAILING.

INTERIOR HANDRAIL HEIGHT SHALL EXCEED 42" FOR PEDESTRIAN USE. TOP OF TRUSS TOP CHORD TO TOP OF DECK DIMENSION SHALL MEET OR EXCEED 42".

- 4.6 FABRICATION
 - A. WELDING OPERATORS SHALL BE CERTIFIED FOR THE 6G POSITION IN ACCORDANCE WITH ANSI/A.W.S.D1.1-LATEST EDITION.
- 4.7 FINISHING

SELF-WEATHERING BRIDGES: ALL BOLDLY EXPOSED SURFACES OF SELF-WEATHERING STEEL SHALL BE CLEANED IN ACCORDANCE WITH STEEL STRUCTURES PAINTING COUNCIL SURFACE PREPARATION SPECIFICATION NO. 6, COMMERCIAL BLAST CLEANING, SSPC-SP 6-LATEST EDITION.

4.8 DELIVERY AND ERECTION

- A. DELIVERY OF THE BRIDGE SHALL BE MADE TO A LOCATION NEAREST THE SITE WHICH IS ACCESSIBLE TO OVER-THE-ROAD TRACKS.
- THE CONTRACTOR WILL BE RESPONSIBLE FOR UNLOADING THE BRIDGE FROM THE TRUCK AT THE TIME OF ARRIVAL THE BRIDGE MANUFACTURER WILL NOTIFY THE OWNER IN ADVANCE OF THE EXPECTED TIME OF ARRIVAL AT THE SITE
- THE MANUFACTURER WILL PROVIDE WRITTEN INSTRUCTIONS TO THE OWNER OR HIS REPRESENTATIVE IN THE PROPER LIFTING PROCEDURE FOR THE UNLOADING OF THE BRIDGE CARE MUST BE TAKEN TO PREVENT DAMAGE TO THE FINISH OF THE BRIDGE.
- THE UNLOADING, SPLICING (IF REQUIRED), AND PLACEMENT OF THE BRIDGE WILL BE THE RESPONSIBILITY OF THE CONTRACTOR. THE PROCEDURE FOR BOLTING FIELD SPLICES WILL BE GIVEN TO THE CONTRACTOR BY THE MANUFACTURER.
- THE CONTRACTOR SHALL BE RESPONSIBLE FOR INSTALLING THE ANCHOR BOLTS IN ACCORDANCE WITH THE DIMENSIONS PROVIDED BY THE BRIDGE MANUFACTURER.

4.9 FOUNDATIONS

- CONSTRUCTION OF THE BRIDGE SUPPORTING FOUNDATION (ABUTMENT, PIER OR FOOTING) WILL BE THE RESPONSIBILITY OF THE CONTRACTOR.
- INFORMATION AS TO THE BRIDGE SUPPORT REACTIONS. ANCHOR BOLT LOCATION AND PLACEMENT WILL BE FURNISHED BY BRIDGE MANUFACTURER.

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CITY OF ALBUQUERQUE CIP DESIGN AND DEVELOPMENT GROUP

MOON ST./BEAR TRIBUTARY ARROYO BRIDGE BRIDGE DESIGN CRITERIA

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