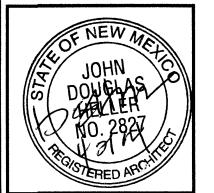


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1015 Tijeras Avenue NW Suite 220 Albuquerque 87102 505 268 4144[p] 505 268 4244 [f]



drawn by mws

broject manager Douglas Heller, AIA

date 4/13/04

OYAL PACIFIC WAREHOUSE

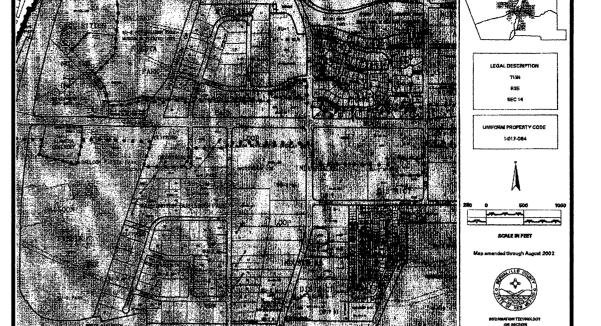
so Del Norte (Frontage Road)

querque, New Mexico

title

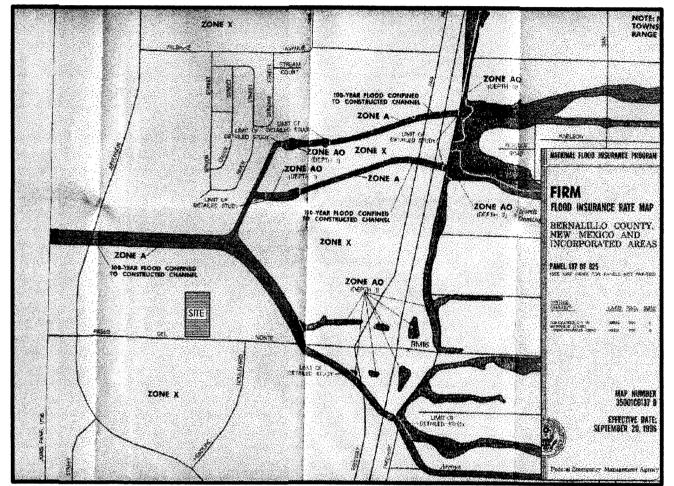
chitectural Site Plan - DRB

sheet-



C-17-Z

Vicinity Map



Flood Map

Site Location - As shown by the Vicinity Map (Zone Atlas Map C-17), the proposed commercial warehouse site is located on a two acre parcel north of the Boulevard. At present, the site is undeveloped and drains roughly from east to west into the existing roadway. The purpose of this project is to construct a 25,400 square foot commercial building, pavement, and parking facilities.

Legal Description - Tract 3-C, Lands of Los Angeles Investors., Albuquerque, New Mexico. Section 14, T 11 N, R 3 E, N.M.P.M.

Benchmark - Basis of elevation is from City of Albuquerque bench mark "15-C17" with elevation stamped 5,148.68 feet.

Flood Zone - As shown by Panel 137of 825 of the National Flood Insurance Program Flood Insurance Rate Maps (FIRM) for the City of Albuquerque, New Mexico, dated September 20, 1996, this site does not lie within a designated flood hazard zone.

Existing Conditions - Currently, the project site drains from east to west across the undeveloped two acre lot to the private north/south roadway located along the western property boundary. The roadway drains to the south from the railroad spur at a slope of 0.4 percent and discharges onto the Paseo del Norte Frontage Road. A 24-inch storm drain is located within the frontage road. A drop inlet is located on the frontage road south of the project site and intercepts runoff from the east. No offsite runoff impacts the project site. The Domingo Baca Arroyo Channel is located north of the project site approximately 400 feet. Developments along the channel tend to drain into the channel. All other offsite runoff is contained within the frontage road and storm drainage system.

Proposed Grading - The Grading and Drainage Plan shows 1) existing and proposed grades indicated by spot elevations; 2) the limit of existing and proposed improvements. The ensuing area will be graded and paved to flow west to the historical drainage outfall points within the existing private roadway. All runoff is to be contained within the private roadway right-of-way then will discharge south onto the frontage road. Additional storm drainage inlets will intercept the runoff in the roadway and will carry it to the Domingo Baca Channel near Jefferson Boulevard.

The warehouse building roof will drain to the south through two roof drains as shown on the Plan.

The loading dock is to be depressed approximately four feet from the finish floor elevation. This area was found to be too deep to gravity drain to the street or existing storm drain system. Therefore, it will be required to install inlets with a sump and pump system that will discharge directly to the sanitary sewer system. Also, under NPDES requirements, the likelihood of fuel contamination related to the trucks in the loading dock should be addressed. By discharging to the sanitary sewer system, offsite contamination can be controlled and held to a

Hydrologic Methods - The drainage basin map shows six separate onsite subbasins A-F and six offsite basins 1-6 to assess peak flow rates at various points around the project site culminating at either the existing roadway or the concrete channel. The calculations which appear hereon analyze both the existing and developed conditions for the 100-year, 6-hour rainfall event. The process outlined in the DPM, Section 22.2 was used to quantify the peak flow rates and volumes. As shown by these calculations, the fully developed commercial improvements will result in an increase in runoff generated by the site. Downstream capacity is sufficient to carry the entire peak runoff generated by the design storm.

The subject property will increase the existing peak runoff by about three cubic feet per second as shown on the calculations. A spreadsheet for Precipitation Zone 2 is included on this plan. This spreadsheet outlines the peak runoff and volume generated for each subbasin for existing and proposed fully developed conditions. Additional calculations show the street capacity for the private roadway and for the Paseo del Norte frontage road and the existing drop inlets and storm drain system.



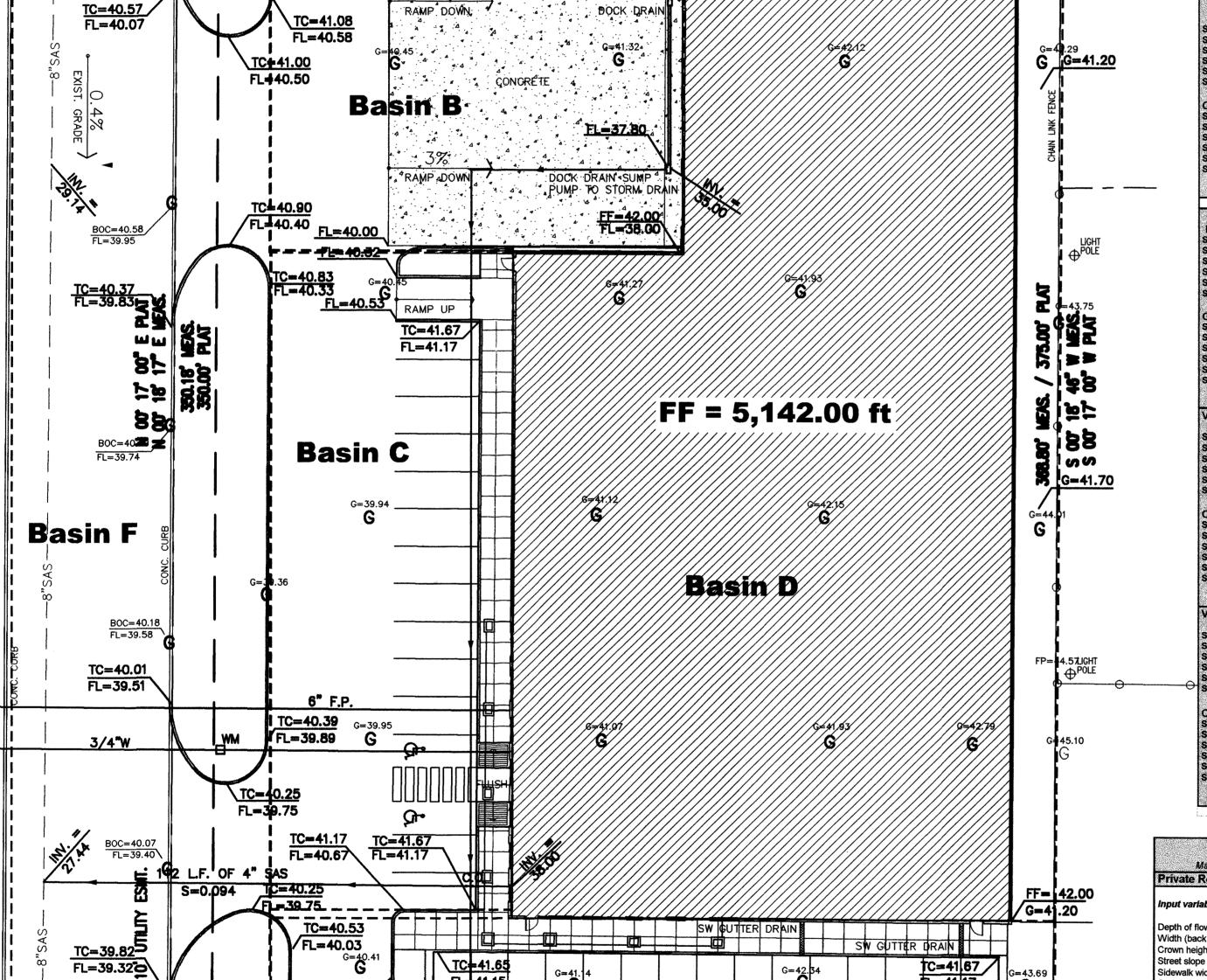
Phototopo Basin Map

lydrologic	Calculation	ons - C	OA DP	M 22.2			Royal Paci Paseo del Nort	ific Wareho		April 2004
recipitation one 2				(inches)	P80 2.01		2.75		10days 3.95	7112001
xcess recipitation	(inches) 0.53	0.78	1.13	2.12		0.53	0.78	1.13	2.12	
eak Discharge	(cfs/acre) 1.56	2.28	3.14	4.7		1.56	2.28	3.14	4.7	
Drainage	Land Treatments -	Proposed .	Developed (Conditions		Land Treatments -	Allowable Develop	ed Conditions		
Areas Subbasin A	A 0.00	₽	c 0.32	0.00	Area (ac) 0.32	A	0.05	c 0.00	D 0.27	Area (ac) 0.32
Subbasin B Subbasin C	0.00 0.00	0.00	0.18 0.42	0.00	0.18 0.42	0.00 0.00	0.03 0.06	0.00	0.15 0.36	0.18 0.42
Subbasin D Subbasin E	0.00	0.00	0.66 0.25	0.00	0.66 0.25	0.00	0.10 0.04	0.00	0.56 0.21	0.66 0.25
Subbasin F	0.00 0 %	0.00 0 %	0.41 100%	0.00 0 %	<u>0.41</u> 2.24	0.00 <i>0</i> %	0.06 15%	0.00 <i>0</i> %	0.35 85%	0.41 2.24
Offsite Subbasin 1	0.00	0.24	0.00	1.36	1.60	0.00	0.24	0.00	1.36	1.60
Subbasin 2 Subbasin 3	0.00 0.00	0.14 0.33	0.00	0.77 1.87	0.90 2.20	0.00 0.00	0.14 0.33	0.00	0.77 1.87	0.90
Subbasin 4 Subbasin 5	0.00	0.27 0.18	0.00	1.53 1.02	1.80 1.20	0.00	0.27	0.00	1.53	2.20 1.80
Subbasin 6	0.00	0.15	0.00	0.85	1.20 1.00 8.70	0.00 0.00	0.18 0.15	0.00 0.00	1.02 0.85	1.20 <u>1.00</u> 8.70
	0%	15%	0%	85%		0%	15%	0%	85%	
Discharge	Peak Flow Rate A	8	nditions C	D	100 yr Q (cfs)	Peak Flow Rate - I A	Developed Conditio	ns C	D	100 yr Q (cfs)
Subbasin A Subbasin B	0.00	0.00	1.00 0.57	0.00 0.00	1.00 0.57	0.00	0.11 0.06	0.00	1.28 0.72	1.39 0.78
Subbasin C Subbasin D	0.00 0.00	0.00	1.32 2.07	0.00 0.00	1.32 2.07	0.00 0.00	0.14	0.00	1.68 2.64	1.82 2.86
Subbasin E Subbasin F	0.00	0.00	0.79 1.29	0.00	0.79	0.00 0.00	0.09 0.14	0.00	1.00 1.64	1.08 1.78
Offsite					7					10
Subbasin 1 Subbasin 2	0.00	0.55 0.31	0.00	6.39 3.60	6.94 3.90	0.00 0.00	0.55 0.31	0.00 0.00	6.39 3.60	6.94 3.90
Subbasin 3 Subbasin 4	0.00	0.75 0.62	0.00	8.79 7.19	9.54 7.81	0.00 0.00	0.75 0.62	0.00	8.79	9.54
Subbasin 5 Subbasin 6	0.00	0.41 0.34	0.00	4.79 4.00	6.20 4.34	0.00 0.00 0.00	0.62 0.41 0.34	0.00	7.19 4.79	7.81 5.20
					38	0.00,	0.34	0.00	4.00	<u>4.34</u> 38
Six Hour	Runoff Volume - E Six Hour Storm				100 yr V (cu-ft)	Runoff Volume - D	eveloped Condition	ıs		100 yr V (cu-ft)
Subbasin A Subbasin B	0	0	1,313 738	0	1,313 738	0 :	136 76	0	2,093 1,177	2,229 1,254
Subbasin C Subbasin D	0	0	1,723 2,707	0	1,723 2,707	0	178 280	0	2,747 4,317	2,926 4,598
Subbasin E Subbasin F	0	0	1,025 1,682	0	1,025 <u>1,682</u>	0	106 174	0	1,635 2,682	1,741 <u>2,856</u>
)ffsite	<u> </u>				9,188					15,604
Subbasin 1 Subbasin 2	0	680 382	0	10,466 5,887	11,146 6,269	0	680 382	0	10,466 5,887	11,146 6,269
Subbasin 3 Subbasin 4	0	934 764	0	14,391 11,774	15,325 12,539	0	934 764	0	14,391 11,774	15,325 12,539
iubbasin 5 iubbasin 6	0	510 425	0	7,850 6,541	8,359 6,966	0	510 425	Ŏ.	7,850 6,541	8,359 6,966
		1			60,604					60,604
	Runoff Volume - E Ten Day Storm E		ditions		100 yr V (cu-ft)	Runoff Volume - De Ten Day Storm Ev	Property of the property of th	5		100 yr V (cu-ft)
iubbasin B iubbasin C					1,313 738	11 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1				3,809 2,142
Subbasin D					1,723 2,707		5			4,999 7,856
Subbasin E Subbasin F					1,025 <u>1,682</u>					2,976 <u>4,880</u>
Offsite					9,188					26,662
ubbasin 1 Subbasin 2					19,044 10,712					19,044
Subbasin 3 Subbasin 4					26,186					10,712 26,186
iubbasin 5		i			21,425 14,283 11,903					21,425 14,283
ubbasin 6						_				11,903

ivate Roadway	at Front	age	Road		
out variables:			Output Parameters:		
pth of flow	0.40	ft	Capacity at d	11.2	cfs
dth (back of curb)	38.0	ft	@ top of curb	23.6	cfs
own height	0.35	ft	@ back of walk	23.6	
eet slope	0.40	%	Velocity at d	1.7	fps
lewalk width	0.0	ft	Hydraulic Jump	0.45	ft
rb height	6.00	in	Gutter width	1.5	ft
dian width	0.0	ft	Gutter depression	1.5	in
back of walk	100.0	%	Asphalt lip	0	in
pack of walk	100.0	%	Manning's n	0.017	

Frontage Road Do	ownstre	am (of Private Road		
Input variables:			Output Parameters:	•	
Depth of flow	0.40	ft	Capacity at d	36.7	cfs
Width (back of curb)	36.0	ft	@ top of curb	72.7	cfs
Crown height	0.31	ft	@ back of walk	72.7	cfs
Street slope	3.40	%	Velocity at d	5.4	fps
Sidewalk width	0.0	ft	Hydraulic Jump	0.84	ft
Curb height	6.00	in	Gutter width	1.5	ft
Median width	0.0	ft	Gutter depression	1.5	in
Rt back of walk	100.0	%	Asphalt lip	0	in
Lt back of walk		%	Manning's n	0.017	
Note: To maintain two Note: Input 100% slo			, depth cannot exceed lik for vertical walls.	0.215	feet BURAK

Frontage Road Dr	op Ink	et Capa	city each side		
Gutter flow, Q	16.0	cfs	Efficiency:		
Flow velocity, V			Frontal:total	25	%
Flow depth, Y	0.4	feet	Side flow, Rs	7	%
Grate length, L	3.0	feet	경상으로 하는 사람이 하는 것을 하게 되는 것 같아. 이 그 그들은 사람이 되었다면 하게 되었다.	30	%
Gutter width, W	2.0	feet	Top width, T	20.0	feet
Cross slope, Sx	2.00	%			
			Intercepted flow	4.8	cfs



POWER POLE W/ELEC. LINE

G = 42.25

210.77 MEAS. / 210.63 PLAT

FF= 42.00 G=41.80

Basin A

- 17' RAILROAD ESM1: 11

TC=41.07

FL=40.57

FL=40.24

FL=40.42

FL=40.65

BOC=42.22

G = 42.89

G = 43.26

TC=41.62

FL=41.12

TC=41.55 FL=41.05

Basin E

AREA TAKEN FOR

_ADDITIONAL___

RIGHT-OF-WAY

G = 39.97

BOC = 40.27

40' WIDE PRIVATE ROADWAY ESMT. FILED:

07/17/78 IN VOL. B15, FOLIO 35.

SCALE: 1" = 20'

Frontage Road 18"SAS —

----(Paseo del Norte)

FL = 40.18

0.00	0.18	0.00	1.02	1.20	0.00	0.18	0.00	1.02	1.20	
0.00	0.15	0.00	0.85	1.00	0.00	0.15	0.00	0.85	1.00	
				8.70					8.70	
0%	15%	0%	85%		0%	15%	0%	85%		
		San San	100 - 11 675 - 475 155 147		i			:		
Peak Flow Rate	Existing Col	CACO TO COLOR		100 yr	Peak Flow Rate - De	PROPERTY OF THE PROPERTY OF TH	and all the second and the parties		100 yr	!
Α	8	C	D	Q (cfs)	Α	В	C	D	Q (cfs)	
0.00	0.00	1.00	0.00	1.00	0.00	0.11	0.00	1.28	1.39	
0.00	0.00	0.57	0.00	0.57	0.00	0.06	0.00	0.72	0.78	
0.00	0.00	1.32	0.00	1.32		0.14	0.00	1.68	1.82	
0.00	0.00	2.07	0.00	2.07	0.00	0.23	0.00	2.64	2.86	
0.00	0.00	0.79	0.00	0.79	0.00	0.09	0.00	1.00	1.08	
0.00	0.00	1.29	0.00	<u>1.29</u>	0.00	0.14	0.00	1.64	1.78	,
									10	
0.00	0.55	0.00	6.39	6.94	l					
0.00	0.33	0.00	3.60	3.90	0.00	0.55	0.00	6.39	6.94	
0.00	0.75	0.00	8.79		0.00	0.31	0.00	3.60	3.90	
0.00	0.73	0.00	7.19	9.54	0.00	0.75	0.00	8.79	9.54	
0.00	0.62	0.00	4.79	7.81	0.00	0.62	0.00	7.19	7.81	
0.00	0.41	0.00		5.20	0.00	0.41	0.00	4.79	5.20	
0.00	0.34	0.00	4.00	4.34 38	0.00	0.34	0.00	4.00	4.34	
									38	
Runoff Volume - E	xistina Con	ditions		100 yr	Runoff Volume - Dev	eloned Conditions			100 yr	e e e e e e e e e e e e e e e e e e e
Six Hour Storm				V (cu-ft)	Six Hour Storm		The second second		V (cu-ft)	
0 ;	0.	1,313	0	1,313	0	136	0	2,093	2,229	
0	0 ;	738	0	738	0	76	0	1,177	1,254	
0	0	1,723	0	1,723	0	178	0	2 747	2,926	
0	0	2,707	0	2,707	0	280	0	2,747 4,317	4,598	
0	0	1,025 1,682	0	1,025	0:	106	0	1,635	1,741	
0	0	1,682	0	1,682	0	174	0	2,682	2,856	
	1			9,188					15,604	
	;				;	,				
Ö	680	0	10,466	11,146	0	680	0	10,466	11,146	
0 :	382	0	5,887	6,269	0	382	Ŏ.	5,887	6,269	
0	934	0	14,391	15.325	Ŏ	934	0		15,325	
0	764	0		15,325 12,539	Ŏ	764	ŏ	14,391 11,774	12,539	
0		0	7,850	8,359	ŏ	510	· - · · · · · · · · · · · · · · · · · ·	7,850	8,359	
0	510 425	0	6,541	6,966	Ŏ	425	ŏ	6,541	6,966	
				60,604		720			60,604	
						2 			00,004	
Runoff Volume - E	vistina Con	ditions		100 yr	Runoff Volume - Dev	etabled Constant				
					Turior Volume - Dev	виреи сопишиль		9C-178 M	100 yr	

ivota Danduvav		1017, 2,041010	apacity in a street section		
ivate Roadway	at From	age	Road		
out variables:			Output Parameters:	•	
pth of flow	0.40	ft	Capacity at d	11.2	cfs
dth (back of curb)	38.0	ft	@ top of curb	23.6	cfs
own height	0.35	ft	@ back of walk	23.6	cfs
eet slope	0.40	%	Velocity at d	1.7	fps
ewalk width	0.0	ft	Hydraulic Jump	0.45	ft
rb height	6.00	in	Gutter width	1.5	ft
dian width	0.0	ft	Gutter depression	1.5	in
back of walk	100.0	%	Asphalt lip	0	in
back of walk	100.0	%	Manning's n	0.017	

T			
25 7	%		
30	%		
20.0			
4.8	cfs		

Mullen Heller

Architecture P.C. 1015 Tijeras Avenue NW Suite 220 Albuquerque 87102

505 268 4144[p] 505 268 4244 [f]

03-37	mws	Douglas Heller, AIA	2/23/04
job number	drawn by	project manager	date

CIFIC (Frontage Mexico Δ.

REHOUSE

WAREHOUSE AREA: 21,662 SF. OFFICE/WAREHOUSE 25,600 SQ. FT. 7500 SF OFFICE AREA: 3,938 SF. FRONTAGE ROAD

(FOR PASEO DEL NORTE)
(RIGHT-OF-WAY VARIES)

Landscape Legend

Common Name

SHOPE THEE 17
HOHETLOCKS(H) CHINEGE PISTACHE(M)

KAPHIOLEPK, CHAMES, CL)
ADOCHE -FLUMES, BLUENES EM)
CHEPET SYSTE(M)

CHAMEHAL SPASSOM) 9

SPRENDING SPORTED EN HONETSIKKLE (M)

HOWE JEWEL FINER FORK

Size

2"2N.

1594

54

Quantity

Landscape Calculations

20,300 \$

65,687.4F TOTAL LOT AREA (sf) TOTAL BUILDING AREA (sf) LANDSCAPE REQUIREMENT (%)

-25,600

TOTAL LANDSCAPE REQUIRED (sf) 9012 TOTAL LANDSCAPE PROVIDED (sf) 2290

Landscape Notes

Landscape Maintenance and Irrigation system maintenance shall be the responsibility of the owner.

All Landscaping shall be watered by a complete underground irrigation system operated by automatic timer. Bubblers to trees and (2) drip emitters per shrub. Point of Connection for irrigation system is unknown at current time and will be coordinated in the field.

It is the intent of this plan to comply with the City of Albuquerque, water conservation and waste water ordinance.

All landscape beds shall be planted so to achieve 75% live ground cover at maturity.

Approval of this plan does not constitute or imply exemption from water waste provisions of the water conservation landscaping and water waste ordinance. Water management is the sole responsibility of the property owner.

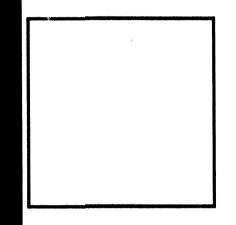


Landscape Plan



Architecture P.C.

1015 Tijeras Avenue NW Suite 220 Albuquerque 87102 505 268 4144[p] 505 268 4244 [f]



PACIFIC WAREHOUSE

LOO 1

