Supplemental Information for Advance Auto

Tract 1, Paradise Plaza



By

ISAACSON & ARFMAN, P.A.

Consulting Engineering Associates

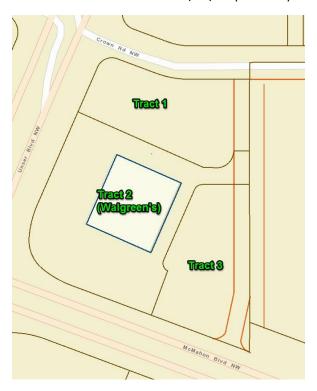
Thomas O. Isaacson, PE(RET.) & LS(RET.) Fred C. Arfman, PE Åsa Nilsson-Weber, PE

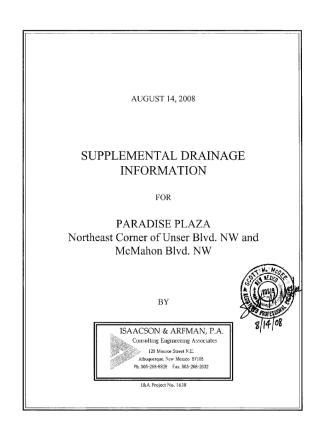
Supplemental Information for Advance Auto Tract 1, Paradise Plaza

Ву



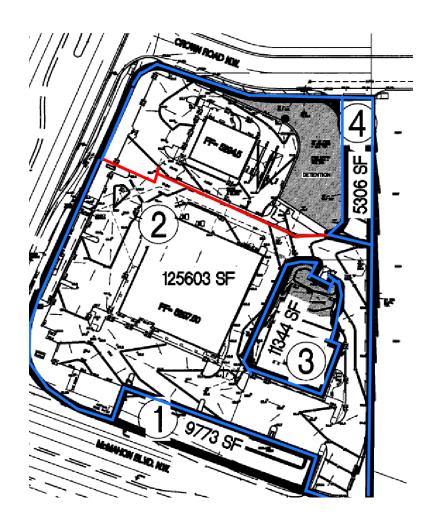
The Supplemental Drainage Information for **PARADISE PLAZA** (Tracts 1, 2 and 3) was prepared by Isaacson & Arfman PA dated 8/14/08 (PP-DMP).



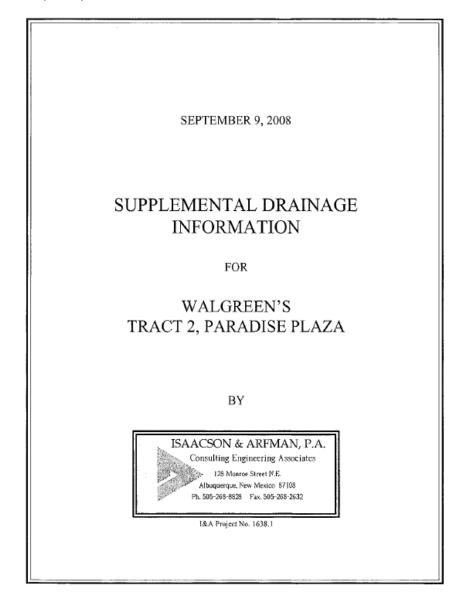


Based on this PP-DMP, the proposed Advance Auto property (Tract 1) falls within two of the master planned basins.

- Basin 4 (0.1 acre) is the existing asphalt access on the northwest side of the property. This road
 was constructed as part of the initial construction on Tract 2 (Walgreens) and will remain. Per
 the PP-DMP, this basin free discharges 0.5 cfs to Crown Road NW.
- The main Advance Auto building and parking fall within Basin 2 (2.9 acre total) which includes the Walgreens building, parking and a portion of the future construction that will occur on Tract 3.



The Supplemental Drainage Information for **WALGREEN's** (Tract 2) was prepared by Isaacson & Arfman PA dated 9/9/08 (W-DP).



Per the W-DP, Tract 2 (Walgreen's: 1.7 acre) fully developed generates 6.7 cfs.

The pond is sized to accept 12.4 cfs from 2.9 acres (Portions of Tracts 1, 2 and 3).

Therefore, the remaining 1.2 acre portion of Basin 2, is permitted to contribute (12.4-6.7) 5.7 cfs or 4.75 cfs per acre.

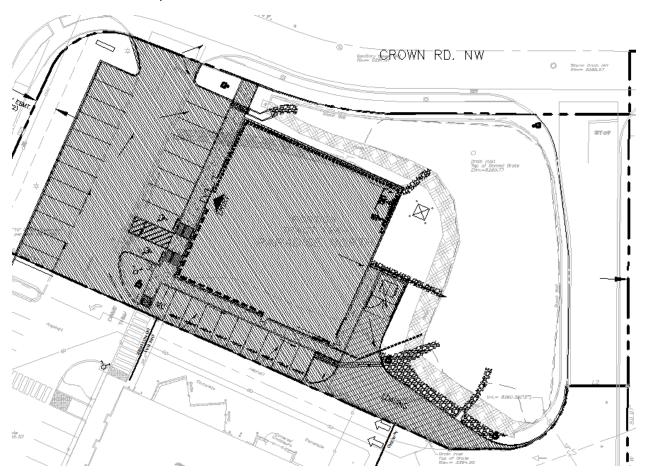
The portion of Tract 1 (Advance Auto) draining to this pond is 0.89 acres (total area) - 0.1 (PP-DMP basin free discharging from PP-DMP Basin 4 - northwest access drive) = .79 acre * 4.75 cfs per acre = 3.75 cfs allowable.

The total of Tract 1 generates 3.0 cfs. Subtracting the 0.5 cfs free discharging at the northwest access drive, the total from Tract 1 will be 2.5 cfs (3.75 cfs allowable).

CALCULATIONS: Advance Auto : April 26, 2016									
Based on Drainage Design Criteria for City	of Albuquerque Section	n 22.2,	, DPM, Vol 2, da	ated Ja	n., 1993				
	ON-SIT	E							
AREA OF SITE:	39160.44	SF	=	0.90					
	100-year, 6-hour								
	ACTUAL				EXCESS PRECIP:				
			Treatment SF	%	Precip. Zone 1				
	Area A	=	11748	30%	$E_A = 0.44$				
	Area B	=	0	0%	$E_{\rm B} = 0.67$				
	Area C	=	3916	10%	$E_{\rm C} = 0.99$				
	Area D	=	23496	60%	$E_{\rm D} = 1.97$				
	Total Area	=	39160.44	100%	_				
Weighted E = $\frac{E_A A_A + E_B A_B + E_C A_C + E_D A_D}{A_A + A_B + A_C + A_D}$									
Weighted E =	$A_A + A_B + A$	$_{\rm C}$ + ${\rm A}_{\rm D}$)		1				
Weighted E =				in.]				
Weighted $E =$ On-Site Volume of Runoff: $V360 =$	$A_A + A_B + A$	$_{\rm C}$ + ${\rm A}_{\rm D}$)	in.]				
· ·	$A_A + A_B + A$ Developed E	$_{\rm C}$ + ${\rm A}_{\rm D}$	1.41	in.]				
· ·	$\begin{array}{c} A_A + A_B + A \\ \hline \text{Developed E} \\ \hline E*A / 12 \\ \hline \text{Developed V}_{360} \\ \hline A+Q_{pB}A_B+Q_{pC}A_C+Q_{pD} \\ \hline \end{array}$	$C + A_{\Gamma}$ $=$	1.41]				
On-Site Volume of Runoff: V360 = On-Site Peak Discharge Rate: $Qp = Q_{pA}A_{pA}$. For Precipitation Zone 1	$A_A + A_B + A$ Developed E $E*A / 12$ Developed V_{360}	$\frac{C + A_{D}}{=}$ $=$ $\frac{A_{D} + A_{D}}{A_{D} + A_{D}}$	1.41 4611 3,560						

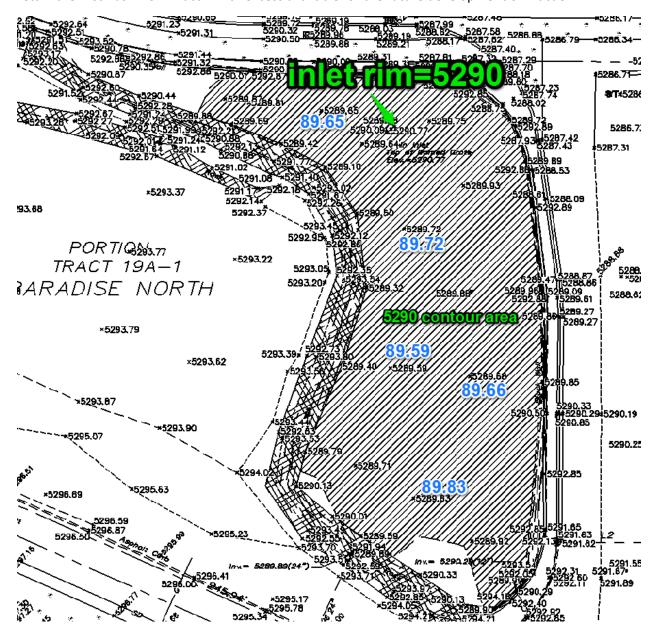
FIRST FLUSH:

The impervious area (building roof and asphalt pavement) draining to the main detention pond is 0.4 acres. Based on this area, the first flush retention volume = 544 cf.



IMPERVIOUS AREA TO POND		DESCRIPTION		MAIN BLDG/PAR	KING TO POND			
Area of basin flows =	19200	SF		=	0.4 Ac.			
The following calculation	ons are based on	Γreatment area	s as shown in t	able to the rig	ght LAND T	REATMENT		
	Sub-basin Weigl	nted Excess Pr	ecipitation (see	formula abov	ve) $A =$	0%		
	Weighted E	=	1.97 i	n.	$\mathbf{B} =$	0%		
	Sub-basin Volun	ne of Runoff (see formula above)			C =	0%		
	V ₃₆₀	=	3152	CF	D =	100%		
	Sub-basin Peak l	Discharge Rate: (see formula above)			FIRST F	FIRST FLUSH VOL.		
	Q_{P}	=	1.9	cfs	•	544 CF		

The existing pond constructed as part of the initial development has an existing outlet with a rim of 5290. In the image below, the area within the 5290 (representing the pond bottom) contour is shaded. Various as-built spot elevations within this area reflect available retention between 0.1′-0.4′. Assuming a 0.2′ average retention over the area of 7277 sf provides an available volume of 1455 cf which will retain the first flush from Tract 1 with excess available for the future development of Tract 3.



In the future, the development of Tract 3 is permitted to discharge 5.7 cfs - 2.5 cfs = 3.2 cfs to the main pond to bring the total to:

2.5 cfs from Tract 2 (proposed)

6.7 cfs from Tract 1 (existing)

3.2 cfs from Tract 3 (future)

12.4 cfs allowable to Main Pond