

4" THICK CONCRETE

2% CROSS SLOPE MAX. ——

SECTION A-A

2% CROSS SLOPE MAX.

H.C RAMP SHALL NOT EXCEED 6', OR 8% SLOPE IN ANY DIRECTION, AND SHALL CONFORM W/ ALL APPLICABLE ANSI REQ'S. G.C SHALL VERIFY CONFORMANCE W/ ALL APPLICABLE ANSI REQ'S

H.C RAMP SHALL NOT EXCEED 6', OR 8% SLOPE IN ANY DIRECTION, AND SHALL CONFORM

VARIES RE: SITE

1V:12H MAX.

H.C RAMP SHALL NOT EXCEED 6', OR 8% SLOPE IN ANY DIRECTION, AND SHALL CONFORM

W/ ALL APPLICABLE ANSI REQ'S. G.C SHALL VERIFY CONFORMANCE W/ ALL APPLICABLE ANSI REQ'S

1V:12H MAX. -

ADA APPROVED TRUNCATED

PROVIDE 1/4" DEEP X 1/4" WIDE GROOVES 2" O.C

EXTEND THE FULL WIDTH AND DEPTH OF THE RAMP.

DOME STRIP

W/ ALL APPLICABLE ANSI REQ'S. G.C SHALL VERIFY CONFORMANCE W/ ALL APPLICABLE ANSI REQ'S

(6' MIN. WIDTH)

DIRECTION OR IF RAMP

PROVIDE 5' SQUARE MINIMUM LANDING AT TOP OF RAMP FOR

6' MAX. TRANSITION

1V:10H MAX.

SIDEWALK

CHANGE IN DIRECTION ——

SLOPE MAX. ---

17 N.T.S

LANDSCAPE AREA

RAMP

SIDEWALK SECTION 1:20 MAX

— PROVIDE ADA APPROVED TRUNCATED DOME

MONOLITHIC CONCRETE HANDICAP RAMP
INTEGRALLY COLORED DAVIS COLORS 160
"BRICK RED"; SMOOTH TEXTURE FINISH
W/ 1/4" DEEP X 1/4" WIDE GROOVES 2"

STRIP 2' IN WIDTH

MONOLITHIC CONCRETE HANDICAP RAMP INTEGRALLY COLORED DAVIS COLORS 160

BRICK RED"; SMOOTH TEXTURE FINISH

- PROVIDE ADA APPROVED TRUNCATED DOME

STRIP 2' IN WIDTH

TTER AS APPLICABLE (SEE

6' MAX. TRANSITION

1V:10H MAX.

HANDICAP RAMP (4000 psi)

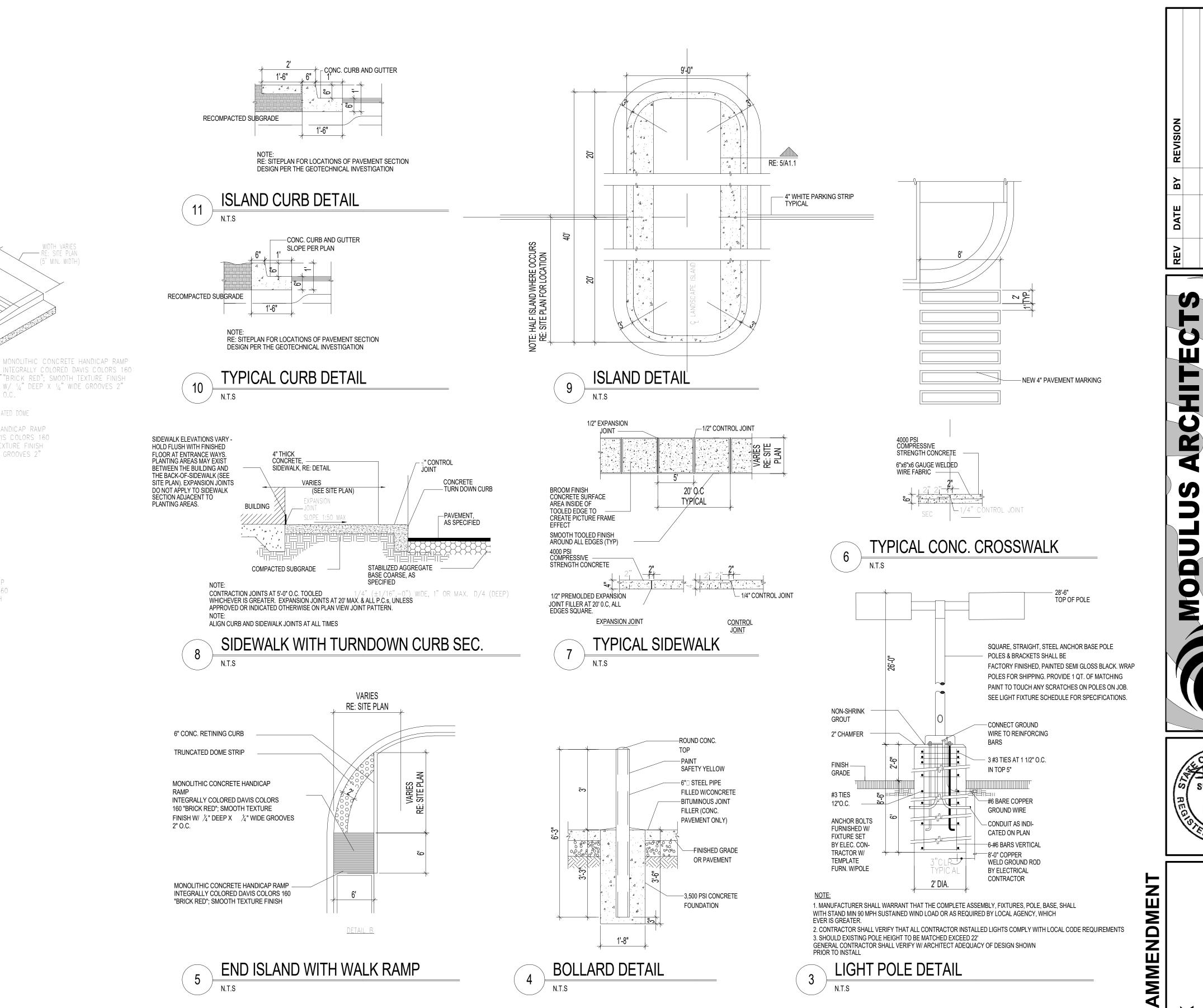
INSTALL 1" TOOLED RADIUS ON ALL EXPOSED EDGES

- MONOLITHIC INTEGRALLY COLORED CONCRETE

PROVIDE CONST. MGR W/ SAMPLE FOR APPROVAL

1/4" DEEP X 1/4" WIDE GROOVES 2"





DUNBAR

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TURA

ARCHITE(

SHEET

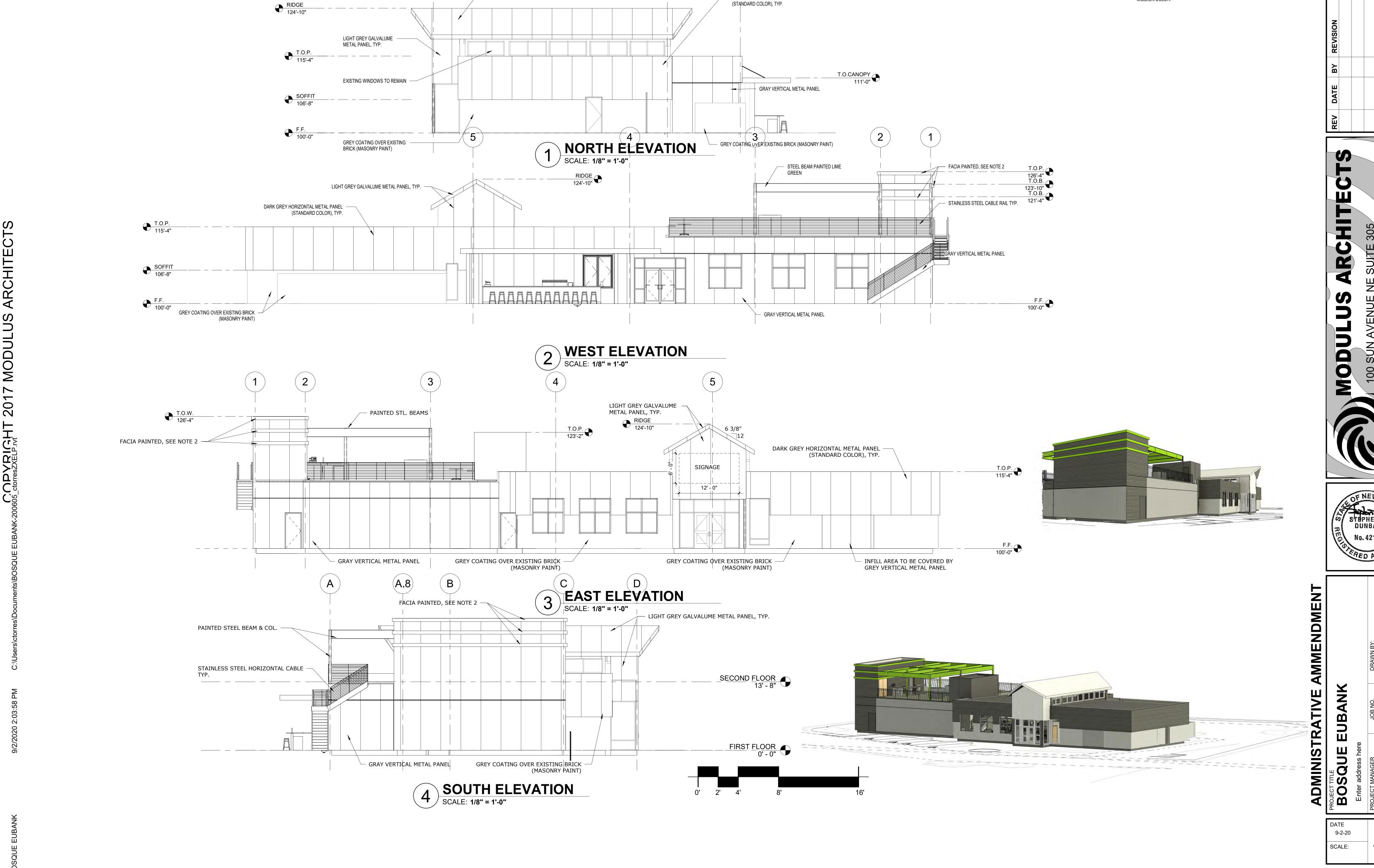
AA1

**ADMINIS** 

DATE

9-2-20

SCALE:



DARK GREY HORIZONTAL METAL PANEL

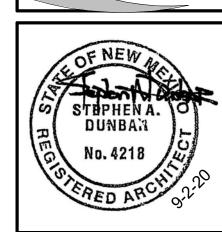
- LIGHT GREY GALVALUME METAL PANEL, TYP.

**GENERAL NOTES:** 

METAL PANEL TO BE RIBBED OR CORRUGATED, TYP.
 COLORS SELECTED FROM STANDARD COLOR CHART.

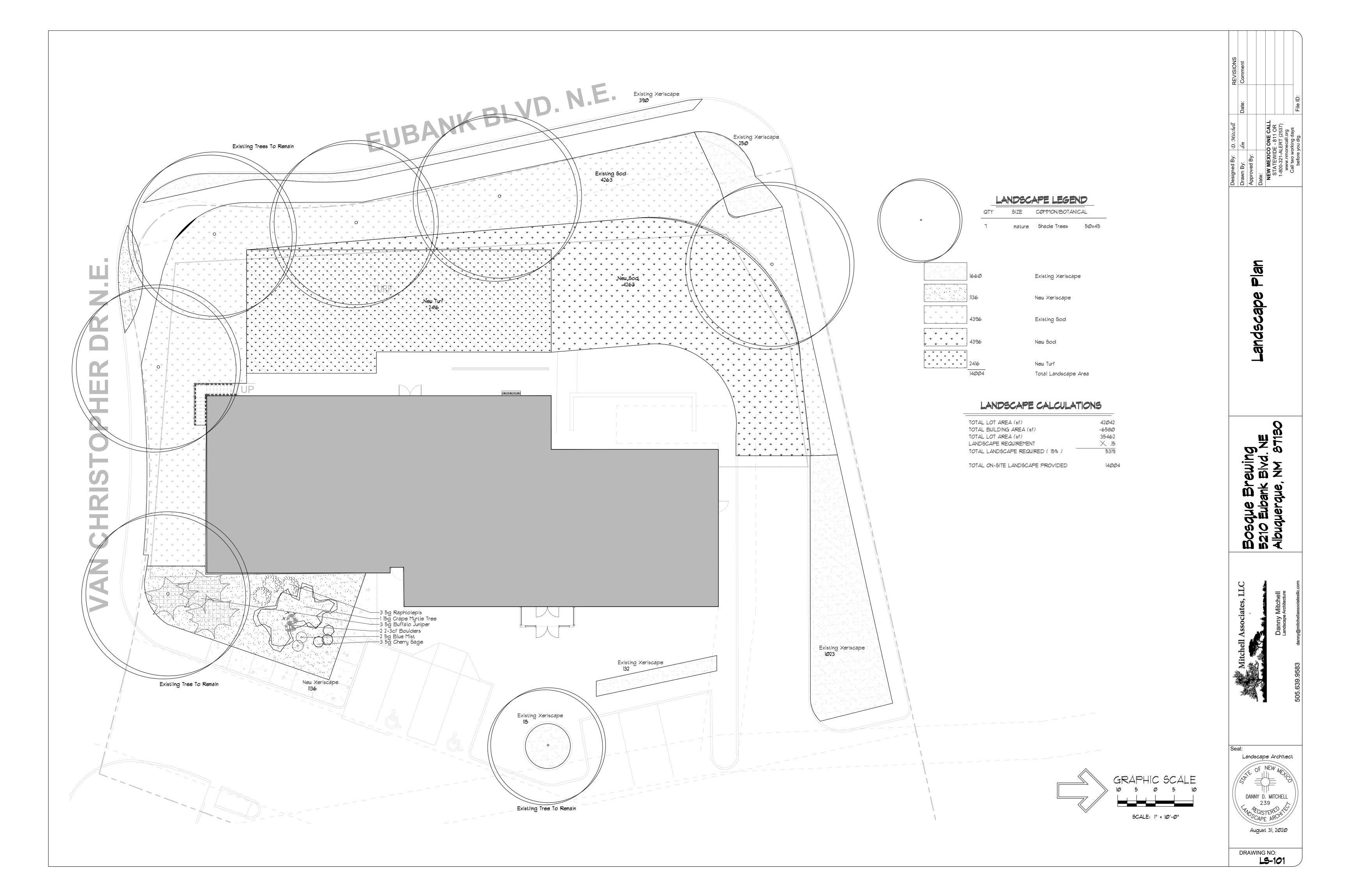
3. METAL CANOPY CAN ONLY BE INSTALLED IF FIRE PROTECTION IS NOT

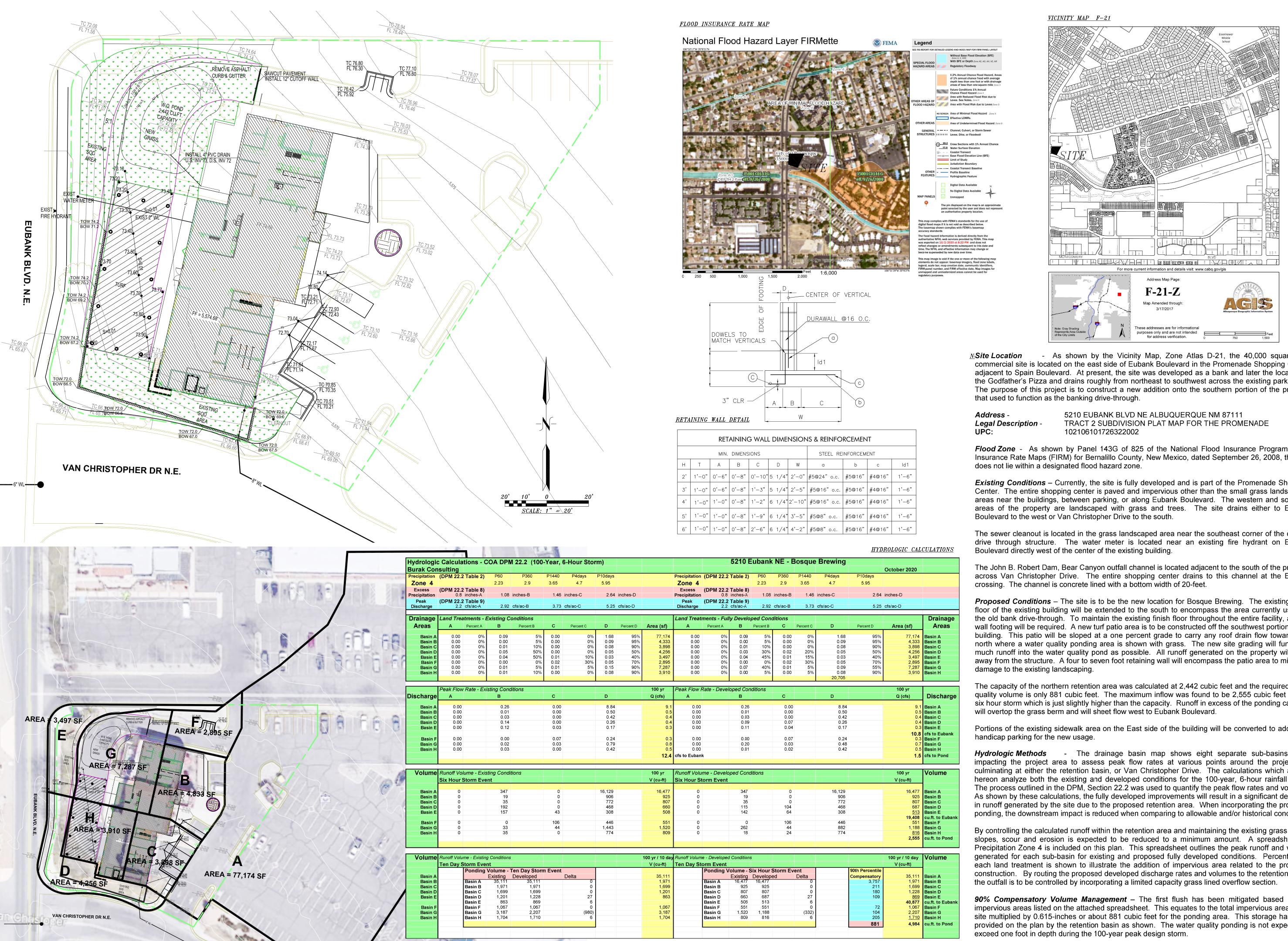
4. WINDOW MULLIONS TO BE STANDARD COLOR TO MATCH EXISTING MULLION COLOR.



SHEET TITLE ELEVATIONS

SHEET **A20** 





- As shown by the Vicinity Map, Zone Atlas D-21, the 40,000 square foot commercial site is located on the east side of Eubank Boulevard in the Promenade Shopping Center adjacent to Spain Boulevard. At present, the site was developed as a bank and later the location of the Godfather's Pizza and drains roughly from northeast to southwest across the existing parking lot. The purpose of this project is to construct a new addition onto the southern portion of the property

Flood Zone - As shown by Panel 143G of 825 of the National Flood Insurance Program Flood Insurance Rate Maps (FIRM) for Bernalillo County, New Mexico, dated September 26, 2008, this site

**Existing Conditions** – Currently, the site is fully developed and is part of the Promenade Shopping Center. The entire shopping center is paved and impervious other than the small grass landscaping areas near the buildings, between parking, or along Eubank Boulevard. The western and southern areas of the property are landscaped with grass and trees. The site drains either to Eubank

The sewer cleanout is located in the grass landscaped area near the southeast corner of the current drive through structure. The water meter is located near an existing fire hydrant on Eubank

The John B. Robert Dam, Bear Canyon outfall channel is located adjacent to the south of the property across Van Christopher Drive. The entire shopping center drains to this channel at the Eubank

Proposed Conditions - The site is to be the new location for Bosque Brewing. The existing finish floor of the existing building will be extended to the south to encompass the area currently used as the old bank drive-through. To maintain the existing finish floor throughout the entire facility, a stem wall footing will be required. A new turf patio area is to be constructed off the southwest portion of the building. This patio will be sloped at a one percent grade to carry any roof drain flow towards the north where a water quality ponding area is shown with grass. The new site grading will funnel as much runoff into the water quality pond as possible. All runoff generated on the property will drain away from the structure. A four to seven foot retaining wall will encompass the patio area to minimize

The capacity of the northern retention area was calculated at 2,442 cubic feet and the required water quality volume is only 881 cubic feet. The maximum inflow was found to be 2,555 cubic feet for the six hour storm which is just slightly higher than the capacity. Runoff in excess of the ponding capacity

Portions of the existing sidewalk area on the East side of the building will be converted to additional

**Hydrologic Methods** - The drainage basin map shows eight separate sub-basins (A-H) impacting the project area to assess peak flow rates at various points around the project site culminating at either the retention basin, or Van Christopher Drive. The calculations which appear hereon analyze both the existing and developed conditions for the 100-year, 6-hour rainfall event. The process outlined in the DPM, Section 22.2 was used to quantify the peak flow rates and volumes. As shown by these calculations, the fully developed improvements will result in a significant decrease in runoff generated by the site due to the proposed retention area. When incorporating the proposed ponding, the downstream impact is reduced when comparing to allowable and/or historical conditions.

By controlling the calculated runoff within the retention area and maintaining the existing grass on the slopes, scour and erosion is expected to be reduced to a minimum amount. A spreadsheet for Precipitation Zone 4 is included on this plan. This spreadsheet outlines the peak runoff and volume generated for each sub-basin for existing and proposed fully developed conditions. Percentage of each land treatment is shown to illustrate the addition of impervious area related to the proposed construction. By routing the proposed developed discharge rates and volumes to the retention basin,

90% Compensatory Volume Management - The first flush has been mitigated based on the impervious areas listed on the attached spreadsheet. This equates to the total impervious area of the site multiplied by 0.615-inches or about 881 cubic feet for the ponding area. This storage has been provided on the plan by the retention basin as shown. The water quality ponding is not expected to

Mark 1512 S Albuqu (505) mbura

October 5, 2020

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DRAWING NUMBER



1 OF 1