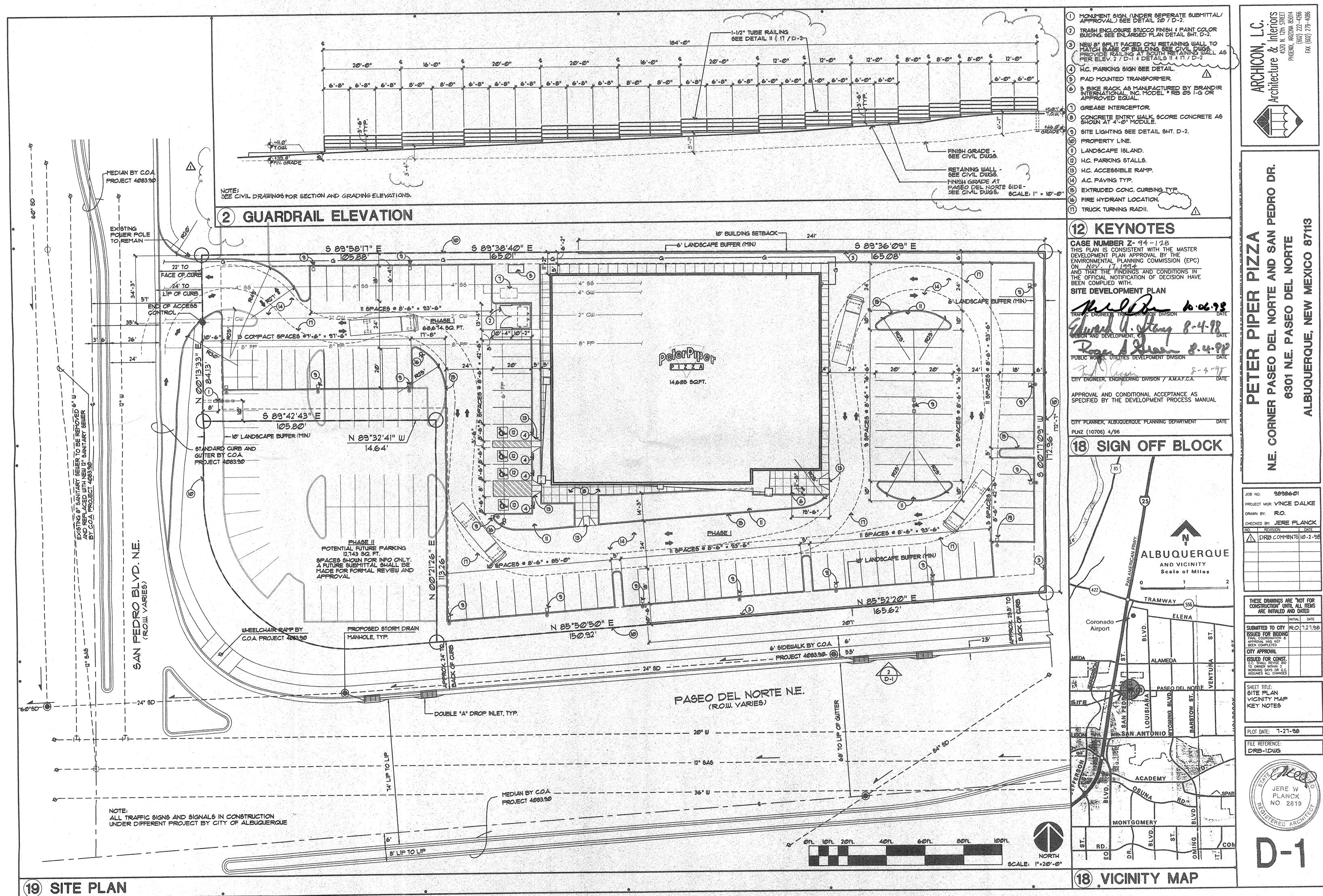
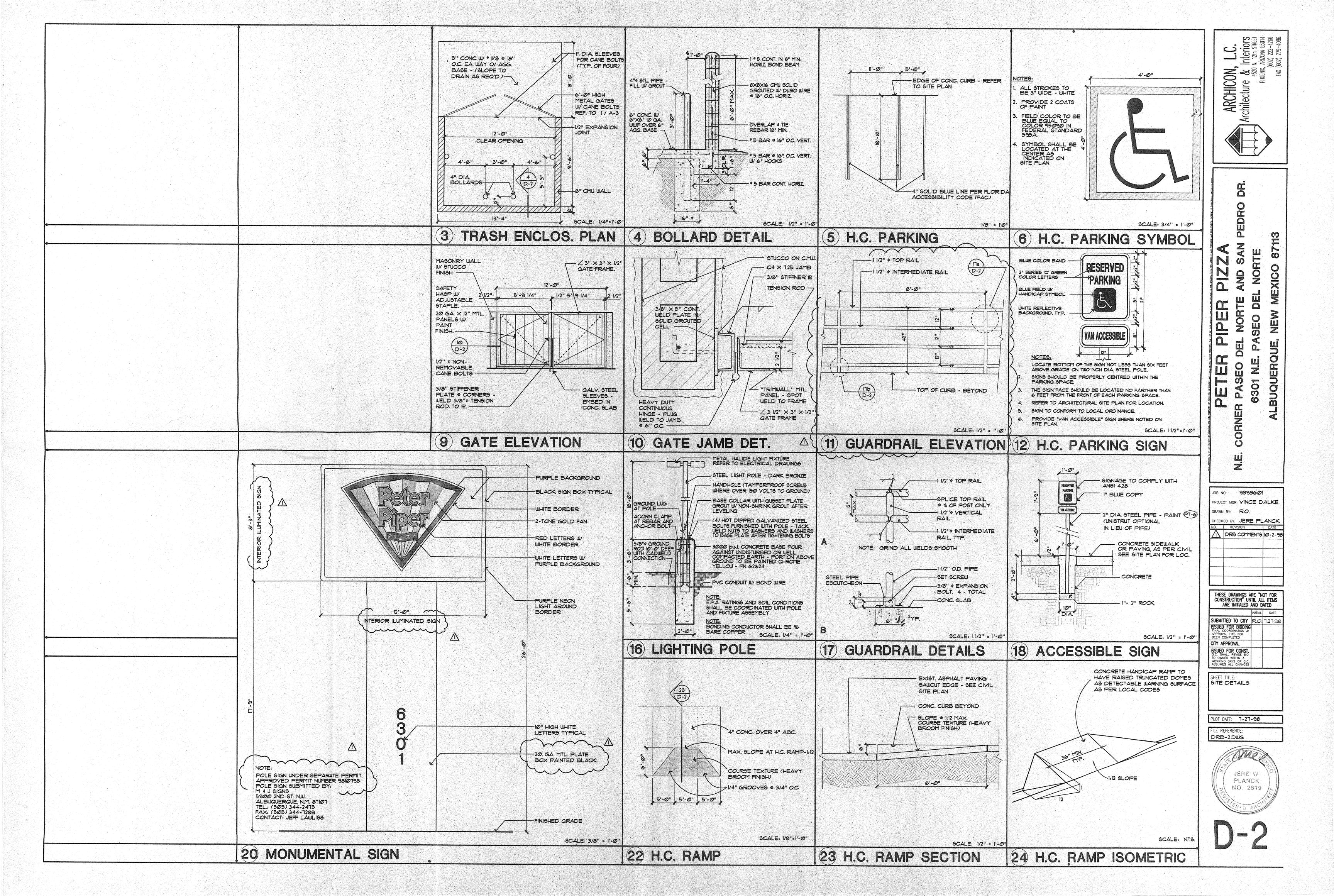
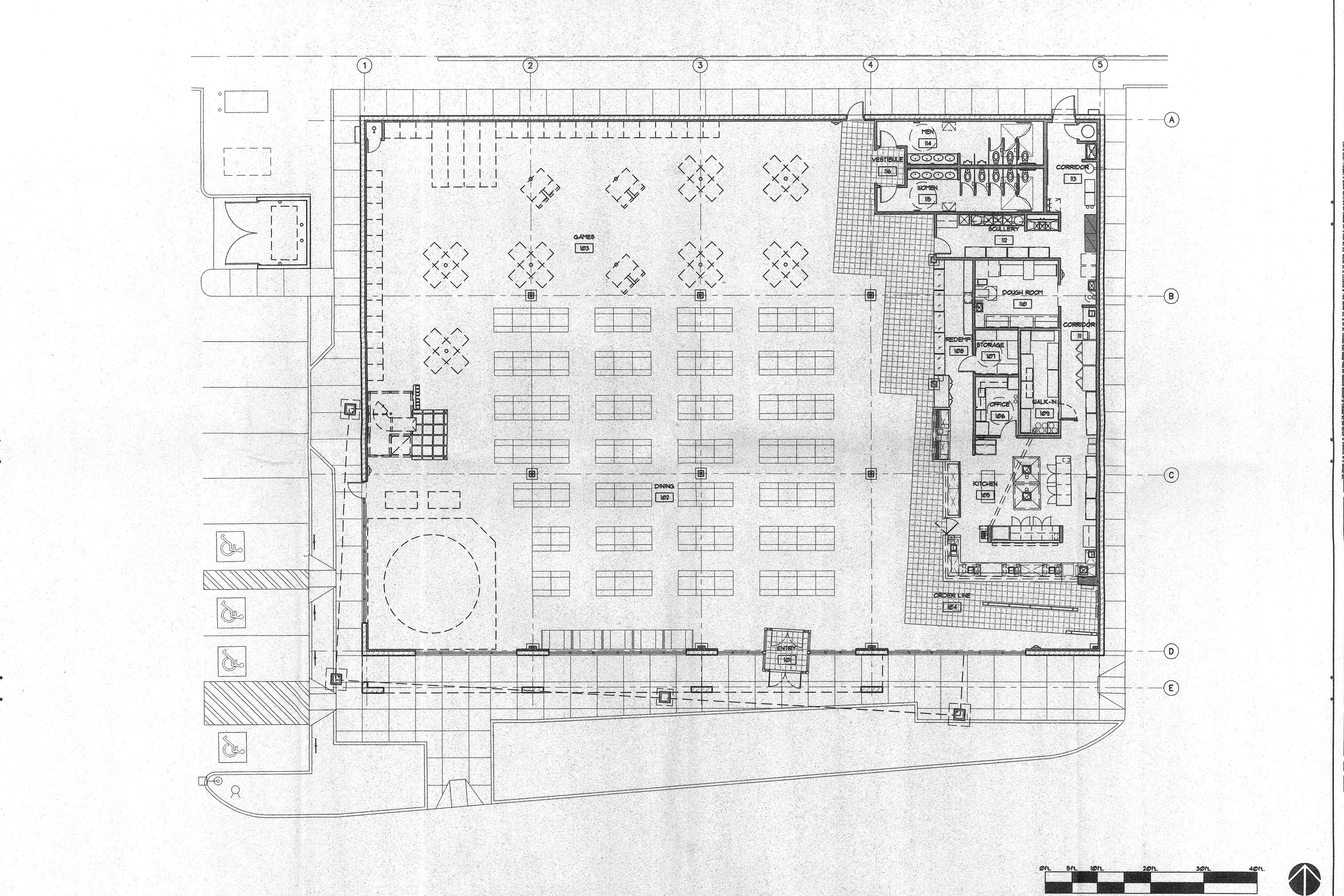
Setback Area = 14,531.75 sf / 19% of site







ARCHICON, L.C.

Architecture & Interiors

4520 N. 12th SIRET

PHOENIX, ARIZONA 85014

(602) 222-4266

JOB NO: 9898601 PROJECT MGR: VINCE DALKE DRAWN BY: R.O. CHECKED BY: JERE PLANCK

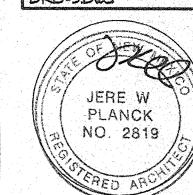
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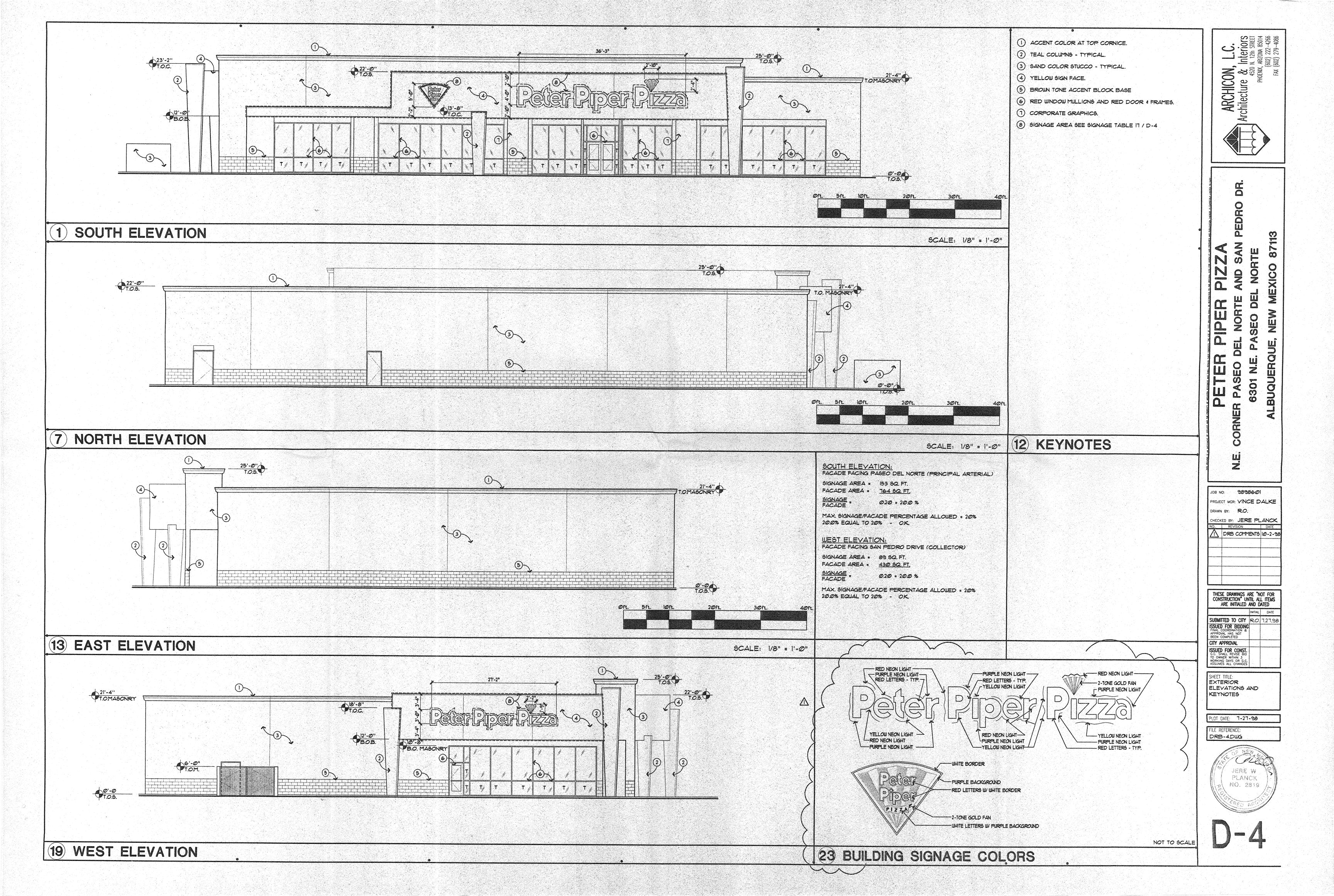
SUBMITTED TO CITY R.O. 7.27.98

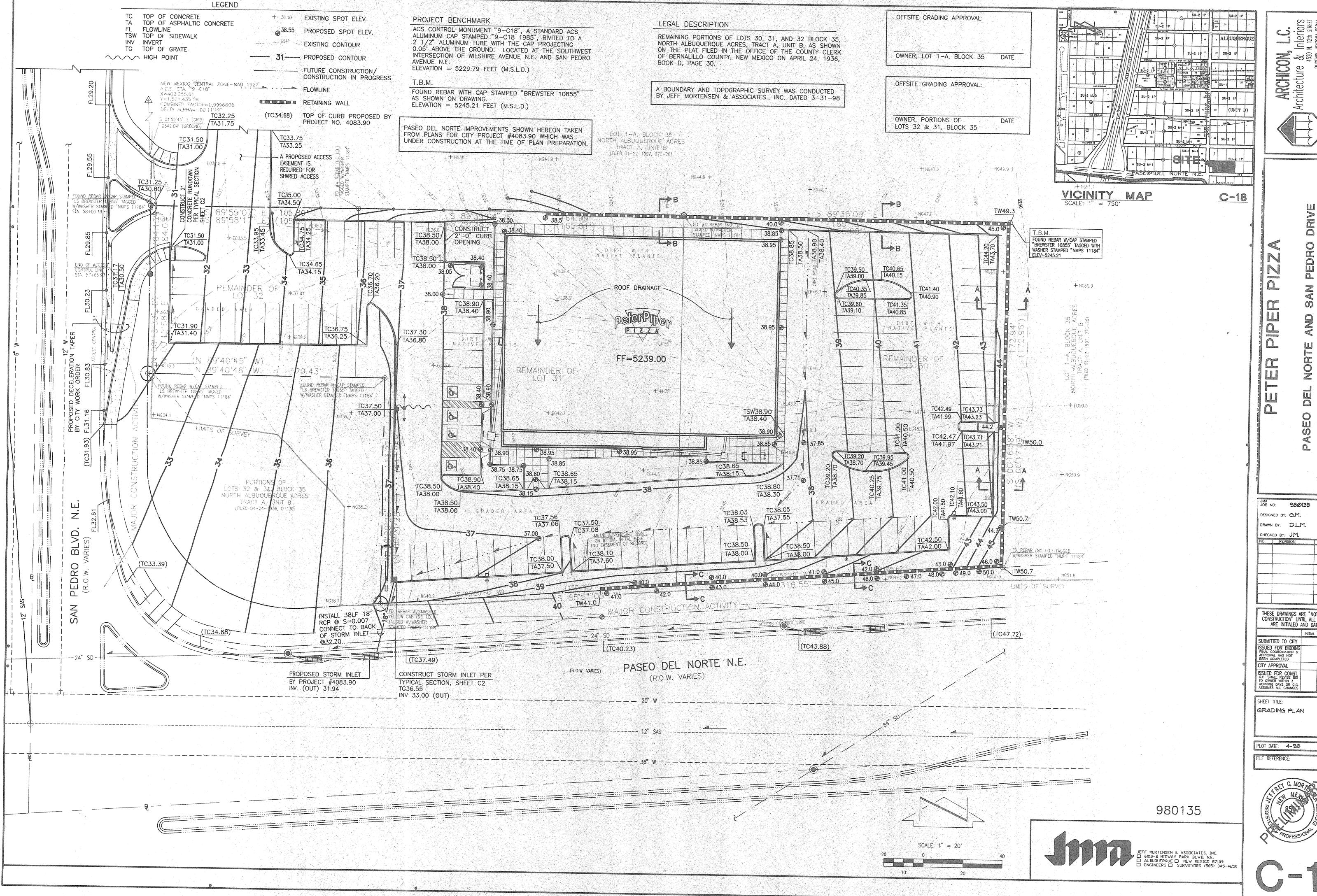
ISSUED FOR BIDDING
FINAL COORDING APPROVAL HAS NOT BEEN COMPLETED CITY APPROVAL ISSUED FOR CONST.
G.C. SHALL REVISE BID
TO OWNER WITHIN 3
WORKING DAYS OR G.C.
ASSUMES ALL CHANGES

SHEET TITLE: FLOOR PLAN

PLOT DATE: 7-27-98 FILE REFERENCE: DRB-3.DWS







ARCHICON, rchitecture & Ir

ALBUQUERQUE

THESE DRAWINGS ARE "NOT FOR CONSTRUCTION" UNTIL ALL ITEMS ARE INITIALED AND DATED

- PRIOR TO CONSTRUCTION, THE CONTRACTOR SHALL EXCAVATE AND VERIFY THE HORIZONTAL AND VERTICAL LOCATION OF ALL POTENTIAL OBSTRUCTIONS. SHOULD A CONFLICT EXIST THE CONTRACTOR SHALL NOTIFY THE ENGINEER IN WRITING SO THAT THE CONFLICT CAN BE RESOLVED WITH A MINIMUM AMOUNT OF DELAY THE CONTRACTOR SHALL BE RESPONSIBLE FOR ALL INTERPRETATIONS IT MAKES WITHOUT FIRST CONTACTING THE ENGINEER AS REQUIRED ABOVE.
- ALL WORK ON THIS PROJECT SHALL BE PERFORMED IN ACCORDANCE WITH APPLICABLE FEDERAL, STATE AND LOCAL LAWS, RULES AND REGULATIONS CONCERNING CONSTRUCTION SAFETY AND HEALTH.
- ALL CONSTRUCTION WITHIN PUBLIC RIGHT-OF-WAY SHALL BE PERFORMED IN ACCORDANCE WITH APPLICABLE CITY OF ALBUQUERQUE STANDARDS AND PROCEDURES.
- 5. IF ANY UTILITY LINES, PIPELINES, OR UNDERGROUND UTILITY LINES ARE SHOWN ON THESE DRAWINGS, THEY ARE SHOWN IN AN APPROXIMATE MANNER ONLY, AND SUCH LINES MAY EXIST WHERE NONE ARE SHOWN. IF ANY SUCH EXISTING LINES ARE SHOWN, THE LOCATION IS BASED UPON INFORMATION PROVIDED BY THE OWNER OF SAID UTILITY AND THE INFORMATION MAY BE INCOMPLETE, OR MAY BE OBSOLETE BY THE TIME CONSTRUCTION COMMENCES. THE ENGINEER HAS CONDUCTED ONLY PRELIMINARY INVESTIGATION OF THE LOCATION, DEPTH, SIZE, OR TYPE OF EXISTING UTILITY LINES, PIPELINES, OR UNDERGROUND UTILITY LINES. THIS INVESTIGATION IS NOT CONCLUSIVE, AND MAY NOT BE COMPLETE, THEREFORE, MAKES NO REPRESENTATION PERTAINING THERETO, AND ASSUMES NO RESPONSIBILITY OR LIABILITY THEREFOR. THE CONTRACTOR SHALL INFORM ITSELF OF THE LOCATION OF ANY UTILITY LINE, PIPELINE, OR UNDERGROUND UTILITY LINE IN OR NEAR THE AREA OF THE WORK IN ADVANCE OF AND DURING EXCAVATION WORK. THE CONTRACTOR IS FULLY RESPONSIBLE FOR ANY AND ALL DAMAGE CAUSED BY ITS FAILURE TO LOCATE, IDENTIFY AND PRESERVE ANY AND ALL EXISTING UTILITIES, PIPELINES, AND UNDERGROUND UTILITY LINES. IN PLANNING AND CONDUCTING EXCAVATION, THE CONTRACTOR SHALL COMPLY WITH STATE STATUTES, MUNICIPAL AND LOCAL ORDINANCES, RULES AND REGULATIONS, IE ANY, PERTAINING TO THE LOCATION OF THESE LINES AND FACILITIES.
- AN EXCAVATION/CONSTRUCTION PERMIT WILL BE REQUIRED BEFORE BEGINNING ANY WORK WITHIN CITY RIGHT-OF-WAY. AN APPROVED COPY OF THESE PLANS MUST BE SUBMITTED AT THE TIME OF APPLICATION FOR THIS PERMIT.
- BACKFILL COMPACTION SHALL BE ACCORDING TO ARTERIAL STREET USE.
- MAINTENANCE OF THESE FACILITIES SHALL BE THE RESPONSIBILITY OF THE OWNER OF THE PROPERTY SERVED.
- THE DESIGN OF PLANTERS AND LANDSCAPED AREAS IS NOT PART OF THIS PLAN. ALL PLANTERS AND LANDSCAPED AREAS ADJACENT TO THE BUILDING(S) SHALL BE PROVIDED WITH POSITIVE DRAINAGE TO AVOID ANY PONDING ADJACENT TO THE STRUCTURE. FOR CONSTRUCTION DETAILS, REFER TO LANDSCAPING PLAN.

## EROSION CONTROL MEASURES:

THE CONTRACTOR SHALL ENSURE THAT NO SOIL ERODES FROM THE SITE INTO PUBLIC RIGHT-OF-WAY OR ONTO PRIVATE PROPERTY.

THE CONTRACTOR SHALL PROMPTLY CLEAN UP

- ANY MATERIAL EXCAVATED WITHIN THE PUBLIC RIGHT-OF-WAY SO THAT THE EXCAVATED MATERIAL IS NOT SUSCEPTIBLE TO BEING WASHED DOWN THE STREET.
- THE CONTRACTOR SHALL SECURE "TOPSOIL DISTURBANCE PERMIT" PRIOR TO BEGINNING CONSTRUCTION.
- ANY AREAS OF EXCESS DISTURBANCE (TRAFFIC ACCESS, STORAGE YARD, EXCAVATED MATERIAL ETC.) SHALL BE RE-SEEDED ACCORDING TO C.O.A. SPECIFICATION 1012 "NATIVE GRASS SEEDING". THIS WILL BE CONSIDERED INCIDENTAL TO CONSTRUCTION, THEREFORE, NO SEPARATE PAYMENT WILL BE MADE.

APPROVALS	NAME	DATE	
A.C.E./DESIGN			
INSPECTOR			
A.C.E./FIELD			

### DRAINAGE PLAN

The following items concerning the Peter Piper Pizza Drainage Plan are contained hereon:

- Vicinity Map Drainage Basin Map
- Grading Plan Calculations
- Sections
- Paseo del Norte Improvements (currently under construction)

As shown by the Vicinity Map, the site is located at the northeast corner of Paseo del Norte N.E. and San Pedro Boulevard N.E. The site is currently undeveloped. Utility relocations and paving and storm drainage improvements were in progress at the time of this plan preparation. These improvements are part of the "Paseo del Norte from I-25 to 500 Feet East of Wyoming" project. City of Albuquerque Project No. 4083.90. The offsite improvements shown on the Gradina Plan are as shown in the Construction Plans for that

As shown by FIRM Panel 137 of 825 published by the National Flood Insurance Program for Bernalillo County, New Mexico, and Incorporated Areas, dated September 20, 1996, this site does not lie within a designated flood hazard

As previously stated, the site is currently undeveloped. As shown by the Drainage Basin Map, and by the Offsite Basin Calculations, offsite flows in the amount of 3.0 cfs enter the site from the east. The proposed grading will not block undeveloped offsite flows. These flows will be accepted and passed through the site in the interim condition until the site to the east is developed, upon which those flows should be directed to public right-of-

The Grading Plan shows: 1) existing and proposed grades indicated by spot elevations and contours at 1'0" intervals, 2) the limit and character of the existing improvements, 3) the limit and character of the proposed improvements, 4) the limit and character of the proposed Paseo del Norte and San Pedro Improvements as shown on the Plans for City Project No. 4083.90, 5) continuity between existing and proposed grades, and 6) continuity between proposed grades and proposed Paseo del Norte and San Pedro Improvements. As shown on the Plan, the proposed improvements consist of a new building with associated paving and landscaping improvements.

As shown by the Drainage Basin Map, the site drains from east to west to San Pedro Boulevard N.E. In the proposed condition, the site will discharge to the public storm drain being constructed by Project No. 4083.90. The majority of the site will drain to the southwest corner of the parking lot and directly into a proposed public storm inlet within Paseo del Norte. The remainder of the site will discharge to San Pedro via the private entrance where it will flow north approximately 140 feet to a proposed public storm inlet. Both of these inlets are part of the new storm drain currently being constructed. The C.L.O.M.R. for the Paseo Del Norte Storm Drains and Related Facilities prepared by R.T.I, Inc. (Case No. 97-06-141R) indicates that this storm drain is sized to accept fully developed runoff for the 100-year, 24hour storm within the Paseo del Norte Watershed. Free discharge, therefore, is justified from this site.

The Calculations which appear hereon analyze both the existing and developed conditions for the 100-year, 6-hour rainfall event. The Procedure for 40acre and Smaller Basins, as set forth in the Revision of Section 22.2. Hydrology of the Development Process Manual, Volume 2, Design Criteria, dated January, 1993, has been used to quantify the peak rate of discharge and volume of runoff generated. As shown by these calculations, the proposed improvements will increase the runoff generated by this site. This runoff will be directed to a proposed storm drain which is currently under construction. Site runoff will enter this storm drain via storm inlet connection and via surface runoff carried within San Pedro Boulevard N.E. The free discharge from this site is appropriate as the proposed storm drain has been sized for free discharge.

## CALCULATIONS

## SITE CHARACTERISTICS

- PRECIPITATION ZONE =
- 2.  $P_{6.100} = P_{360} = 2.60 \text{ IN}$
- TOTAL AREA  $(A_{\tau}) = 68,670 \text{ SF}/1.58 \text{ AC}$
- EXISTING LAND TREATMENT

-	ΑT	MEN.	Γ.	437			13	AR	EA 5,37	(SF	/AC	;)	
. 7	A.		'nij,					35	5,37	0/	8.0	1	
ġ	C							3	3,30	0/	0.7	6	
					130				- (4)				

DEVELOPED LAND TREATMENT TREATMENT

AREA (SF/AC) 13,170/0.30 55,500/1.27

#### EXISTING CONDITION VOLUME

 $E_W = (E_A^A + E_B^A + E_C^A + E_D^A) / A_T$ 

 $E_W = [(0.66)(0.81) + (1.29)(0.76)]/1.57 = 0.96 \text{ IN}.$ 

19

 $V_{100} = (E_W/12)A_T$ 

 $V_{100} = (0.96/12)(68,670) = 5,490 \text{ CF}$ 

2. PEAK DISCHARGE

 $Q_{D} = Q_{PA}A_{A} + Q_{PB}A_{B} + Q_{PC}A_{C} + Q_{PD}A_{D}$ 

 $Q_{\rm p} = Q_{100} = (1.87)(0.81) + (3.45)(0.76) = 4.1 CFS$ 

### DEVELOPED CONDITION VOLUME

 $E_W = (E_A A_A + E_B A_B + E_C A_C + E_D A_D) / A_T$ 

 $E_W = [(0.92)(0.30) + (2.36)(1.27)]/(1.57) = 2.08 \text{ IN}.$ 

 $V_{100} = (E_W/12)A_T$ 

 $V_{100} = (2.08/12)(68,670) = 11,900 CF$ 

2. PEAK DISCHARGE

 $Q_{D} = Q_{PA}^{A}_{A} + Q_{PB}^{A}_{B} + Q_{PC}^{A}_{C} + Q_{PD}^{A}_{D}$  $Q_D = Q_{100} = (2.60)(0.30) + (5.02)(1.27) = 7.2 CFS$ 

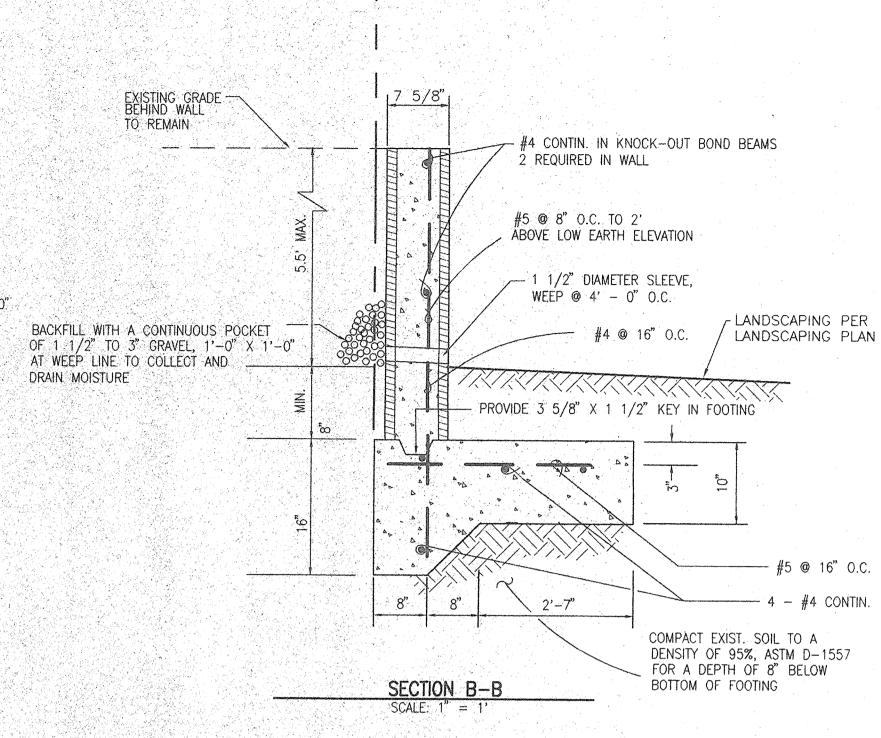
## COMPARISON

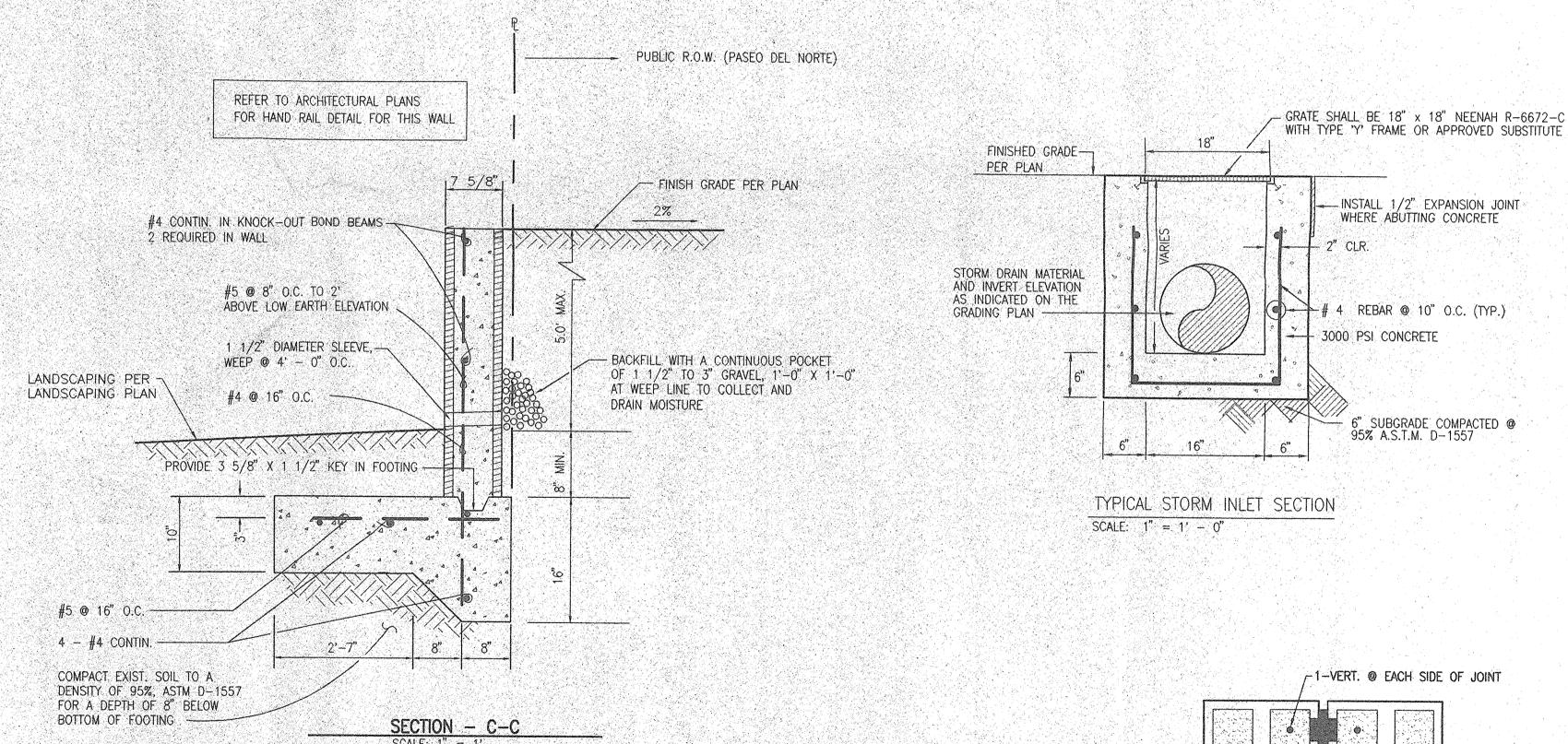
1.  $\Delta V_{100} = 11,900 - 5,490 = 6,410 CF (INCREASE)$ 

2.  $\Delta Q_{100} = 7.2 - 4.1 = 3.1$  CFS (INCREASE)

OFFSITE BASIN: 1.61 AC TREATMENT A,  $Q_{100} = (1.61)(1.87) = 3.0$  CFS

ROTATE WALL BLOCK SIDEWAYS @ 12' INTERVALS TO ACCEPT OFFSITE FLOWS EXISTING GRADE BEHIND WALL TO REMAIN TO REMAIN #4 CONTIN. IN KNOCK-OUT BOND BEAMS-2 REQUIRED IN WALL #5 @ 8" O.C. TO 2' ABOVE LOW EARTH ELEVATION 1 1/2" DIAMETER SLEEVE. -- BACKFILL WITH A CONTINUOUS POCKET WEEP ◎ 4' - 0" O.C. LANDSCAPING PER OF 1 1/2" TO 3" GRAVEL, 1'-0" X 1'-0" LANDSCAPING PLAN AT WEEP LINE TO COLLECT AND #4 @ 16" O.C. DRAIN MOISTURE DRAIN MOISTURE PROVIDE 3 5/8" X 1 1/2" KEY IN FOOTING #5 @ 16" O.C. — 4 - #4 CONTIN. -2'-7' COMPACT EXIST. SOIL TO A DENSITY OF 95%, ASTM D-1557 FOR A DEPTH OF 8" BELOW BOTTOM OF FOOTING -





# RETAINING WALL NOTES:

1. 8"X8"X16" CMU OF UBC STD, 24-4 OR 24-5.

2. USE KNOCK-OUT BOND BEAM BLOCK AT 4'-0" MAX C.C., VERTICALLY, AND 1 #4 CONTINUOUS.

3. FILL ALL BLOCK VOIDS WITH 3000 PSI CONCRETE.

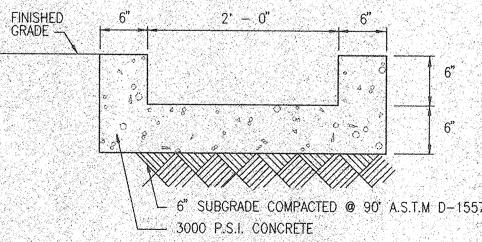
4. REINFORCING TO BE INTERMEDIATE GRADE STEEL. fs=20,000 psi

5. IN LIEU OF CONTINUOUS KNOCK-OUT BOND BEAMS, CONTRACTOR MAY INSTALL DUR-O-WALL REINFORCING EVERY SECOND COURSE.

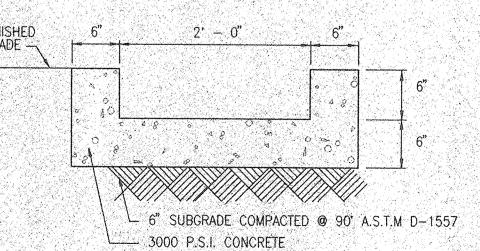
6. SPLICE SHALL BE 40 BAR DIA, MINIMUM FOR VERTICAL BARS. ALL OTHER SHALL BE 20 BAR DIA. MINIMUM.

7. CONCRETE FILL SHALL BE 21 DAYS OLD PRIOR TO BACKFILLING.

8. INSTALL MASONRY CONTROL JOINTS PER TYPICAL DETAIL AT UNIFORM SPACINGS OF 20' (MIN.) TO 24' (MAX.).



TYPICAL RUNDOWN SECTION SCALE: 1'' = 1' - 0''

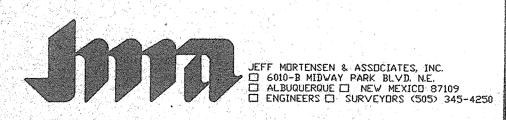


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PREFORMED CONTINUOUS

TYPICAL MASONRY CONTROL JOINT DETAIL

NEOPRENE EXPANSION JOINT



Interiors
N. 12th STREET
ARIZONA 85014
(602) 222-4266
(602) 279-4086 ARCHICON,
Architecture & I

> OZ 0

98@135 JOB NO: DESIGNED BY: GM. DRAWN BY: DLM CHECKED BY: JGM. NO. | REVISION

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SHEET TITLE: DRAINAGE PLAN CALCULATIONS, SECTIONS AND DETAILS

PLOT DATE: 4-98 FILE REFERENCE:

THESE DRAWINGS ARE "NOT FOR CONSTRUCTION" UNTIL ALL ITEMS ARE INITIALED AND DATED SUBMITTED TO CITY

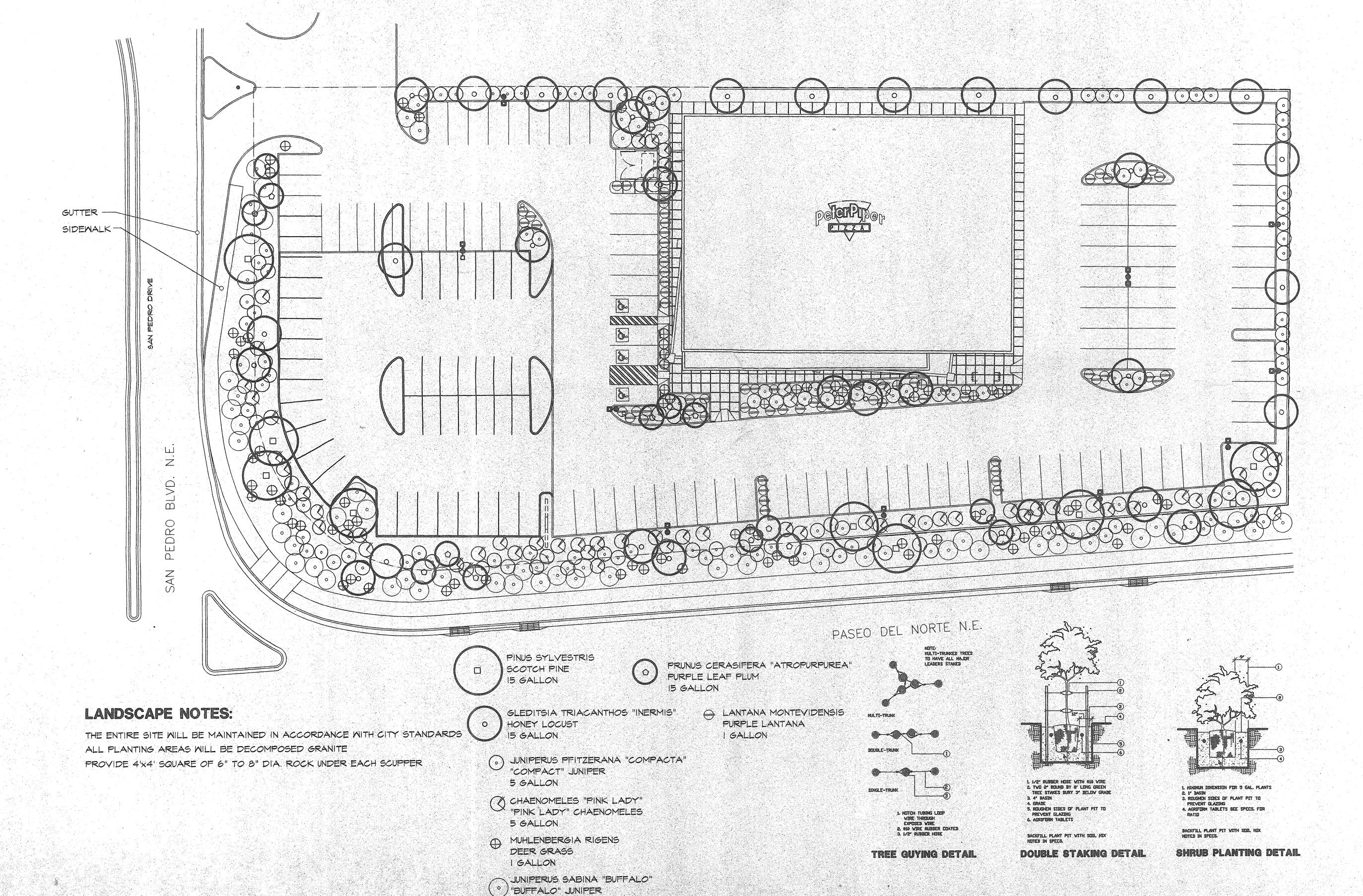
ISSUED FOR BIDDING FINAL COORDINATION & APPROVAL HAS NOT BEEN COMPLETED CITY APPROVAL

SHEET TITLE: PLANTING

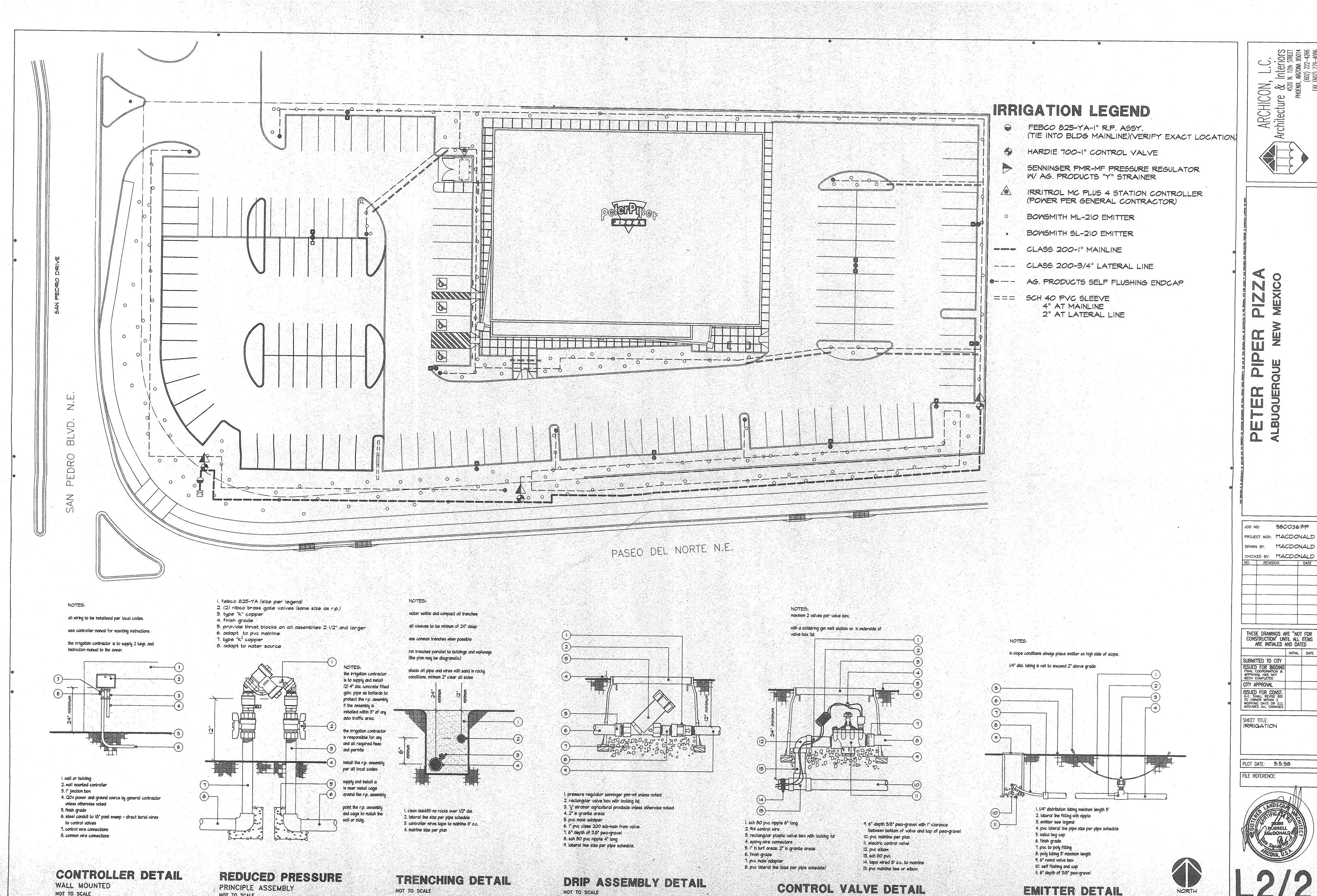
PLOT DATE: 5.5.98

FILE REFERENCE:





5 GALLON



NOT TO SCALE

SCALE: 1"=20'-0"

NOT TO SCALE