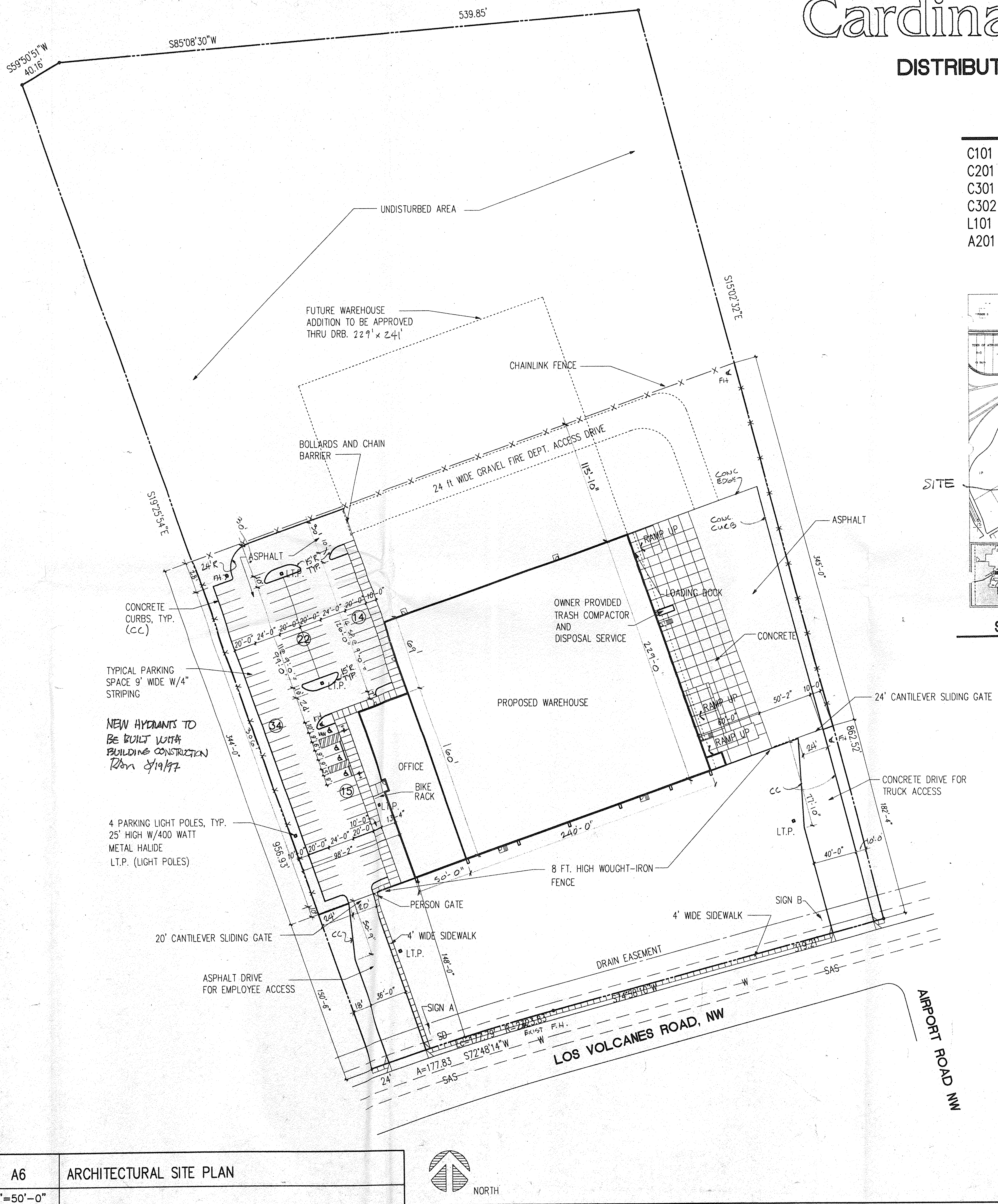
# Cardinal Health

## DISTRIBUTION FACILITY

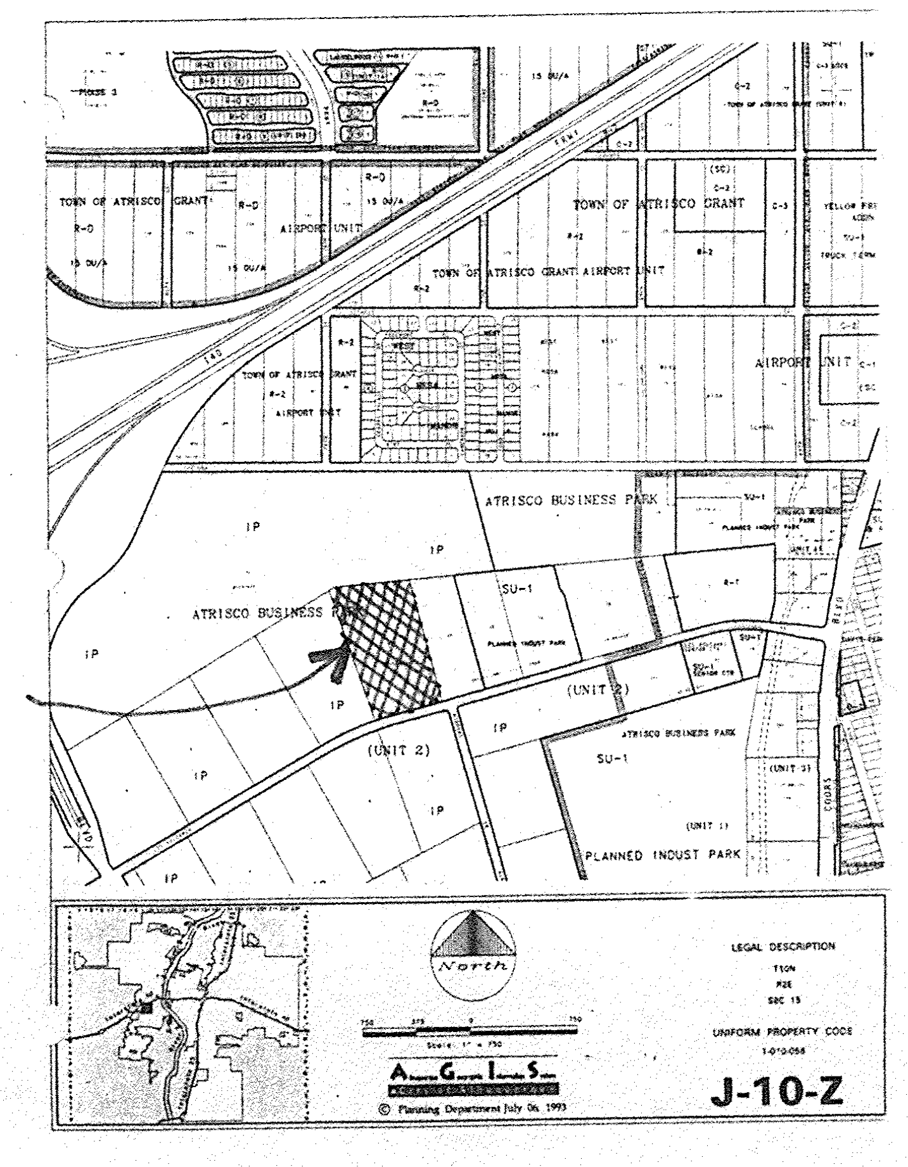


NEW HYDRANTS TO BE BUILT WITH BUILDING CONSTRUCTION Ron 8/19/97

| A6 | ARCHITECTURAL SITE PLAN |
|---|---|
| 1"=50'-0" | |

### INDEX OF DRAWINGS

| | |
|---|---|
| C101 | ARCHITECTURAL SITE PLAN |
| C201 | UTILITY SITE PLAN |
| C301 | GRADING PLAN |
| C302 | DRAINAGE CALCULATIONS |
| L101 | LANDSCAPE PLAN |
| A201 | EXTERIOR ELEVATIONS |



### SITE LOCATION MAP

### DESIGN DATA

Occupancy Group: B division 2
Construction Type: VN
Property Zone: I-P

Building Area

| | | | |
|---|---|---|---|
| Office | 8,004 S.F. | | (50' X 160') |
| Warehouse | 54,960 SF | (Ground Floor) | (240 ' x 229') |
| | 34,226 SF | (Second Floor) | |
| TOTAL | 97,190 S.F. | | |

### OCCUPANT LOAD

Office/Corridor
Ground 8,004 s.f.
TOTAL 8,004 s.f./100 = 80 occupants
Warehouse
Ground 54,960 s.f. Second
Second 34,226 s.f.
TOTAL 89,186 s.f./500 = 179 occupants

Occupant Load
TOTAL BUILDING OCCUPANCY = 259

### PARKING REQUIREMENTS

Office : 1 space per 200 s.f. net leaseable space
8000 s.f. = 40 spaces
Warehouse : 1 space per 2,000 sf. net leaseable space
89,186 s.f. = 45 spaces
Total spaces = 85 provided
Accessible parking spaces : 4 required (one to be Van parking)

Bicycle: 1 space per 20 vehicle spaces, but not less than 2.
85 spaces = 5 spaces provided

### LEGAL DESCRIPTION

LOT 2 OF TRACT S-1, UNIT 2,
ATRISCO BUSINESS PARK
ALBUQUERQUE, BERNALILLO COUNTY
NEW MEXICO

### FLOOR AREA RATIO

LOT AREA – 488,806 SF
BUILDING AREA – 97,190 SF
F.A.R.= .19

0' 25' 100' 150'
GRAPHIC SCALE: 1"=50'-0"

### DRB CASE NUMBER DRB – 97-326

SITE DEVELOPMENT PLAN APPROVAL

THE PLAN IS CONSISTENT WITH THE CONCEPTS OF THE ORIGINAL ATRISCO BUSINISS PARK MASTER DEVELOPMENT PLAN FOR IP USES APPROVED BY THE ENVIRONMENTAL PLANNING COMMISSION ON (OCT. 15, 1992) EPC CASE #Z-92-57

IT CONFORMS WITH THE COMMENTS RENDERED BY THE DEVELOPMENT REVIEW BOARD ON 7-29-97 AS REFLECTED IN DRB-97-326

N/A
PLANNING DIRECTOR DATE

Ira Gibson 8-12-97
SOLID WASTE DATE

Lynn C. Doni 8/20/97
PLANNING DEPARTMENT DATE

8-19-97
TRANSPORTATION DEVELOPMENT DATE

Robert W. Kane 8-19-97
UTILITY DEVELOPMENT DIVISION DATE

Edward R. Stang 8-19-97
PARKS & GENERAL SERVICES DATE

8-19-97
CITY ENGINEER/AMAFCA DATE

SMPC ARCHITECTS
ARCHITECTS ◦ PLANNERS ◦ INTERIOR DESIGNERS
115 AMHERST DRIVE SE., ALBUQUERQUE, NEW MEXICO 87106
TELE. (505) 255-8668 FAX (505) 268-6665

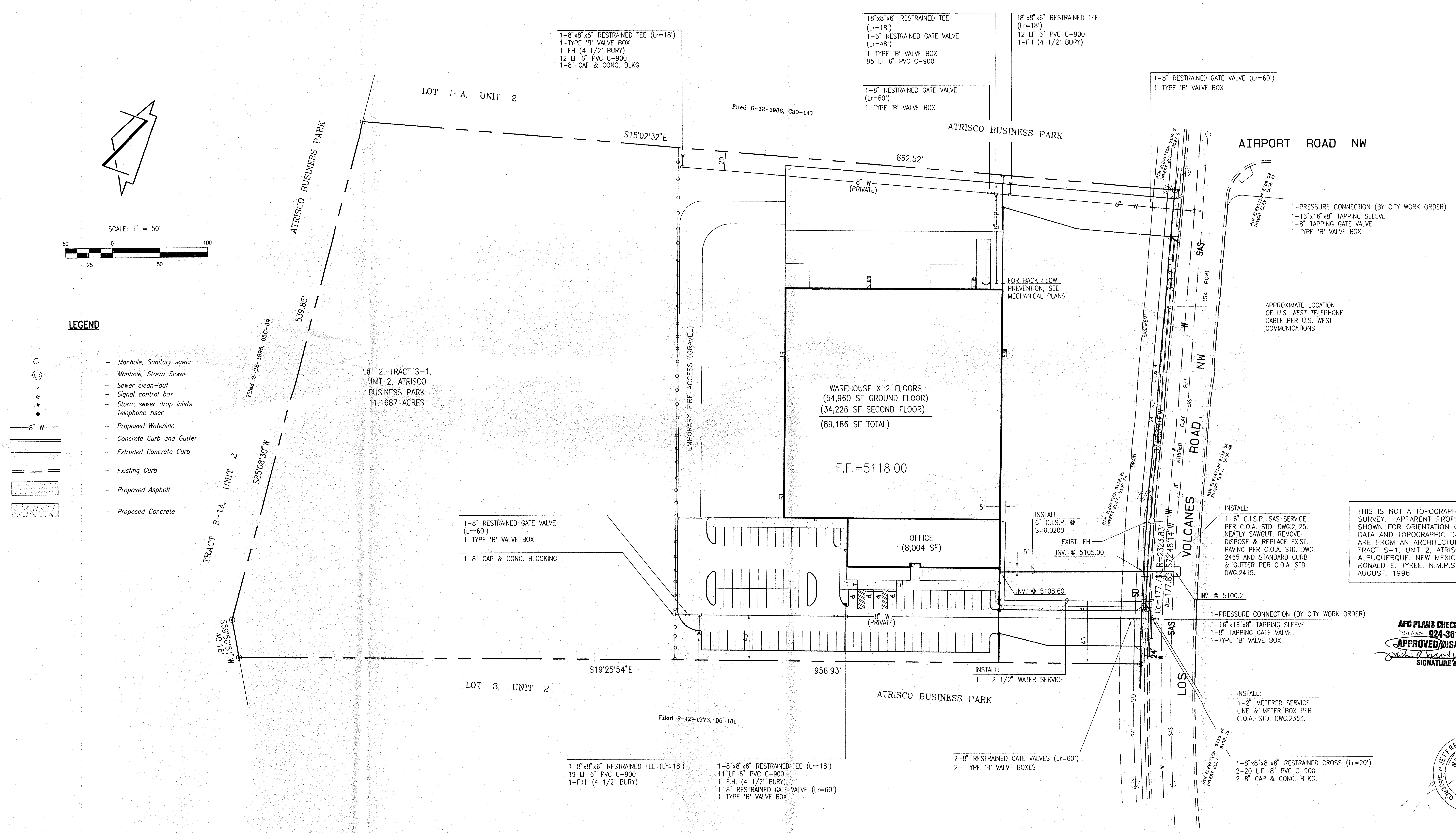
CARDINAL HEALTH
ARCHITECTURAL SITE PLAN

| REV. # | DATE | |
|---|---|---|
| 1 | 8/4/97 | PROJECT #: 97039 |
| 2 | 8/12/97 | DWN BY: SAS/PHH |
| | | CHK BY: PHH |
| | | DATE: 7/23/97 |

C101
1 OF 6

DRB-97-326

DRB-97-326



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**AFD PLANS CHECKING OFFICE**
**924-3611**
**[✓ APPROVED]/DISAPPROVED**
8-18-97
**SIGNATURE & DATE**
Hydrant

JEFFREY G. MORTENSEN 8547
08-06-97

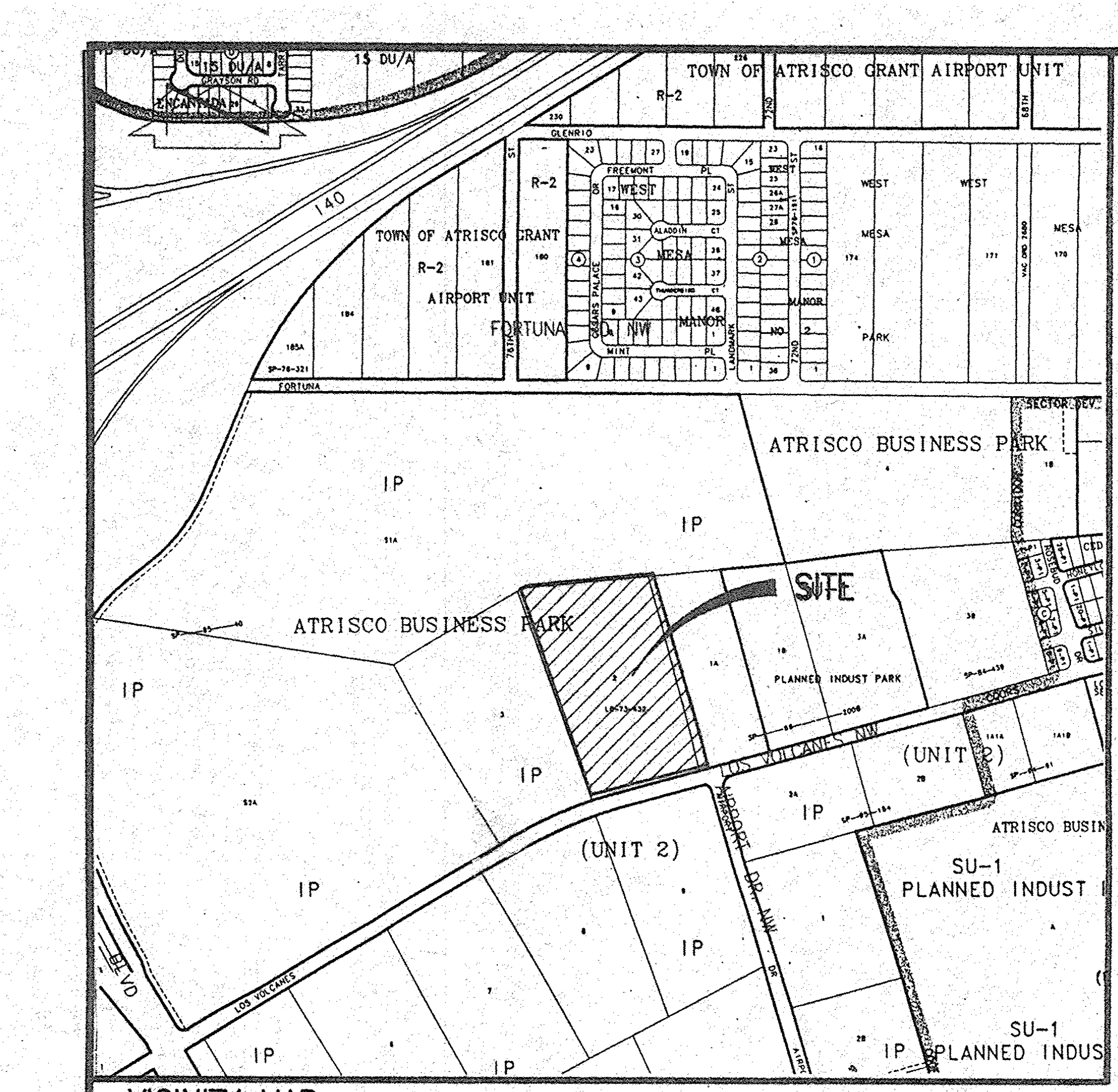
JEFF MORTENSEN & ASSOCIATES, INC.
☐ 6010-B MIDWAY PARK BLVD. N.E.
☐ ALBUQUERQUE ☐ NEW MEXICO 87109
☐ ENGINEERS ☐ SURVEYORS (505) 345-4250

# WATER AND SANITARY SEWER SITE PLAN
# CARDINAL HEALTH

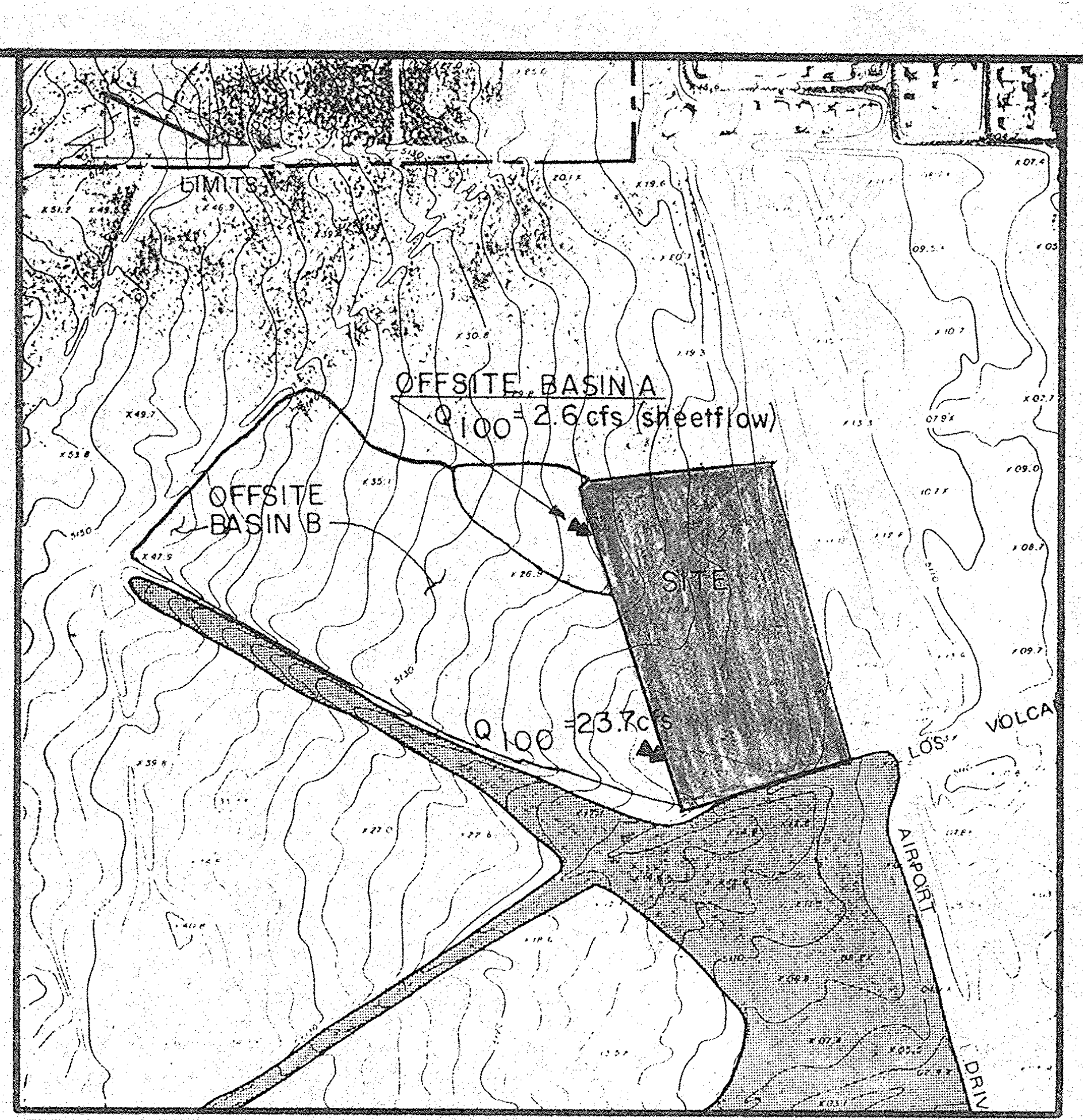
DESIGNED BY J.A.P.
DRAWN BY J.M.A.
APPROVED BY J.G.M.

| NO. | DATE | BY | REVISIONS |
|---|---|---|---|
| | | | |

JOB NO. 960923
DATE 08-97
SHEET 2 OF 6



VICINITY MAP
SCALE: 1" = 750'
J10



FIRM MAP
SCALE: 1" = 500'
PANEL 328 OF 825
PANEL 27 OF 50

## LEGAL DESCRIPTION

Lot 2 of Tract S-1, Unit 2, Atrisco Business Park
Albuquerque, Bernalillo County, New Mexico.

## PROJECT BENCHMARK:

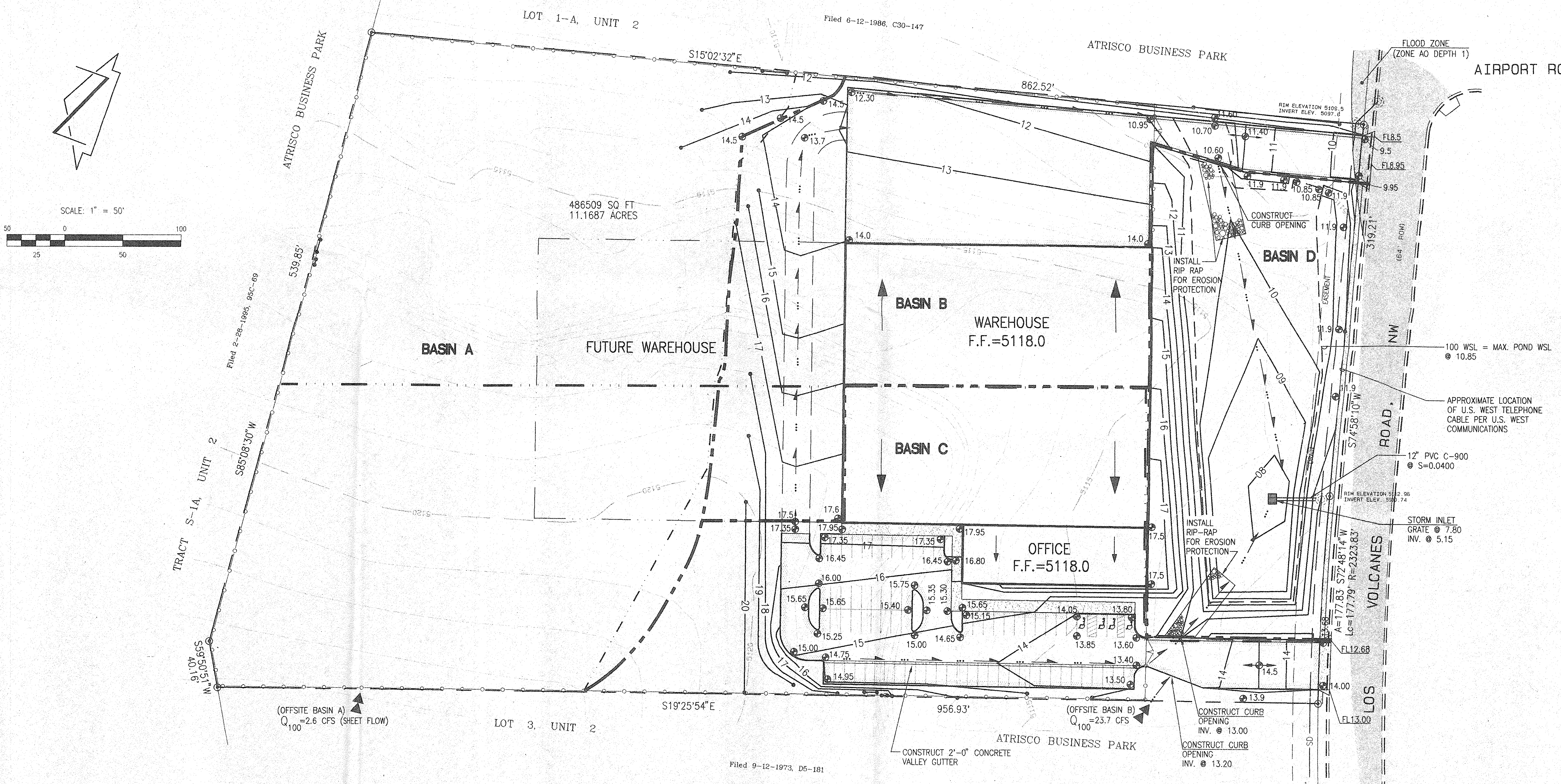
Project Bench mark is Albuquerque City Survey Station "10 - K10" having a mean sea level, 1929 datum of 5142.790.

## T.B.M.:

A temporary bench mark was established on the North bonnet bolt (marked with a chiseled " X ") of the fire hydrant located just south of the southerly property line and approximately 150 feet East of the Southwest property corner, Elevation being 5115.65.

## LEGEND

| | |
|---|---|
| ┄┄┄ | – *Existing Contour* |
| ○ | – *Manhole, Sanitary sewer* |
| ◎ | – *Manhole, Storm Sewer* |
| ∘ | – *Sewer clean-out* |
| ▫ | – *Signal control box* |
| ▤ | – *Storm sewer drop inlets* |
| ■ | – *Telephone riser* |
| ⊕17.35 | – *Proposed Spot Elevation* |
| —10— | – *Proposed Contour* |
| ··· → | – *Flowline* |
| ← | – *Proposed Roof Drainage* |
| ←+→ | – *Proposed High Point* |
| — · · · — | – *Existing Basin* |
| ━ ━ ━ | – *Proposed Basin* |
| ━ · · · ━ | – *Future Basin* |
| ▭ | – *Proposed Asphalt* |
| ▨ | – *Proposed Concrete* |
| ▩ | – *Flood Zone* |



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JEFFREY G. MORTENSEN
8547

11-01-96

C301

JEFF MORTENSEN & ASSOCIATES, INC.
☐ 6010-B MIDWAY PARK BLVD. N.E.
☐ ALBUQUERQUE ☐ NEW MEXICO 87109
☐ ENGINEERS ☐ SURVEYORS (505) 345-4250

# CONCEPTUAL GRADING PLAN
# CARDINAL HEALTH

DESIGNED BY M.F.D.
DRAWN BY T.N.T./S.G.H.
APPROVED BY J.G.M.

| NO. | DATE | BY | REVISIONS |
|---|---|---|---|
| | | | |

JOB NO. 960921
DATE 10-1996
SHEET 3 OF ~~8~~ 6

C:\DATA\ACADDWG\960921G Fri Nov 1 13:34:55 1996 -MLS

## CONCEPTUAL DRAINAGE PLAN

The following items concerning the Cardinal Health Conceptual Drainage Plan are contained hereon:

1. Vicinity Map
2. Floodplain Map
3. Conceptual Grading Plan
4. Calculations

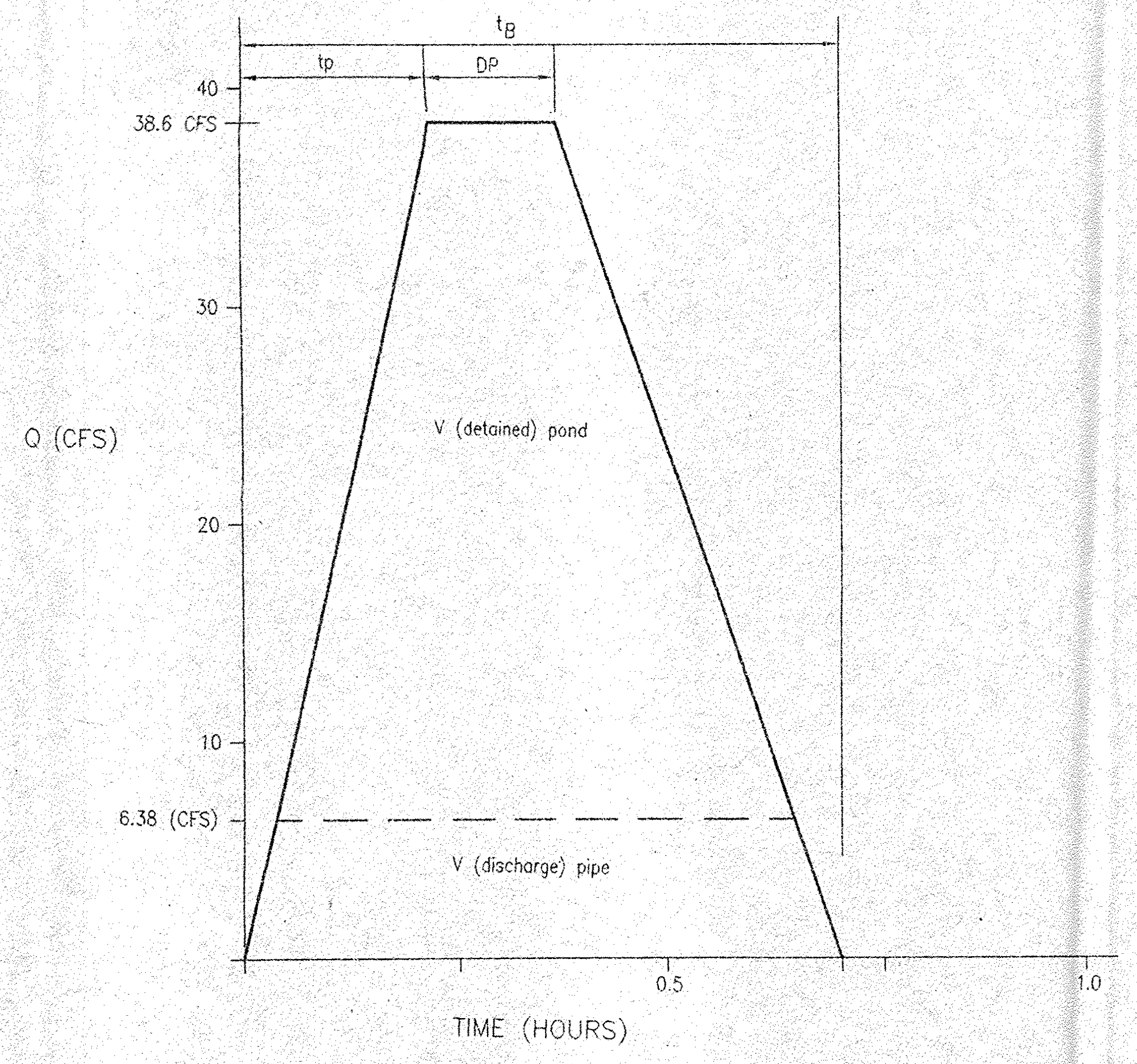
As shown by the Vicinity Map, the site is located northwest of the intersection of Los Volcanes Road N.W. and Airport Drive N.W. The present legal description is Lot 2 of Tract S-1, Unit 2, Atrisco Business Park. At present, the site is vacant.

As shown by Panel 328 of 825 of the National Flood Insurance Program Flood Insurance Rate Maps published by F.E.M.A. for the City of Albuquerque, New Mexico dated September 20, 1996, a small portion of the site, the southeast corner, lies within a designated flood hazard zone, AO depth 1. As can be noted on the F.E.M.A. map, this floodplain was defined before construction of the adjacent street with associated storm drain system, thereby placing the floodplain within the street right-of-way.

The Conceptual Grading Plan shows: 1) existing and proposed grades at 1'0" intervals, 2) proposed spot elevations, 3) the limit and character of the existing and proposed improvements, and 4) continuity between existing and proposed grades. As shown by the conceptual grading plan, the proposed construction consists of an office structure and a warehouse with associated landscaping, parking, and loading docks, with the possibility of a future warehouse which will be located north of the proposed warehouse. An onsite detention pond, with associated drop inlet, storm drain pipe, and emergency spillway, will be constructed to handle the ultimate buildout condition for the runoff generated by the site. The spillway is located at the eastern portion of the site. The spillway was designed to discharge the Q~s100~S from the the fully developed future condition basins plus the existing offsite flows. The storm drain pipe will connect to an existing storm drain manhole located at the south end of the site which discharges into the existing storm drain system.

Offsite flows enter the site from the west from two locations and will not be blocked. In the developed condition, Offsite Basin A discharges into Basin A, while Offsite Basin B discharges into Basin C. Offsite flows do not enter from the south because a fully developed street exists in that direction. Finally, offsite flows do not enter from the east and north portions of the site because they lie topographically lower than the site. Per the Atrisco Business Park Development plan, the offsite flows entering the site will be removed only when the site to the west is developed.

The Calculations which appear hereon analyze both the existing and developed conditions for the 100-year, 6-hour rainfall event. The Procedure for 40-acre and Smaller Basins, as set forth in the Revision of Section 22.2, Hydrology of the Development Process Manual, Volume 2, Design Criteria, dated January, 1993, has been used to quantify the peak rate of discharge and volume of runoff generated. As shown by these calculations, there will be an increase for total runoff for the site with a decrease in the discharge rate.



UNIT HYDROGRAPH

## CALCULATIONS

Site Characteristics

1. Precipitation Zone = 1
2. $P_{6,100} = P_{360} = 2.20$ in.
3. Total Area ($A_T$) = 11.17 acres
4. Existing Land Treatment

| | | |
|---|---|---|
| Basin A | 202,000/4.64 | 100 |
| Treatment | Area (sf/ac) | % |
| A | 202,000/4.64 | 100 |
| Basin B | 284,510/6.53 | 100 |
| A | 284,510/6.53 | 100 |

5. Developed Land Treatment

| | | |
|---|---|---|
| Basin A | 202,210/4.64 | 100 |
| Treatment | Area (sf/ac) | % |
| A | 197,660/4.53 | 97.7 |
| C | 4,550/0.11 | 2.3 |
| Basin B | 111,360/2.56 | 100 |
| B | 29,160/0.67 | 26.2 |
| C | 13,400/0.31 | 12.0 |
| D | 68,800/1.58 | 61.8 |
| Basin C | 104,750/2.40 | 100 |
| A | 15,000/0.34 | 14.3 |
| B | 10,850/0.25 | 10.4 |
| C | 4,500/0.10 | 4.3 |
| D | 74,400/1.71 | 71.0 |
| Basin D | 68,190/1.57 | 100 |
| B | 59,360/1.36 | 87.0 |
| C | 8,830/0.21 | 13.0 |

6. Future Land Treatment

| | | |
|---|---|---|
| Basin B | 210,890/4.84 | 100 |
| Treatment | Area (sf/ac) | % |
| B | 59,890/1.37 | 28.4 |
| C | 13,400/0.31 | 6.4 |
| D | 137,600/3.16 | 65.2 |
| Basin C | 207,430/4.76 | 100 |
| B | 54,130/1.24 | 26.1 |
| C | 4,500/0.11 | 2.2 |
| D | 148,800/3.41 | 71.7 |
| Basin D | 68,190/1.57 | 100 |
| B | 59,360/1.36 | 87.0 |
| C | 8,830/0.21 | 13.0 |

Existing Condition

Basin A

1. Volume

$E_W = (E_A A_A + E_B A_B + E_C A_C + E_D A_D)/A_T$

$E_W = (0.44)(4.64)/(4.64) = 0.44$ in.

$V_{100} = (E_W/12)A_T$

$V_{100} = (0.44/12)4.64 = 0.1701$ ac.ft.; 7,410 cf

2. Peak Discharge

$Q_p = Q_{PA}A_A + Q_{PB}A_B + Q_{PC}A_C + Q_{PD}A_D$

$Q_p = Q_{100} = (1.29)(4.64) = 6.0$ cfs

Basin B

1. Volume

$E_W = (E_A A_A + E_B A_B + E_C A_C + E_D A_D)/A_T$

$E_W = (0.44)(4.64)/(4.64) = 0.44$ in.

$V_{100} = (E_W/12)A_T$

$V_{100} = (0.44/12)6.53 = 0.2394$ ac.ft.; 10,430 cf

2. Peak Discharge

$Q_p = Q_{PA}A_A + Q_{PB}A_B + Q_{PC}A_C + Q_{PD}A_D$

$Q_p = Q_{100} = (1.29)(6.53) = 8.4$ cfs

Developed Condition

Basin A

1. Volume

$E_W = (E_A A_A + E_B A_B + E_C A_C + E_D A_D)/A_T$

$E_W = [(0.44)(4.53)+(0.99)(0.11)]/(4.64) = 0.45$ in.

$V_{100} = (E_W/12)A_T$

$V_{100} = (0.45/12)4.53 = 0.1740$ ac.ft.; 7,580 cf

2. Peak Discharge

$Q_p = Q_{PA}A_A + Q_{PB}A_B + Q_{PC}A_C + Q_{PD}A_D$

$Q_p = Q_{100} = (1.29)(4.53)+(2.87)(0.11) = 6.2$ cfs

Basin B

1. Volume

$E_W = (E_A A_A + E_B A_B + E_C A_C + E_D A_D)/A_T$

$E_W = [(0.67)(0.67)+(0.99)(0.31)+(1.97)(1.58)]/(2.56) = 1.51$ in.

$V_{100} = (E_W/12)A_T$

$V_{100} = (1.51/12)(2.56) = 0.3224$ ac.ft.; 14,040 cf

2. Peak Discharge

$Q_p = Q_{PA}A_A + Q_{PB}A_B + Q_{PC}A_C + Q_{PD}A_D$

$Q_p = Q_{100} = (2.03)(0.67)+(2.87)(0.31)+(4.37)(1.58) = 9.2$ cfs

Basin C

1. Volume

$E_W = (E_A A_A + E_B A_B + E_C A_C + E_D A_D)/A_T$

$E_W = [(0.44)(0.34)+(0.67)(0.25)$

$+(0.99)(0.10)+(1.97)(1.71)]/(2.40) = 1.58$ in.

$V_{100} = (E_W/12)A_T$

$V_{100} = (1.58/12)(2.40) = 0.3160$ ac.ft.; 13,770 cf

2. Peak Discharge

$Q_p = Q_{PA}A_A + Q_{PB}A_B + Q_{PC}A_C + Q_{PD}A_D$

$Q_p = Q_{100} = (1.29)(0.34)+(2.03)(0.25)$

$+(2.87)(0.10)+(4.37)(1.71) = 8.7$ cfs

Basin D

1. Volume

$E_W = (E_A A_A + E_B A_B + E_C A_C + E_D A_D)/A_T$

$E_W = [(0.67)(1.36)+(0.99)(0.21)]/(1.57) = 0.71$ in.

$V_{100} = (E_W/12)A_T$

$V_{100} = (0.71/12)(1.57) = 0.0932$ ac.ft.; 4,060 cf

2. Peak Discharge

$Q_p = Q_{PA}A_A + Q_{PB}A_B + Q_{PC}A_C + Q_{PD}A_D$

$Q_p = Q_{100} = (2.03)(1.36)+(2.87)(0.21) = 3.4$ cfs

Future Condition

Basin B

1. Volume

$E_W = (E_A A_A + E_B A_B + E_C A_C + E_D A_D)/A_T$

$E_W = [(0.67)(1.37)+(0.99)(0.31)+(1.97)(3.16)]/(4.84) = 1.54$ in.

$V_{100} = (E_W/12)A_T$

$V_{100} = (1.54/12)(4.84) = 0.6208$ ac.ft.; 27,040 cf

2. Peak Discharge

$Q_p = Q_{PA}A_A + Q_{PB}A_B + Q_{PC}A_C + Q_{PD}A_D$

$Q_p = Q_{100} = (2.03)(1.37)+(2.87)(0.31)+(4.37)(3.16) = 17.5$ cfs

Basin C

1. Volume

$E_W = (E_A A_A + E_B A_B + E_C A_C + E_D A_D)/A_T$

$E_W = [(0.67)(1.24)+(0.99)(0.11)$

$+(1.97)(3.41)]/(4.76) = 1.61$ in.

$V_{100} = (E_W/12)A_T$

$V_{100} = (1.61/12)(4.76) = 0.6381$ ac.ft.; 27,800 cf

2. Peak Discharge

$Q_p = Q_{PA}A_A + Q_{PB}A_B + Q_{PC}A_C + Q_{PD}A_D$

$Q_p = Q_{100} = (2.03)(1.24)+(2.87)(0.11)$

$+(4.37)(3.41) = 17.7$ cfs

Basin D

1. Volume
   No Change

2. Peak Discharge
   No Change

Orifice Equation (12" PVC)

$Q = CA(gh)^{1/2}$

$C = 0.6$

$A = \Pi(0.5)^{1/2}$

$g = 32.2$

$h = 10.85 - 5.15 = 5.7'$

$Q = 0.6(0.7854)[(32.2)(5.7)]^{1/2}$

$Q = 6.38$ cfs

Weir Equation (Emergency Spillway)

$Q = CLH^{3/2}$

$C = 2.6$
$L = 48'$
$H = 0.65'$

$Q = 65.4 \text{ cfs} > Q_{100}$ (Future) + $Q_{100}$ (Offsite)

Pond Volume Calculations

| Elevation (ft) | Area (sf) | Volume (cf) | Σ Volume (cf) |
|---|---|---|---|
| 7.8 | 0 | | |
| | | 100 | 100 |
| 8.0 | 1,000 | | |
| | | 6,512.5 | 6,612.5 |
| 9.0 | 12,025 | | |
| | | 16,617.5 | 23,230.0 |
| 10.0 | 21,210 | | |
| | | 25,814.5 | 49,044.5 |
| 10.85 | 39,530 | | |

Unit Hydrograph

$t_B = (2.017 * E * A_T/Q_p) - (0.25 * A_D/A_T)$

$E = [(0.67)(3.97)+(0.99)(0.63)+(1.97)(6.57)]/(11.17) = 1.45$ in.

$A_T = 11.17$ ac.

$Q_p = 17.5 + 17.7 + 3.4 = 38.6$ cfs

$A_D = 6.57$ ac.

$t_B = 0.6993$ hrs $= 41.96$ min.

$t_p = (0.7 * t_c) + \{[(1.6 - A_D/A_T)]/12\}$

$= 0.2243$ hrs $= 13.46$ min

$D_p =$ peak $= (0.25 * A_D/A_T)$

$= (0.25 * 6.57/11.17)$

$= 0.1470$ hrs $= 8.82$ min.

$V_{discharge} = [(0.5)(6.38)(0.2243)/38.6 + (0.5)(6.38)(0.3280)/38.6 +$

$0.6993 - (6.38)/38.6(0.2243 + 0.3280)]3600(6.38)$

$= 10,236$ cf

$V_{hydrograph} = [(0.5)(38.6)[(0.2243) + 0.3280] + 38.6(0.1470)](3600)$

$= 58,800$ cf

$V_{hydrograph} > V_{100}$

Use $V_{hydrograph}$

$V_{pond} = 49,044.5$ cf

$V_{discharge} = 10,236$ cf

$V_{capacity} = V_{pond} + V_{discharge} = 49,044.5 + 10,236 = 59,280.5$ cf

$V_{capacity} > V_{hydrograph}$

$V_{detained} = 58,800 - 10,236 = 48,564$ cf

Volume @ 10.85 = 49,044.5 cf = $V_{detained}$

$V_{100}$ WSL @ 10.85

Comparison

(Existing vs. Proposed)
1. $\Delta V_{100} = 7580+14040+13770+4060-7410-10430 = 21610$ cf (increase)
2. $\Delta Q_{100} = 8.4 + 6.0 - 6.38 = 8.02$ cfs (decrease)

(Proposed vs. Future)
1. $\Delta V_{100} = 27040+27800+4060-7580-14040-13770-4060 = 19450$ cf (increase)
2. $\Delta Q_{100} = 6.38 - 6.38 = 0$ cfs (no change)

### OFFSITE CALCULATIONS

Existing Land Treatment

| | | |
|---|---|---|
| Offsite Basin A | 88,190/2.02 | 100 |
| Treatment | Area (sf/ac) | % |
| A | 88,190/2.02 | 100 |
| Offsite Basin B | 799,650/18.36 | 100 |
| Treatment | Area (sf/ac) | % |
| A | 799,650/18.36 | 100 |

Existing Condition

Offsite Basin A

1. Volume

$E_W = (E_A A_A + E_B A_B + E_C A_C + E_D A_D)/A_T$

$E_W = (0.44)(2.02)/(2.02) = 0.44$ in.

$V_{100} = (E_W/12)A_T$

$V_{100} = (0.44/12)2.02 = 0.0741$ ac.ft.; 3,230 cf

2. Peak Discharge

$Q_p = Q_{PA}A_A + Q_{PB}A_B + Q_{PC}A_C + Q_{PD}A_D$

$Q_p = Q_{100} = (1.29)(2.02) = 2.6$ cfs

Offsite Basin B

1. Volume

$E_W = (E_A A_A + E_B A_B + E_C A_C + E_D A_D)/A_T$

$E_W = (0.44)(18.36)/(18.36) = 0.44$ in.

$V_{100} = (E_W/12)A_T$

$V_{100} = (0.44/12)18.36 = 0.6732$ ac.ft.; 29,320 cf

2. Peak Discharge

$Q_p = Q_{PA}A_A + Q_{PB}A_B + Q_{PC}A_C + Q_{PD}A_D$

$Q_p = Q_{100} = (1.29)(18.36) = 23.7$ cfs

JEFFREY G. MORTENSEN
8547

11-01-96

C302

JEFF MORTENSEN & ASSOCIATES, INC.
☐ 6010-B MIDWAY PARK BLVD. N.E.
☐ ALBUQUERQUE ☐ NEW MEXICO 87109
☐ ENGINEERS ☐ SURVEYORS (505) 345-4250

# CONCEPTUAL DRAINAGE PLAN AND CALCULATIONS
# CARDINAL HEALTH

DESIGNED BY M.F.D.
DRAWN BY S.G.H.
APPROVED BY J.G.M.

| NO. | DATE | BY | REVISIONS |
|---|---|---|---|
| | | | |
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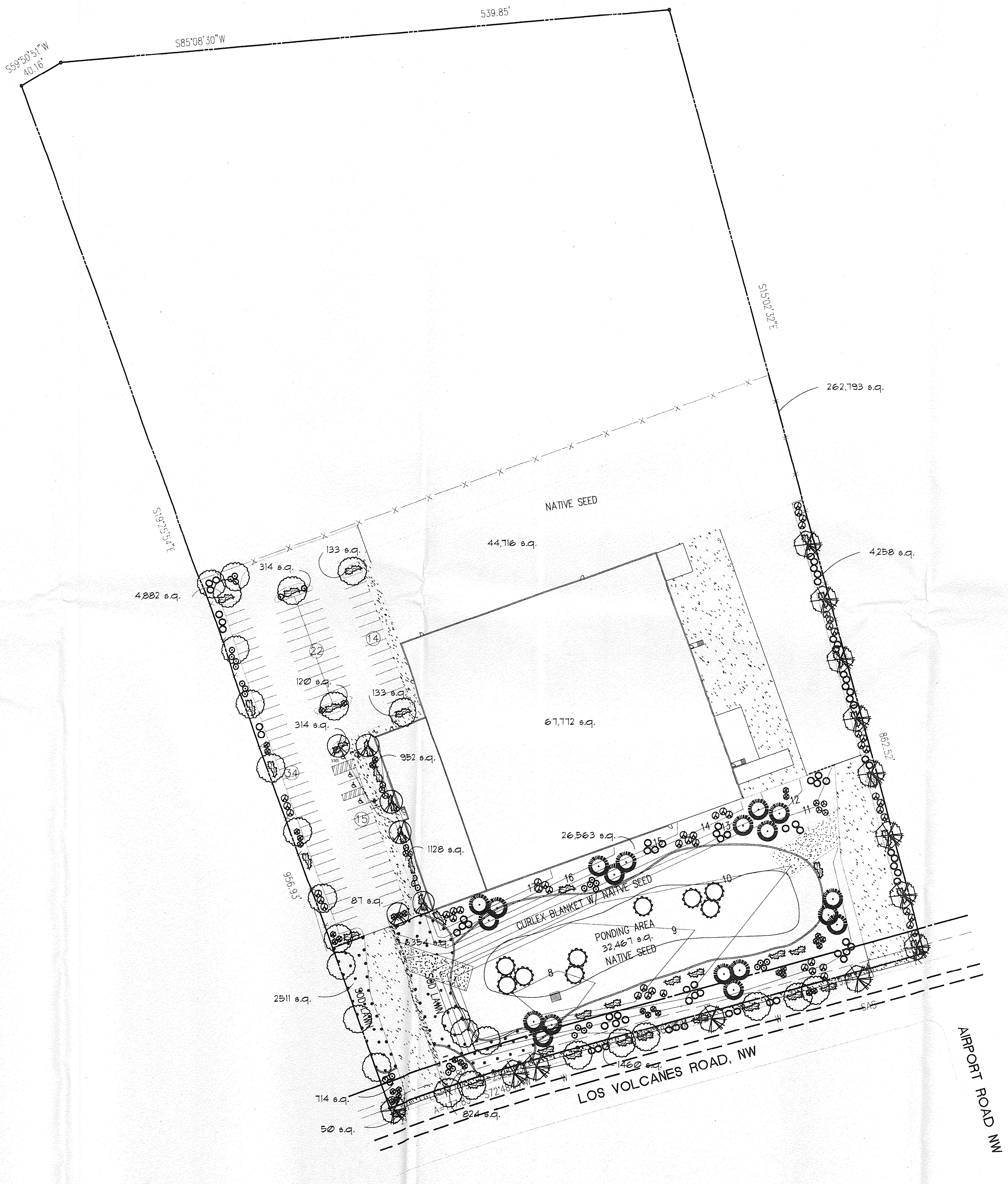
JOB NO. 960921
DATE 10-1996
SHEET 4 OF 6

C:\DATA\ACADDWG\960921C Fri Nov 1 13:21:01 1996 -MLS



ASH(H)
Fraxinus pennsylvanica
2 1/2 - 3"-Cal

HONEY LOCUST(H)
Gleditsia triacanthos
2 1/2 - 3"-Cal

FLOWERING PEAR(H)
Pyrus calleryana
20 GAL

DESERT WILLOW(L)
Fraxinus velutina
5 GAL.

AUSTRIAN PINE(M)
Pinus nigra
6-8

YUCCA PALM(L)
FAXONIANA

RUSSIAN SAGE(M)
5 GAL PEROVSKIA ATRIPLICIFOLIA

APACHE PLUME(L)
5 GAL Fallugia paradoxa

CHAMISA(L)
5 GAL Chrysothamnus nauseosus

AUTUMN SAGE(M)
5 GAL Salvia greggii

COREOPSIS(M)
5 GAL Coreopsis verticillata

5-Gal BUFFALO JUNIPER(M)
5 GAL JUNIPERUS SABINA(M)

OVERSIZED GRAVEL
& BOULDERS(8)

▭ SOD

▭ GRAVEL W/FF

∿ COMMERCIAL GRADE STEEL EDGING

## LANDSCAPE NOTES:

All landscaping shall be watered by a complete underground irrigation system operated by automatic timer. Pop Up spray heads to sod lawn. Bubblers to trees and drip irrigation to shrub areas. Irrigation system maintenance shall be the responsibility of the Property Owner.

Landscape maintenance shall be the responsibility of the Property Owner.

It is the intent of this plan to comply with the City Of Albuquerque, water conservation Landscaping and waste water ordinance, planting restriction approach.

Approval of this plan does not constitute or imply exemption from water waste provisions of the water conservation landscaping and water waste ordinance. Water management is the sole responsibility of the Property Owner.

**IRRIGATION NOTES:**
Trees to receive .25GPM Bubblers
Shrubs to receive 1.0 GPH Drip Emitters
Drip and Bubbler systems to be tied to 1/2" polypipe with flush caps at each end

Run time per each drip valve will be approximately 15 minutes per. week , to be adjusted according to the season.

Point of connection for irrigation system is unknown at current time and will be coordinated in the field.

Irrigation will be operated by automatic controller. Location of controller to be field determined and power source for controller to be provided by others.

NOTE:
All disturbed areas to be re-seeded.

DANNY D. MITCHELL
239
8-4-97

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08/04/97

## LANDSCAPE CALCULATIONS

NET LANDSCAPE AREA

| | | |
|---|---|---|
| TOTAL LOT AREA | 262,793 | square feet |
| TOTAL BUILDINGS AREA | 67,772 | square feet |
| OFFSITE AREA | 1,510 | square feet |
| NET LOT AREA | 193,510 | square feet |
| LANDSCAPE REQUIREMENT | .15 | square feet |
| TOTAL LANDSCAPE REQUIREMENT | 29,027 | square feet |
| TOTAL LANDSCAPE PROVIDED | 128,991 | square feet |
| TOTAL BED PROVIDED | 49,663 | square feet |
| TOTAL SEED LAWN PROVIDED | 2,145 | square feet |
| TOTAL NATIVE SEED PROVIDED | 77,183 | square feet |

ARCHITECTS • PLANNERS • INTERIOR DESIGNERS
115 AMHERST DRIVE SE., ALBUQUERQUE, NEW MEXICO 87106
TELE. (505) 255-8668 FAX (505) 268-6665
SMPC
ARCHITECTS

CARDINAL HEALTH

LANDSCAPE PLAN

| REV. # | DATE | |
|---|---|---|
| | | PROJECT #: 97039 |
| | | DWN BY: |
| | | CHK BY: |
| | | DATE: 08/04/97 |

L101

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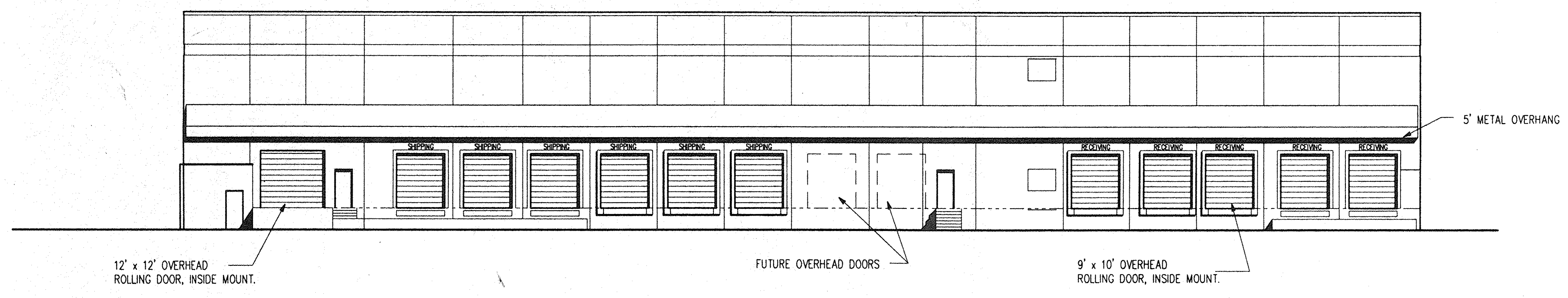
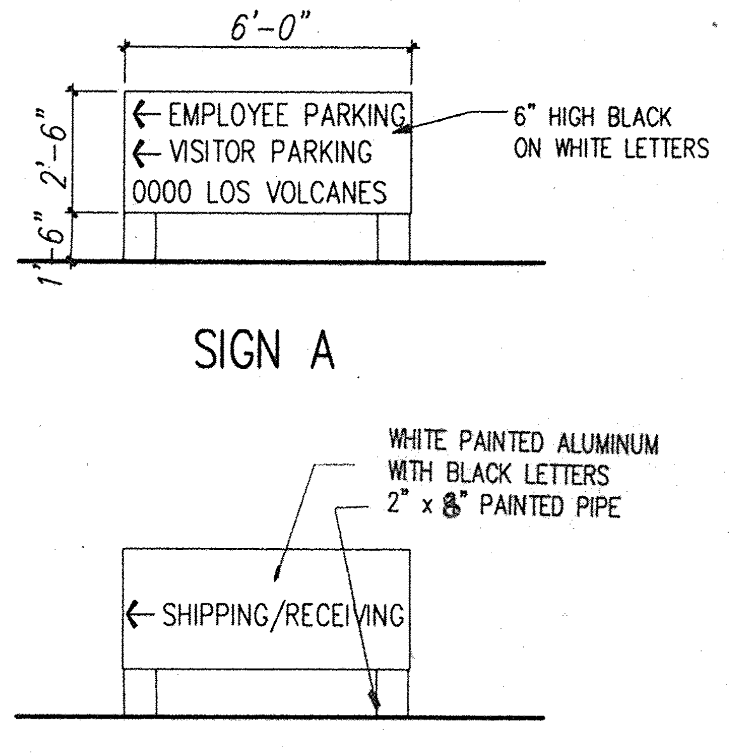
LANDSCAPE PLAN
1"=50'-0"



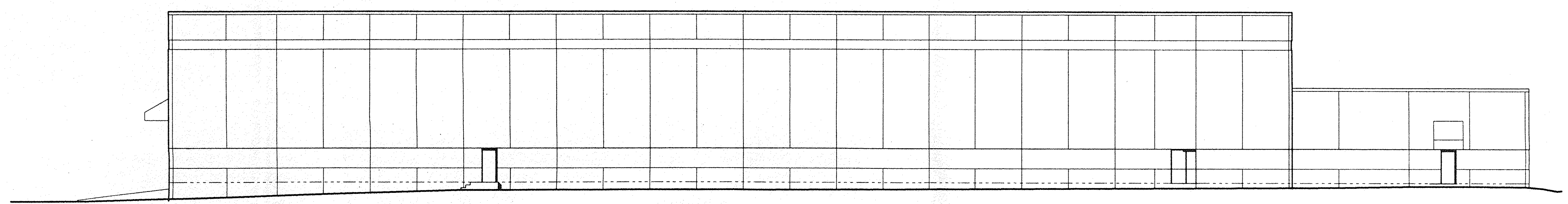
| L1 | EAST ELEVATION |
|---|---|
| 1/6"=1'-0" | C101 |

## DIRECTIONAL SIGNAGE

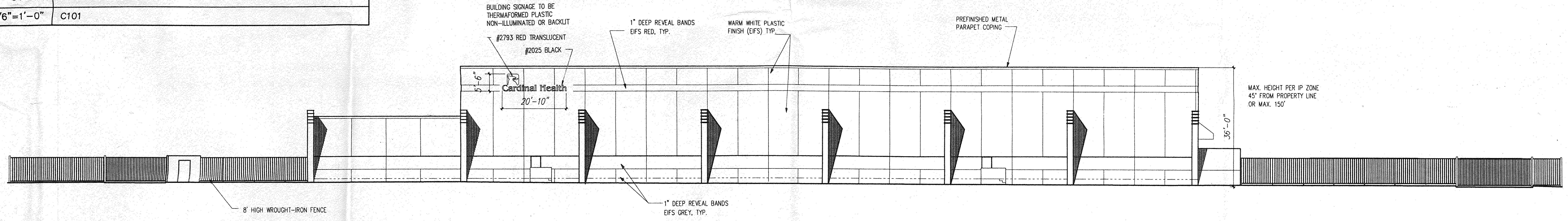


SIGN A

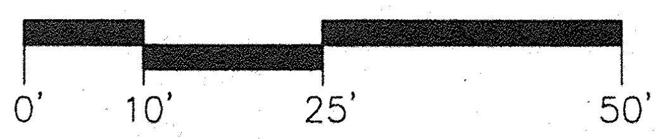
SIGN B



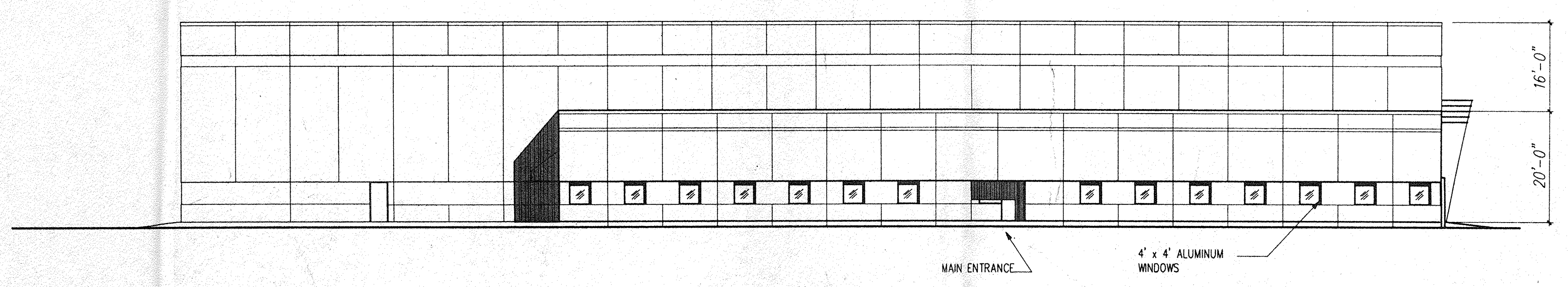
| G1 | NORTH ELEVATION |
|---|---|
| 1/6"=1'-0" | C101 |



| D1 | SOUTH ELEVATION |
|---|---|
| 1/6"=1'-0" | C101 |



0' 10' 25' 50'
GRAPHIC SCALE: 1/16"=1'-0"



| A1 | WEST ELEVATION |
|---|---|
| 1/6"=1'-0" | C101 |

NAME: Q:\97039\201.DWG DATE: AUG 04, 1997 TIME: 8:32 AM PLOT SCALE: 1" = 16'-0"

ARCHITECTS ∘ PLANNERS ∘ INTERIOR DESIGNERS
115 AMHERST DRIVE SE., ALBUQUERQUE, NEW MEXICO 87106
TELE. (505) 255-8668 FAX (505) 268-6665

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CARDINAL HEALTH

# EXTERIOR ELEVATIONS

| REV. # | DATE |
|---|---|
| 1 | 8/4/97 |
| 2 | 8/12/97 |

PROJECT #: 97039
DWN BY: PHH
CHK BY:
DATE: 7/23/97

**A201**
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