CONCEPTUAL DRAINAGE BASINS	OVERALL CALCULATIONS
BASIN NO. A DESCRIPTION Area of basin flows = 103990 SF = 2.4 Ac.	CALCULATIONS: MILAGRO :
Area of basin flows = 103990 SF = 2.4 Ac. The following calculations are based on Treatment areas as shown in table to the right LAND TREATMENT	Based on Drainage Design Criteria for City of Albuquerque Section 22.2, DPM, Vol 2, dated Jan., 1993
Sub-basin Weighted Excess Precipitation (see formula above) A = 0%	ON-SITE AREA OF SITE: 377532 SF = 8.67 AC
Weighted E = 1.74 in. B= 10%	100-year, 6-hour
Sub-basin Volume of Runoff (see formula above) C = 10% V360 = 15096 CF D = 80%	PREVIOUS DEVELOPED FLOWS: DEVELOPED FLOWS: EXCESS PRECIP:
Sub-basin Peak Discharge Rate: (see formula above) FIRST FLUSH VOL.	Treatment SF % Precip. Zone 1
$Q_P = 9.5 \text{ cfs}$ 2357 CF	Area A = 0 0% Area A = 0 0% E _A = 0.44
BASIN NO. B DESCRIPTION Area of basin flows = 175039 SF = 4.0 Ac.	Area B = 377532 100% Area B = 33978 9% E _B = 0.67
The following calculations are based on Treatment areas as shown in table to the right LAND TREATMENT	Area C = 0 0% Area C = 33978 9% E _C = 0.99
Sub-basin Weighted Excess Precipitation (see formula above) A = 0%	Area D = 0 0% Area D = 309576 82% E _D = 1.97 Total Area = 377532 100% Total Area = 377532 100%
Weighted E = 1.86 in. B= 5% Sub-basin Volume of Runoff (see formula above) C= 5%	Total Area - 37/332 100% Total Area - 37/332 100%
V ₅₆₀ = 27073 CF D= 90%	On-Site Weighted Excess Precipitation (100-Year, 6-Hour Storm)
Sub-basin Peak Discharge Rate: (see formula above) FIRST FLUSH VOL.	Weighted E = $\underline{E}_A A_A + \underline{E}_B A_B + \underline{E}_C A_C + \underline{E}_D A_D$
QP = 16.8 cfs 4463 CF	$A_A + A_B + A_C + A_D$
BASIN NO. C DESCRIPTION Area of basin flows = 98503 SF = 2.3 Ac.	Historic E = 0.67 in. Developed E = 1.76 in.
The following calculations are based on Treatment areas as shown in table to the right LAND TREATMENT	O. C. W.L. CD. C. W.Z.
Sub-basin Weighted Excess Precipitation (see formula above) A = 0%	On-Site Volume of Runoff: V360 = E*A / 12 Historic V360 = 21079 CF Developed V360 = 55522 CF
Weighted E = 1.64 in. B= 14% Sub-bas in Volume of Runoff (see formula above) C= 15%	Historic V_{360} = 21079 CF Developed V_{360} = 55522 CF
V ₃₆₀ = 13470 CF D= 71%	On-Site Peak Discharge Rate: Qp = QpAAA+QpBAB+QpCAC+QpDAD / 43,560
Sub-basin Peak Discharge Rate: (see formula above) FIRST FLUSH VOL.	For Precipitation Zone 1
CONCEPTUAL DRAINAGE BASINS BASIN W 19.5 CFS SAME STATE OF THE STATE	$Q_{pA} = 1.29$ $Q_{pC} = 2.87$
	$Q_{pB} = 2.03$ $Q_{pD} = 4.37$
	Historic Q_p = 17.6 CFS Developed Q_p = 34.9 CFS
	CONCEPTUAL DRAINAGE BASINS
	DISCHARGE LOCATIONS AND FINAL GRADES. THIS PROPERTY WAS PREVIOUSLY APPROVED (CURTIS A. CHERNE P.E. LETTER DATED JANUARY 25, 2010) FOR PLATTING AND SITE PLAN ACTION BY THE DRB. THE SAD 222 PROJECT PUBLIC STORM DRAIN FACILITIES INCLUDED A MONETARY ASSESSMENT AGAINST THIS PROPERTY (PAID). FREE DISCHARGE OF DEVELOPED CONDITION STORM WATER IS PROVIDED. SURFACE DISCHARGE TO THE ADJACENT STREETS AS WELL AS DIRECT STORM DRAIN CONNECTIONS TO PUBLIC STORM DRAIN SYSTEM WILL BE DESIGNED AT EACH PHASE AS NEEDED. FINAL DESIGN FOR PUBLIC STORM DRAIN IMPROVEMENTS WITHIN SUNSET GARDENS R/W WILL BE PROVIDED AS PART OF THE PUBLIC WORK ORDER SUBMITTAL. 18" DIA. STUBS FROM THE PROPERTY LINE TO THE BACK OF THE IDENTIFIED PUBLIC STORM DRAIN IMPROVEMENTS WITHIN THE PUBLIC R/W WILL BE CONSTRUCTED AS PART OF THE PUBLIC WORK ORDER CONSTRUCTION. THESE STUBS WILL BE CAPPED AT THE PROPERTY LINE FOR FUTURE PRIVATE CONNECTION.
BASIN 'B' 4.02 AC. 16.8 CFS	

1-TYPE A & 1-TYPE C INLET

OFFSITE UNDEVELOPED

23.7 CFS

STORMWATER QUALITY

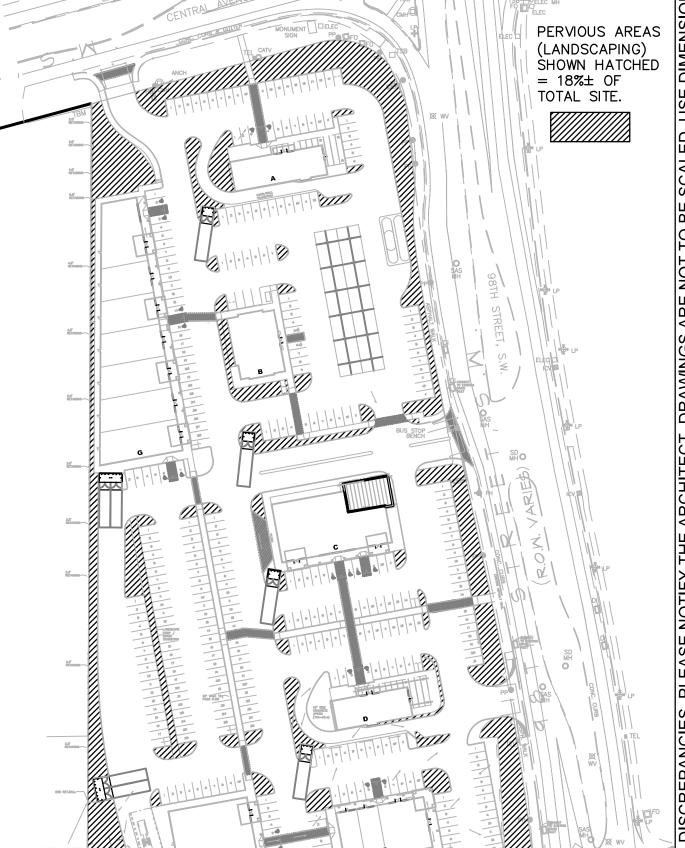
FOR NEW CONSTRUCTION, THE CABQ STORMWATER QUALITY (S.Q.) VOLUME IS BASED ON THE 90TH PERCENTILE STORM EVENT OR 0.34" [0.44" LESS 0.1" FOR INITIAL ABSTRACTION] OF STORMWATER WHICH DISCHARGES DIRECTLY TO A PUBLIC STORM DRAINAGE SYSTEM).

THE ESTIMATED IMPERVIOUS AREA FOR THIS PROPERTY IS CALCULATED AS 82% OF TOTAL AREA: (0.82 * 8.67 AC) = 309,576 SF. THE TOTAL REQUIRED S.Q. RETENTION VOLUME = 0.34" * TYPE 'D' AREA: 0.34/12 * (309,576 SF) = 8,772 CF. THIS MAY VARY DEPENDING ON THE FINAL IMPERVIOUS AREA TO BE CONSTRUCTED WITH EACH PHASE.

S.Q. RETENTION PONDS WILL BE CONSTRUCTED THROUGHOUT THE PROPERTY AS THE SITE DEVELOPS. FINAL LOCATIONS AND DESIGN WILL BE PROVIDED AS PART OF THE INDIVIDUAL BUILDING PERMIT CONSTRUCTION DOCUMENTS. AREAS ARE SHOWN (LABELED WITH *) TO INDICATE POTENTIAL S.Q. POND LOCATIONS. PRIVATE FACILITY DRAINAGE COVENANT(S) WILL BE PROVIDED FOR THE S.Q. PONDS IF REQUIRED.

PRIOR TO EACH BUILDING PERMIT APPROVAL, A "PAYMENT IN-LIEU FOR S.Q. VOLUME REQUIREMENT" TREASURY DEPOSIT SLIP WILL BE PROVIDED BY C.O.A. HYDROLOGY BASED ON THE PORTION OF S.Q. VOLUME (@ \$8.00 PER CF) THAT IS NOT RETAINED ON-SITE. A COPY OF α THE PAID RECEIPT WILL BE REQUIRED PRIOR TO RECEIVING HYDROLOGY APPROVAL FOR BUILDING PERMIT.

PERVIOUS AREA EXHIBIT







PETERSON PROPERTIES



VERIFIED BY: **REVISIONS**

O DRAWN BY:

CHECKED BY:

CONCEPTUAL GRADING + DRAINAGE NOTES AND CALCULATIONS

ISAACSON & ARFMAN, P.A. Consulting Engineering Associates 128 Monroe Street N.E. Albuquerque, New Mexico 87108

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