

FORM P3: ADMINISTRATIVE DECISIONS AND MINOR AMENDMENTS

A single PDF file of the complete application including all plans and documents being submitted must be emailed to PLNDRS@cabq.gov prior to making a submittal. Zipped files or those over 9 MB cannot be delivered via email, in which case the PDF must be provided on a CD.

INFORMATION REQUIRED FOR ALL ADMINISTRATIVE DECISIONS OR AMENDMENTS

- Letter of authorization from the property owner if application is submitted by an agent
- Zone Atlas map with the entire site clearly outlined and labeled

ARCHEOLOGICAL CERTIFICATE

- Archaeological Compliance Documentation Form with property information section completed
- Only the information above is required unless the City Archaeologist determines that the application does not qualify for a Certificate of No Effect, in which case a treatment plan prepared by a qualified archaeologist that adequately mitigates any archeological impacts of the proposed development must be submitted and reviewed for a Certificate of Approval per the criteria in IDO Section 14-16-6-5(A)(3)(b)

MINOR AMENDMENT TO SITE PLAN – ADMIN, EPC, or DRB

- Justification letter describing, explaining, and justifying the request per the criteria in IDO Section 14-16-6-4(X)(2)
- Three (3) copies of all applicable sheets of the approved Site Plan being amended, folded
- Copy of the Official Notice of Decision associated with the prior approval
- Three (3) copies of the proposed Site Plan, with changes circled and noted
Refer to the Site Plan Checklist for information needed on the proposed Site Plan.

Minor Amendments must be within the thresholds established in IDO TABLE 6-4-5. Any amendment beyond these thresholds is considered a Major Amendment and must be processed through the original decision-making body for the request.



MINOR AMENDMENT TO SITE DEVELOPMENT PLAN APPROVED PRIOR TO THE EFFECTIVE DATE OF THE IDO

- Justification letter describing, explaining, and justifying the request per the criteria in IDO Section 14-16-6-4(Y)(1)(a)
- Three (3) copies of all applicable sheets of the approved Site Development Plan being amended, folded
- Copy of the Official Notice of Decision associated with the prior approval
- Three (3) copies of the proposed Site Development Plan, with changes circled and noted
Refer to the Site Plan Checklist for information needed on the proposed Site Plan.

Minor Amendments must be within the thresholds established in IDO TABLE 6-4-5. Any amendment beyond these thresholds is considered a Major Amendment and must be processed through the original decision-making body for the request.

ALTERNATIVE SIGNAGE PLAN

- Proposed Alternative Signage Plan compliant with IDO Section 14-16-5-12(F)(5)
- Justification letter describing, explaining, and justifying the request per the criteria in IDO Section 14-16-6-5(F)(4)(c)
- Required notices with content per IDO Section 14-16-6-4(K)(6)
- Office of Neighborhood Coordination notice inquiry response and proof of emailed notice to affected Neighborhood Association representatives
- Sign Posting Agreement

<i>I, the applicant or agent, acknowledge that if any required information is not submitted with this application, the application will not be scheduled for a public meeting or hearing, if required, or otherwise processed until it is complete.</i>		
Signature:	Date: 8/4/2020	
Printed Name: Pam Kearney, As Agent	<input type="checkbox"/> Applicant or <input checked="" type="checkbox"/> Agent	
FOR OFFICIAL USE ONLY		
Project Number:	Case Numbers	
	-	
	-	
	-	
Staff Signature:		
Date:		

Bernalillo County Assessor Parcels:

UPC 101706148301640115
Tax Year 2020
Owner MONTGOMERY PLAZA
PARTNERS LLC
Legal Description TR D-1-B PLAT OF TRS D-1-A &
D-1-B MONTGOMERY PLAZA
(BEING A REPL OF TR D-1
MONTGOMERY PLAZA) CONT
1.0171 AC
Complete Owner Address 100 SUN AVE NE SUITE 100
ALBUQUERQUE NM 87109
Complete Site Address 5009 MONTGOMERY BLVD NE
ALBUQUERQUE NM 87109



IDO Zone Info

IDO Zone District	MX-M
IDO District Definition	Moderate Intensity
IDO Category	Mixed-Use
IDO Use Table URL	More info
Last Update Date	7/28/2017
Old Zoning Designation	SU-1
Old Zoning Description	C-2 WITH WAREHOUSING

Platted Parcels (AGIS)

Full Address	5001 MONTGOMERY BLVD NE
Lot	D1A
Block	0000
Subdivision	MONTGOMERY PLAZA
PIN	ABQ61202

Platted Parcels (AGIS)

Full Address	5009 MONTGOMERY BLVD NE
Lot	D1B
Block	0000
Subdivision	MONTGOMERY PLAZA
PIN	ABQ213744



September 27, 2019
Letter of Authorization

Chick-Fil-A Inc.
5200 Buffington Rd.
Atlanta, GA 30349

To Whom It May Concern,

Chick-fil-A, Inc. authorizes Scout Services and their representatives, specifically Pam Kearney, to represent the company in matters associated with the permitting of Drive Thru Canopies at its various locations around the country

Sincerely,

M. Troy Tripp
Sr. Principal Project Lead, Existing Restaurants

State of Georgia
County of DeKalb

Signed and sworn to (or affirmed) before me on October 7, 2019 (date) by
M. Troy Tripp (name(s) of individual(s) making statement), who proved to
me on the basis of satisfactory evidence to be the person(s) who appeared before me.

Personally Known
 Produced Identification
Type of ID _____

Seal/Stamp



Signature of Notary Public

Sydney H Guest

(Notary name, typed, stamped or
printed)
Notary Public, State of Georgia

My commission expires: 8/8/2023

From: [Rodenbeck, Jay B.](#)
To: [Pam Kearney](#)
Cc: [Trujillo, Concetta M.](#)
Subject: RE: Help Needed with Administrative Amendment BP-2020-25270
Date: Wednesday, July 22, 2020 6:47:50 PM
Attachments: [image003.png](#)
[image004.png](#)
[image005.png](#)
[DRB_EPC_application.pdf](#)
[FormP3.pdf](#)

Pam,

For an Administrative Amendment application, you need to submit the following to our office **via email** at this time due to the Coronavirus:

1. Filled out and signed/dated application sheets (attached to this message).
2. Letter of authorization from the property owner if application is submitted by an agent.
3. Zone Atlas map with the entire site clearly outlined and labeled (go to the following web-site to obtain the required map: <https://cabq.maps.arcgis.com/home/webmap/viewer.html?webmap=f12cf8ebe9514a2d8d09e2c7a095d2f4>).
4. Justification letter describing, explaining, and justifying the request per the criteria in IDO Section 14-16-6-4(X)(2).
5. A copy of all applicable sheets of the approved Site Plan being amended (that Concetta sent you).
6. Copy of the Official Notice of Decision associated with the prior approval.
7. A copy of the proposed Site Plan, with changes circled and noted.
8. A PDF file of the complete application, including all plans and documents, emailed to PLNDRS@cabq.gov, mgould@cabq.gov, and jrodenbeck@cabq.gov. If the file size is over 9 megabytes in size, please send us the submission via WeTransfer at <https://wetransfer.com/>.

The application fee is \$50. You can pay the fee online (the preferable option at this time) or pay via check, with the check mailed to:

City of Albuquerque Planning Department
C/O Maggie Gould or Jay Rodenbeck
600 2nd Street NW
Ground Floor
Albuquerque, NM 87102

You can pay online once you have submitted your Administrative Amendment application and our office has confirmed reception. Our clerks will direct you to this option at that time if desired.

WeTransfer Instructions:

You will initially be directed to a page which gives you the option of going to the free version of the site (send up to 2 GB at a time), or the subscription-based version of the site (send up to 20 GB at a time). Click on the link to the free version of the site, and you will be directed to the free version of

the site (a screenshot of the WeTransfer webpage is attached). You just add the email address(es) to send files to, add your files and a message of what you're sending, then click the "Transfer" icon.

When you receive files from a sender on WeTransfer, you will receive an email from the sender. Within the email message is an icon labeled "Get your files" that you click on to download the files from the sender (a screenshot of an email from a sender is attached). Once you click on that icon, a webpage appears with a "Download" icon. Just click on the "Download" icon, and the files sent to you will then download onto your computer. Once you download the files, you can place them where you want. If the sender sends more than one file to you, you will be send a Zip file which you will have extract the individual files from.



Jay Rodenbeck

Planner

o 505.924.3994

e jrodenbeck@cabq.gov

cabq.gov/planning

From: Pam Kearney <pkearney@scoutservices.com>

Sent: Wednesday, July 22, 2020 3:34 PM

To: Rodenbeck, Jay B. <jrodenbeck@cabq.gov>

Cc: Trujillo, Concetta M. <cmtrujillo@cabq.gov>

Subject: Help Needed with Administrative Amendment BP-2020-25270

Jay,

Can you help me with this Information today?

Thank you,

Pam

Pam Kearney
302-318-1230

866-504-3888 x106

Scout Services

490 Quail Ridge Dr. Westmont. IL 60559



FIND US ONLINE

Visit us at www.scoutservices.com



From: Trujillo, Concetta M. <cmtrujillo@cabq.gov>
Sent: Wednesday, July 22, 2020 12:41 PM
To: Pam Kearney <pkearney@scoutservices.com>
Cc: Rodenbeck, Jay B. <jrodenbeck@cabq.gov>
Subject: RE: Resubmit Task Submission Complete for BP-2020-25270

Good Morning Pam,

You need to apply for an Administrative Amendment to the site plan on file. I am added Jay Rodenbeck to the email. He will be able to assist you with this process.

Jay- Can you assist Pam with an AA to add a Canopy to the Drive-Thru. She currently has this project in for permitting.

Thanks,
Concetta



CONCETTA TRUJILLO
zoning plan examiner
o 505.924.3833
e cmtrujillo@cabq.gov
cabq.gov/planning

From: Pam Kearney [<mailto:pkearney@scoutservices.com>]
Sent: Wednesday, July 22, 2020 11:05 AM
To: Trujillo, Concetta M.
Subject: FW: Resubmit Task Submission Complete for BP-2020-25270
Importance: High

Hi, Concetta.

I need your help with this please.
It was rejected on ProjectDox with our resubmittal.

This is the Site Plan that was originally approved in 2011.

What is needed in order to clear your comments?
I have not received a reply to any of the emails that I've sent.

Your help would really be appreciated,
Pam

Pam Kearney
302-318-1230
866-504-3888 x106
Scout Services
490 Quail Ridge Dr. Westmont. IL 60559



FIND US ONLINE

Visit us at www.scoutservices.com



From: Pam Kearney
Sent: Monday, July 20, 2020 4:07 PM
To: jrodenbeck@cabq.gov
Cc: Gomez, Ernest P. <epgomez@cabq.gov>; rbrito@cabq.gov; Tena, Victoria C. <vtena@cabq.gov>
Subject: Resubmit Task Submission Complete for BP-2020-25270
Importance: High

Good Afternoon, Jay.
I was told that you might be able to help us.
We have an application in for a canopy in the drive-thru of a Chick-fil-A.
When we did the due diligence for this project, we were told that there were not any Planning or

Zoning reviews needed.

I just resubmitted our project to ProjectDox.
One of the comments referred to an Administrative Amendment.

Concetta advised us that we needed to locate the site plan that was approved originally in 2011.
Chick-fil-A was able to locate the attached that was submitted then and provide it.

I am attaching that here, but also included it in the resubmittal.

Is this enough to take care of these comments referring to the Administrative Amendment?

Your help in solving would be greatly appreciated,
Pam

Pam Kearney
302-318-1230
866-504-3888 x106
Scout Services
490 Quail Ridge Dr. Westmont. IL 60559

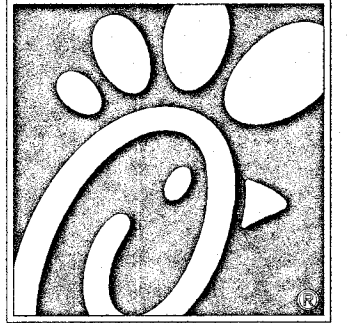


FIND US ONLINE
Visit us at www.scoutservices.com



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This message has been analyzed by Deep Discovery Email Inspector.

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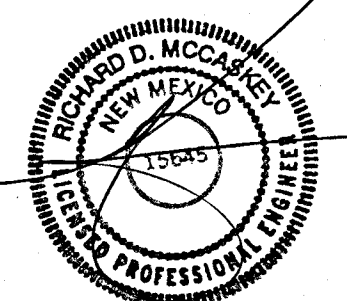


Chick-fil-A

5200 Buffington Rd.
Atlanta Georgia,
30349-2998

Revisions:
Mark Date By

Seal



APR 20 2020

LANE
SUPPLY, INC.
120 FAIRVIEW
ARLINGTON, TX. 76010
(817) 261-9116

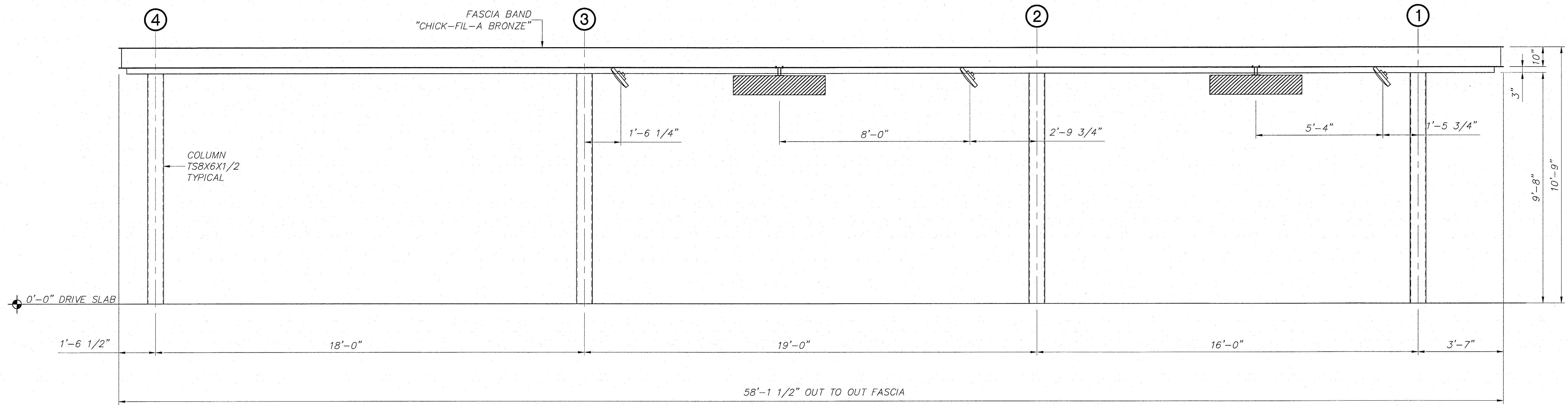
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STORE
Chick-fil-A #2793
5009 MONTGOMERY
BLVD NE
ALBUQUERQUE, NW
87109

SHEET TITLE
**CANOPY
ELEVATION
PLAN**
10'-6 1/2" X 58'-1 1/2"

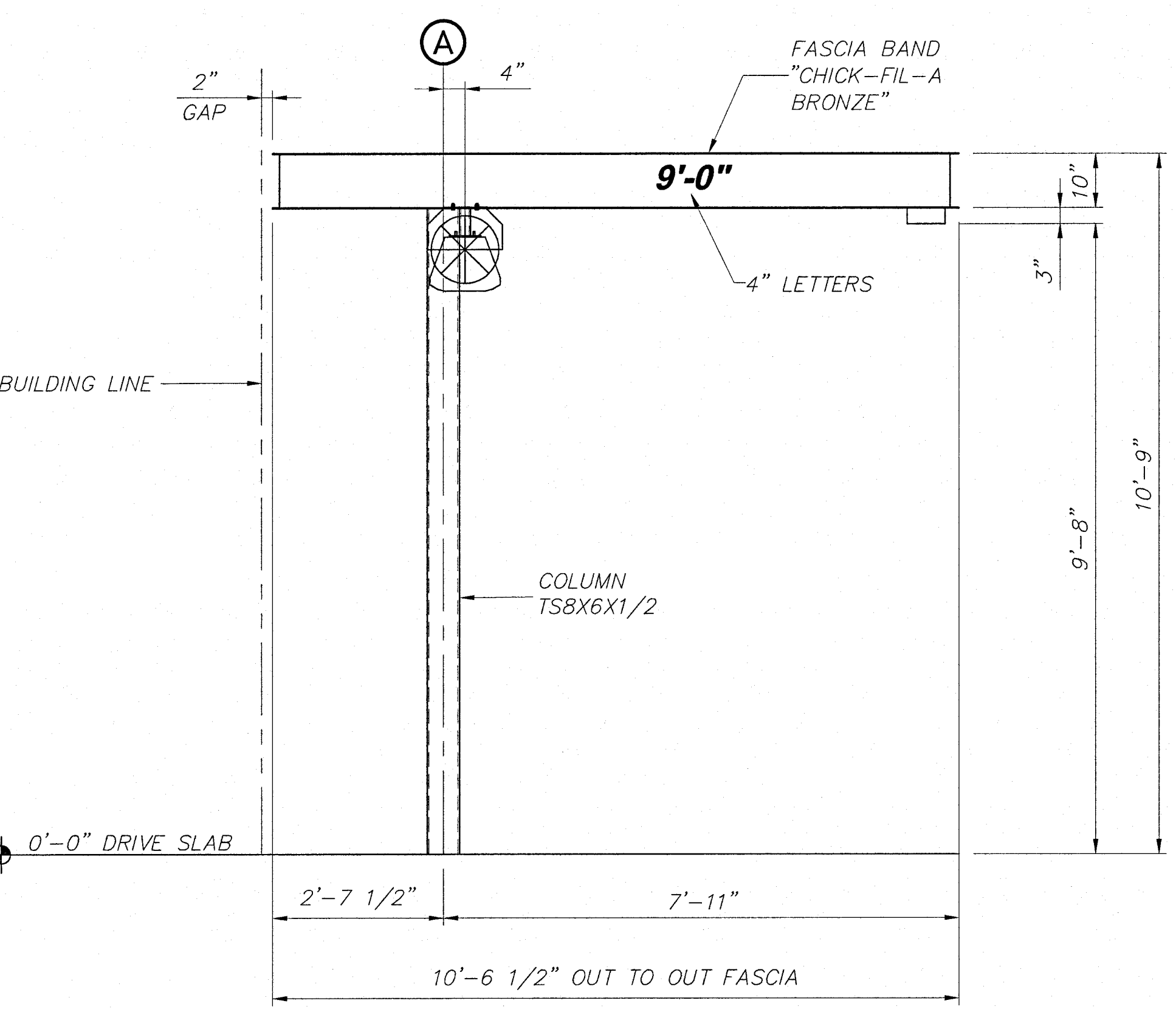
Job No.: LSC: 66258
Store : 2793
Date : 04.16.20
Drawn By : AFG
Checked By: ELM

Sheet
OMD-7
ELI OF 1



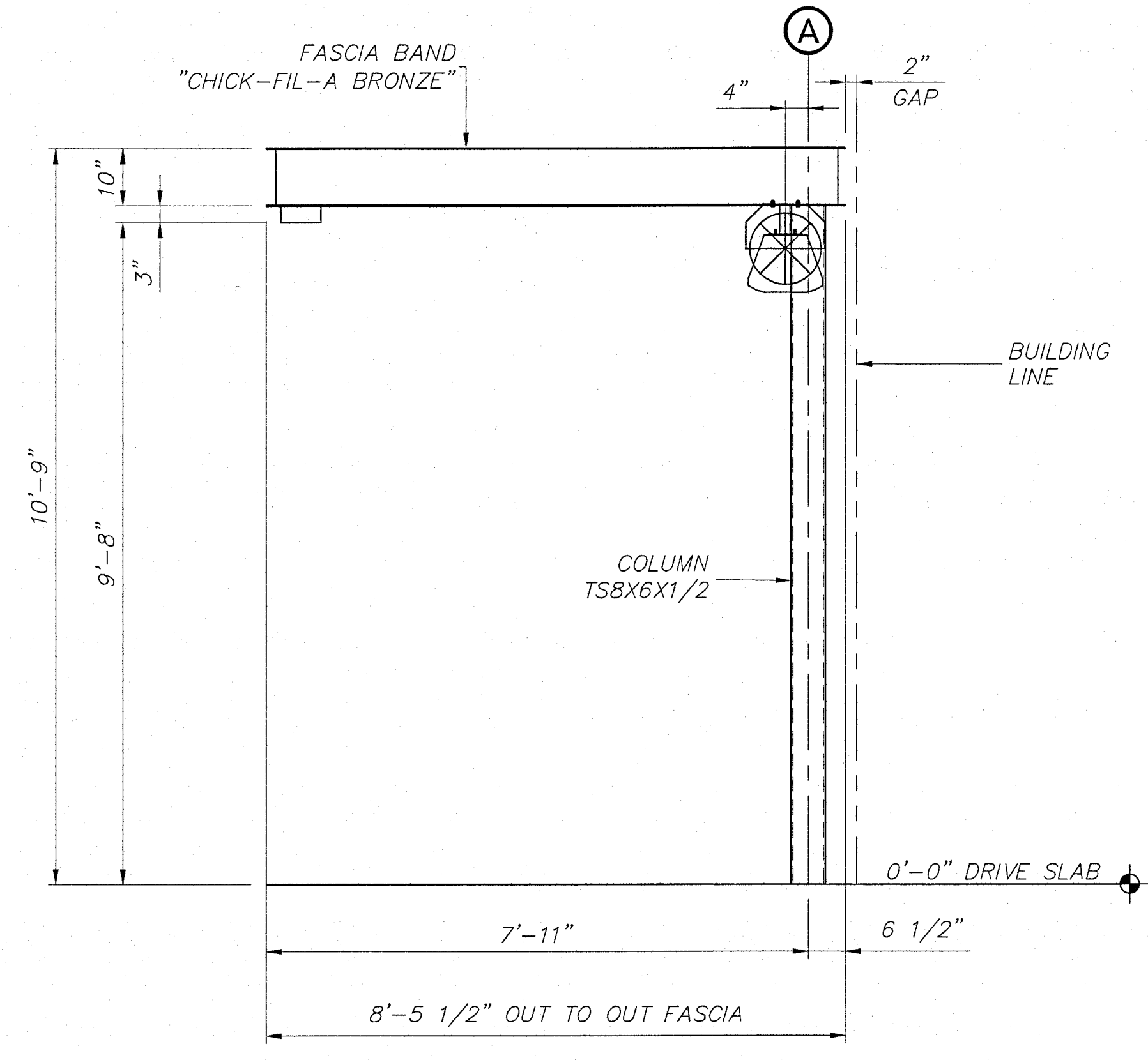
HI CANOPY SIDE ELEVATION

1/4" = 1'-0" FI-ABI, FI-EI, FI-LLI



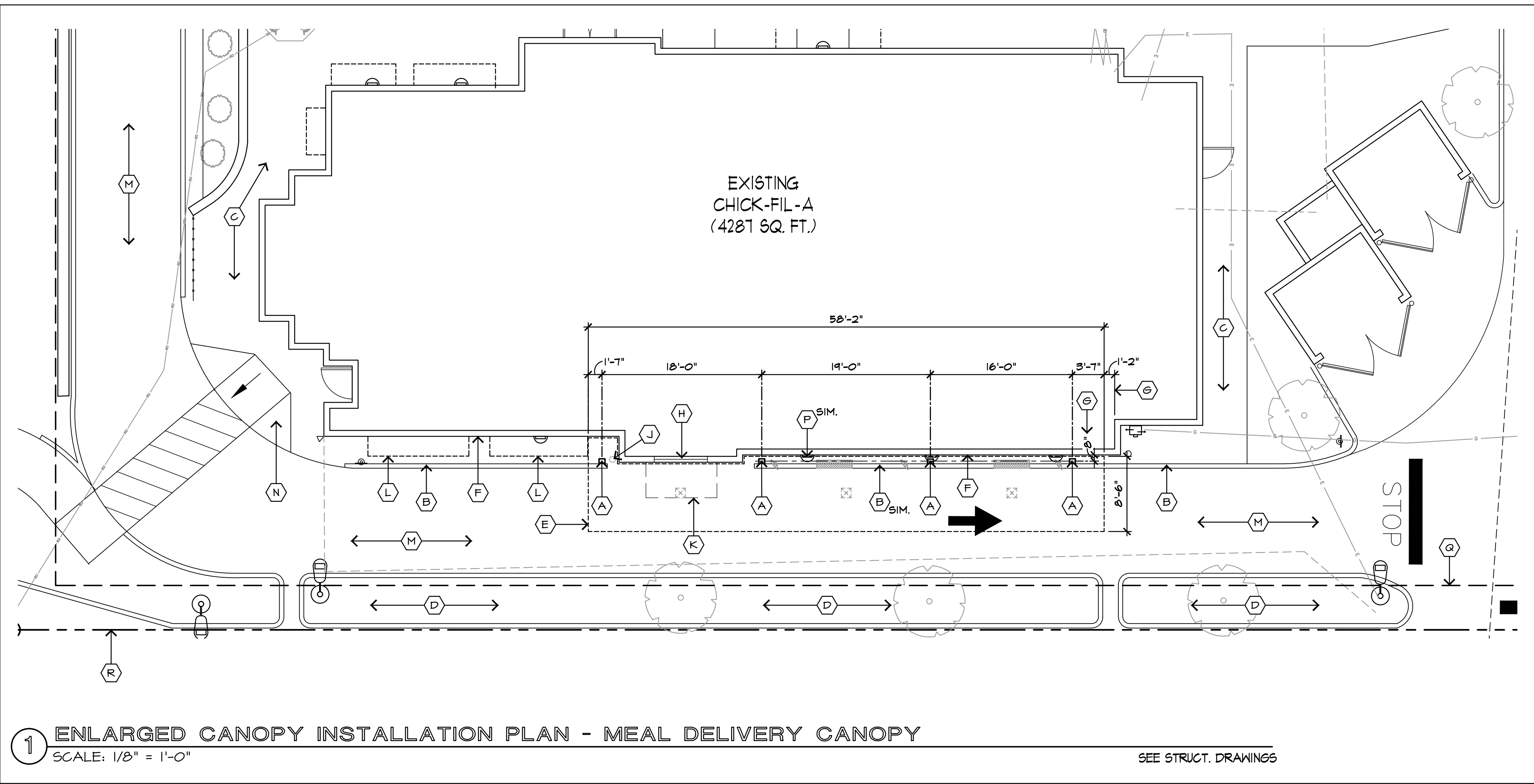
AI CANOPY END ELEVATION

1/4" = 1'-0" FI-ABI, FI-EI, FI-LLI



A9 CANOPY END ELEVATION

1/4" = 1'-0" FI-ABI, FI-EI, FI-LLI



1 ENLARGED CANOPY INSTALLATION PLAN - MEAL DELIVERY CANOPY
SCALE: 1/8" = 1'-0"

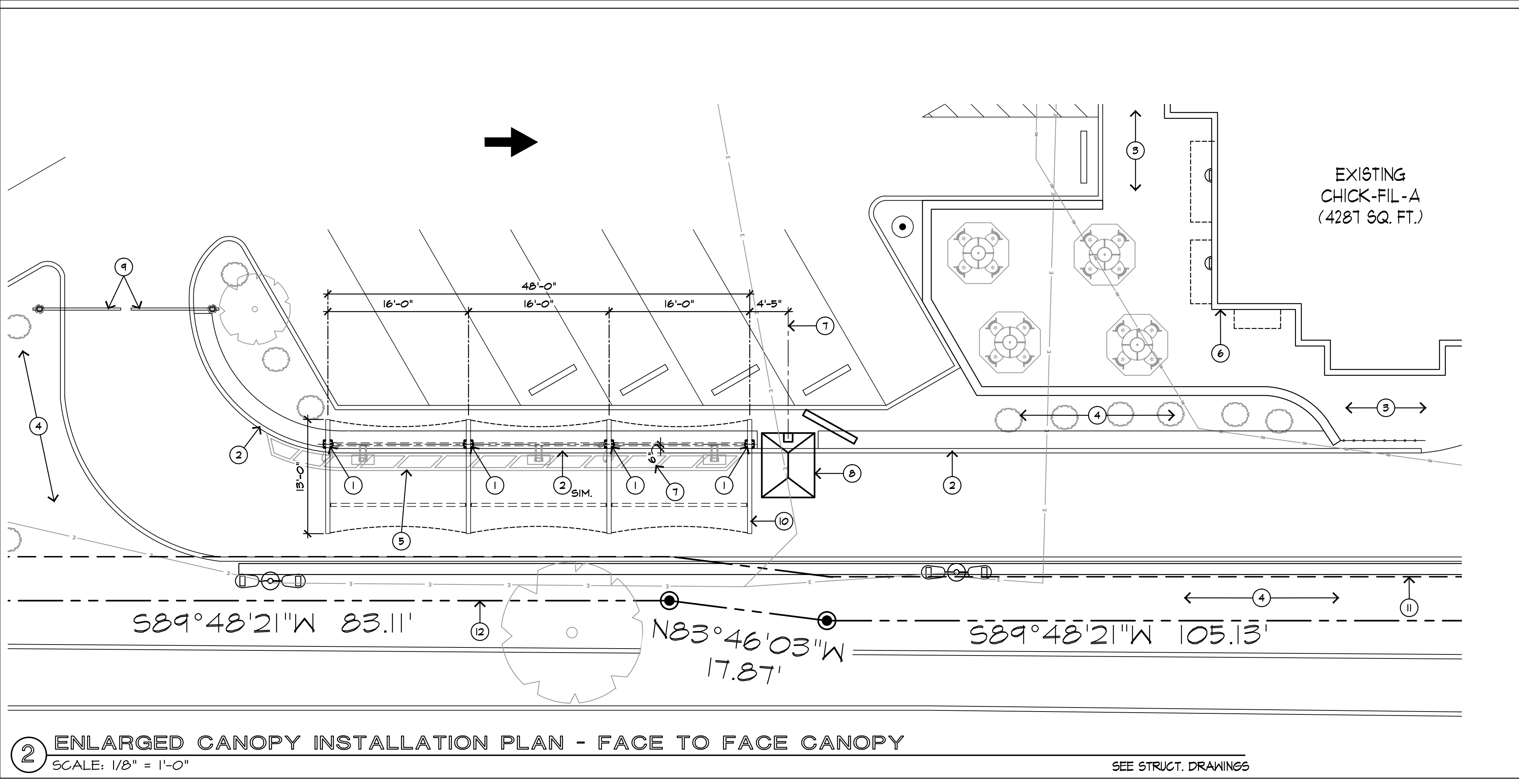
SEE STRUCT. DRAWINGS

- GENERAL NOTES**
- THE DIMENSIONS SHOWN ON THESE DRAWINGS ARE TO CENTER OF COLUMN LINE ONLY AND HAVE BEEN ESTABLISHED IN ACCORDANCE WITH CHICK-FIL-A STANDARD DESIGN GUIDELINES.
 - COLUMNS SHALL BE INSTALLED IN A MANNER AS NOT TO DISTURB THE EXISTING EXTERIOR BUILDING FOUNDATIONS NOR UNDERMINE THE EXISTING EXTERIOR BUILDING STEM WALLS/FOOTINGS.
 - WHERE AN OFFSET CAISSON FOOTING OR SPREAD FOOTING IS REQUIRED, THE G.C. SHALL COORDINATE THE EXISTING FIELD CONDITIONS TO THE CANOPY FABRICATOR FOR APPROVAL PRIOR TO INSTALLATION OF THE FOOTINGS.
 - COLUMNS NEEDING TO BE RELOCATED INTO THE DRIVE LANE OR TEAM WALK WAY, THE G.C. SHALL NOTIFY THE FIELD CONSTRUCTION MANAGER TO GAIN PROPER APPROVAL FROM CHICK-FIL-A PRIOR TO COMMENCEMENT OF SAID CONSTRUCTION WORK.

- CONSTRUCTION NOTES - MEAL DELIVERY**
- NEW SHADE CANOPY STEEL SUPPORT COLUMN WITH CONCRETE FOOTING BELOW. SEE STRUCTURAL DRAWINGS FOR FURTHER DETAILS. EXISTING CONCRETE / LANDSCAPING TO BE REPAIRED OR REPLACED AS REQUIRED PROVIDE FLUSH TRANSITION BETWEEN NEW AND EXIST. CONG. SURFACES.
 - EXISTING CONCRETE CURB TO REMAIN. SAW CUT AND REMOVE PORTION OF CURB AS REQUIRED FOR INSTALLATION OF NEW FOOTING. UPON FOOTING INSTALLATION G.C. SHALL PATCH AND REPAIR EXIST. CURB AS REQUIRED. PROVIDE SMOOTH TRANSITION BETWEEN NEW AND EXIST. CONG. CURB. G.C. SHALL REPLACE ANY KNOWN CRACKED OR DAMAGED ON SITE CONG. CURBING (TYP.).
 - EXISTING CONCRETE WALKWAY TO REMAIN.
 - EXISTING LANDSCAPING AREA TO REMAIN.
 - DASHED LINE INDICATING OUTLINE OF OVERHEAD CANOPY. REFER TO CANOPY SHOP DRAWINGS WITH-IN THIS SET.
 - FACE OF EXISTING EXTERIOR BUILDING WALL. DO NOT DISTURB.
 - XY DIMENSION SHOWN FOR ILLUSTRATIVE PURPOSES ONLY. G.C. TO FIELD VERIFY EXACT LOCATION AND NOTIFY CHICK-FIL-A PROJECT TEAM IF CANOPY LOCATION CHANGES AND IMPACTS OTHER CONSTRUCTION RELATED CONDITIONS.
 - EXISTING MEAL DELIVERY PICK UP WINDOW. DO NOT DISTURB.
 - EXISTING BOLLARD TO BE REMOVED.
 - DASHED LINE INDICATING EXISTING AWNING TO BE REMOVED. G.C. TO REPAIR EXIST. EXTERIOR WALL AS REQUIRED APPLY TOUCH-UP PAINT FIELD MATCH EXIST. COLOR AND WALL FINISH. PROVIDE FLUSH TRANSITION FOR ALL PATCH WORK.
 - EXISTING AWNING TO REMAIN. NO CHANGE.
 - EXISTING DRIVE THRU LANE TO REMAIN.
 - EXIST. A.D.A. COMPLIANT CONG. CURB RAMP TO REMAIN. DO NOT DISTURB.
 - EXISTING SECURITY LIGHTS TO REMAIN.
 - EXISTING BUILDING SETBACK LINE SHOWN FOR REFERENCE.
 - EXISTING PROPERTY LINE SHOWN FOR REFERENCE.

REFERENCE:

SCALE: 1/8" = 1'-0"



2 ENLARGED CANOPY INSTALLATION PLAN - FACE TO FACE CANOPY
SCALE: 1/8" = 1'-0"

SEE STRUCT. DRAWINGS

- GENERAL NOTES**
- THE DIMENSIONS SHOWN ON THESE DRAWINGS ARE TO CENTER OF COLUMN LINE ONLY AND HAVE BEEN ESTABLISHED IN ACCORDANCE WITH CHICK-FIL-A STANDARD DESIGN GUIDELINES.
 - COLUMNS SHALL BE INSTALLED IN A MANNER AS NOT TO DISTURB THE EXISTING EXTERIOR BUILDING FOUNDATIONS NOR UNDERMINE THE EXISTING EXTERIOR BUILDING STEM WALLS/FOOTINGS.
 - WHERE AN OFFSET CAISSON FOOTING OR SPREAD FOOTING IS REQUIRED, THE G.C. SHALL COORDINATE THE EXISTING FIELD CONDITIONS TO THE CANOPY FABRICATOR FOR APPROVAL PRIOR TO INSTALLATION OF THE FOOTINGS.
 - COLUMNS NEEDING TO BE RELOCATED INTO THE DRIVE LANE OR TEAM WALK WAY, THE G.C. SHALL NOTIFY THE FIELD CONSTRUCTION MANAGER TO GAIN PROPER APPROVAL FROM CHICK-FIL-A PRIOR TO COMMENCEMENT OF SAID CONSTRUCTION WORK.

- CONSTRUCTION NOTES - FACE TO FACE**
- NEW SHADE CANOPY STEEL SUPPORT COLUMN WITH CONCRETE FOOTING BELOW. SEE STRUCTURAL DRAWINGS FOR FURTHER DETAILS. EXISTING CONCRETE / LANDSCAPING TO BE REPAIRED OR REPLACED AS REQUIRED PROVIDE FLUSH TRANSITION BETWEEN NEW AND EXIST. CONG. SURFACES.
 - EXISTING CONCRETE CURB TO REMAIN. SAW CUT AND REMOVE PORTION OF CURB AS REQUIRED FOR INSTALLATION OF NEW FOOTING. UPON FOOTING INSTALLATION G.C. SHALL PATCH AND REPAIR EXIST. CURB AS REQUIRED. PROVIDE SMOOTH TRANSITION BETWEEN NEW AND EXIST. CONG. CURB. G.C. SHALL REPLACE ANY KNOWN CRACKED OR DAMAGED ON SITE CONG. CURBING (TYP.).
 - EXISTING CONCRETE WALKWAY TO REMAIN.
 - EXISTING LANDSCAPING AREA TO REMAIN.
 - NEW 24" WIDE TEAM MEMBER DEMARCATION STRIPING. USE TRAFFIC RATED PAINT. MIN. (2) COATS.
 - FACE OF EXISTING EXTERIOR BUILDING WALL - DO NOT DISTURB.
 - XY DIMENSION SHOWN FOR ILLUSTRATIVE PURPOSES ONLY. G.C. TO FIELD VERIFY EXACT LOCATION OF COLUMN STARTING POINT. NOTIFY CHICK-FIL-A PROJECT TEAM OF ANY DISCREPANCIES IMPACTING OTHER CONSTRUCTION PHASES OR RELATED CONDITIONS.
 - EXISTING DRIVE THRU ORDER POINT MENU BOARD. DO NOT DISTURB.
 - EXISTING CLEARANCE BARS TO REMAIN. DO NOT DISTURB.
 - DASHED LINE INDICATING OUTLINE OF OVERHEAD CANOPY. REFER TO CANOPY DRAWINGS WITH-IN THIS SET.
 - EXISTING BUILDING SETBACK LINE SHOWN FOR REFERENCE.
 - EXISTING PROPERTY LINE SHOWN FOR REFERENCE.

REFERENCE:

SCALE: 1/8" = 1'-0"

Chick-fil-A
5200 BUFFINGTON ROAD
ATLANTA, GEORGIA 30349
(404) 765-8000

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esencia

1743 E. McNair Drive, Suite 200
Tempe, Arizona 85283
Telephone 480-755-0959

STATE OF NEW MEXICO
JEFFREY W. WINTER
No. 004961
REGISTERED ARCHITECT
05-29-2020

Revision Schedule

Rev	Date	By	Description

Project Name: CANOPY PROGRAM

CHICK-FIL-A
STORE #2793

5009 MONTGOMERY BLVD. NE
ALBUQUERQUE, NM 87109

Issue Date: 05-29-2020
Drawn By: GA
Job Number: 19060

Sheet Title: **Enlarged Canopy Plan**

Scale: As indicated

Sheet Number: **A-2**



LANE SUPPLY, INC.

120 Fairview
Arlington, TX 76010
817-261-9116

DESIGN CALCULATIONS FOR :

Chick-fil-A #2793 Outside Meal Delivery Canopy
5009 Montgomery Blvd NE
Albuquerque, NM

Four-Column Canopy :	10'-2" X 57'-9" Canopy
Lane Reference Number :	LSC-66258
Date :	20-Apr-20

TABLE OF CONTENTS :

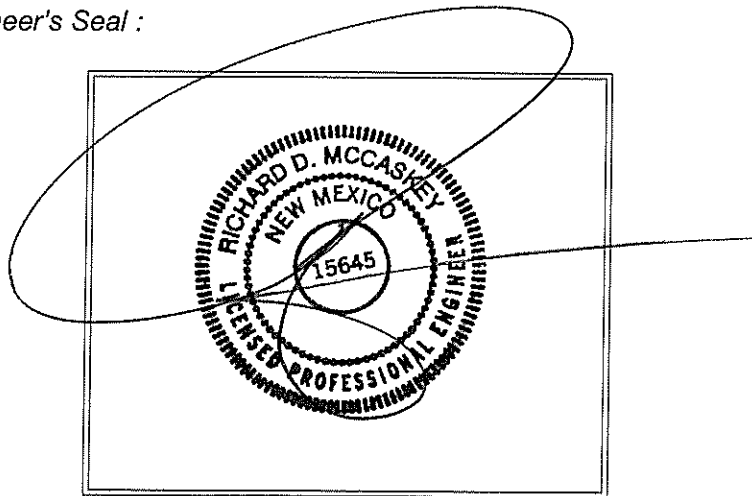
Canopy Calculations :

Design Loads :	1-2
Deck Design :	3-5
Purlin Design :	6-9
Header Design :	10-13
Column & Foundation Design :	14-16

Attachments :

- Lane SL-316 Deck Panel Properties
- Lane Standard Cap Plate Design
- Lane Standard Base Plate Design
- Design Sketch

Engineer's Seal :



APR 20 2020

Calculations By:

Lane Supply, Inc.

LSC - 66258

Customer:

Chick-fil-A #2793 Outside Meal Delivery Canopy

By: JMCP

Project:

10'-2" X 57'-9" Canopy

Check:

Code: 2015 International Building Code

Roof Loads: Dead Load = 3.00 psf (SL-316 Deck)

 Live/Snow Load = 20.00 psf

 TOTAL = 23.00 psf

Fascia Load: Height = 10.00 in.

 Dead Load = 5.83 plf

Wind Loads: Risk Category = II

 V, ULT Speed = 116 m.p.h. Exp C

 V, ASD Speed = 90 m.p.h. Exp C

 Height = 15 ft

 Kd = 0.85

 Kh = 0.85

 G = 0.85

 qz = 14.93 psf

Lateral Load = 1.0 (H)•qz = 16.00 psf

Deck Uplift = -1.7 (V)•G•qz = -21.58 psf

Frame Uplift = -1.1 (V)•G•qz = -13.96 psf

Base Shear : V = CS • W = 0.340 • W

 Site Class = D

 Ss(0.2) = 0.44

 S1(1.0) = 0.13

 Fa = 1.45

 Fv = 2.27

 SM1 = Fv•S1 = 0.30

 SMS = Fa•Ss = 0.64

 SD1 = 2/3•SM1 = 0.20

 SDS = 2/3•SMS = 0.43

 R = 1.25

 Risk Category = II

 CS = (SDS/R) = 0.340 (12.8-2)

Seismic Design Category Based on SDS : C

Seismic Design Category Based on SD1 : D

Design Category : D

Section 7.1--Symbols & Notation

- Ce = 1.2 Exposure Factor as determined from Table 7-2
- Ct = 1.2 Thermal factor as determined from Table 7-3
- D = Snow Density in pcf as determined from Eq. 7-4
- hb = Height of balanced snow load determined by dividing Pf by D, in feet.
- hd = Height of snow drift, in feet
- hc = Clear height from top of balanced snow to top of parapet, ft
- hr = 0.83 = Fascia height, ft
- Is = 1.0 = Importance factor (see Table 7-4).
- Pf = Snow load on flat roofs, psf.
- Pg = 20 =ground snow, psf.
- Pd = Maximum intensity of drift surcharge load, psf.
- lu = 10.541667 = Length of roof upwind of the drift, feet
- w = Width of snow drift, in feet

Section 7.3--Flat-Roof Snow Loads, Pf

The snow load, Pf, on a roof with a slope equal to or less than 15° shall be calculated in psf using equation 7.3-1, but not less than the following minimum values for low slope roofs: where Pg is 20 psf or less Pf = I(Pg), where Pg exceeds 20 psf, Pf = 20 (I).

Section 7.7 & Section 7.8

The geometry of the surcharge load due to snow drifting shall be approximated by a triangle as shown in figure 7-8. Drift loads shall be superimposed on the balanced snow load. If hc/hb is less than 0.2, drift loads are not required to be applied. The height of such drifts shall be taken as 0.75 x hd as determined from Fig 7-9, with lu equal to the length of the roof upwind of the projection or parapet wall. If the side of a roof projection is less than 15 ft long, a drift load is not required to be applied to that side. If the height, hd, is equal to or less than hc, the drift width shall equal 4hd and the drift height shall equal hd. If this height exceeds hc, the drift width, w, shall equal 4 hd²/hc and the drift height shall equal hc. However, the drift width w shall not exceed 8hc. The maximum intensity of the drift surcharge load, pd, equals hd x D where the snow density, D, is defined by Eq 7.7-

Section 7.10--Rain-On-Snow Surcharge Load

For locations where Pg is 20 psf or less but not zero, all roofs with a slope less than W/50, shall have a 5 psf rain-on-snow surcharge load applied to establish the design snow loads. This rain-on-snow augmented design load applies only to the balanced load case and need not be used in combination with drift, sliding, unbalanced, or partial loads.

$$\begin{aligned}
 P_f &= 0.7 \times C_e \times C_t \times I_s \times P_g && \text{Eq 7.3-1} \\
 P_f &= 20.0 \text{ psf} \\
 h_d &= 0.75 \times (0.43(l_u)^{1/3} \times (P_g+10)^{1/4-1.5}) \\
 h_d &= 0.53 \text{ ft} \\
 D &= 0.13P_g + 14 < 30 \text{ psf} && \text{Eq 7.7-1} \\
 D &= 16.60 \text{ psf} \\
 h_b &= 1.20 \text{ ft} \\
 h_c = h_r - h_b &= -0.37 \text{ ft} \\
 h_c/h_b &= -0.31 \\
 &\text{Drift Loads Not Considered} \\
 w &= -3.02 \text{ ft} \\
 P_d &= D \times h_d < D \times h_c \\
 P_d &= -6.17 \text{ psf}
 \end{aligned}$$

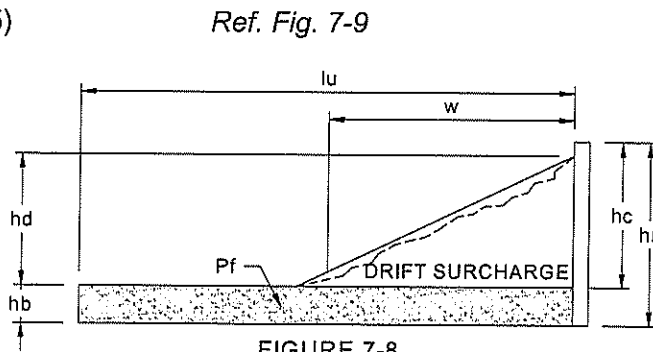
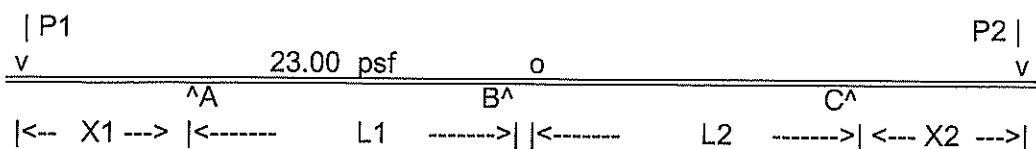


FIGURE 7-8 Configuration of Snow Drifts on Lower Roofs



Wd=	3.00 psf	X1=	1.00 ft
Wl=	20.00 psf	L1=	2.71 ft
Deck : Ww=	-21.58 psf	L2=	4.42 ft
Frame : Ww=	-13.96 psf	X2=	2.00 ft
P1=	5.83 plf		
P2=	5.83 plf		

MAd=	7.33 ft-lbs/ft	RA d=	15.60 plf
MAI=	10.00 ft-lbs/ft	RAI=	50.78 plf
Deck : MAw=	-10.79 ft-lbs/ft	Frame : RAw=	-35.45 plf
MA(d+l)=	17.33 ft-lbs/ft	Deck : RAw=	-54.78 plf
Deck : MA(d+w)=	-3.46 ft-lbs/ft	RA(d+l)=	66.38 plf
		Frame : RA(d+w)=	-19.84 plf
		Deck : RA(d+w)=	-39.18 plf

MAB(d+l)=	17.58 ft-lbs/ft	RBd=	3.98 plf
Deck : MAB(d+w)=	-15.35 ft-lbs/ft	RBI=	71.25 plf
		Frame : RBw=	-40.84 plf
		Deck : RBw=	-63.12 plf
MBC(d+l)=	47.60 ft-lbs/ft	RB(d+l)=	75.23 plf
Deck : MBC(d+w)=	-33.45 ft-lbs/ft	Frame : RB(d+w)=	-36.86 plf
		Deck : RB(d+w)=	-59.14 plf

MCd=	17.67 ft-lbs/ft	RCd=	22.46 plf
MCI=	40.00 ft-lbs/ft	RCI=	93.22 plf
Deck : MCw=	-43.16 ft-lbs/ft	Frame : RCw=	-65.08 plf
MC(d+l)=	57.67 ft-lbs/ft	Deck : RCw=	-100.58 plf
Deck : MC(d+w)=	-25.49 ft-lbs/ft	RC(d+l)=	115.68 plf
		Frame : RC(d+w)=	-42.62 plf
		Deck : RC(d+w)=	-78.12 plf

USE 20 GAUGE GRADE C DECK

+S=.3961 in³ -S=.3036 in³ FY=40 ksi

DECK DESIGN: Deck 2

Wd=	3.00 psf	P1	P2
Wl=	20.00 psf	v	v
Deck : Ww=	-21.58 psf	23.00 psf	
Frame : Ww=	-13.96 psf	^A ^B	
P1=	5.83 plf	<-- X1 --> <----- L -----> <-- X2 ->	
P2=	5.83 plf		
L=	4.42 ft		
X1=	0.79 ft		
X2=	2.00 ft		

MA(d)=	5.56 ft-lbs/ft	RA d=	12.09 plf
MA(l)=	6.27 ft-lbs/ft	RA l(MAX)=	61.42 plf
Deck: MA(w)=	-6.76 ft-lbs/ft	Frame: RA w=	-36.55 plf
MA(d+l)=	11.83 ft-lbs/ft	Deck: RA w=	-56.49 plf
Deck : MA(d+w)=	-1.20 ft-lbs/ft	RA(d+l)=	73.51 plf
		Frame: RA(d+w)=	-24.46 plf
		Deck: RA(d+w)=	-44.40 plf

MB(d)=	17.67 ft-lbs/ft	RB d=	21.20 plf
MB(l)=	40.00 ft-lbs/ft	RB l(MAX)=	93.22 plf
Deck: MB(w)=	-43.16 ft-lbs/ft	Frame: RB w=	-64.09 plf
MB(d+l)=	57.67 ft-lbs/ft	Deck: RB w=	-99.05 plf
Deck : MB(d+w)=	-25.49 ft-lbs/ft	RB(d+l)=	114.42 plf
		Frame: RB(d+w)=	-42.89 plf
		Deck: RB(d+w)=	-77.85 plf

MAB(d+l)=	44.63 ft-lbs/ft
Deck : MAB(d+w)=	-32.77 ft-lbs/ft

USE 20 GAUGE GRADE C DECK

+S=.3961 in^3 -S=.3036 in^3 FY=40 ksi

DECK DESIGN: Deck 3

Wd=	3.00 psf	P1	P2
Wl=	20.00 psf	v	v
Deck : Ww=	-21.58 psf	23.00 psf	
Frame : Ww=	-13.96 psf	^A ^B	
P1=	5.83 plf	<-- X1 --> <----- L -----> <-- X2 -->	
P2=	5.83 plf		
L=	4.42 ft		
X1=	1.63 ft		
X2=	2.00 ft		

MA(d)=	13.44 ft-lbs/ft
MA(l)=	26.41 ft-lbs/ft
Deck: MA(w)=	-28.49 ft-lbs/ft
MA(d+l)=	39.85 ft-lbs/ft
Deck : MA(d+w)=	-15.05 ft-lbs/ft

RA(d)=	16.38 plf
RAI(MAX)=	82.65 plf
Frame: RAw=	-51.37 plf
Deck: RAw=	-79.40 plf
RA(d+l)=	99.02 plf
Frame: RA(d+w)=	-35.00 plf
Deck: RA(d+w)=	-63.02 plf

MB(d)=	17.67 ft-lbs/ft
MB(l)=	40.00 ft-lbs/ft
Deck: MB(w)=	-43.16 ft-lbs/ft
MB(d+l)=	57.67 ft-lbs/ft
Deck : MB(d+w)=	-25.49 ft-lbs/ft

RB(d)=	19.42 plf
RBI(MAX)=	93.22 plf
Frame: RBw=	-60.91 plf
Deck: RBw=	-94.13 plf
RB(d+l)=	112.64 plf
Frame: RB(d+w)=	-41.49 plf
Deck: RB(d+w)=	-74.71 plf

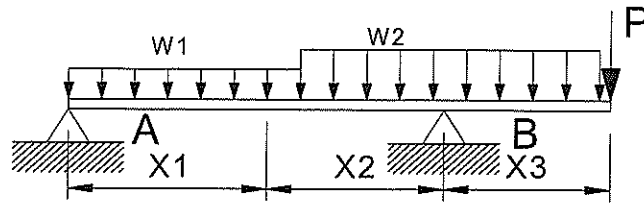
MAB(d+l)=	40.55 ft-lbs/ft
Deck : MAB(d+w)=	-25.18 ft-lbs/ft

USE 20 GAUGE GRADE C DECK
 +S=.3961 in^3 -S=.3036 in^3 FY=40 ksi

Wd1 = 24.09 plf
 WI1 = 61.42 plf
 Ww1 = -36.55 plf

Wd2 = 15.98 plf
 WI2 = 71.25 plf
 Ww2 = -40.84 plf
 Pd = 31.87 lbs
 Pl = 0.00 lbs
 Pw = 0.00 lbs

X1 = 16.46 ft
 X2 = 1.54 ft
 X3 = 1.33 ft



RA _d =	213 lbs	RB _d =	261 lbs
RA _l =	550 lbs	RB _l =	666 lbs
RA _w =	-327 lbs	RB _w =	-392 lbs
<hr/>		<hr/>	
RA(d+l) =	763 lbs	RB(d+l) =	927 lbs
RA(d+w) =	-114 lbs	RB(d+w) =	-131 lbs

MB(d+l) = -120 ft-lbs lu = 1.33 ft OK
 MB(d+w) = -20 ft-lbs lu = 1.33 ft OK

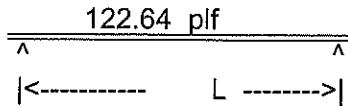
MAB(d+l) = 3384 ft-lbs lu = 9.00 ft OK
 MAB(d+w) = -519 ft-lbs lu = 1.33 ft OK

Deflections: (inches)	Overhang	Midspan
DL =	-0.019	0.083
DL+LL =	-0.071	0.302

USE: W6X12 Fy = 50 ksi

BEAM DESIGN: P-d

Wd= 29.42 plf
 Wl= 93.22 plf
 Ww= -60.91 plf
 L= 19.00 ft



		Rd=	279 lbs	
		RI=	886 lbs	
Md=	1327.4 ft-lbs	Rw=	-579 lbs	
MI=	4206.7 ft-lbs	R(d+l)=	1165 lbs	
Mw=	-2748.4 ft-lbs	R(d+w)=	-299 lbs	
M(d+l)=	5534.1 ft-lbs	Lu=	9.50 ft	OK
M(d+w)=	-1421.0 ft-lbs	Lu=	1.33 ft	OK

USE: W8X10 Fy = 50 ksi

Deflections: (inches)	Midspan
DL=	0.097
DL+LL=	0.403
(+downward, -upward)	

BEAM DESIGN: P-e

Wd= 31.38 plf | P
 Wl= 82.65 plf v 114.02 plf
 Ww= -51.37 plf ^A B^
 Pd= 21.46 lbs |<----X----> |<----- L ----->|
 Pl= 0.00 lbs
 Pw= 0.00 lbs

Deflections: (inches)	Overhang	Midspan
DL=	-0.026	0.047
DL+LL=	-0.104	0.176
(+downward, -upward)		

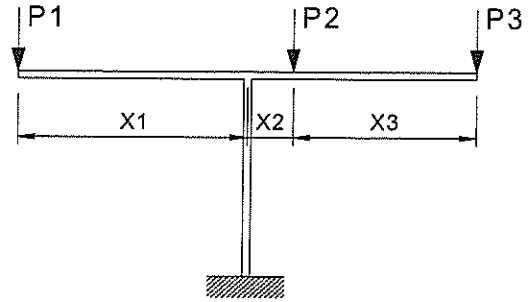
MA(d) =	251 ft-lbs		RA d=	394 lbs
MA(l) =	471 ft-lbs		RA l=	970 lbs
MA(w) =	-293 ft-lbs		RA w=	-603 lbs
MA(d+l) =	722 ft-lbs lu=	1.33 ft	RA(d+l)=	1364 lbs
MA(d+w) =	-41 ft-lbs lu=	3.38 ft	RA(d+w)=	-209 lbs
			RB d=	235 lbs
			RB l(Max)=	661 lbs
			RB w=	-393 lbs
MAB(d+l)=	3524 ft-lbs lu=	8.00 ft	RB(d+l)=	896 lbs
MAB(d+w)=	-619 ft-lbs lu=	1.33 ft	RB(d+w)=	-157 lbs

USE: W6X15 Fy = 50 ksi

HEADER DESIGN:

H-a

P1d =	45 lbs
P1I =	146 lbs
P1w =	-102 lbs
P2d =	261 lbs
P2I =	666 lbs
P2w =	-392 lbs
P3d =	366 lbs
P3I =	968 lbs
P3w =	-676 lbs



Beam Wt = 20 plf

X1 =	1.42 ft
X2 =	1.29 ft
X3 =	4.42 ft

BEAM:			
Left Hand Side			
Md =	-84 ft-lbs		
MI =	-207 ft-lbs		
Mw =	144 ft-lbs		
Md+MI =	-290 ft-lbs	lu =	1.42 ft
Md+Mw =	61 ft-lbs	lu =	1.42 ft
Md+0.75•I+0.75•Mw =	347 ft-lbs	lu =	1.42 ft
Right Hand Side			
Md =	-2754 ft-lbs		
MI =	-6385 ft-lbs		
Mw =	4363 ft-lbs		
Md+MI =	-9139 ft-lbs	lu =	4.42 ft
Md+Mw =	1610 ft-lbs	lu =	4.42 ft
Md+0.75•I+0.75•Mw =	10815 ft-lbs	lu =	4.42 ft

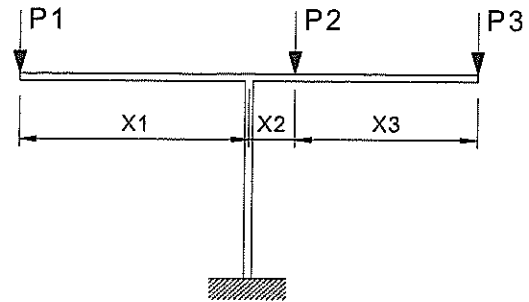
USE: W6X20 Fy = 50 ksi

COLUMN:		Pd =	815 lbs
Md =	2670 ft-lbs	PI =	1780 lbs
M(ULL) =	6648 ft-lbs	Pw =	-1169 lbs
M(D+L) =	9318 ft-lbs	Pd + PI =	2594 lbs
		Pd + Pw =	-355 lbs

HEADER DESIGN:

H-b

P1d = 0 lbs
 P1l = 0 lbs
 Plw = 0 lbs
 P2d = 511 lbs
 P2l = 1335 lbs
 P2w = -815 lbs
 P3d = 568 lbs
 P3l = 1725 lbs
 P3w = -1161 lbs



Beam Wt = 21 plf

X1 = 0.00 ft
 X2 = 1.29 ft
 X3 = 4.42 ft

BEAM:			
Left Hand Side			
Md =	-0 ft-lbs		
MI =	-0 ft-lbs		
Mw =	-0 ft-lbs		
Md+MI =	-0 ft-lbs	lu =	0.00 ft
Md+Mw =	-0 ft-lbs	lu =	0.00 ft
Md+0.75•I+0.75•Mw =	0 ft-lbs	lu =	0.00 ft
Right Hand Side			
Md =	-4245 ft-lbs		
MI =	-11569 ft-lbs		
Mw =	7681 ft-lbs		
Md+MI =	-15814 ft-lbs	lu =	4.42 ft
Md+Mw =	3437 ft-lbs	lu =	4.42 ft
Md+0.75•I+0.75•Mw =	18682 ft-lbs	lu =	4.42 ft

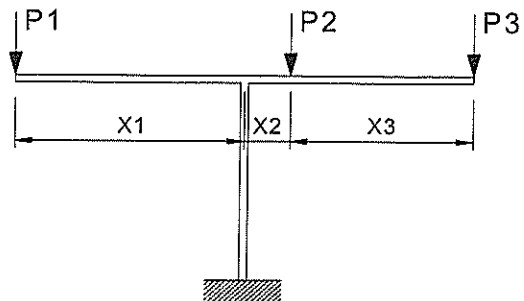
USE: W8X21 Fy = 50 ksi

COLUMN:		Pd =	1199 lbs
Md =	4245 ft-lbs	Pl =	3060 lbs
M(ULL) =	11595 ft-lbs	Pw =	-1976 lbs
M(D+L) =	15839 ft-lbs	Pd + Pl =	4259 lbs
		Pd + Pw =	-777 lbs

HEADER DESIGN:

H-c

P1d =	0 lbs
P1l =	0 lbs
P1w =	0 lbs
P2d =	533 lbs
P2l =	1446 lbs
P2w =	-881 lbs
P3d =	499 lbs
P3l =	1631 lbs
P3w =	-1044 lbs



Beam Wt = 21 plf

X1 =	0.00 ft
X2 =	1.29 ft
X3 =	4.42 ft

BEAM:			
Left Hand Side			
Md =	-0 ft-lbs		
MI =	-0 ft-lbs		
Mw =	-0 ft-lbs		
Md+MI =	-0 ft-lbs	lu =	0.00 ft
Md+Mw =	-0 ft-lbs	lu =	0.00 ft
Md+0.75•I+0.75•Mw =	0 ft-lbs	lu =	0.00 ft
Right Hand Side			
Md =	-3879 ft-lbs		
MI =	-11181 ft-lbs		
Mw =	7098 ft-lbs		
Md+MI =	-15060 ft-lbs	lu =	4.42 ft
Md+Mw =	3219 ft-lbs	lu =	4.42 ft
Md+0.75•I+0.75•Mw =	17588 ft-lbs	lu =	4.42 ft

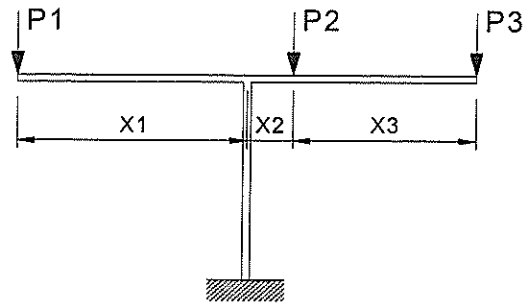
USE: W8X21 Fy = 50 ksi

COLUMN:		Pd =	1152 lbs
Md =	3879 ft-lbs	Pl =	3078 lbs
M(ULL) =	10968 ft-lbs	Pw =	-1925 lbs
M(D+L) =	14847 ft-lbs	Pd + Pl =	4230 lbs
		Pd + Pw =	-773 lbs

HEADER DESIGN:

H-d

P1d = 0 lbs
 P1l = 0 lbs
 P1w = 0 lbs
 P2d = 394 lbs
 P2l = 970 lbs
 P2w = -603 lbs
 P3d = 376 lbs
 P3l = 1094 lbs
 P3w = -714 lbs



Beam Wt = 20 plf

X1 = 0.00 ft
 X2 = 1.29 ft
 X3 = 4.42 ft

BEAM:			
Left Hand Side			
Md =	-0 ft-lbs		
MI =	-0 ft-lbs		
Mw =	-0 ft-lbs		
Md+MI =	-0 ft-lbs	lu =	0.00 ft
Md+Mw =	-0 ft-lbs	lu =	0.00 ft
Md+0.75•I+0.75•Mw =	0 ft-lbs	lu =	0.00 ft
Right Hand Side			
Md =	-2981 ft-lbs		
MI =	-7495 ft-lbs		
Mw =	4857 ft-lbs		
Md+MI =	-10475 ft-lbs	lu =	4.42 ft
Md+Mw =	1876 ft-lbs	lu =	4.42 ft
Md+0.75•I+0.75•Mw =	12244 ft-lbs	lu =	4.42 ft

USE: W6X20 Fy = 50 ksi

COLUMN:		Pd =	884 lbs
Md =	2981 ft-lbs	PI =	2063 lbs
M(ULL) =	7511 ft-lbs	Pw =	-1317 lbs
M(D+L) =	10492 ft-lbs	Pd + PI =	2947 lbs
		Pd + Pw =	-433 lbs

Column Design

AISC 14th ed, Use First Order Analysis Criteria

P DL =	1.20 kips	Clr. Ht.=	9.50 ft
P LL =	3.06 kips	Fascia Ht.=	1.00 ft
P WL =	-1.98 kips	Col. Trib=	18.43 ft
Base Shear =	0.41 kips	Wind Load=	16.00 psf
Total Base Shear =	1.38 kips	# of COL.=	1.00
M WL =	$w(\text{Fascia Ht} \cdot 2.5 \cdot \text{Col Trib} / \# \text{ of col} \cdot L) + w(\text{Wrap} \cdot 1/2 \text{ Clr. Ht}^2)$	Max All. Defl =	1.20 in
M Seis =	Base Shear x L	Max Defl Ratio =	L / 100
M Unbal =	Live Load x Col. Trib.x (Canopy Width/2)^2/2	Max Defl. =	0.26 in, OK
L =	Clr. Ht. + Fascia Ht/2		
Pr =	4.26 kips	1.6Pr < 0.5Py First-Order Analysis Allowed (A-7-1)	
Py =	533.60 kips		
N =	0.01 • Yi (A-7-2)		
B2 =	1.02 OK, A-8-6		
M WL =	8.10 kip-ft		
M Seis =	4.08 kip-ft		
M DL(Nod) =	0.09 kip-ft		
M LL(Nod) =	0.22 kip-ft		
M Unbal DL=	4.24 kip-ft		
M Unbal LL=	11.59 kip-ft		
M Unbal WL=	7.68 kip-ft		

Use: TS8X6X1/2	
Fy =	46.00 ksi
K =	1.00
L, Col =	10.00 ft
A =	11.60 in ²
I =	62.50 in ⁴
Cm =	1.00
Pe1 =	395.43 kips
B1 =	1.02 (A-8-3)
P, All =	266.89 kips
M, All =	57.30 kip-ft

Load Combination	Pr, Kips	Mr, Kip-ft	Equation	Result
D+L	4.26	16.43	0.29	OK
D+W	1.20	20.46	0.36	OK
D+0.7E	1.20	7.31	0.13	OK
D+0.75W+0.75L	3.49	25.44	0.45	OK
D+0.75(0.7E)+0.75L	3.49	15.58	0.28	OK

Top Connection : Welded Moment	Base Plate : MODLBP8 - 30
--------------------------------	---------------------------

Spread Footing Design

From Column

P dl = 1.20 kips
 P ll = 3.06 kips
 P wl = -1.98 kips

Soil Density = 110 pcf
 Width = 6.50 ft
 Length = 6.50 ft
 Depth = 3.00 ft
 e = 2.58 ft
 a = 0.00 ft
 b = 0.00 ft
 c = 0.00 ft

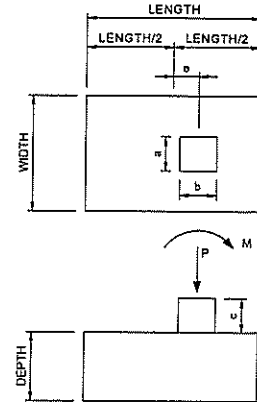
Kern = L/6 = 1.08 ft
 Footing Weight = 19.01 kips
 Soil Weight = 0.00 kips

Unbalanced Load to Column

M dl = 4.24 kip-ft
 M ll = 11.59 kip-ft
 M wl = 7.68 kip-ft

From Lateral

M wl = 8.10 kip-ft
 M el = 6.63 kip-ft



Total Loads to Spread Footing

PDL = 20.21 kips	MDL = 7.34 kip-ft
PLL = 3.06 kips	MLL = 19.50 kip-ft
PWL = -1.98 kips	MWL = 10.67 kip-ft
	MEL = 6.63 kip-ft

Load Combination	Pr, Kips	Mr, Kip-ft	ecc, ft	Soil Pressure psf
D+L	23.27	26.84	1.15	808.41
D+W	18.24	18.01	0.99	495.15
0.6D+W	10.15	15.08	1.49	259.94
0.6D+0.7E	12.13	9.05	0.75	154.70
D+0.7E	20.21	11.98	0.59	410.22
D+0.75W+0.75L	21.02	29.97	1.43	851.85
D+0.75(0.7E)+0.75L	22.51	25.45	1.13	759.20

q(ALLOW)= 1500.00 psf OK

REINFORCING:

M = 20911.77 ft-lbs/ft

Assume: f'c=3000 psi, Fy=40000 psi

d = 32.63 in

As(REQ'D)= 0.44 in²

As(PROV.)= 0.59 in² OK

USE #6's AT 9"O.C. T&B, EACH WAY

Foundation: (Restrained at Grade)

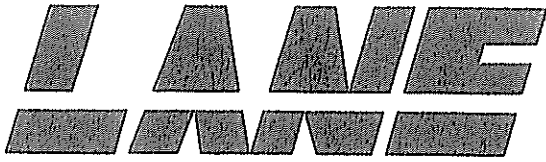
$$d^2 = (4.25 * M) / (S3 * b)$$

M(MAX)=	32897 ft-lbs	Pmax=	4.26 kips
S3=	100 PCF X d	Footing Area=	7.07 ft ²
b=	3.000 ft	Bearing=	602.48 psf
d=	7.753 ft		
Footing=	Round		
USE:	3.00 FT.RND. X	8.00 ft deep footing	

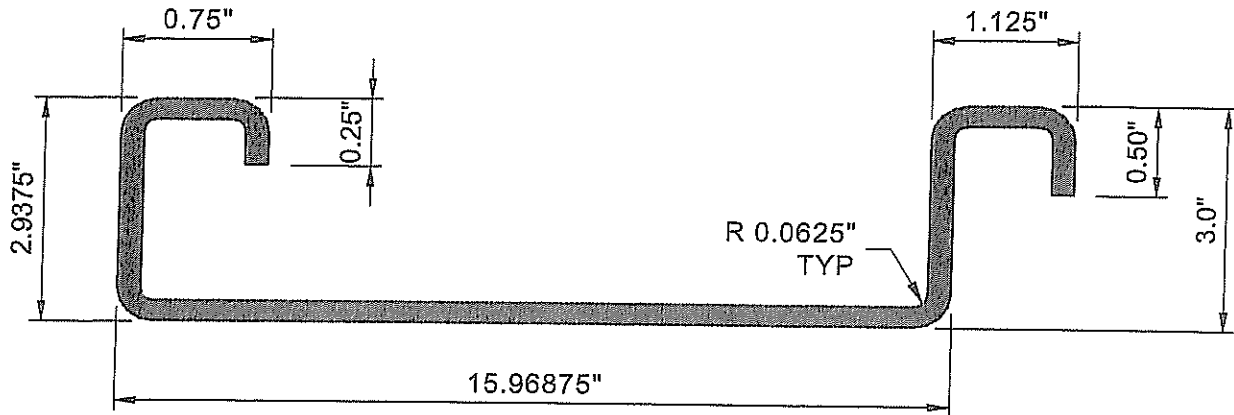
$$A_s = 12 * M / (j * d * 24000) = 0.4406 \text{ in}^2$$

USE: 10 #8's (RND. Cage) w/ #4 Ties @ 4" O.C. w/135 degree hooks In The Top
3'-0" of The Footing, #4 Ties @ 12" o.c w/ 135 Degree Hooks In The Balance of Footing

Footing design to allow offset column placement of 1'-9" from centerline of footing.



120 Fairview
Arlington, Texas 76010
817-261-9116



SL-316 DECK PANEL

Section Properties

Gage.	Wt, psf	Thickness, in	ASTM 653	+I, in ⁴	-I, in ⁴	+S, in ³	-S, in ³	+M, ft-lbs/ft	-M, ft-lbs/ft
20	2.20	0.0359	Grade 40	0.9346	0.4680	0.3961	0.3036	592.70	454.44
			Grade 50	0.9208	0.4522	0.3879	0.2880	725.86	538.92
18	2.93	0.0478	Grade 40	1.2486	0.6827	0.5329	0.4377	797.77	655.28
			Grade 50	1.2129	0.6518	0.5141	0.4296	962.09	803.92

Notes:

- 1 Designed per AISI Cold Formed Steel Manual, 2007 ed.
- 2 Complete calculations are available upon request.
- 3 ± M is allowable bending moment.

Issued 12-5-14

Cap Plate Moment Capacity

Wide Flange Beam Over Tube Column

Design Moment = 18.70 Kip-Ft

Assumes only Moment Loading

BOLT
 Material: A325
 Diameter: 0.88 in.
 Tensile Capacity: 26.50 kips

PLATE
 Fy = 36.00 ksi
 t = 2.25 in.
 a = 1.50 in.
 b = 1.50 in.

COLUMN: TS8X6X1/2
 Fy = 46.00 ksi
 w = 6.00 in.
 tw = 0.29 in.

BEAM: W6X20
 Fy = 36.00 ksi
 bf = 6.02 in.
 tf = 0.37 in.
 tw = 0.26 in.
 gage = 3.50 in.

Bending w/Prying on the Flange of the WF

B = 26.50 kips/Bolt
 T = 14.96 kips/Bolt
 b = 1.62 in.
 a = 1.26 in.
 b' = 1.18 in.
 a' = 1.70 in.
 p = 3.00 in.
 d' = 0.94 in.
 delta = 0.69 in.
 rho = 0.70 in.
 Beta = 1.11
 alpha' = 1.00
 t'reqd = 0.88 in.

**Use 3/8" Stiffeners In Header
 W/ 5/16" Fillet Welds**

STIFFENERS REQ'D IN BEAM

Bending w/Prying on the Cap Plate

B = 26.50 kips/Bolt
 T = 14.96 kips/Bolt
 b = 1.50 in.
 a = 1.50 in.
 b' = 1.35 in.
 a' = 1.94 in.
 p = 1.75 in.
 d' = 0.94 in.
 delta = 0.46 in.
 rho = 0.70 in.
 Beta = 1.10
 alpha' = 1.00
 t'reqd = 1.33 in.

Cap Plate Moment Capacity

Wide Flange Beam Over Tube Column

Design Moment = 18.70 Kip-Ft

Assumes only Moment Loading

BOLT
 Material: A325
 Diameter: 0.88 in.
 Tensile Capacity: 26.50 kips

PLATE
 Fy = 36.00 ksi
 t = 2.25 in.
 a = 1.50 in.
 b = 1.50 in.

COLUMN: TS8X6X1/2
 Fy = 46.00 ksi
 w = 6.00 in.
 tw = 0.29 in.

BEAM: W8X21
 Fy = 36.00 ksi
 bf = 5.27 in.
 tf = 0.40 in.
 tw = 0.25 in.
 gage = 2.75 in.

Bending w/Prying on the Flange of the WF

B = 26.50 kips/Bolt
 T = 14.96 kips/Bolt
 b = 1.25 in.
 a = 1.26 in.
 b' = 0.81 in.
 a' = 1.70 in.
 p = 3.00 in.
 d' = 0.94 in.
 delta = 0.69 in.
 rho = 0.48 in.
 Beta = 1.61
 alpha' = 1.00
 t'reqd = 0.73 in.

**Use 3/8" Stiffeners In Header
 W/ 5/16" Fillet Welds**

STIFFENERS REQ'D IN BEAM

Bending w/Prying on the Cap Plate

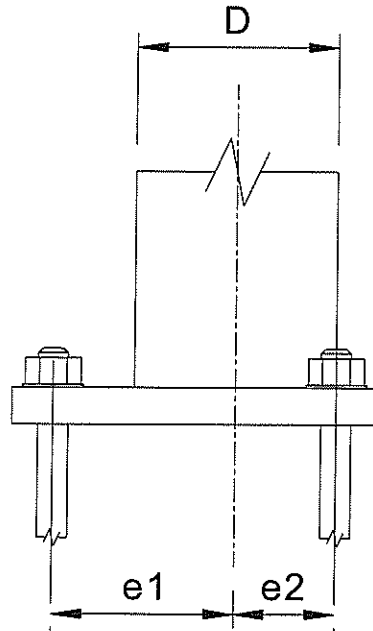
B = 26.50 kips/Bolt
 T = 14.96 kips/Bolt
 b = 1.50 in.
 a = 1.50 in.
 b' = 1.35 in.
 a' = 1.94 in.
 p = 1.38 in.
 d' = 0.94 in.
 delta = 0.32 in.
 rho = 0.70 in.
 Beta = 1.10
 alpha' = 1.00
 t'reqd = 1.58 in.

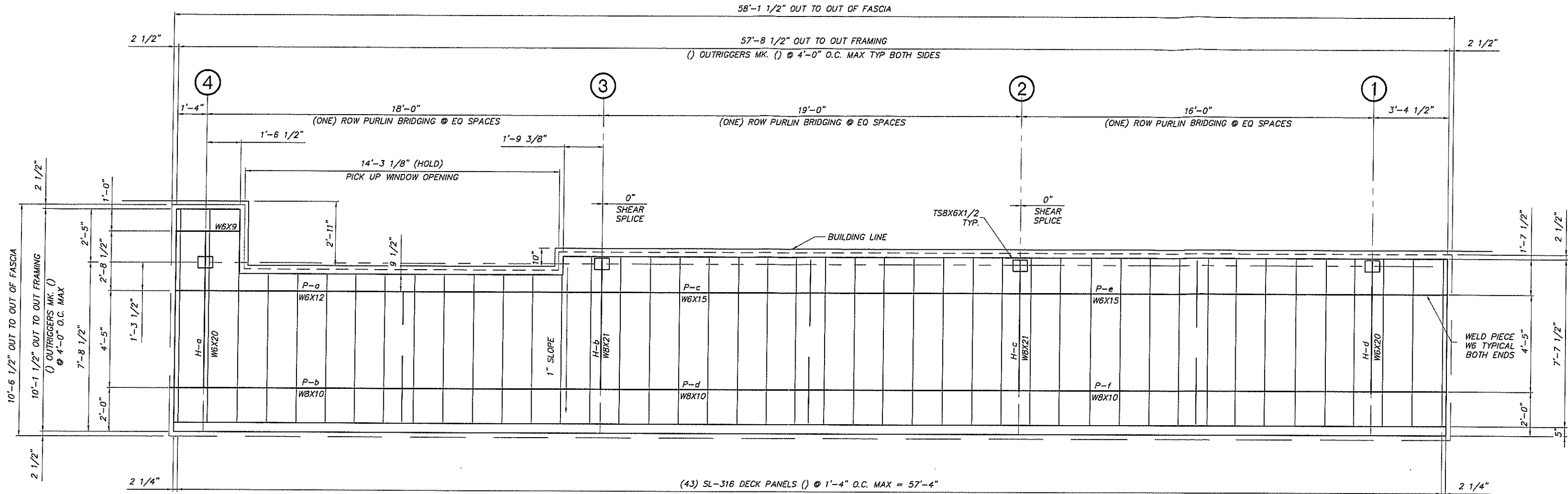
Moment = 30.00 kip-ft
Column = TS8X6X1/2
D = 6 in.
e1 = 9 in.
e2 = 3 in.
Anchor Bolts = 1 1/2 in
t plate = 2.00 in

A36 Steel Plate Fy = 36 ksi
E70 Electrode Fw = 0.928 kips / in / 16th
A307 Anchor Bolts Ft = 20 ksi

Clockwise Moment
Pbolt = 12.86 kips < 35.3, OK
t(req'd) = 1.69 in
t(actual) = 2.00 in
Weld(req'd) = 4.62 /16th's
Weld(actual) = F.P. /16th's

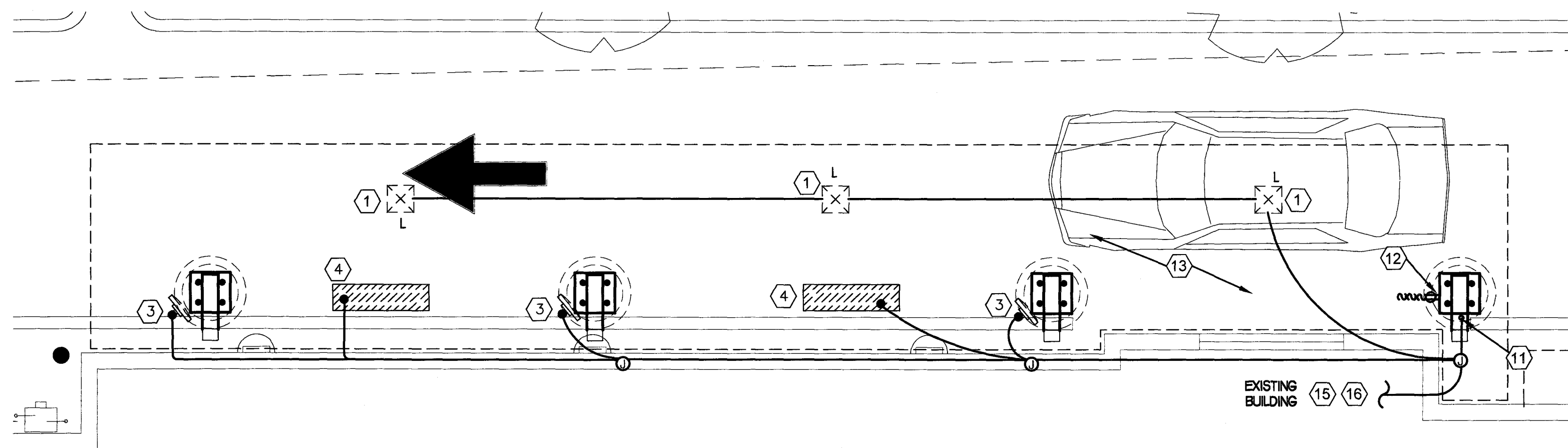
Counter-Clockwise Moment
Pbolt = 22.50 kips < 35.3, OK



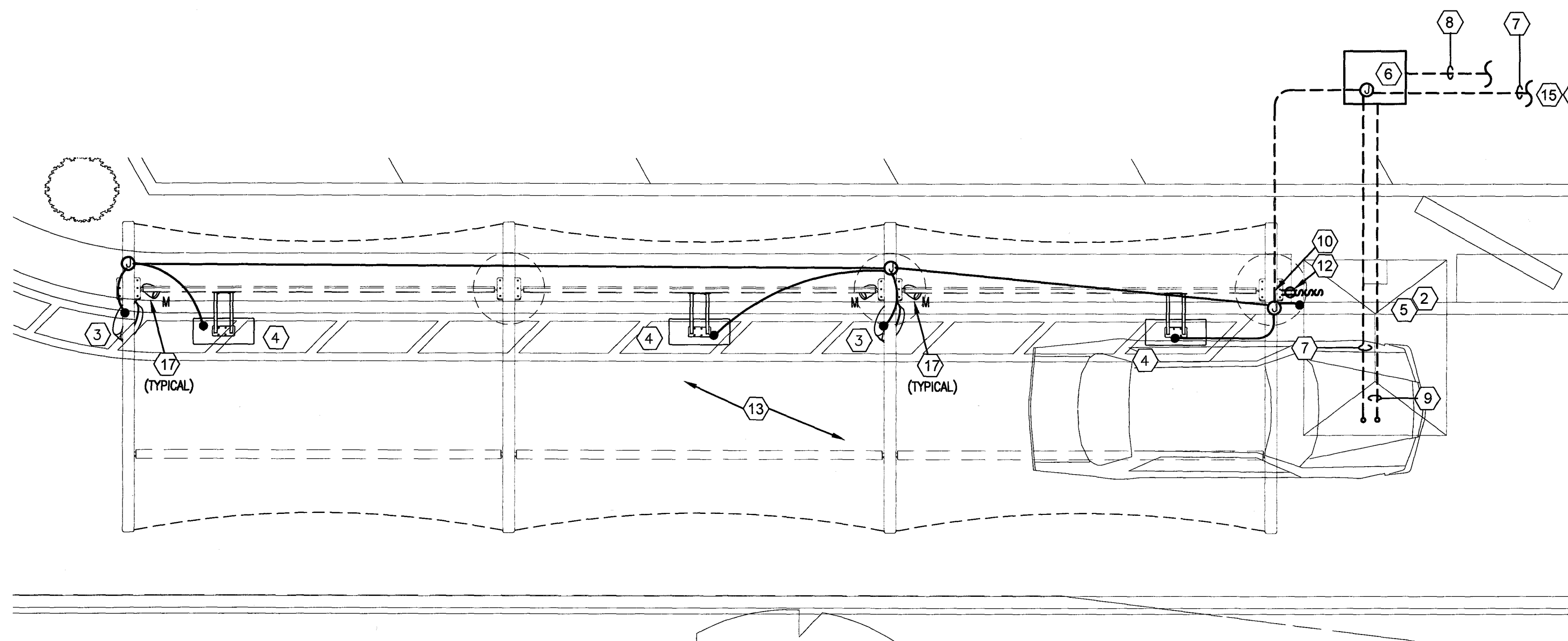


DESIGN LOADS:

DEAD LOAD = 3 p.s.f.(DECK + LIGHTS) + WEIGHT OF STRUCTURAL COMPONENTS
 LIVE LOAD = 20 p.s.f.
 SNOW LOAD = 20 p.s.f.
 V, ULT = 116 m.p.h. EXP. C
 V, ASD = 90 m.p.h. EXP. C
 BLDG CODE = 2015 INTERNATIONAL BUILDING CODE
 EQUIVALENT LATERAL FORCE PROCEDURE
 LATERAL FORCE RESISTING SYSTEM = CANTILEVERED COLUMN SYSTEM—ORDINARY STEEL MOMENT FRAME
 Pf = 20 p.s.f.
 Ce = 1.2
 Ct = 1.2
 Is = 1.0
 W = DRIFT LOADS NOT CONSIDERED
 Pd = DRIFT LOADS NOT CONSIDERED
 SITE CLASS = D
 Ss (0.2) = 0.44
 S1 (1.0) = 0.13
 SDS = 0.43
 SD1 = 0.20
 Fa = 1.45
 Fv = 2.27
 R = 1.25
 SEISMIC IMPORTANCE FACTOR = 1.0
 RISK CATEGORY = II
 SEISMIC DESIGN CATEGORY = D
 CS = 0.340
 CONSTRUCTION TYPE = IIB
 OCCUPANCY CATEGORY = M
 TOTAL SEISMIC BASE SHEAR BOTH DIRECTIONS = 1.38 KIPS



3 MEAL DELIVERY CANOPY POWER PLAN
SCALE: 1/4" = 1'-0"



2 ORDER CANOPY POWER PLAN
SCALE: 1/4" = 1'-0"

LEGEND	
---	UNDERGROUND
---	ABOVE GROUND

KEYED NOTES

- 1 CEILING LIGHT PROVIDED BY CANOPY SUPPLIER AND INSTALLED BY E.C.
- 2 EXISTING OPEN/ENCLOSED LIGHTS ON ORDER CANOPY.
- 3 AIR CIRCULATING FAN (WITH INTEGRAL ON-OFF SWITCH) PROVIDED BY OTHERS. FANS TO BE HARDWIRED IN APPROPRIATE CONDUIT ABOVE CANOPY
- 4 INFRARED GAS HEATER WITH INTEGRAL ON-OFF SWITCH PROVIDED BY OTHERS.
- 5 EXISTING MENUBOARD WITH CANOPY.
- 6 EXISTING IN-GROUND QUAZITE PULLBOX FOR SLOP DATA CABLES WITH POWER NEMA 3R JUNCTION BOX MOUNTED INSIDE THE PULLBOX.
- 7 EXISTING 2" UNDERGROUND SCH40 PVC CONDUIT WITH POWER CONDUCTORS, SEE WIRING SCHEMATIC.
- 8 EXISTING 2" UNDERGROUND SCH40 PVC CONDUIT FOR AUDIO SYSTEM/DETECTOR LOOP CABLES.
- 9 EXISTING 1" EMPTY UNDERGROUND SCH40 PVC CONDUIT FOR AUDIO SYSTEM/DETECTOR LOOP CABLES.
- 10 INSTALL UNDERGROUND 3" SCH40 PVC CONDUIT UP INTO THE CANOPY COLUMN WITH TYPE MC CABLE (GALVANIZED STEEL WITH PVC JACKET) RUN WITHIN FOR THE 120V POWER FOR LIGHTS, 120 VOLT POWER FOR FANS, AND 24 VOLT POWER FOR THE INFRARED GAS HEATERS.
- 11 AT EXISTING BUILDINGS STUB A 3" CHASE THRU THE EXTERIOR WALL FROM THE CEILING SPACE ABOVE THE KITCHEN TO ABOVE THE CANOPY'S COLUMN FOR THE MC CABLE POWER CIRCUITS TO GO THRU THE COLUMN MOUNTED SWITCHES AND OUTLET.
- 12 PROVIDE ONE DUPLEX GFCI (WITH IN-USE WP COVER PLATE) AND THREE 120V SINGLE-POLE SWITCHES (EACH WITH HUBBELL #RW51550 WP COVER PLATE) MOUNTED ON THE COLUMN IN FLUSH MOUNTED METAL SINGLE GANG BOXES FOR LOCAL ON-OFF CONTROL OF THE FAN, HEATERS, AND CANOPY LIGHTS. SEE WIRING SCHEMATIC AND CANOPY COLUMN DETAILS FOR FURTHER INFORMATION. ALL SURFACE MOUNTED ITEMS AND COVER PLATES TO BE FIELD PAINTED MATTE BLACK. INSTALL BLANK COVERPLATE WHEN HEATERS ARE NOT INSTALLED AND THE THIRD SWITCH IS NOT REQUIRED.
- 13 ALL CONDUIT AND BOXES SHALL BE CONCEALED FROM NORMAL VIEW, UNDERGROUND, IN COLUMNS, OR ABOVE THE CANOPY (ON THE ROOF). MC CABLE (GALVANIZED STEEL WITH PVC JACKET) TO BE USED INSIDE THE COLUMNS, BUT MUST CONVERT BACK TO IMC ABOVE THE ROOF. ALL EXPOSED ELECTRICAL BOXES TO BE NEMA 3R CAST-METAL.

- 14 NOT USED
- 15 PROVIDE ONE (1) 20A/1P CIRCUIT FOR LIGHTS AND ONE (1) 20A/1P CIRCUIT FOR FANS. CONNECT TO NEXT AVAILABLE SPARE/SPACE IN LIGHTING PANEL FOR EACH. CONTRACTOR SHALL PROVIDE APPROPRIATE BREAKER SIZE AND LOAD. LIGHTING LOAD ON CIRCUIT NOT TO EXCEED 1.8 KW. FIELD VERIFY NUMBER OF SPARES/SPACES PRIOR TO BID. REPORT ANY DISCREPANCIES TO ENGINEER PRIOR TO CONSTRUCTION.
- 16 PROVIDE ONE (1) #8 CU EQUIPMENT GROUND TO BE BONDED TO CANOPY STRUCTURE AND GAS PIPING PER MANUFACTURER'S RECOMMENDATIONS.
- 17 COLUMN MOUNTED SINGLE LIGHT PROVIDED BY OTHERS/GC AND INSTALL BY E.C.

NOTES: FOR ALL CONDUITS, REFER TO PLANS FOR OTHER CONDUITS, REFER TO VENDOR DRAWINGS FOR CONDUIT AND WIRING REQUIREMENTS FOR LOW VOLTAGE SYSTEMS AND CONTROL WIRING.

MULTIPLE 1PH CIRCUITS MAY OCCUPY THE SAME CONDUIT IN ACCORDANCE WITH THE NEC. MAXIMUM OF THREE AND OF DIFFERENT PHASES.

LOW VOLTAGE AND CONTROL WIRING SHALL BE IN SEPARATE CONDUIT FROM BUILDING TO CANOPY.

1 LIGHTING FIXTURE (LUMINAIRE) SCHEDULE - CHCK-FL-A

MARK	MANUFACTURER	CATALOG NUMBER	NO. LAMPS/TYPE	STY. LAMP NO.	WATTS	VOLTS	MOUNTING	REMARKS
M	ACCUSERV	HBLD28YB4.0/R14-3B	LED	-	26	120	MOUNTED ON COLUMN	REFER TO NOTE #2 IN THIS SCHEDULE
L	INDUSTRIES	LSI CRUS_SM-SC-LED-LW-30-CW-UE-WHT	LED	-	74	120	MOUNTED ON CANOPY	

NOTES:
1. LUMINAIRES UTILIZING DOUBLE-ENDED LAMPS AND CONTAIN BALLASTS THAT CAN BE SERVICED IN PLACE SHALL HAVE A DISCONNECTING MEANS EITHER INTERNAL OR EXTERNAL TO EACH LUMINAIRE PER NEC 410.130(G).
2. THE LIGHTING FIXTURE PACKAGE IS AVAILABLE THROUGH A NATIONAL ACCOUNT PROGRAM.

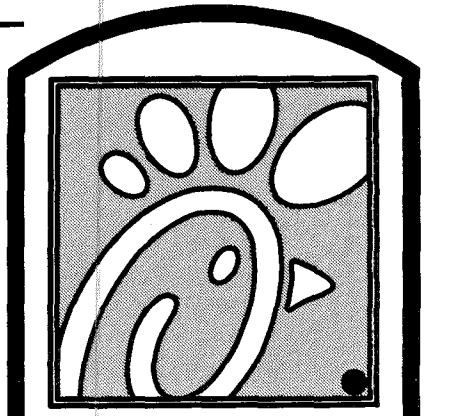
FIELD VERIFY ALL CONDITIONS

NOTE! AS NOTED IN THE SPECIFICATIONS, ALL WIRING LAYOUTS, LAYOUTS ARE SCHEMATIC. EXACT LOCATIONS SHALL BE DETERMINED BY THE CONSTRUCTION AND STRUCTURE OF THE BUILDING AND SHALL BE VERIFIED AND COORDINATED IN THE FIELD. EACH TRADE CONTRACTOR SHALL VERIFY WITH THE GENERAL CONTRACTOR THAT HE HAS THOROUGHLY REVIEWED AND COORDINATED ALL LOCATIONS AND ROUTINGS WITH ALL OTHER TRADES PRIOR TO FABRICATION OF CONDUITS, DUCTS, OR PIPING, AND START OF INSTALLATION OF SAME (INCLUDING SPRINKLER PIPING WHEN PRESENT ON JOB). ANY INSTALLATION OR CONSTRUCTION CONFLICTS WHICH OCCUR IN THE FIELD SHALL BE RESOLVED BY THE TRADE CONTRACTOR TO THE SATISFACTION OF THE OWNER AND ARCHITECT AND AT NO EXPENSE TO THE OWNER, ARCHITECT AND/OR GENERAL CONTRACTOR.

THE CONTRACTOR SHALL CONTACT THE ARCHITECT, ENGINEER OR OWNER PRIOR TO BIDDING FOR INTERPRETATIONS AND CLARIFICATIONS OF THE DESIGN AND INCLUDE IN HIS BID ALL COSTS TO MEET THE DESIGN INTENT. CLARIFICATIONS MADE BY THE ARCHITECT, ENGINEER OR OWNER AFTER BIDDING WILL BE FINAL AND SHALL BE IMPLEMENTED AT CONTRACTORS COST.

BIDDING CONTRACTORS SHALL HAVE A WORKING KNOWLEDGE OF LOCAL CODES AND ORDINANCES AND SHALL INCLUDE IN THEIR BIDS THE COSTS FOR ALL WORK INSTALLED IN STRICT ACCORDANCE WITH GOVERNING CODES. THE PLANS AND SPECIFICATIONS NOT WITHSTANDING, THE CONTRACTOR SHALL ALERT ARCHITECT, ENGINEER OR OWNER OF ANY APPARENT DISCREPANCIES BETWEEN GOVERNING CODES AND DESIGN INTENT.

COMPARISON LOAD SUMMARY			
EXISTING LOAD BASED ON DATA RECEIVED			
			899.77 AMPS
	QTY.	WATTS	AMPS
-DRIVE THRU CANOPY LIGHTS	3	X 26	0.2 AMPS
-MEAL DELIVERY CANOPY LIGHTS	3	X 74	0.6 AMPS
-DRIVE THRU CANOPY FANS	2	X 264	1.5 AMPS
-MEAL DELIVERY CANOPY FANS	3	X 264	2.2 AMPS
TOTAL ADDED AMPS			4.5 AMPS
NEW CONNECTED AMPS			904.3 AMPS
EXISTING SERVICE SIZE IS 1000 AMPS BASED ON EXISTING DATA RECEIVED			

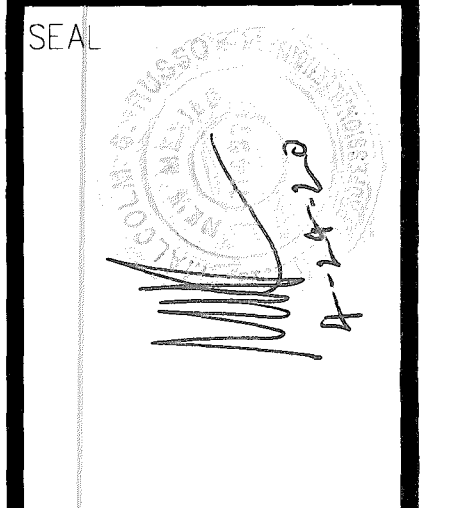


Chick-FL-A

5200 Buffington Rd.
Atlanta Georgia,
30349-2998

Revisions:
Mark Date By

△			
△			
△			
△			



INTERPLANE
ARCHITECTURE
ENGINEERING
INTERIOR DESIGN
PROJECT MANAGEMENT

604 COURTLAND STREET SUITE 100
ORLANDO, FLORIDA 32804
PH 407.645.5008
FX 407.659.9124

STORE
CANOPY ROLL OUT
PROGRAM FSU 2793

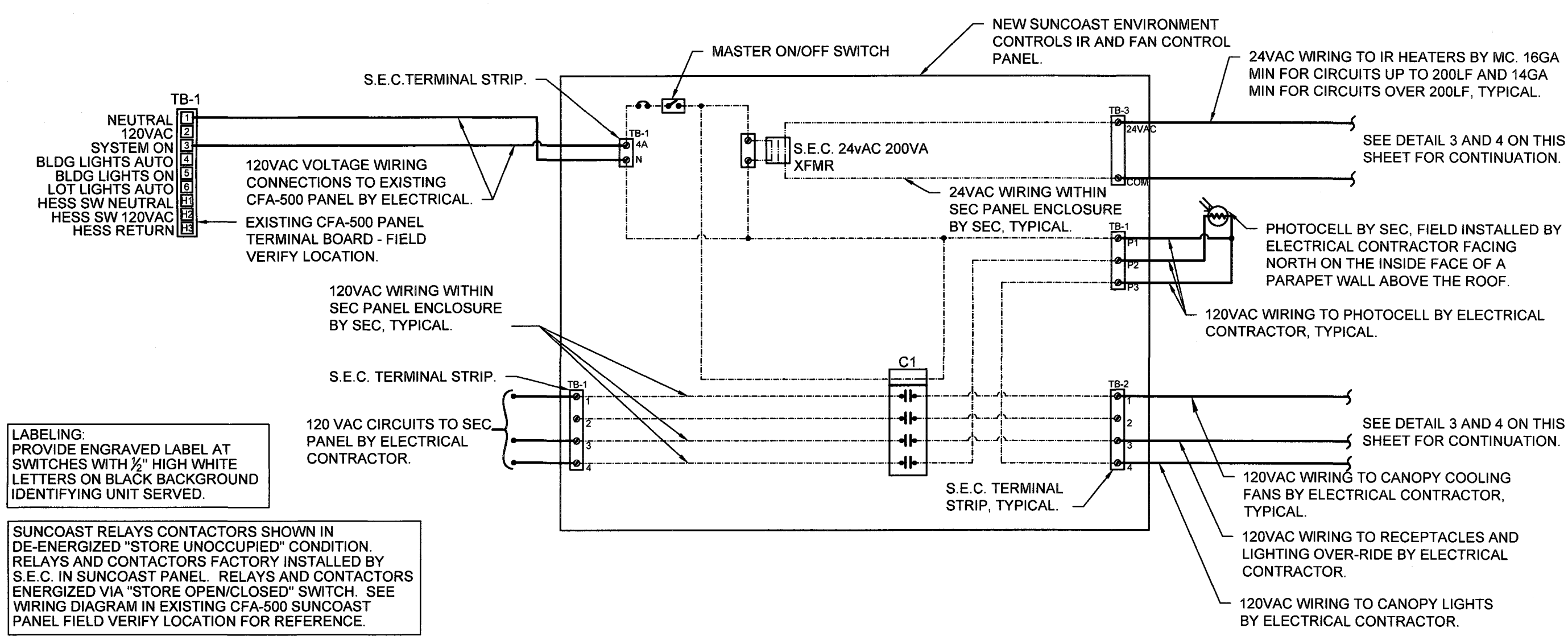
5009 MONTGOMERY
BLVD NE,
ALBUQUERQUE, NM
87109

SHEET TITLE
**CANOPY POWER
AND LGT PLAN**

VERSION: V7.025
ISSUE DATE: 04-2020

Job No. : 20.0388
Store : 2793
Date : 04.2020
Drawn By : WA2
Checked By : SN





SEQUENCE OF OPERATION

STORE SWITCH IN "STORE OPEN" POSITION STORE SWITCH IN "STORE CLOSED" POSITION
 A. INFRARED HEATERS ARE ENABLED. A. INFRARED HEATERS ARE DISABLED.
 B. COOLING FANS ARE ENABLED. B. COOLING FANS ARE DISABLED.
 C. MASTER ON/OFF SWITCH FOR IR HEATERS AND COOLING FANS PROVIDE SINGLE POINT OF ON/OFF CONTROL.

NOTES

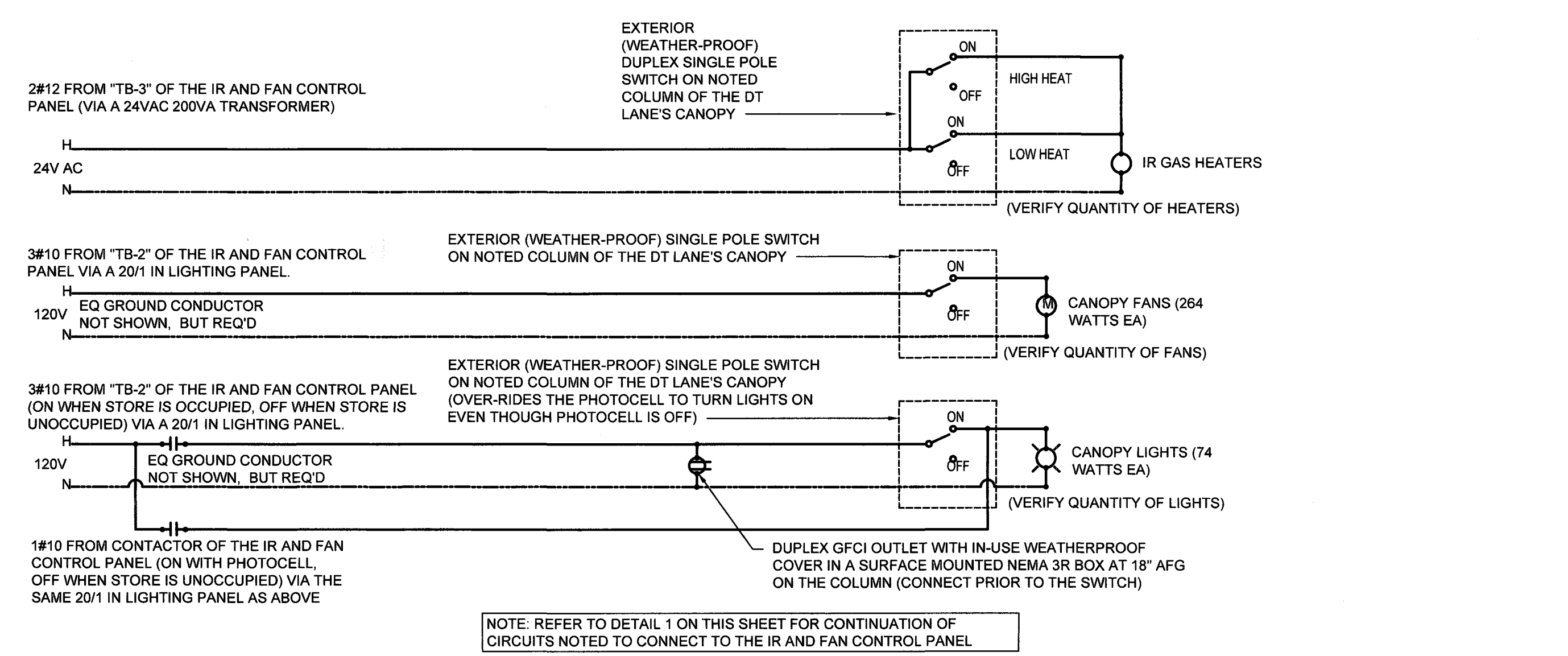
- CONTRACTOR SHALL PURCHASE CONTROL PANEL DIRECT FROM SUNCOAST ENVIRONMENTAL CONTROLS. TELE NO 727-544-6679.
- COORDINATE WITH GC TO ESTABLISH LOCATION TO MOUNT PANEL IN A CONDITIONED SPACE INSIDE THE BUILDING.
- INFORM SEC ASAP AS TO WHETHER THE PANEL IS TO BE SURFACE OR FLUSH MOUNTED.
- PROVIDE LAMINATED LEGEND SHOWING NAMED LOCATIONS OF FAN AND IR HEATERS. MOUNT LEGEND AT PANEL.

LEGEND

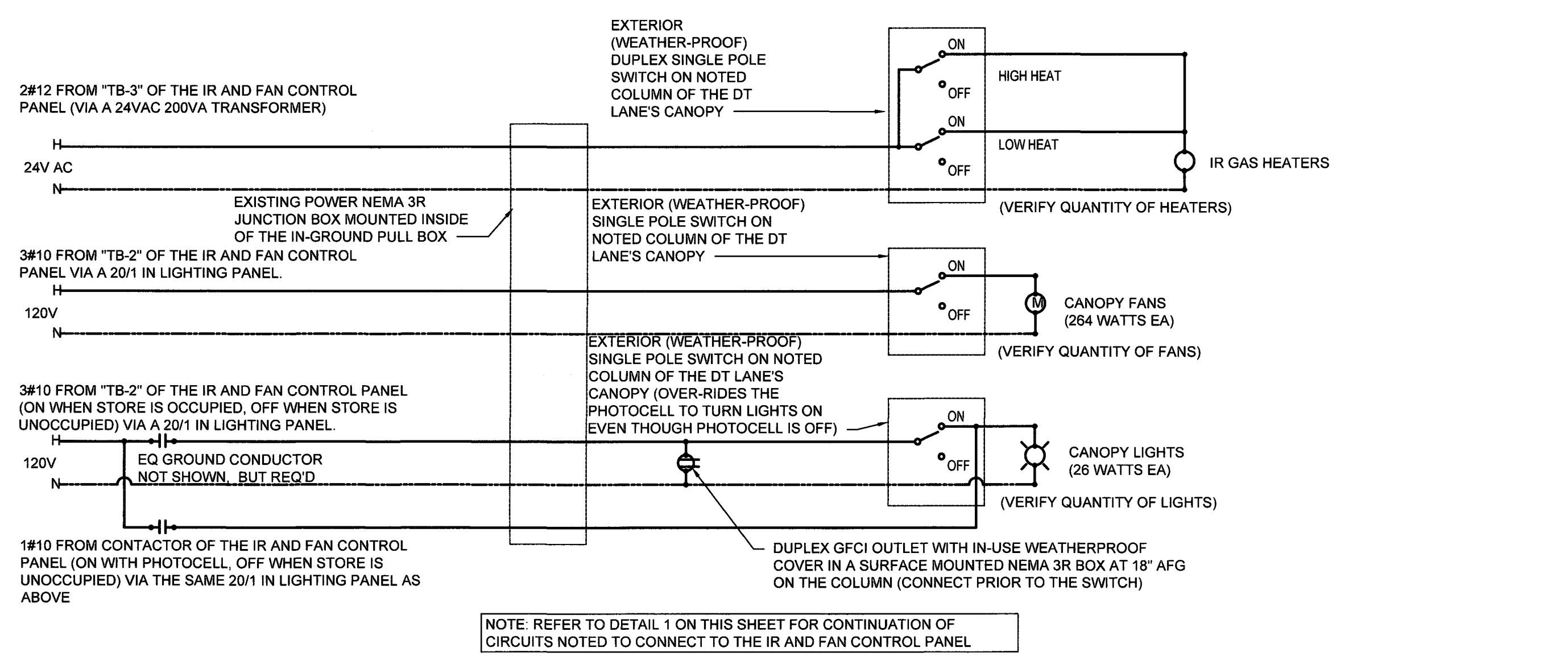
S E C	SUNCOAST ENVIRONMENTAL CONTROLS (SUPPLIER OF TEMP/FAN CONTROL PANELS)
MC	MECHANICAL CONTRACTOR
EC	ELECTRICAL CONTRACTOR
---	LOW VOLTAGE BY MECHANICAL CONTRACTOR
---	18 GA. MIN LOW VOLTAGE WIRING BY SUNCOAST
---	HIGH VOLTAGE BY ELECTRICAL CONTRACTOR
---	HIGH VOLTAGE BY SUNCOAST

IMPORTANT:
 CHANGE OUT 2 AMP CIRCUIT BREAKER IN THE EXISTING CFA-500 S.E.C. PANEL FIELD VERIFY LOCATION TO A 6 AMP CB. THE FUSE IS LOCATED IN THE UPPER HALF OF THE CFA-500 PANEL IMMEDIATELY TO THE RIGHT OF THE PHOTOCELL CONTROLLER. FAILURE TO DO SO WILL CAUSE CONTROL CIRCUIT TO BLOW FUSE.

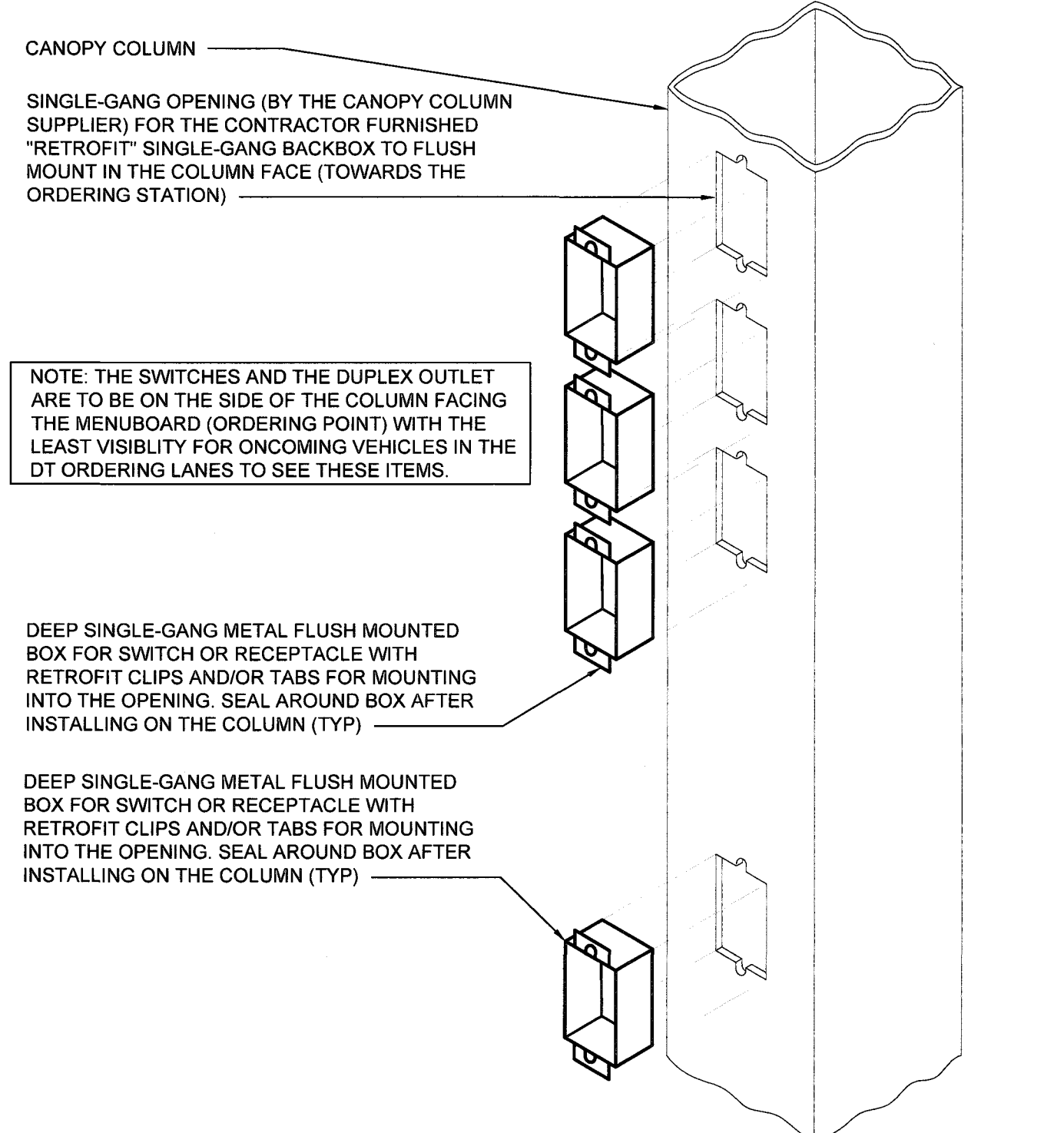
1 SUNCOAST PANEL WIRING DIAGRAM - INFRARED HEATERS, COOLING FANS
 SCALE: NTS



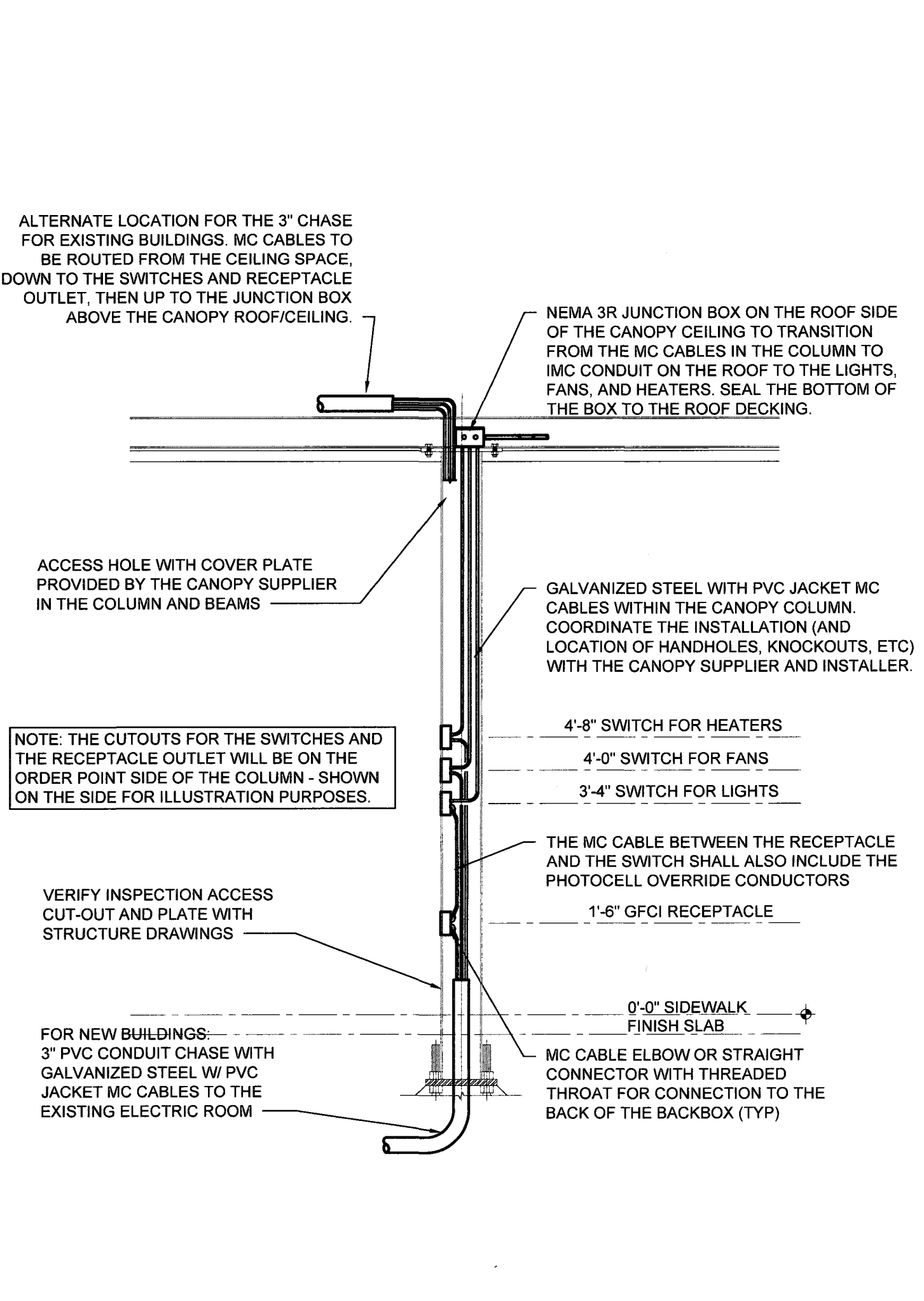
3 MEAL DELIVERY CANOPY POWER WIRING SCHEMATIC
 SCALE: NTS



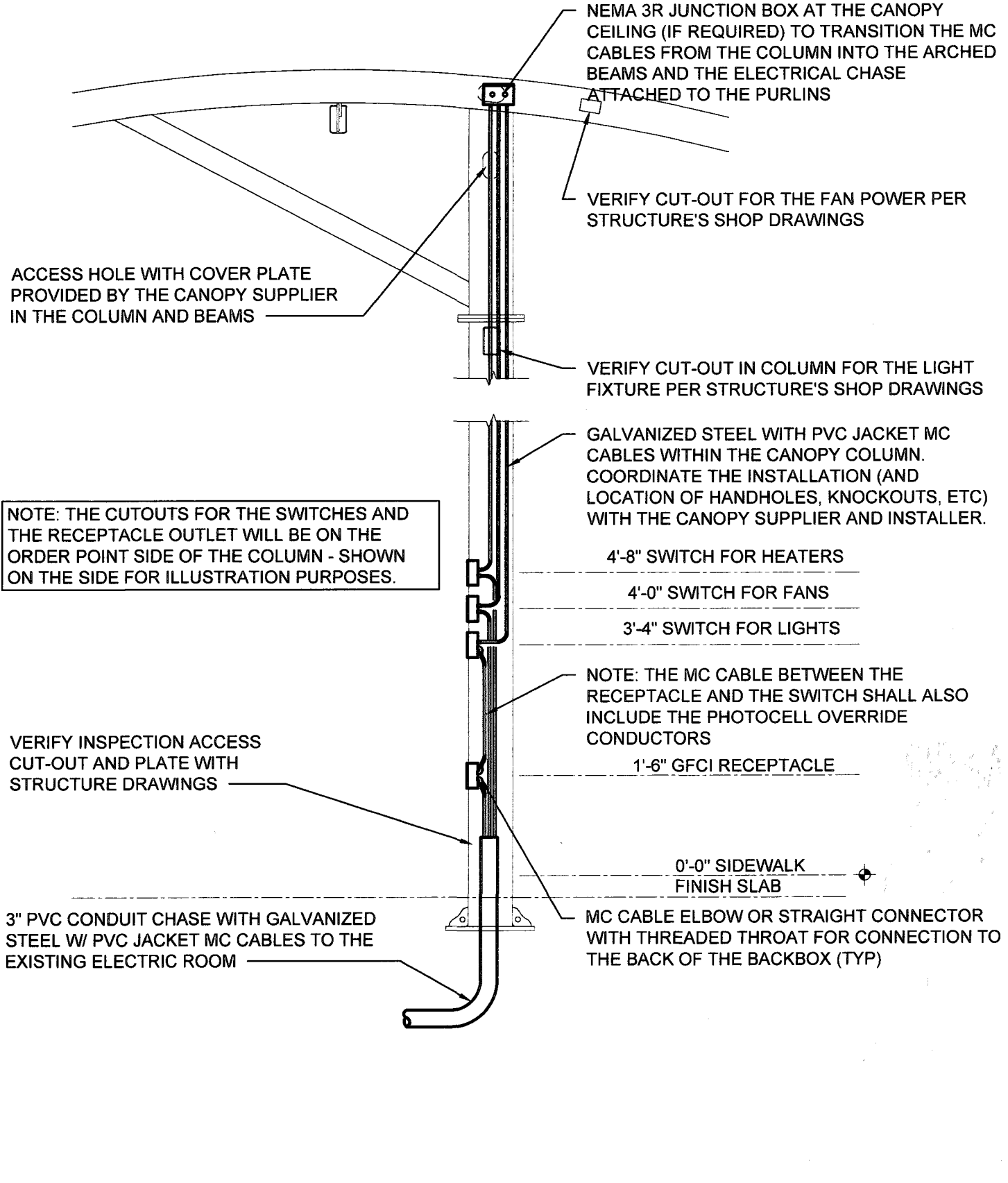
4 DUAL LANE ORDER CANOPY POWER WIRING SCHEMATIC
 SCALE: NTS



5 CANOPY COLUMN ISOMETRIC
 SCALE: NTS



2 SECTION - MEAL DELIVERY CANOPY COLUMN
 SCALE: NTS



6 SECTION - ORDER CANOPY COLUMN
 SCALE: NTS

5200 Buffington Rd.
 Atlanta Georgia,
 30349-2998

Revisions:

Mark	Date	By
△		
△		
△		
△		

INTERPLAN
 ARCHITECTURE
 ENGINEERING
 INTERIOR DESIGN
 PROJECT MANAGEMENT

404 COURTLAND STREET
 SUITE 100
 ORLANDO, FLORIDA 32804
 PH 407.645.5008
 FX 407.629.9124

STORE
CANOPY ROLL OUT PROGRAM FSU 2793

5009 MONTGOMERY BLVD NE,
 ALBUQUERQUE, NM 87109

SHEET TITLE
CANOPY ELEC. DETAILS

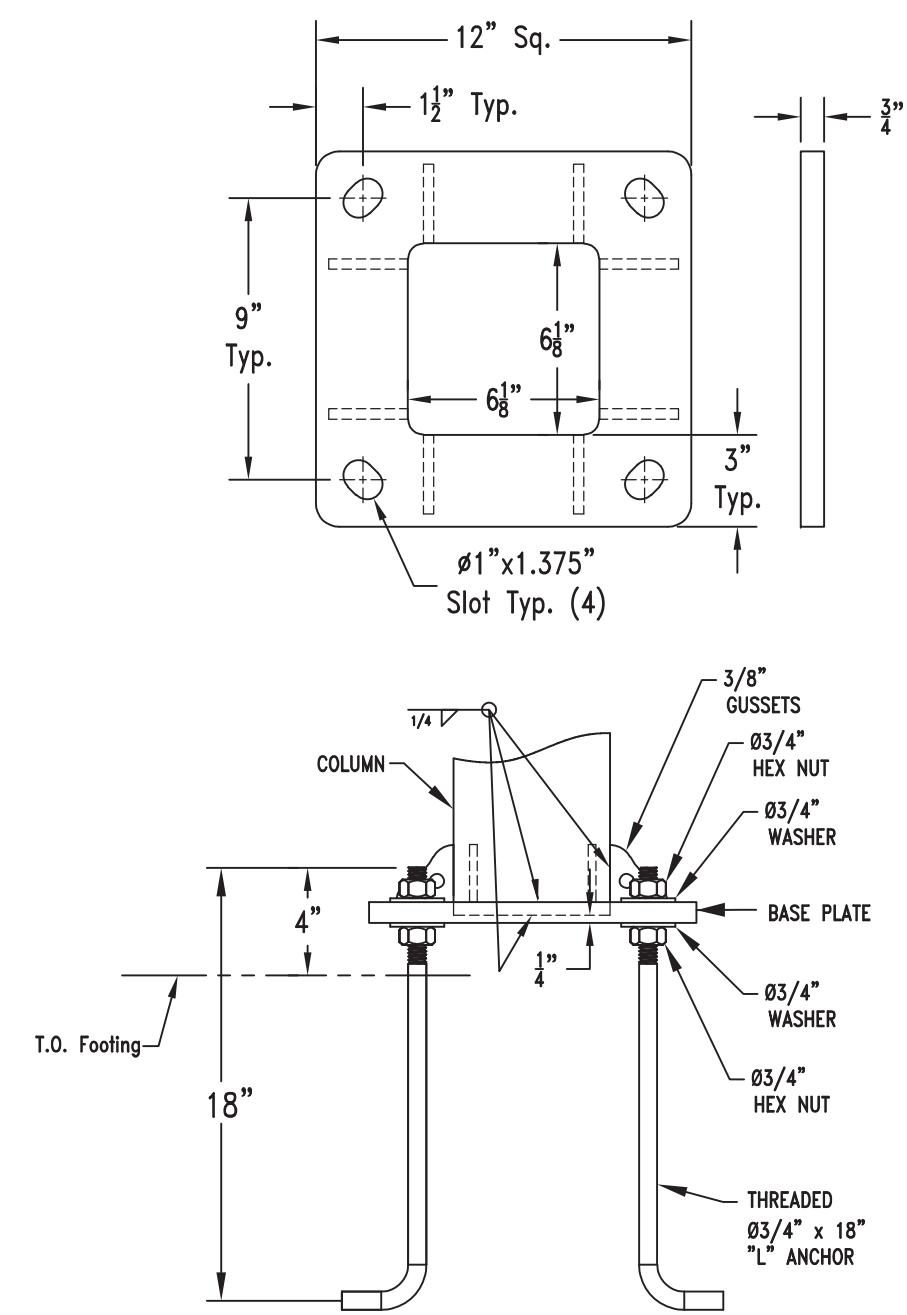
VERSION: V7.025
 ISSUE DATE: 04-2020

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 Store : 2793
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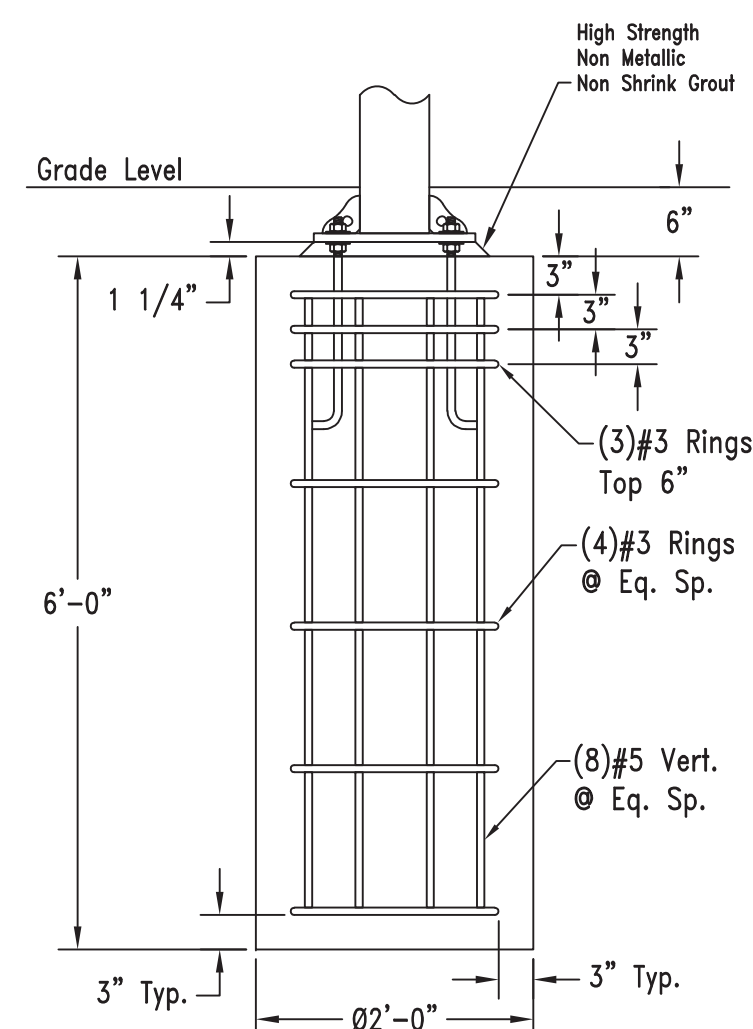
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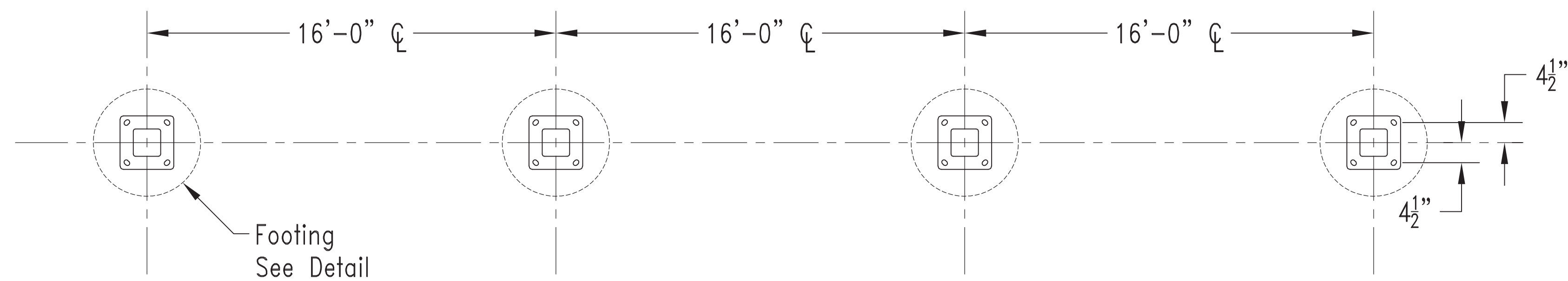
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12 BASE PLATE DETAILS
2 ANCHOR DETAILS



13 AUGER FOOTING DETAIL
2



14 BASE PLATE/FOOTING LAYOUT
2

SEAL:



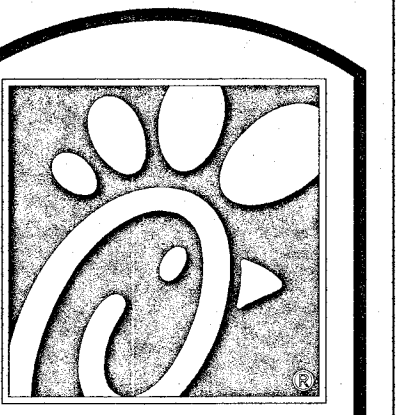
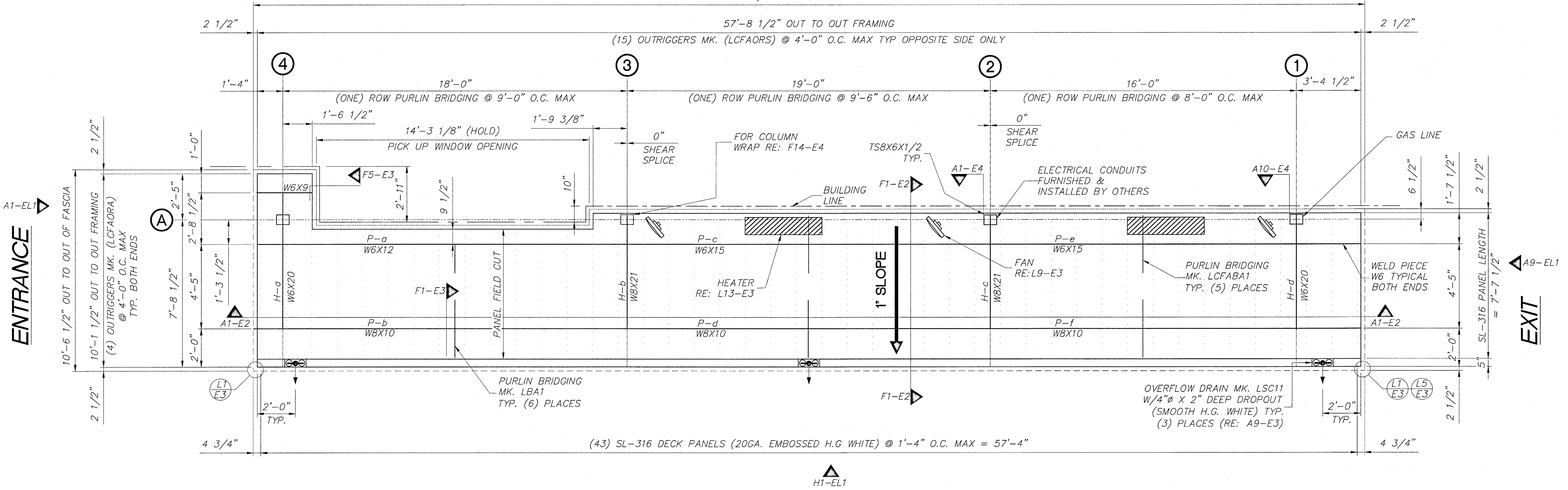
Revisions:
Date: By:

Drawn: K. Keithley
Date: 5/6/2020
Chkd:
Date:

#203113

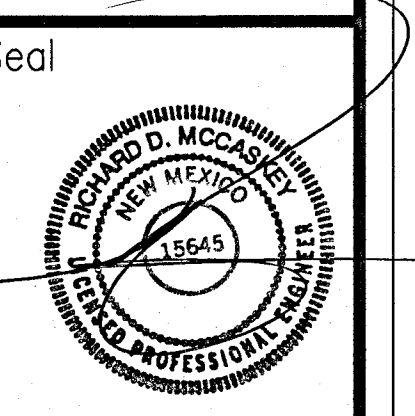
BUILDING

58'-1 1/2" OUT TO OUT OF FASCIA



5200 Buffington Rd.
Atlanta Georgia,
30349-2998

Revisions:
Mark Date By



APR 20 2020

LANE
SUPPLY, INC.
120 FAIRVIEW
ARLINGTON, TX. 76010
(817) 261-9116

FI CANOPY FRAMING LAYOUT

STRUCTURAL STEEL SHALL MEET THE AISC 2011 SPECIFICATION 14TH EDITION AND THE AISC CODE OF STANDARD PRACTICE, 2011
COLUMNS TO BE ASTM A500, GRADE B
BOLTS TO BE ASTM A325 OR ASTM F1852 (A325-TC)
INSTALLATION OF BOLTS SHALL BE BROUGHT TO A SNUG TIGHT CONDITION-DEFINED AS THE CONDITION THAT EXISTS WHEN ALL OF THE PLIES IN A CONNECTION HAVE BEEN PULLED INTO FIRM CONTACT BY THE BOLTS IN THE JOINT AND ALL OF THE BOLTS IN THE JOINT HAVE BEEN TIGHTENED SUFFICIENTLY TO PREVENT THE REMOVAL OF THE NUTS WITHOUT THE USE OF A WRENCH.

WIDE FLANGE BEAMS TO BE ASTM A992
ANGLES, & PLATES TO BE ASTM A36
REINFORCING STEEL TO BE ASTM 615, GRADE 60
DECK PANELS TO BE ASTM 653, GRADE C MINIMUM
WELDING SHALL MEET THE REQUIREMENTS OF THE AWS FOR BUILDING CONSTRUCTION USING E70XX ELECTRODES
ALL STRUCTURAL STEEL TO BE PAINTED WITH ONE SHOP COAT PRIMER
CANOPY FABRICATOR SHALL BE AISC CERTIFIED
LANE SUPPLY INC. IS AN AISC CERTIFIED FABRICATOR (AISC #208101101-02R1T)

1. REFERENCE SEALANT SCHEDULE FOR ALL APPLICATIONS
2. SEAL ALL JOINTS WITH A SMOOTH, CLEAN APPLICATION
3. APPLY CAULK CLEAR AROUND THE COLUMNS ON THE TOP SIDE OF THE DECK AFTER BOTTOM SIDE HAS BEEN CAULKED.
4. DECK PANELS AND TRIM WILL BE WIPED CLEAN AFTER INSTALLATION
5. ALL TRASH AND EXTRA MATERIALS WILL BE HAULED OFF JOBSITE
6. CHECK WITH GENERAL CONTRACTOR FOR DRAIN ORIENTATION
7. FURNISH & INSTALL LANE DESIGNED AND ENGINEERED "HUNG" DECK
8. FURNISH & INSTALL SUPPORT FRAMING FOR (2) FANS & (2) HEATERS (FANS & HEATERS FURNISHED & INSTALLED BY OTHERS.)
9. FURNISH & INSTALL (3) LSI CRUS-CS-LED-LW-30-UE-WHT CANOPY DECK LIGHTS.
10. FURNISH & INSTALL SHEET METAL FASCIA "CHICK-FIL-A BRONZE".

SEALANT SCHEDULE		
SEALANT	COLOR	APPLICATION
SOUDEASEAL FC	WHITE	GUTTER TO DECK
SOUDEASEAL FC	WHITE	DECK TO COLUMN @ BOTTOM
SOUDEASEAL FC	WHITE	GUTTER JOINTS
SOUDEASEAL FC	WHITE	DECK TO COLUMN @ TOP
SOUDEASEAL FC	WHITE	SEAL @ OVERFLOW DROPOUTS
SOUDEASEAL FC	WHITE	SEAL BOLTS @ HEATER SUPPORTS
SOUDEASEAL FC	WHITE	DAM UP DECK @ DECK CLOSURE
SOUDEASEAL FC	WHITE	SEAL FASCIA @ DECK CLOSURE

DEAD LOAD = 3 p.s.f.(DECK + LIGHTS) + WEIGHT OF STRUCTURAL COMPONENTS
LIVE LOAD = 20 p.s.f.
SNOW LOAD = 20 p.s.f.
V, ULT = 116 m.p.h. EXP. C
V, ASD = 90 m.p.h. EXP. C
BLDG CODE = 2015 INTERNATIONAL BUILDING CODE
EQUIVALENT LATERAL FORCE PROCEDURE
LATERAL FORCE RESISTING SYSTEM = CANTILEVERED COLUMN SYSTEM-ORDINARY STEEL MOMENT FRAME
Pf = 20 p.s.f., Ce = 1.2, Ct = 1.2, Is = 1.0
W = DRIFT LOADS NOT CONSIDERED
Pd = DRIFT LOADS NOT CONSIDERED
SITE CLASS = D
Ss (0.2) = 0.44
S1 (1.0) = 0.13
SDS = 0.43
SD1 = 0.20
Fa = 1.45
Fv = 2.27
R = 1.25
SEISMIC IMPORTANCE FACTOR = 1.0
RISK CATEGORY = II
SEISMIC DESIGN CATEGORY = D
CS = 0.340
CONSTRUCTION TYPE = IIB
OCCUPANCY CATEGORY = M
TOTAL SEISMIC BASE SHEAR BOTH DIRECTIONS = 1.38 KIPS

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STORE
Chick-fil-A #2793
5009 MONTGOMERY
BLVD NE
ALBUQUERQUE, NW
87109

SHEET TITLE
CANOPY FRAMING PLAN
10'-6 1/2" X 58'-1 1/2"

Job No.: LSC: 66258
Store : 2793
Date : 04.16.20
Drawn By : AFG
Checked By : ELM

Sheet
OMD-3
EI OF 4

AI GENERAL NOTES

N.T.S.

A6 ERECTOR'S NOTES

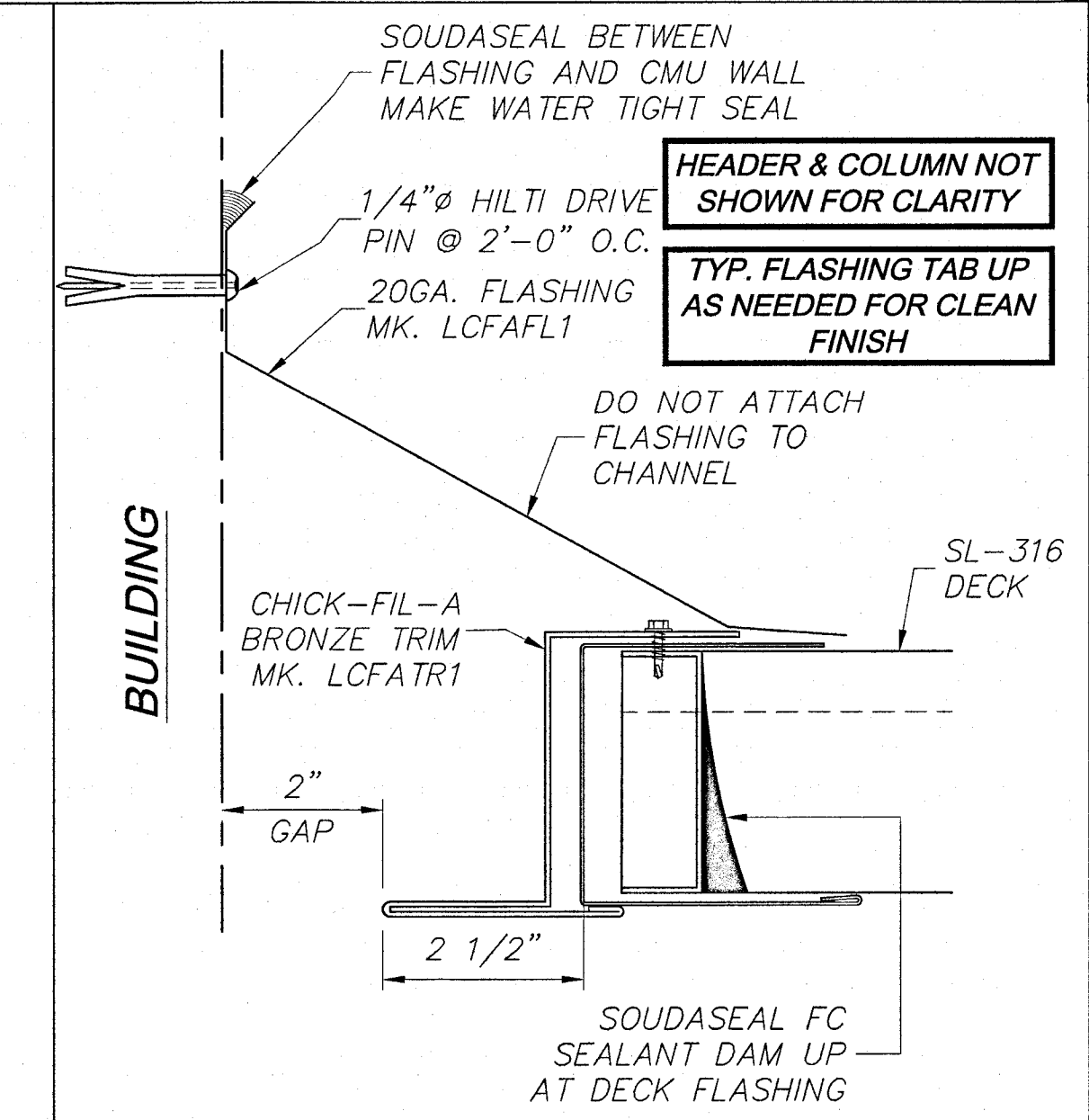
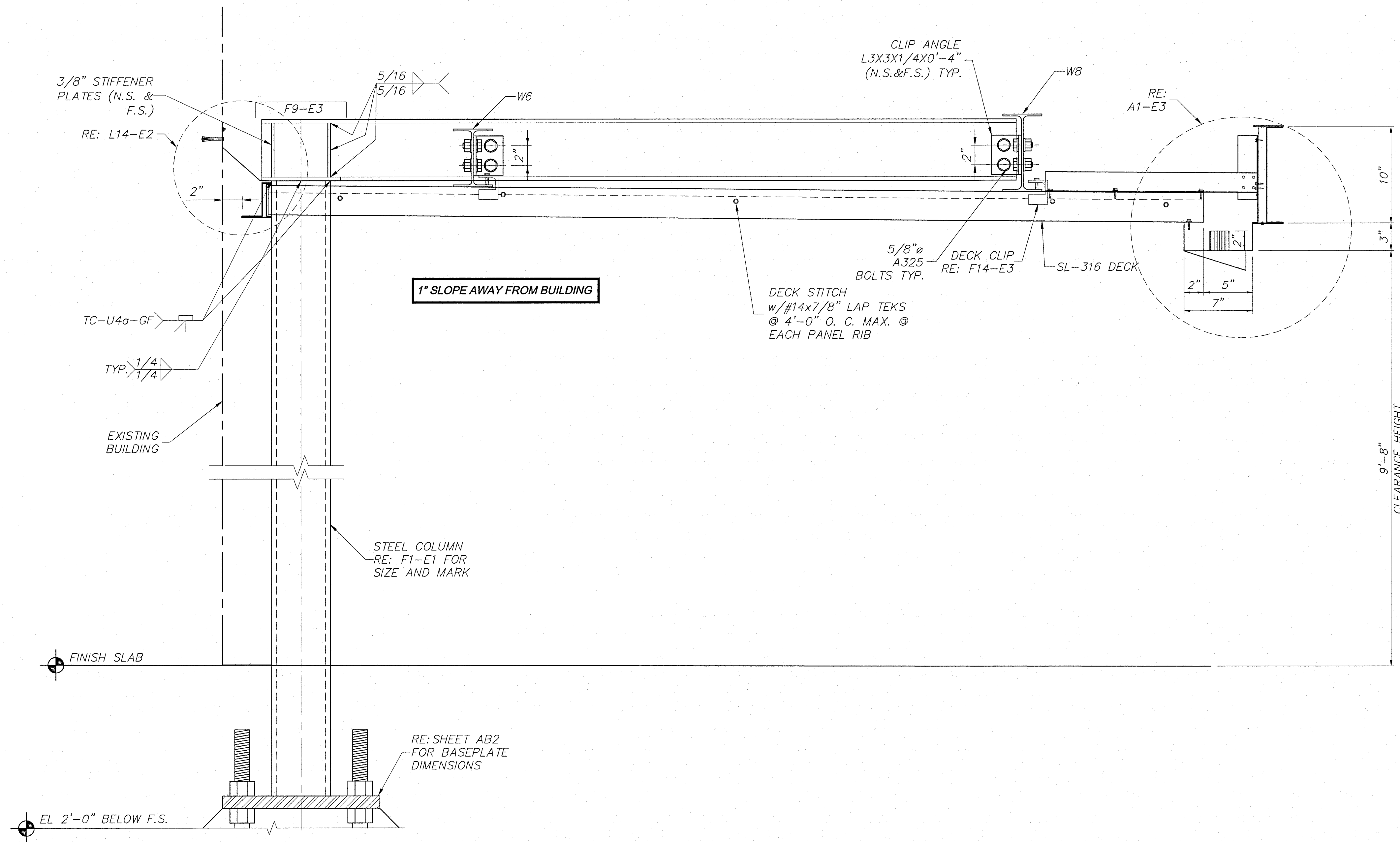
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AII NOT USED

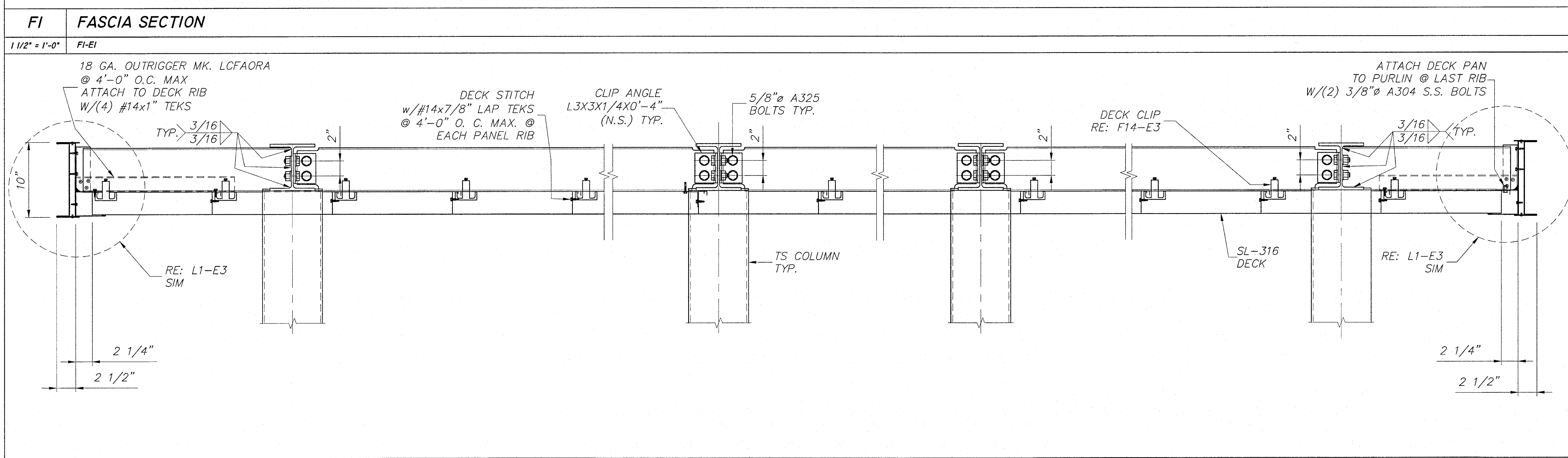
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A14 DESIGN LOADS

N.T.S.



L14 SECTION AT BUILDING
N.T.S. FI-E2



AI4 FASCIA SECTION
N.T.S. FI-E1

F14 NOT USED

DEAD LOAD = 3 p.s.f.(DECK + LIGHTS) + WEIGHT OF STRUCTURAL COMPONENTS
 LIVE LOAD = 20 p.s.f.
 SNOW LOAD = 20 p.s.f.
 V, ULT = 116 m.p.h. EXP. C
 V, ASD = 90 m.p.h. EXP. C
 BLDG CODE = 2015 INTERNATIONAL BUILDING CODE
 EQUIVALENT LATERAL FORCE PROCEDURE
 LATERAL FORCE RESISTING SYSTEM = CANTILEVERED COLUMN SYSTEM - ORDINARY STEEL MOMENT FRAME
 $P_f = 20$ p.s.f., $C_e = 1.2$, $C_t = 1.2$, $I_s = 1.0$
 $W =$ DRIFT LOADS NOT CONSIDERED
 $P_d =$ DRIFT LOADS NOT CONSIDERED
 SITE CLASS = D
 $S_s(0.2) = 0.44$
 $S_1(1.0) = 0.13$
 $SDS = 0.43$
 $SD1 = 0.20$
 $F_a = 1.45$
 $F_v = 2.27$
 $R = 1.25$
 SEISMIC IMPORTANCE FACTOR = 1.0
 RISK CATEGORY = II
 SEISMIC DESIGN CATEGORY = D
 $CS = 0.340$
 CONSTRUCTION TYPE = IIB
 OCCUPANCY CATEGORY = M
 TOTAL SEISMIC BASE SHEAR BOTH DIRECTIONS = 1.38 KIPS

AI4 DESIGN LOADS
N.T.S.

5200 Buffington Rd.
Atlanta Georgia,
30349-2998

Revisions:
Mark Date By

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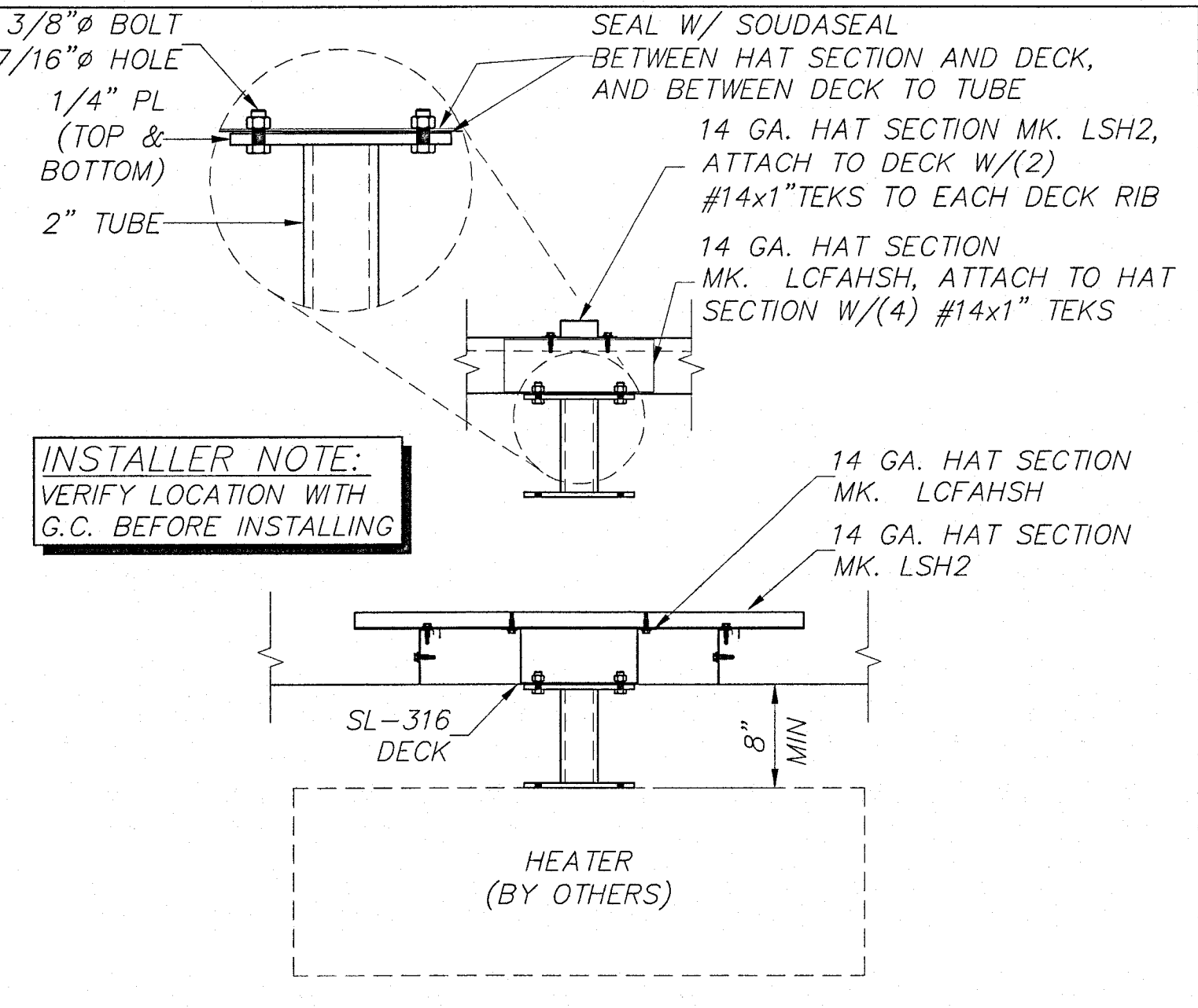
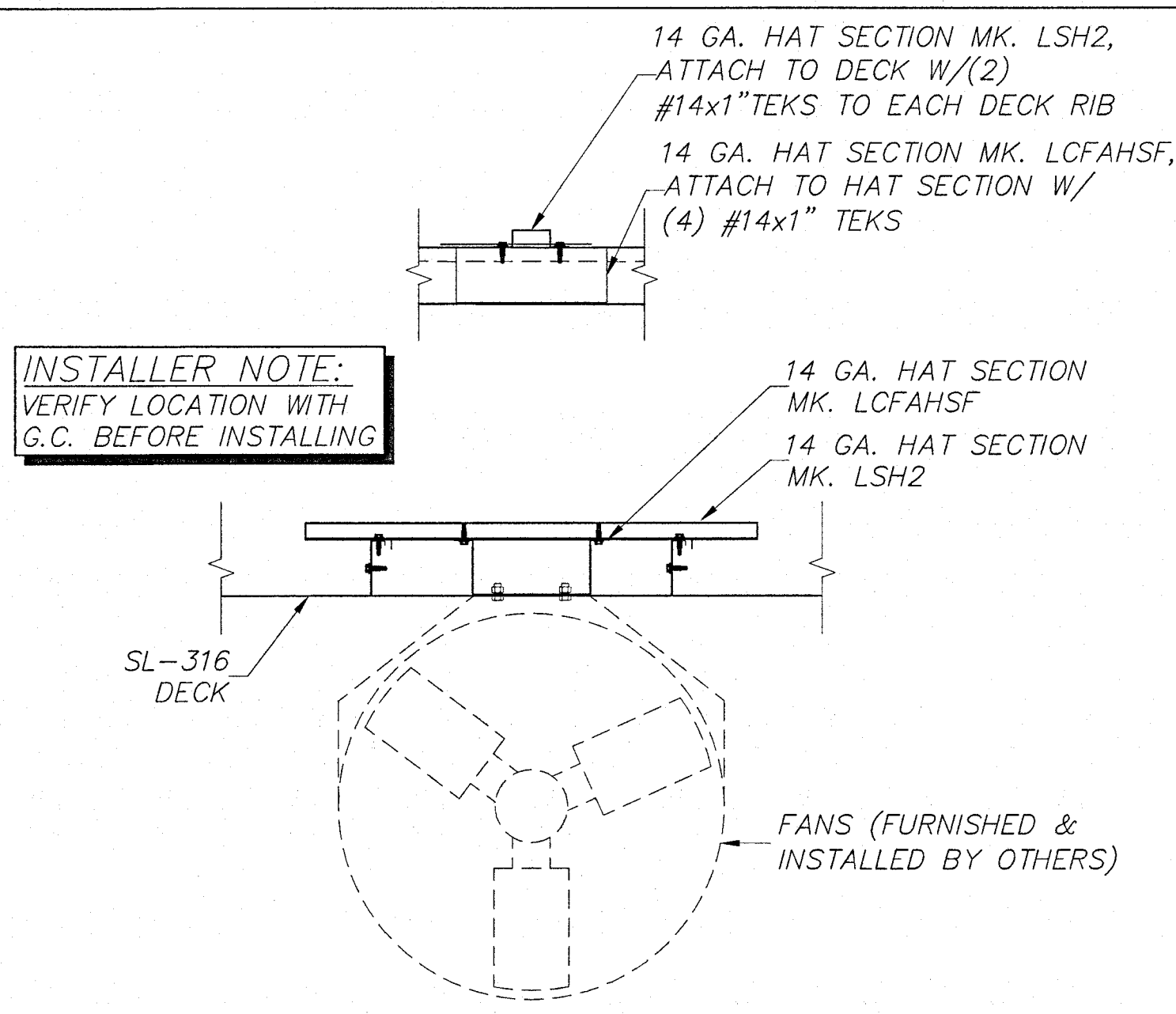
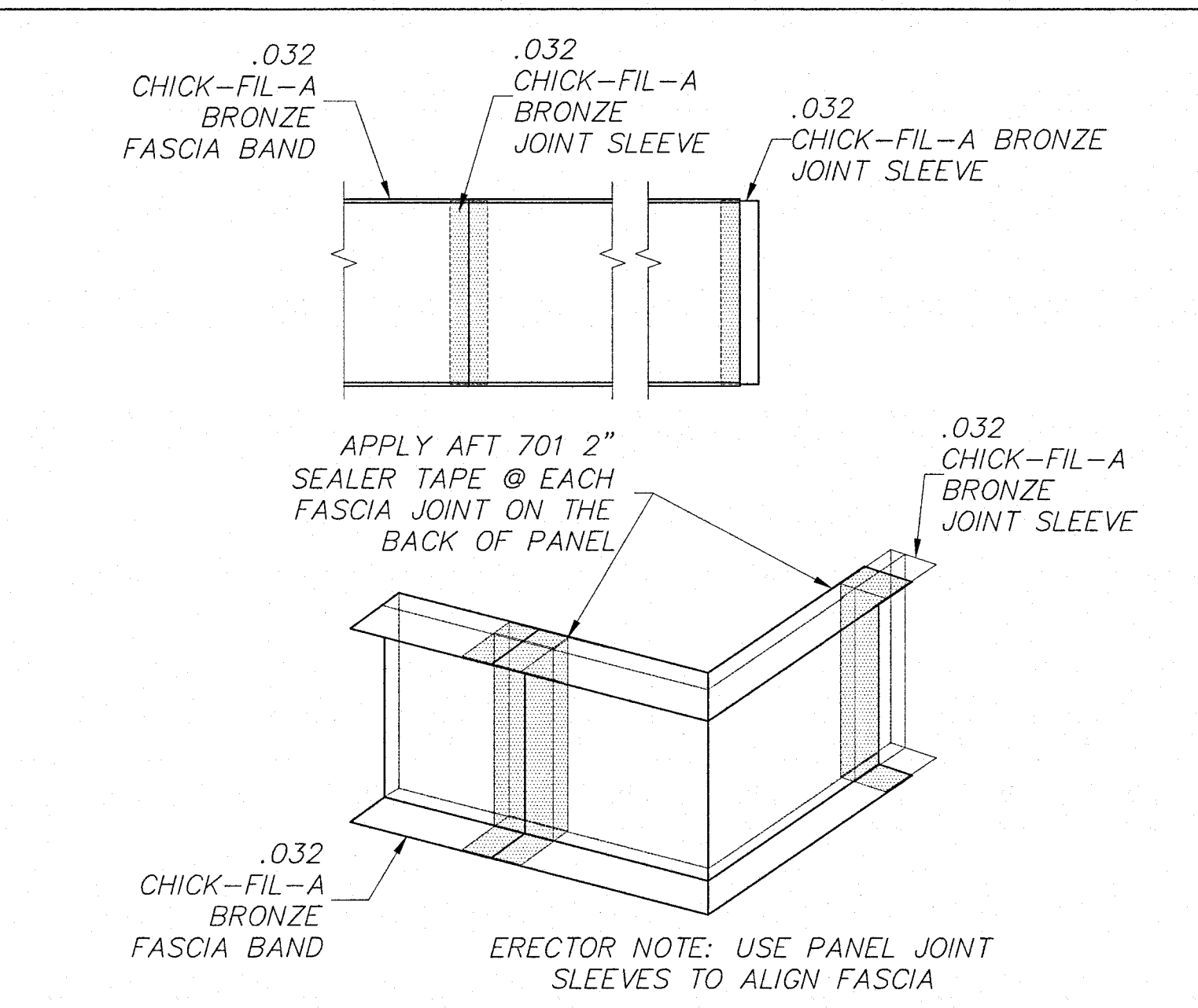
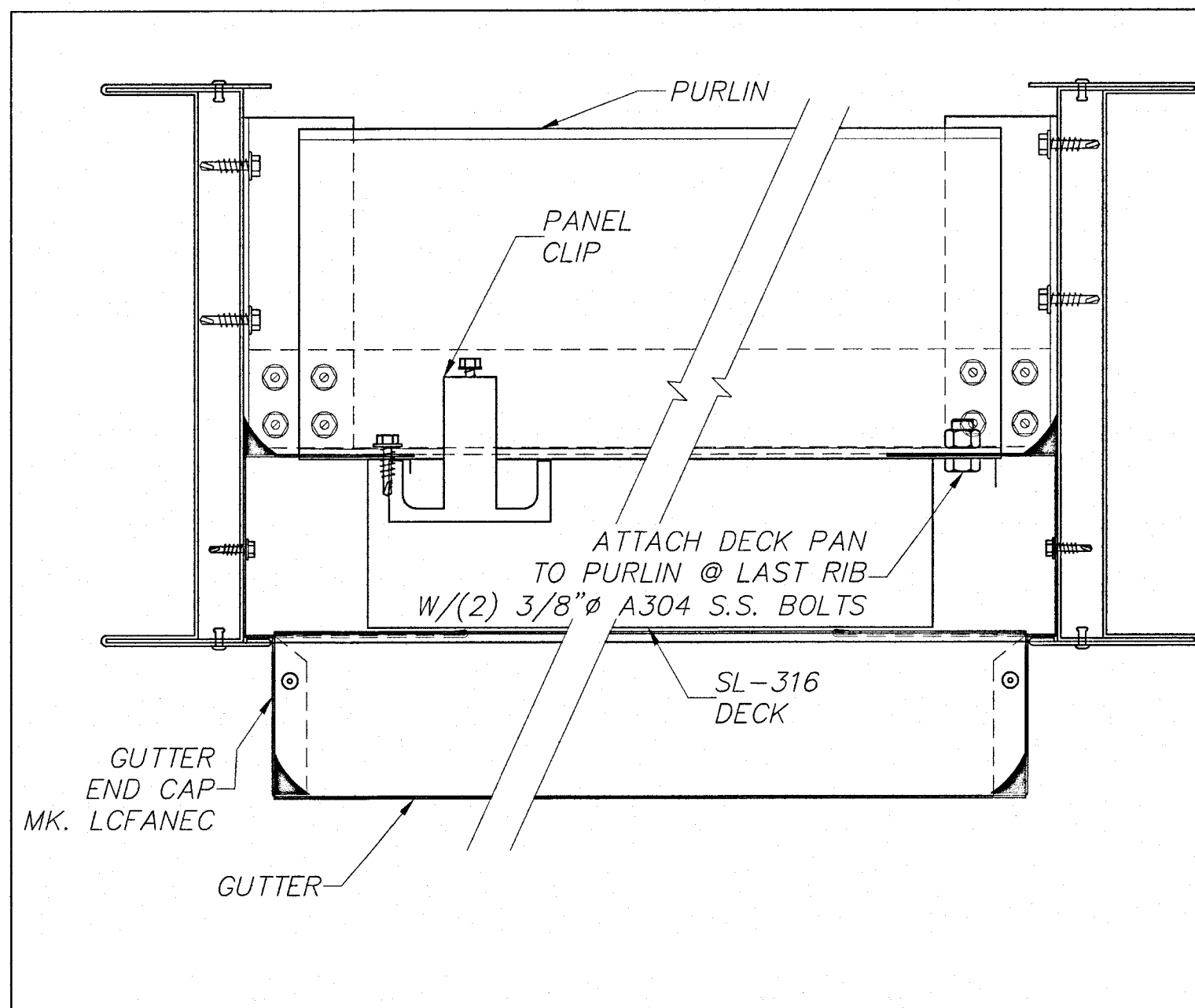
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ARLINGTON, TX. 76010
(817) 261-9116

STORE
Chick-fil-A #2793
5009 MONTGOMERY
BLVD NE
ALBUQUERQUE, NW
87109

SHEET TITLE
CANOPY SECTIONS
10'-6 1/2" X 58'-1 1/2"

Job No.: LSC: 66258
Store : 2793
Date : 04.16.20
Drawn By : AFG
Checked By: ELM

Sheet
**OMD-4
E2 OF 4**

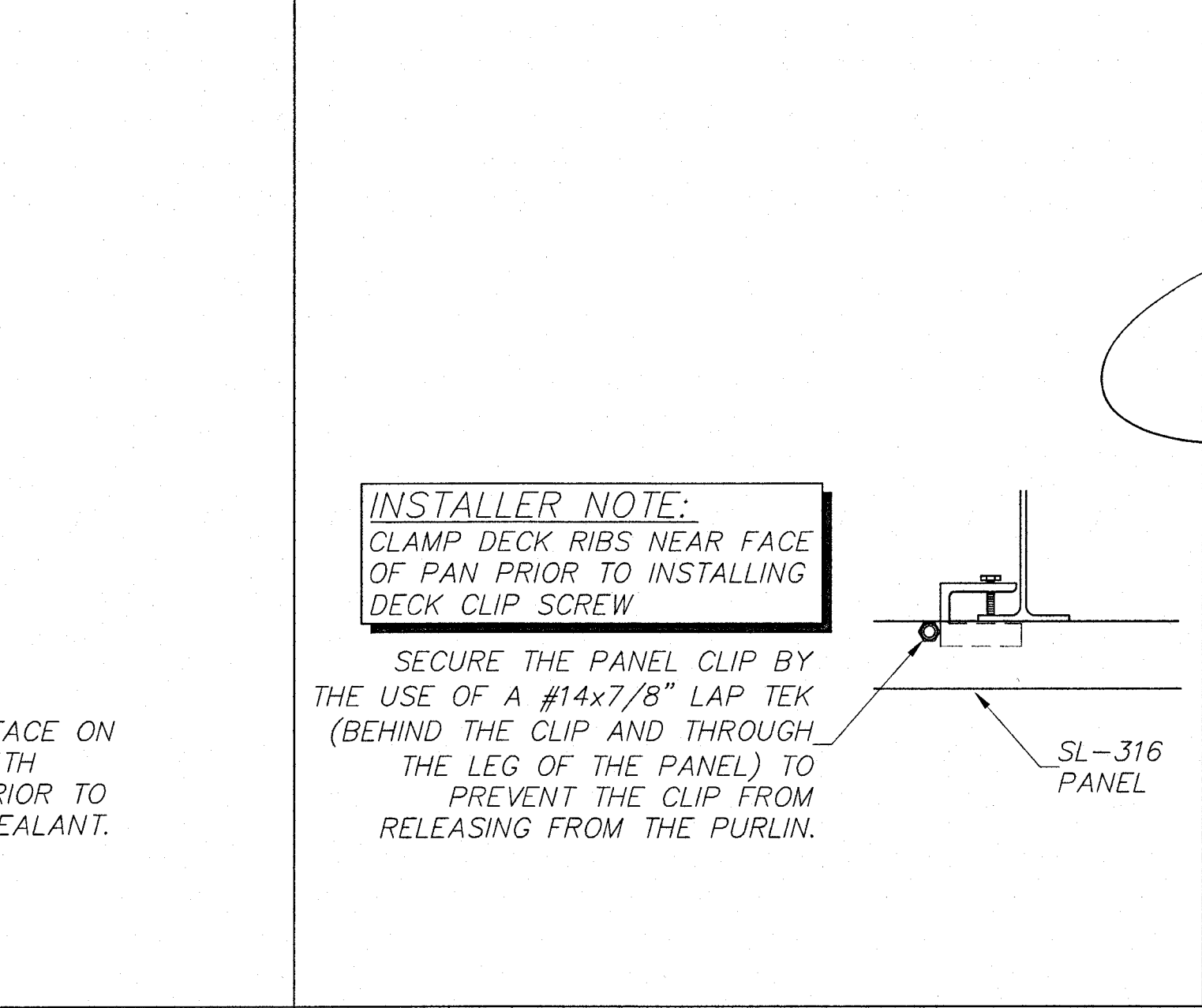
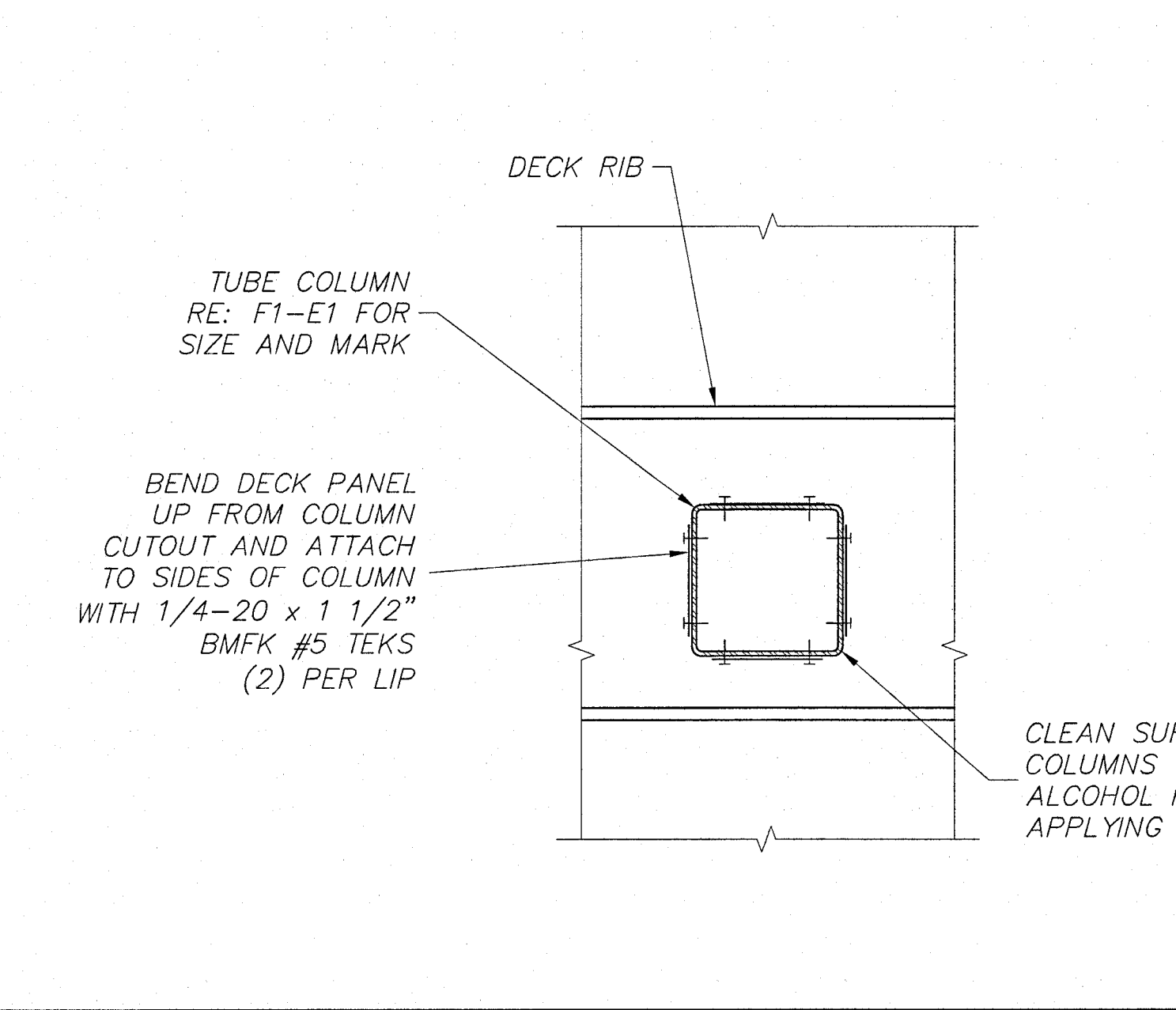
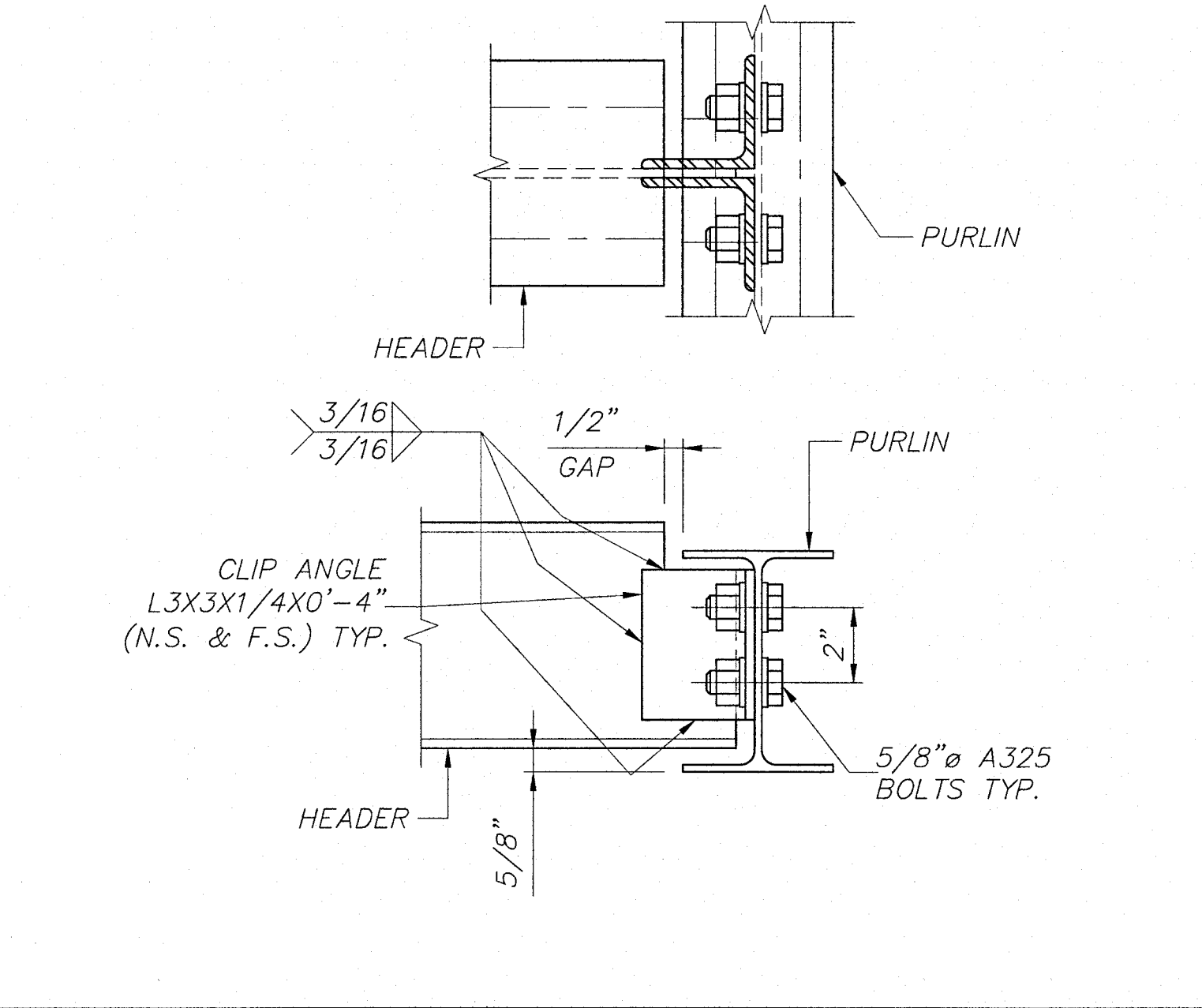
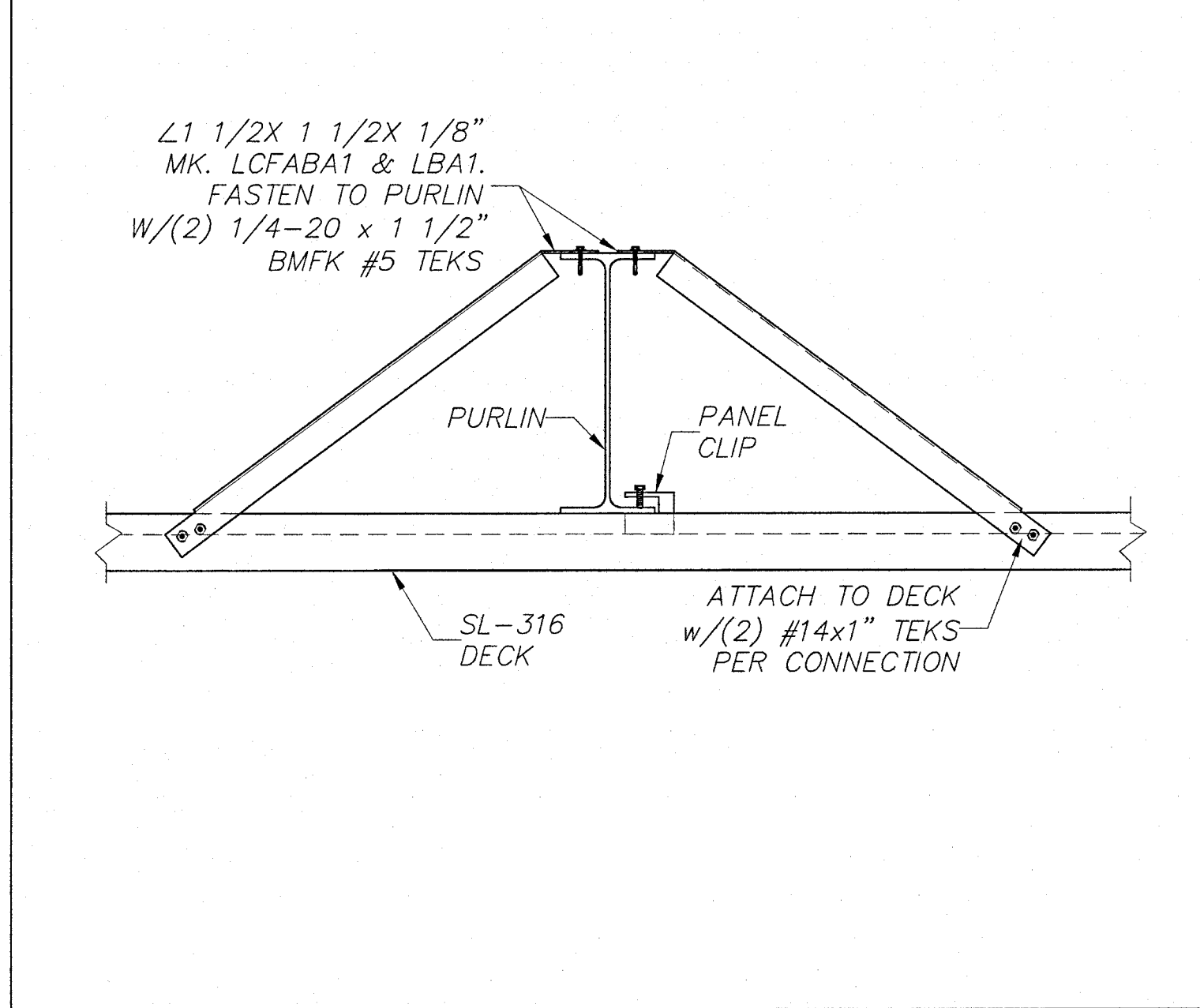


L1 DETAIL AT SIDES OF CANOPY
N.T.S. FI-EI, AI-E2

L5 DETAIL AT FASCIA CORNER AND SPLICE
N.T.S. FI-EI

L9 SECTION AT FAN SUPPORT
N.T.S. FI-EI

LI3 SECTION AT HEATER SUPPORT
N.T.S. FI-EI

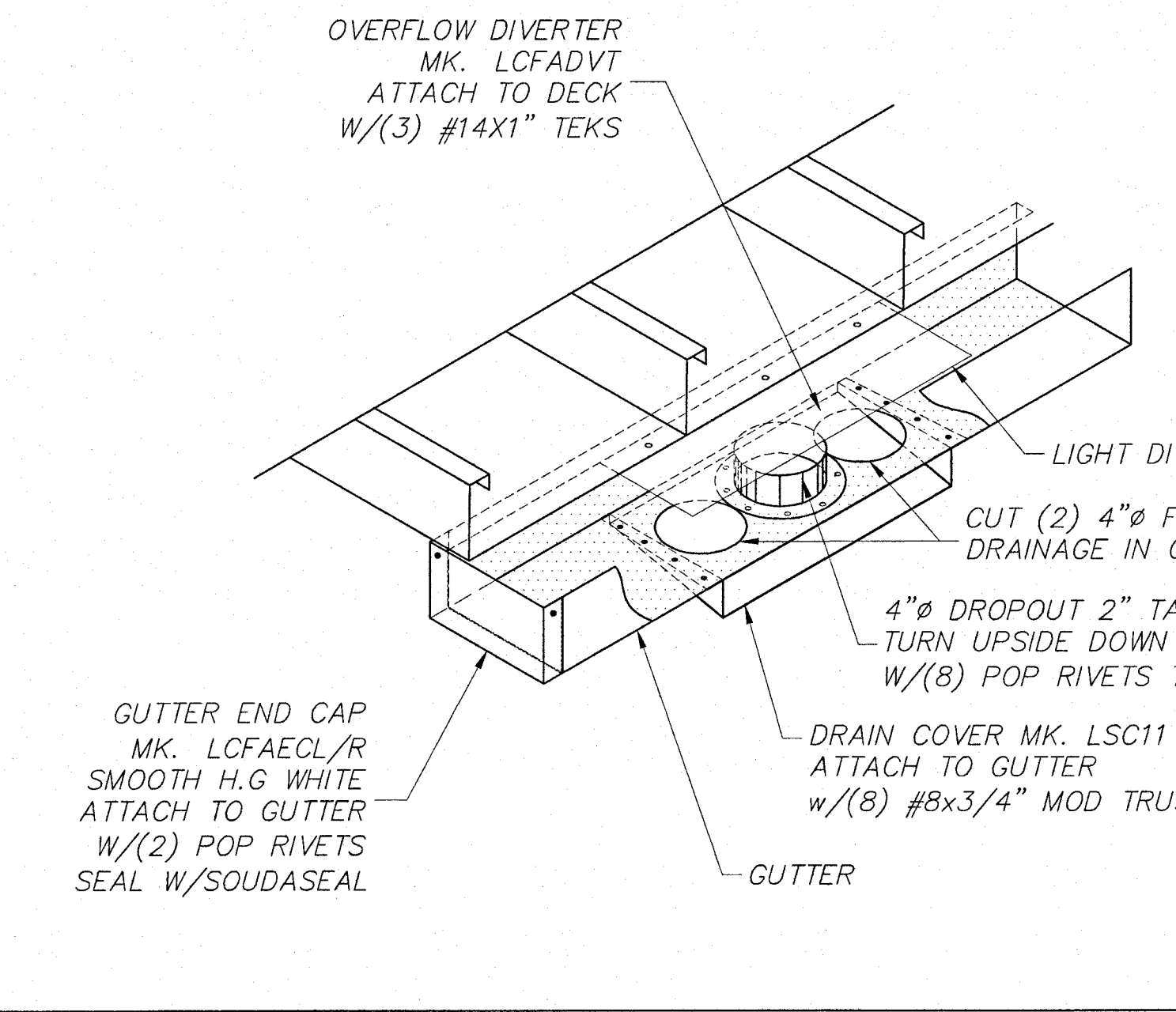
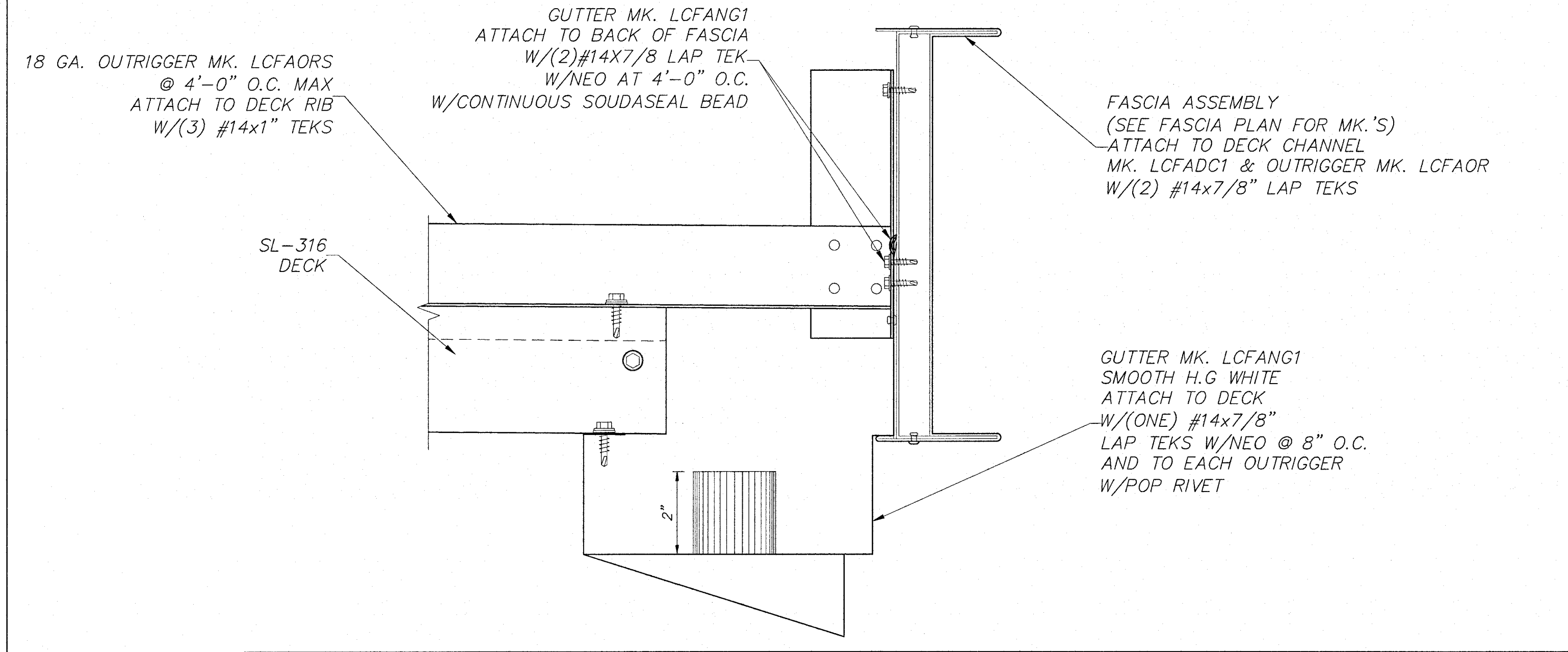


FI SECTION AT PURLIN BRIDGING
N.T.S. FI-EI

F5 SECTION AT CONNECTION
N.T.S. FI-E2

F9 DECK TO COLUMN DETAIL
1 1/2" = 1'-0" FI-E2

FI4 DETAIL AT DECK CLIP
N.T.S. AI-E2, FI-E2



DEAD LOAD = 3 p.s.f.(DECK + LIGHTS) + WEIGHT OF STRUCTURAL COMPONENTS
LIVE LOAD = 20 p.s.f.
SNOW LOAD = 20 p.s.f.
V, ULT = 116 m.p.h. EXP. C
V, ASD = 90 m.p.h. EXP. C
BLDG CODE = 2015 INTERNATIONAL BUILDING CODE
EQUIVALENT LATERAL FORCE PROCEDURE
LATERAL FORCE RESISTING SYSTEM = CANTILEVERED COLUMN SYSTEM - ORDINARY STEEL MOMENT FRAME
Pf = 20 p.s.f., Ce = 1.2, Ct = 1.2, Is = 1.0
W = DRIFT LOADS NOT CONSIDERED
Pd = DRIFT LOADS NOT CONSIDERED
SITE CLASS = D
Ss (0.2) = 0.44
S1 (1.0) = 0.13
SDS = 0.43
SD1 = 0.20
Fa = 1.45
Fv = 2.27
R = 1.25
SEISMIC IMPORTANCE FACTOR = 1.0
RISK CATEGORY = II
SEISMIC DESIGN CATEGORY = D
CS = 0.340
CONSTRUCTION TYPE = IIB
OCCUPANCY CATEGORY = M
TOTAL SEISMIC BASE SHEAR BOTH DIRECTIONS = 1.38 KIPS

AI DETAIL AT END OF CANOPY
N.T.S. FI-EI, FI-E2

A9 DETAIL OF OVERFLOW DRAIN
N.T.S. FI-EI

AI4 DESIGN LOADS
N.T.S.

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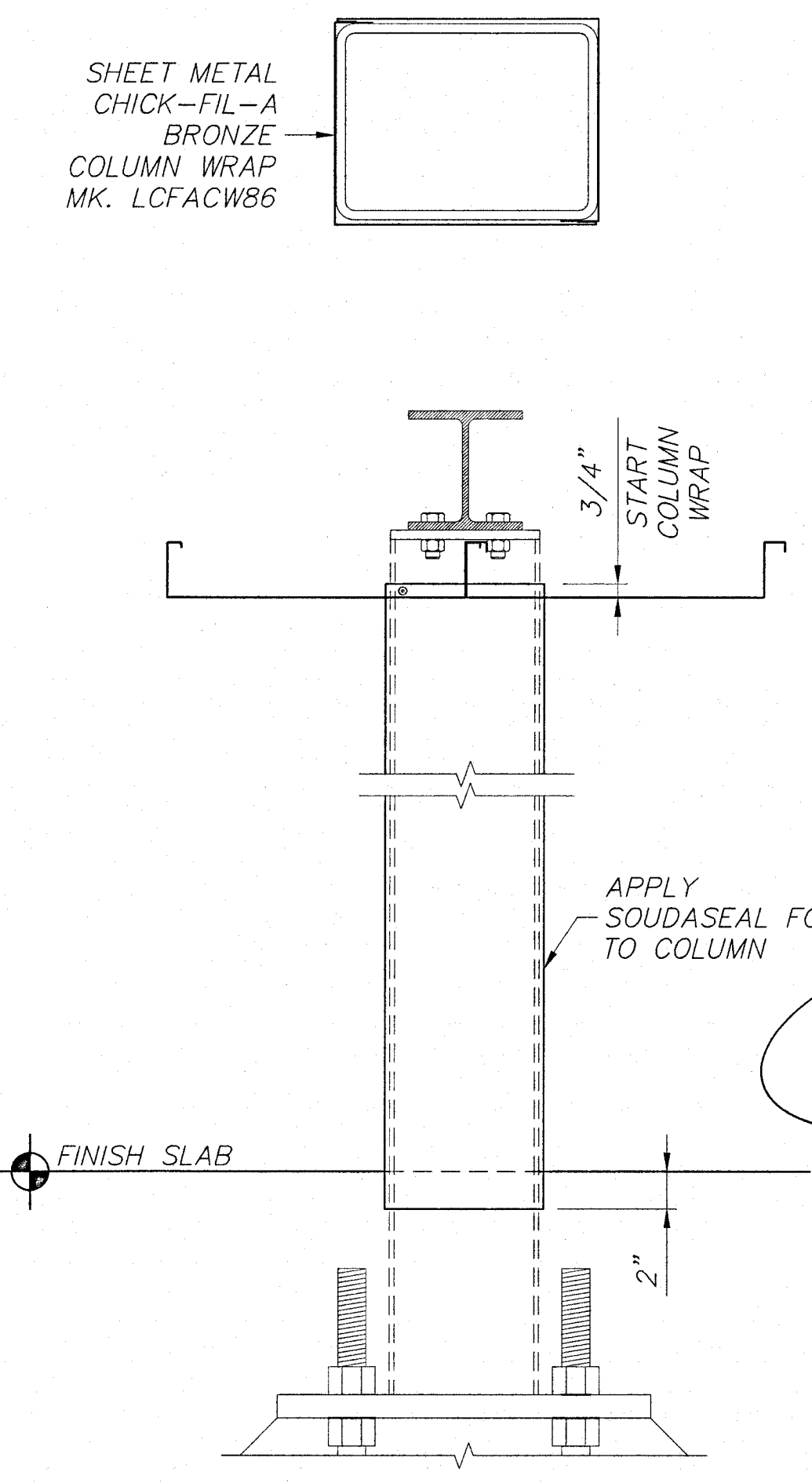
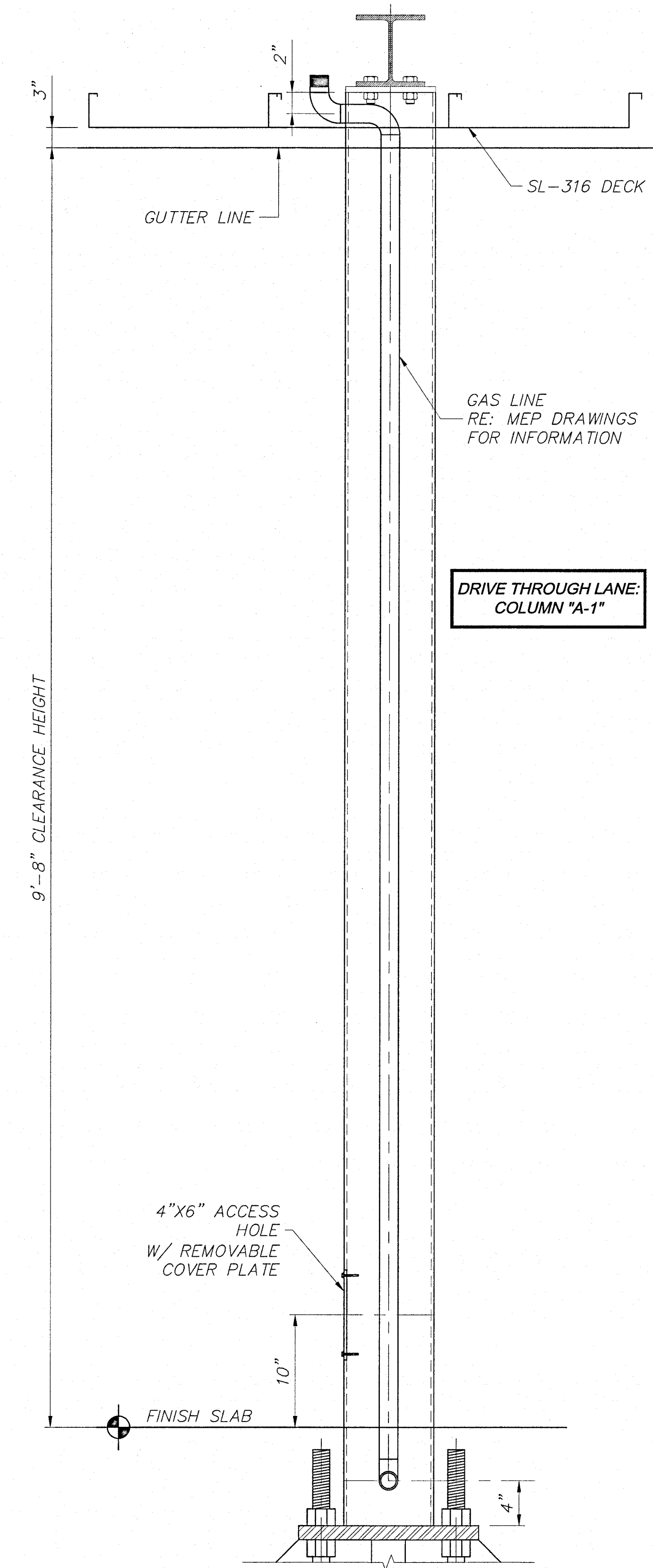
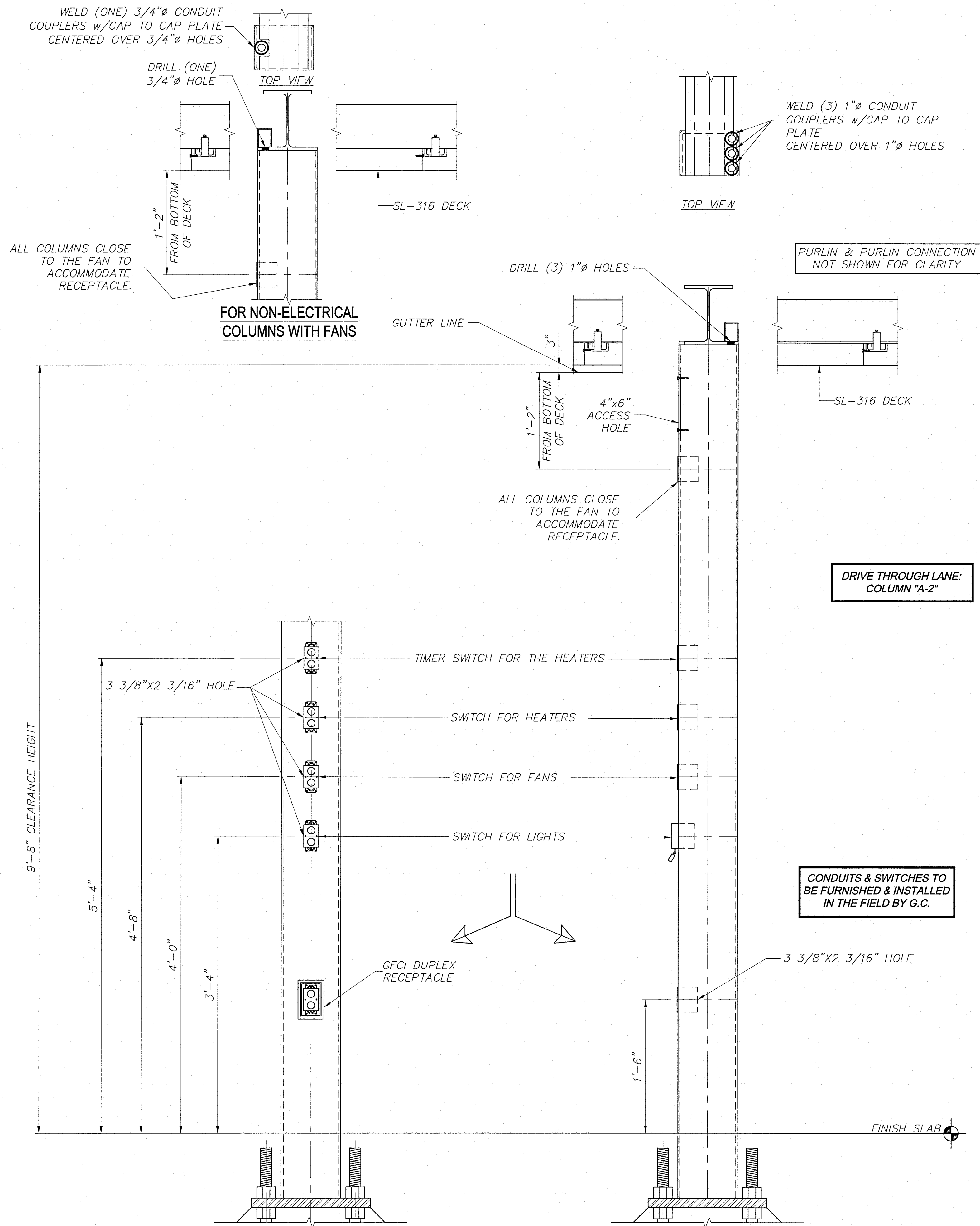
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ALBUQUERQUE, NW
87109

SHEET TITLE
CANOPY SECTIONS
10'-6 1/2" X 58'-1 1/2"

Job No.: LSC: 66258
Store #: 2793
Date: 04.16.20
Drawn By: AFG
Checked By: ELM

Sheet
OMD-5
E3 OF 4



FI4 SECTION AT COLUMN WRAP

N.T.S. FI-EI

DEAD LOAD = 3 p.s.f.(DECK + LIGHTS) + WEIGHT OF STRUCTURAL COMPONENTS
 LIVE LOAD = 20 p.s.f.
 SNOW LOAD = 20 p.s.f.
 V, ULT = 116 m.p.h. EXP. C
 V, ASD = 90 m.p.h. EXP. C
 BLDG CODE = 2015 INTERNATIONAL BUILDING CODE
 EQUIVALENT LATERAL FORCE PROCEDURE
 LATERAL FORCE RESISTING SYSTEM = CANTILEVERED COLUMN SYSTEM - ORDINARY STEEL MOMENT FRAME
 Pf = 20 p.s.f., Ce = 1.2, Ct = 1.2, Is = 1.0
 W = DRIFT LOADS NOT CONSIDERED
 Pd = DRIFT LOADS NOT CONSIDERED
 SITE CLASS = D
 Ss (0.2) = 0.44
 S1 (1.0) = 0.13
 SDS = 0.43
 SD1 = 0.20
 Fa = 1.45
 Fv = 2.27
 R = 1.25
 SEISMIC IMPORTANCE FACTOR = 1.0
 RISK CATEGORY = II
 SEISMIC DESIGN CATEGORY = D
 CS = 0.340
 CONSTRUCTION TYPE = IIB
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 TOTAL SEISMIC BASE SHEAR BOTH DIRECTIONS = 1.38 KIPS

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STORE
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SHEET TITLE
CANOPY SECTIONS
10'-6 1/2" X 58'-1 1/2"

Job No.: LSC: 66258
Store : 2793
Date : 04.16.20
Drawn By : AFG
Checked By: ELM

Sheet
OMD-6
E4 OF 4

AI	SECTION @ COLUMN "A-2"	A10	SECTION @ COLUMN "A-1"	A14	DESIGN LOADS
1 1/2" = 1'-0"	FI-EI	1 1/2" = 1'-0"	FI-EI	N.T.S.	