



# DEVELOPMENT FACILITATION TEAM (DFT) APPLICATIONS

Effective 12/15/2022

Please check the appropriate box(es) and refer to supplemental forms for submittal requirements. All fees must be paid at the time of application.				
MISCELLANEOUS APPLICATION	NS	□ Extension of Infrastructure L	ist or IIA <i>(Form S3)</i>	
□ Site Plan Administrative DFT (Forms P & P2)		P	RE-APPLICATIONS	
Final EPC Sign-off for Master Development/Site Plance	ans - EPC <i>(Form P2)</i>	□ Sketch Plat Review and Con	nment <i>(Form</i> S3)	
□ Amendment to Infrastructure List ( <i>Form S3</i> )		Sketch Plan Review and Co	mment (Form S3)	
□ Temporary Deferral of S/W (Form S3)			APPEAL	
□ Extension of IIA: Temp. Def. of S/W (Form S3)		Administrative Decision (For	m A)	
BRIEF DESCRIPTION OF REQUEST				
Final Sign off for an ECP approved S	ite Developmen	t Plan to add in a new	restaurant and drive through	
use (Starbucks).				
APPLICATION INFORMATION				
Applicant/Owner:Smith' s Food & Drug Center Inc. Phone:			Phone:	
Address:1014 VINE ST FLOOR 7TH		Email:		
City:CINCINNATI		State:OH	Zip:45202	
Professional/Agent (if any): Modulus Architects &	Land Use Planning	, Inc.	Phone:	
Address:8220 SAN PEDRO DR. NE, SUITE	520		Email:rokoye@modulusarchitects.com	
City:Albuquerque		State:NM	Zip:87109	
Proprietary Interest in Site: Agent		List <u>al</u> l owners:		
SITE INFORMATION (Accuracy of the existing lega	I description is crucial	! Attach a separate sheet if ne	cessary.)	
Lot or Tract No.:J-1		Block:	Unit:	
Subdivision/Addition: FOUR HILLS VILLAGE SHOPPING CTR &	APT COMPLEX	MRGCD Map No.:	UPC Code: 102305602134521316	
Zone Atlas Page(s):L- 23- Z and L- 22- Z	Existing Zoning:MX-	M	Proposed ZoningN/A	
# of Existing Lots:1	# of Proposed Lots:1		Total Area of Site (Acres):+/- 8.5	
LOCATION OF PROPERTY BY STREETS				
Site Address/Street:200 TDAM/MAY DLVD SE	Botwoon: Tromouro		d: Control Aug	

Site Address/Street:200 TRAMWAY BLVD SE	Between: Tramway Blvd	<sup>and:</sup> Central Ave		
CASE HISTORY (List any current or prior project and case number(s) that may be relevant to your request.)				
PR-2023-008767-SI-2023-001056				
I certify that the information I have included here and sent in the required notice was complete, true, and accurate to the extent of my knowledge.				
Signature: Regina Okoye		Date:9/13/2023		
Printed Name: Regina Okoye		□ Applicant or ■ Agent		

## FORM P2: SITE PLAN ADMINISTRATIVE – Development Facilitation Team (DFT) as of 12/25/2022

### SITE PLAN ADMINISTRATIVE – DFT

A Single PDF file of the complete application including all documents being submitted must be emailed to <u>PLNDRS@cabq.gov</u>. Zipped files or those over 9 MB cannot be delivered via email, in which case the PDF must be provided to City Staff using other online resources such as Dropbox or FTP. <u>The PDF shall</u> <u>be organized in the number order below</u>. Divide the PDF with a title sheet for each of the three documentation sections in **bold** below.

### SITE PLAN DOCUMENTATION

- \_\_\_\_\_1) DFT Application form completed, signed, and dated
- \_\_\_\_\_ 2) Form P2 with all the submittal items checked/marked
- \_\_\_\_\_ 3) Form P with signatures from Hydrology, Transportation, and ABCWUA
- \_\_\_\_\_ 4) Zone Atlas map with the entire site clearly outlined and labeled
- \_\_\_\_\_ 5) Site Plan and related drawings (include a Site Plan key of the sheets submitted)
- \_\_\_\_\_ 6) Copy of the original approved Site Plan or Master Development Plan (for amendments to or Extensions of the Site Plan)
  - \_\_\_\_\_7) Infrastructure List, if required for building of public infrastructure
- 8) Sensitive Lands Site Analysis for new site design in accordance with IDO Section 5-2(C) (The Sensitive Lands Site Analysis form can be obtained online at): <u>https://documents.cabq.gov/planning/development-review-</u> board/Sensitive lands analysis form.pdf
- 9) Responses to climatic and geographic responsive design considerations (Recommended to promote sustainability, but not required. The Climatic and Geographic Responsiveness form can be obtained online at): <u>https://documents.cabq.gov/planning/IDO/SubmittalFormIDO5-</u> <u>2(D)ClimaticGeographic\_Responsiveness.pdf</u>

### SUPPORTIVE DOCUMENTATION

- \_\_\_\_\_ 10) Completed Site Plan Checklist
- \_\_\_\_\_ 11) Letter of authorization from the property owner if application is submitted by an agent
- 12) Justification letter describing and justifying the request per the criteria in IDO Section 16-6-5(G)(3)
- 13) Explanation and justification of requested deviations, if any, in accordance with IDO Section 14-16-6-4(P). Note: If requesting more than allowed by deviation, a Variance – ZHE or Waiver –DHO will be required, as applicable
- \_\_\_\_\_ 14) Sites 5 acres or greater: Archaeological Certificate in accordance with IDO Section 14-16-6-5(A) *(not required for Extension)*

- 15) Landfill disclosure statement per IDO Section 14-16-6-4(S)(5)(d)(2)(d) if site is within a designated landfill buffer zone
- 16) Architectural Review Committee approval letter if the site is located within a Master Development Plan area or a Framework Plan area

### PUBLIC NOTICE DOCUMENTATION

- \_\_\_\_\_ 17) Sign Posting Agreement
- 18) Proof of a <u>Pre-Submittal</u> Neighborhood Meeting per IDO 6-4(C)(1)(b) for new building or multiple new buildings that include a total of more than 100 multi-family residential dwelling units or more than 50,000 square feet of non-residential development
  - \_\_\_ Office of Neighborhood Coordination neighborhood meeting inquiry response
  - Proof of email with read receipt OR Certified Letter offering meeting to applicable associations
  - Completed neighborhood meeting request form(s)
  - \_\_\_\_ If a meeting was requested or held, copy of sign-in sheet and meeting notes
  - 19) Required Content of Notice <u>at Submittal</u> per IDO Section 14-16-6-4(K)(1) (not required for an extension)
    - Office of Neighborhood Coordination notice inquiry response
    - Completed notification form(s), proof of additional information provided in accordance with IDO Section 6-4(K)(1)(b)
    - \_\_\_ Proof of emailed notice to affected Neighborhood Association representatives

# $\overline{\mathrm{X}}$ final sign-off for master development plans and site plans – epc

A Single PDF file of the complete application including all documents being submitted must be emailed to <u>PLNDRS@cabq.gov</u> prior to making a submittal. Zipped files or those over 9 MB cannot be delivered via email, in which case the PDF must be provided to City Staff using other online resources such as Dropbox or FTP. <u>The PDF shall be organized in the number order below.</u>

- $\underline{\mathbf{X}}$  1) DFT Application form completed, signed, and dated
- $\underline{\mathrm{X}}$  2) Form P2 with all the submittal items checked/marked
- ${f X}$  3) Zone Atlas map with the entire site clearly outlined and labeled
- X \_\_\_\_\_ 4) Site Plan and related drawings
- $\underline{\mathrm{X}}$  5) Infrastructure List, if require
- ${f X}$  6) Copy of EPC Notice of Decision and letter explaining how each EPC condition has been met
- $\underline{\mathrm{X}}$  7) Letter of authorization from the property owner if application is submitted by an agent
- $\underline{\mathrm{X}}$  8) Solid Waste Department signature on Site Plan
- X 9) Signed Form DRWS Drainage Report, Grading and Drainage Plan, and Water & Sewer Availability Statement filing information
- ${
  m X}$  10) Approved Grading and Drainage Plan
- X 11) Copy of Site Plan with Fire Marshal's stamp, i.e. "Fire 1" plan (not required for Master Development Plans)





	GENERAL NOTES			AN
	A. "G" SERIES SHEETS APPLY TO THE ENTIRE SET OF DRAWINGS. B. INDICATED DIMENSIONS ARE TO FACE OF FINISH; UNLESS OTHERWISE NOTED.			MENT PI
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	SITE DATA		REVISI	XISTING
	IDO ZONING DESIGNATION – MX–M COMPREHENSIVE PLAN CORRIDOR – MAJOR TRANSIT CORRIDOR & PREMIUM TRANSIT CORRIDOR	(central ave)		IT TO E
	COMPREHENSIVE PLAN CENTER – FOUR HILLS VILLAGE – ACTIVITY CENTER CITY DEVELOPMENT AREA – AREA OF CHANGE			IENDMEI
				AJOR AN
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			DATE	JUL 20
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			5	198
				8-1
	PROJECT NUMBER:APPLICATION NUMBER:			9 33
	THIS PLAN IS CONSISTENT WITH THE SPECIFIC SITE DEVELOPMENT PLAN APPROVED BY THE ENV (EPC), DATED AND THE FINDINGS AND CONDITIONS IN THE DECISION ARE SATISFIED	(IRONMENTAL PLANNING COMMISSION E OFFICIAL NOTIFICATION OF		7100
	IS AN INFRASTRUCTURE LIST EQUIRED? () YES () NO. IF YES, THEN A SET OF APPROVED	DRC PLANS WITH A WORK ORDER		
	IS REQUIRED FOR ANT CONSTRUCTION WITHIN FUBLIC RIGHT-OF-WAT OR FOR CONSTRUCTION O	F FUDLIG IMPROVEMENTS.		
	Traffic Engineering, Transportation Division	Date		
			S	-14 -14
		Date		ИUЕ JE, N 338
	Parks and Recreation Department	Date		AVE RQL 05)
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	Hydrology	Date		
	Code Enforcement			- IA F
	Code Linordement	Date		
	Environmental Health Department (conditional)	Date		
	Solid Waste Management	Date		
	Planning Department			
		540	OF N	EWARE
	EXISTING SMITH'S PARKING		STEP	HENA.
	USE/PARKING REQUIREMENT TOTALS		HEG NO.	
	TOTAL PARKING REQUIRED     1 SPACE PER 300 SQ. FT. GFA = 286		STERED	ARCHI
	EXISTING PARKING PROVIDED     383 SPACES       HC PARKING REQUIRED     7 SPACES		05 JUL	Y 2023
	HC PARKING REQUIRED10 SPACESBIKE SPACE PROVIDED12 SPACES		RU	÷
	NO. OF PARKING REMOVED FOR COFFEE SHOP W/ DRIVE-THRU 48 SPACES REMOVED			AWN B DTN )23
	REMAIN PARKING SPACES PROVIDED 335 SPACES		- T	DR, 20, 20
			DR	T JULY
	PROPOSED COFFEE SHOP WITH DRIVE-THRU PARKING		ith	ob ng S-trawwa ENDMEN
	USE/PARKING REQUIREMENT TOTALS		⊃ w 8712	or AME
	TOTAL PARKING REQUIRED 8 SPACES PER 1,000 SF GFA = 10 SPACES		MEXICC	Ċ, MAJ
	OUTDOOR PATIO SIZE725 SFTOTAL PARKING REQ FOR PATIO3 SPACES PER 1,000 SF GFA = 2 SPACES			
	20% PARKING REDUCTION (AC, MT)9 SPACESHC PARKING REQUIRED1 SPACE		TE FE Verque	ANAGER NGUYE IF PLA
	HC PARKING PROVIDED 2 SPACES TOTAL PARKING PROVIDED 11 SPACES		COF 200 TI ALBUQI	ROJECT M. DEVIN HEET TITLE SI
	BIKE SPACES REQUIRED 3 SPACES		≏ DATE:	⊶ ∽ sheet-
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GENERAL N	DTES		
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		REVISION	
1. SIGNAGE SHA POWER UNDE 2. SURFACE MO	NOTES L BE PROVIDED AND INSTALLED BY TENANT UNDER A SEPARATE PERMIT; CONTRACTOR THIS CONTRACT. INTED LIGHT FIXTURE: SEE ELECTRICAL PLAN ON SHEET E101	TO PROVIDE ELECTRICAL	
3. DRIVE-THRU 4. COVERED CAI 5. SCHEDULED I 6. LEADER HEAD 7. DASHED LINE	GHELF. OPY. OOR AND FRAME: SEE FLOOR PLAN ON SHEET A101. AND DOWNSPOUT. INDICATES <b>%"</b> FIRE RETARDANT PLYWOOD BACKING AT SIGNAGE LOCATION.	DATE	
8. ALUMINUM ST 9. BRICK VENEE 10. HEATED AIR ( 11. EXPANSION J 12. SCHEDULED S 13. GALVANIZED	JCCO REVEAL: SEE XXX/AXXX. CONTROL JOINT: SEE XXX/AXX. URTAIN: AA300. JINT: SEE XX/AXXX. TOREFRONT: SEE FLOOR PLAN ON SHEET A101 FOR ADDITIONAL INFORMATION. IETAL COPING.	REV	4
14. DASHED LINE 15. SURFACE MO	INDICATES MASONRY SCREEN IN FRONT. INT DOWNWARD FOCUSED LIGHT FIXTURE.		S
SIGNAGE DA	TA TABLE		
WALL SIGN	I REQUIREMENT           15% OF FACADE AREA (528 SF) INCLUSIVE OF DOORS AND	PROPOSED	
'DRIVE-THRU'	WINDOWS: 79 SF ALLOWED 15% OF FACADE AREA (528 SE) INCLUSIVE OF DOORS AND		te 600
WEST ELEVATION: ROUND SIGNAGE WALL SIGN	WINDOWS: 79 SF ALLOWED	12.5 SF	LE. St
SOUTH ELEVATION 'STARBUCKS' WALL SIGN	WINDOWS: 218 SF ALLOWED	21 SF	
SOUTH ELEVATION ROUND SIGNAGE	WINDOWS: 218 SF ALLOWED	4 SF	
*Signs will be per	nitted separately and the colors and material will be determined at the	e time of sign permit.	100 SUN
			Â.
		AS HEL	STEPHE DUNB No. 42
		AG HEL	STEPHE DUNBA No. 421 28 Jul 2
		AG REC	No. 421 28 Jul 2
EXTERIOR	ATERIALS	AG	AMMAY STEPHE DUNBY No. 421 28 Jul 2 28 Jul 2
EXTERIOR N STUCCO-1	' <mark>ATERIALS</mark> COAT EXTERIOR STUCCO SYSTEM JLOR - SHERWIN WILLIAMS SW#7030 ANEW GRAY, SANDED TEXTURE RICK VENEER JTUAL MATERIALS - PEWTER MISSION, RUNNING BOND WITH SM100 GRAY COLOR MORT/	AR	COE NEW STEPPHE DUNBA No. 421 58 Jul 2 58 Jul 2
EXTERIOR N STUCCO-1 2 BRICK-1 5 MASONRY N PAINT-1 5	ATERIALS COAT EXTERIOR STUCCO SYSTEM JLOR - SHERWIN WILLIAMS SW#7030 ANEW GRAY, SANDED TEXTURE NCK VENEER JLUAL MATERIALS - PEWTER MISSION, RUNNING BOND WITH SM100 GRAY COLOR MORT/ ISONRY SCREEN JLUAL MATERIALS - PEWTER MISSION, RUNNING BOND WITH SM100 GRAY COLOR MORT/ TUAL MATERIALS - PEWTER MISSION, RUNNING BOND WITH SM100 GRAY COLOR MORT/ TEL CANOPY STRUCTURE JLOR: SHERWIN WILLIAMS SW#7033 BRAIN STORM RPONZE	AR AR	UCKS - TRAMWAY BLVD. NE NEW MEXICO 87123 NEW MEXICO 87123 JOB NO. DRAWN BY: JOB NO. DRAWN BY:
EXTERIOR N STUCCO-1 BRICK-1 MASONRY PAINT-1 CLAZ-1 V	ATERIALS COAT EXTERIOR STUCCO SYSTEM JUR - SHERWIN WILLIAMS SW#7030 ANEW GRAY, SANDED TEXTURE RICK VENEER JULA MATERIALS - PEWTER MISSION, RUNNING BOND WITH SM100 GRAY COLOR MORTI ASONRY SCREEN JTUAL MATERIALS - PEWTER MISSION, RUNNING BOND WITH SM100 GRAY COLOR MORTI TUAL MATERIALS - PEWTER MISSION, RUNNING BOND WITH SM100 GRAY COLOR MORTI TUAL MATERIALS - PEWTER MISSION, RUNNING BOND WITH SM100 GRAY COLOR MORTI TUAL MATERIALS - PEWTER MISSION, RUNNING BOND WITH SM100 GRAY COLOR MORTI TEEL CANOPY STRUCTURE JLOR: SHERWIN WILLIAMS SW#7033 BRAIN STORM BRONZE JUMINUM STOREFRONT SYSTEM WITH CLEAR LOW-E JLOR: DARK BRONZE		TARBUCKS - TRAMWAY TRAWAY BLVD. NE JQUERQUE, NEW MEXICO 87123 MANAGER JOB NO. DRAWN BY: 12 TO 12
EXTERIOR N         STUCCO-1       2         BRICK-1       1         MASONRY       1         MASONRY       1         GLAZ-1       2         WOOD-1       1         COPING-1       2	ATERIALS         COAT EXTERIOR STUCCO SYSTEM         JLOR - SHERWIN WILLIAMS SW#7030 ANEW GRAY, SANDED TEXTURE         RUCK VENEER         JULAL MATERIALS - PEWTER MISSION, RUNNING BOND WITH SM100 GRAY COLOR MORTI         ASORRY SCREEN         JULAL MATERIALS - PEWTER MISSION, RUNNING BOND WITH SM100 GRAY COLOR MORTI         FEEL CANOPY STRUCTURE         JLOR: SHERWIN WILLIAMS SW#7033 BRAIN STORM BRONZE         JUMINUM STOREFRONT SYSTEM WITH CLEAR LOW-E         JOR: DARK BRONZE         JOD CLADDING         CHIHA FIBER CEMENT - VINTAGEWOOD- CEDER         LIVANUED METAL COPING         JLOR: SHERWIN WILLIAMS SW#7030 ANEW GRAY         JLOR: SHERWIN WILLIAMS SW#7030 ANEW GRAY         JLOR: BLOREY	AR AR INTERNET	STARBUCKS - TRAMWAY 200 TRAWWAY BLVD. NE ALBUQUERQUE, NEW MEXICO 87123 PROJECT MANAGER JOB NO. DRAWN BY:
EXTERIOR N STUCCO-1 BRICK-1 MASONRY PAINT-1 GLAZ-1 WOOD-1 X COPING-1 SCREEN SOFFIT-1	ATERIALS         COAT EXTERIOR STUCCO SYSTEM         JCOR - SHERWIN WILLIAMS SW#7030 ANEW GRAY, SANDED TEXTURE         RCK VENER         JTUAL MATERIALS - PEWTER MISSION, RUNNING BOND WITH SM100 GRAY COLOR MORTI         ASONRY SCREEN         JTUAL MATERIALS - PEWTER MISSION, RUNNING BOND WITH SM100 GRAY COLOR MORTI         ASONRY SCREEN         JTUAL MATERIALS - PEWTER MISSION, RUNNING BOND WITH SM100 GRAY COLOR MORTI         SLOR: SHERWIN WILLIAMS SW#7033 BRAIN STORM BRONZE         JUAN: STOREFRONT SYSTEM WITH CLEAR LOW-E         JCDC: CARE BRONZE         JOD CLADDING         CHIHA FIBER CEMENT - VINTAGEWOOD- CEDER         LVANZED METAL COPING         JCD: SHERWIN WILLIAMS SW#7030 ANEW GRAY         -T/2" METAL B-DECK         JCD CLADDING         CHIHA FIBER CEMENT - VINTAGEWOOD- CEDER         LVANZED METAL COPING         JCD: SHERWIN WILLIAMS SW#7030 ANEW GRAY, SANDED TEXTURE         JCD CLADDING         CHIHA FIBER CEMENT - VINTAGEWOOD- CEDER		STARBUCKS - TRAMWAY 200 TRAWWAY BLVD. NE 200 TRAWWAY BLVD. NE 200 TRAWWAY BLVD. NE ALBUQUERQUE, NEW MEXICO 87123 PROJECT MANAGER JOB NO. DRAWN BY: 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7









# A. "G" SERIES SHEETS APPLY TO THE ENTIRE SET OF DRAWINGS. B. INDICATED DIMENSIONS ARE TO FACE OF FINISH; UNLESS OTHERWISE NOTED.

INDICATED ELEVATIONS ARE BASED ON A FINISHED FLOOR OF 100'-0".

# > KEYED NOTES

- SIGNAGE SHALL BE PROVIDED AND INSTALLED BY TENANT UNDER A SEPARATE PERMIT; CONTRACTOR TO PROVIDE ELECTRICAL POWER UNDER THIS CONTRACT. SURFACE MOUNTED LIGHT FIXTURE: SEE ELECTRICAL PLAN ON SHEET E101
- DRIVE-THRU SHELF.
- COVERED CANOPY.
- SCHEDULED DOOR AND FRAME: SEE FLOOR PLAN ON SHEET A101. LEADER HEAD AND DOWNSPOUT.
- DASHED LINE INDICATES 5/6" FIRE RETARDANT PLYWOOD BACKING AT SIGNAGE LOCATION.
- ALUMINUM STUCCO REVEAL: SEE XXX/AXXX.
- BRICK VENEER CONTROL JOINT: SEE XXX/AXX.
   HEATED AIR CURTAIN: AA300.
- EXPANSION JOINT: SEE XX/AXXX.
- . SCHEDULED STOREFRONT: SEE FLOOR PLAN ON SHEET A101 FOR ADDITIONAL INFORMATION.
- . GALVANIZED METAL COPING. DASHED LINE INDICATES MASONRY SCREEN IN FRONT.
   MASONRY SCREEN WALL AREA.

# SIGNAGE DATA TABLE

SIGNAGE LOCATION	REQUIREMENT	PROPOSED
VALL SIGN TAST ELEVATION DRIVE—THRU'	15% OF FACADE AREA (537 SF) INCLUSIVE OF DOORS AND WINDOWS: 81 SF ALLOWED	4 SF
VALL SIGN TAST ELEVATION: ROUND SIGNAGE	15% OF FACADE AREA (537 SF) INCLUSIVE OF DOORS AND WINDOWS: 81 SF ALLOWED	12.5 SF
VALL SIGN NORTH ELEVATION: STARBUCKS'	15% OF FACADE AREA (1,415 SF) INCLUSIVE OF DOORS AND WINDOWS: 212 SF ALLOWED	21 SF

\*Signs will be permitted separately and the colors and material will be determined at the time of sign permit.

EXTERIOR	MATERIALS
STUCCO-1	2 COAT EXTERIOR STUCCO SYSTEM COLOR – SHERWIN WILLIAMS SW#7030 ANEW GRAY, SANDED TEXTURE
BRICK-1	BRICK VENEER MUTUAL MATERIALS – PEWTER MISSION, RUNNING BOND WITH SM100 GRAY COLOR MORTAR
(MASONRY)	MASONRY SCREEN MUTUAL MATERIALS – PEWTER MISSION, RUNNING BOND WITH SM100 GRAY COLOR MORTAR
(PAINT-1)	STEEL CANOPY STRUCTURE COLOR: SHERWIN WILLIAMS SW#7033 BRAIN STORM BRONZE
GLAZ-1	ALUMINUM STOREFRONT SYSTEM WITH CLEAR LOW-E COLOR: DARK BRONZE
(WOOD-1)	WOOD CLADDING NICHIHA FIBER CEMENT – VINTAGEWOOD– CEDER
(COPING-1)	GALVANIZED METAL COPING COLOR: SHERWIN WILLIAMS SW#7030 ANEW GRAY
SCREEN	1–1/2" METAL B–DECK COLOR – SHERWIN WILLIAMS SW#7030 ANEW GRAY, SANDED TEXTURE
(SOFFIT-1)	WOOD CLADDING NICHIHA FIBER CEMENT – VINTAGEWOOD– CEDAR

WOOD CLADDING NICHIHA FIBER CEMENT – VINTAGEWOOD– CEDAR









# LANDSCAPE LEGEND

	QTY	SIZE	CØMMON/BOTANICAL
			Trees
	2	4 - 6'	Pinon 30x20 Pinus edulis
	2	6 - 8'	Pinon 30x20 Pinus edulis
	2	2" Cal. 6-8'	Desert Willow 20x25 Chilopsis linearis
	6	2" Cal 6-8'	Oklahoma Redbud 15x12 Cercis reniformis
TOTAL TREES	12		TOTAL TREE COVERAGE
	QTY	SIZE	CØMMON/BOTANICAL
		Shr	ubs & Groundcov
3 <sup>2373</sup> }3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3	11	5 Gal	Indía Hawthorne 3x5 Raphiolepis indica
*	26	5 Gal	Feather Reed Grass 2.5x2 Calamoarostis arudin
*	11	5 Gal	<b>Red Yucca 3x6</b> Hesperaloe parviflord
	1Ø	5 Gal	Buffalo Juniper 1x12 Juniperus sabina 'Bufi
	5	5 Gal	Fern Bush 5x6 Chamaebatiaria millefo
Z Z	5	5 Gal	Blue Rug Juniper Ix6 Juniperus horizontalis
	3	5 Gal	Apache Plume 6x1 Fallugia paradoxa
	3	5 Gal	Chamisa 5x1 Chrysothamnus nause
TOTAL SHRUBS	74		TOTAL SHRUB COVERAG
73	8	2-3cf	Boulders Bury 1/3, Moss Roc

			5869
	· · ·	·	812
			6681

3/4"	Crusher	Fin
Landscap	e Gravel /	Filte
2"-4"	´ Gravel	to
Totsl Lanc	dscape Ar	ea

# LANDSCAPE CALCULATIONS

TOTAL LOT AREA (sf) TOTAL BUILDING AREA (sf) TOTAL LOT AREA (sf) LANDSCAPE REQUIREMENT TOTAL LANDSCAPE REQUIRED

TOTAL ON-SITE LANDSCAPE PROVIDED TOTAL GROUNDCOVER REQUIRED TOTAL GROUNDCOVER PROVIDED TOTAL LIVE PLANTS REQUIRED TOTAL LIVE PLANTS PROVIDED

	NOISI
	REV
LANDSCAPE NOTES: Landscape maintenance shall be the responsibility of the Property Owner. The Property Owner shall maintain street trees in a living, healthy, and attractive condition.	BY
It is the intent of this plan to comply with Bernalillo County's requirements for landscape construction.	
Water management is the sole responsibility of the Property Owner.	DA
All landscaping will be in conformance with Bernalillo County Zoning Code.	
In general, water conservative, environmentally sound landscape principles will be followed in design and installation.	
Landscaping shall achieve 75% live ground cover at maturity.	<b>N</b>
A minimum of 50% of the landscape area are low water use or precipitation supported plant material.	
Green Stormwater Infrastructure and Low Impact Development techniques will be implemented to direct surface flows to landscape areas to slow down run off, and provide additional rain water to landscape areas. Curb cuts, sunken parking islands, and alternative mulches may be used.	<b>11</b> 338-14
PLANTING NOTES	<b>5</b> )
I. IT SHALL BE THE CONTRACTOR'S RESPONSIBILITY TO PREVENT PLANTS FROM FALLING OR BEING BLOWN OVER AND TO STRAIGHTEN OR REPLANT ALL PLANTS WHICH ARE DAMAGED DUE TO WIND. PLANTS BLOWN OVER BY HIGH WINDS SHALL NOT BE A	<b>CH</b> 00 871 871 871
THE FINANCIAL RESPONSIBILITY OF CONTRACTOR.	
CLODS, STIFF CLAY, HARD PAN, STONES LARGER THAN I" IN DIAMETER, NOXIOUS WEEDS AND PLANTS, SOD, PARTIALLY DISINTEGRATED DEBRIS INSECTS OR ANY OTHER INDESIRABLE	MEX
MATERIAL INCLUDING PLANTS OR SEEDS THAT WOULD BE TOXIC OR HARMEUL TO GROWTH	
3. CONTRACTOR IS RESPONSIBLE FOR VERIFICATION OF MATERIAL	
4. IN THE EVENT OF VARIATION BETWEEN THE PLANT QUANTITIES	лU П ЛЕ, 333
PLANS, THE PLANT LEGEND AND THE QUANTITIES SHOWN ON THE PLANS, THE PLANS SHALL CONTROL IMPROPER PLANT COUNT LIGTED ON THE PLANT LEGEND MADE BY THE LANDGGADE	
ARCHITECT SHALL BE NO CAUSE FOR ADDITIONAL COSTS TO THE	
5. THE CONTRACTOR SHALL MEET BOTH THE CONTAINER SIZE AND CALIPER SIZE, AS WELL AS HEIGHT AND SPREAD SPECIFICATIONS	
6. EXCAVATE TWO TIMES GREATER THAN THE ROOT BALL-DIAMETER OF THE SHRUB, TWO TIMES GREATER THAN THE ROOT BALL FOR	ALB PHC
TREES, SCARIFY BOTTOM OF PLANTING PIT BEFORE PLACING PLANT, PLACEMENT OF PLANT SHALL BE PERPENDICULAR TO GROUND	
<ol> <li>CONTRACTOR WILL NOT PLANT MATERIAL SHOWN ON PLANS WHEN IT IS EVIDENT THAT FIELD CONDITIONS HAVE CHANGED SINCE</li> </ol>	
PLANS WERE DRAWN, ANY CHANGES ARE TO BE BROUGHT TO THE ATTENTION OF THE OWNER'S REPRESENTATIVE BEFORE ANY	
PLANTING IS DONE IN THE AREA. 8. STEMS AND LEAVES TO BE REMOVED FROM LOWER PORTION OF	
TRUNKS OF TREES TO LEAVE A CLEAN APPEARANCE AND SO TREES APPEAR LESS LIKE SHRUBS AND MORE LIKE TREES.	
9. PLANT SUBSTITUTIONS WILL BE PERMITTED WITH WRITTEN APPROVAL OF OWNER'S REPRESENTATIVE REQUEST SUBSTITUTIONS	
IN WRITING GIVING REASONS FOR SUCH SUBSTITUTIONS, DOCUMENT THAT REASONABLE EFFORT HAS BEEN MADE TO LOCATE SPECIES	
ORIGINALLY SPECIFIED. NO PLANT OR TREE SUBSTITUTIONS ALLOWED UNLESS PRIOR APPROVAL FROM OWNER'S	Landscape Architect August <u>Ø3,</u> 2023
REPRESENTATIVE. 10. REMOVE ALL WIRE, STRING, WIRE BASKETS, BURLAP, CONTAINERS,	EOFNER
ETC., FROM THE ROOTBALL OF PLANTS BEFORE BACKFILLING THE PLANTING HOLE.	Shir Hander
II. FINAL LOCATION OF ALL PLANT MATERIAL SHALL BE SUBJECT TO THE APPROVAL OF THE LANDSCAPE ARCHITECT.	
12. CONTRACTOR SHALL NOTIFY OWNER'S REPRESENTATIVE 48 HOURS IN ADVANCE OF COMMENCEMENT OF WORK TO COORDINATE	DANNA, D. MITCHELL
PROJECT INSPECTION SCHEDULES. 13. CONTRACTOR SHALL PROVIDE ONE YEAR WARRANTY ON PLANT	SCAPE ARCH
MATERIAL FROM DATE OF SUBSTANTIAL COMPLETION, DEAD OR DAMAGED PLANT MATERIAL SHALL BE REPLACED AT NO COST	
TO THE OWNER UNLESS CAUSED BY FACTORS OUTSIDE THE	
14. ALL AREAS TO RECEIVE LANDSCAPE ROCK WITH WEED BARRIER EABRIC SHALL RECEIVE PENDULUM OR OTHER APPROVED FOULD	
PRE-EMERGENT OR COMBINATION OF PRE-EMERGENTS. APPLICATION SHALL TAKE PLACE AFTER SOIL PREPARATION AND	DTN
PRIOR TO INSTALLATION OF WEED BARRIER FABRIC AND SHALL BE ARRIED BY LICENSED ARRIVATOR PROVIDED BY	
CONTRACTOR. PRE-EMERGENT TO PROVIDE PREVENTION OF ALL INVAGIVE	
WEEDS AND GRASSES, INCLUDING BERMUDA GRASS, BUT SHALL	A M MMAY
PROTECTION OF EXISTING PLANTS AND TREE ROOTS SYSTEMS SHALL BE PROVIDED PRIOR TO APPLICATION AND AS PER	B-TRAM
MANUFACTURER RECOMMENDATIONS, SUBMIT PRE-EMERGENT MSDS	8712 s
IN UNHATION TO OWNER.	XICO I
	▼ WE: V NE (∪
	PLAN UEV
Mitchell Associates inc	APE APE
	TTTLE TAL TAL TRA UQUE TMANA TMANA TILE



lter Fabric Match Existing

21253 -131Ø 19943 X.15 2991	
668   67Ø 2869 5Ø   6583	

SCALE: 1" = 20'-0"

GRAPHIC SCALE

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2Ø

505.639.9583

and the statistics

ndscape Architect lugust 03, 2023 OF WANKIN DANNY, D. MITCHELL 239 WHY CISTERED IT SCAPE ARCH 123 ΞЩ BLVD. NEW A 1 24 May 2023 LS-101 AS NOTED

# Property Owner shall maintain street trees in a condition. 3Øx2Ø 4ØØ 8ØØ M It is the intent of this plan to comply with Bei landscape construction. Water management is the sole responsibility of 3*0*x2*0 400 800* M All landscaping will be in conformance with Be In general, water conservative, environmentall 20x25 625 1250 M be followed in design and installation. Landscaping shall achieve 75% live ground 144 864 M A minimum of 50% of the landscape area are l supported plant material. 3714 Green Stormwater Infrastructure and Low Impa be implemented to direct surface flows to la off, and provide additional rain water to land parking islands, and alternative mulches may 4. IN THE EVENT OF VARIATION BETWEEN

H20 USE

20 Landscape Architecture Danny Mitchell danny@mitchellassociatesinc.com

1Ø



Wenonah Ave SE

# POINT OF CONNECTION IRRIGATION CONTROLLER BACKFLOW PREVENTION DEVIC MASTERVALVE ASSEMBLY MAINLINE ELECTRIC ZONE VALVE SLEEVES Irrigation Lateral for tree rings Drip Line, Shrub Drip Emitter Line Drip Emmiter, TREE IRRIGATION

# IRRIGATION NOTES

- CONTRACTOR SHALL VERIFY ALL DIMENSIONS, ELEVATIO EQUIPMENT QUANTITIES, AND UTILITY LOCATIONS PRIOR T WORK.
- CONTRACTOR SHALL NOTIFY LANDSCAPE ARCHITECT OF DISCREPANCIES IN PLANS OR SPECIFICATIONS PRIOR TO OR CONTINUING WORK.
- THE IRRIGATION CONTRACTOR SHALL MAKE NO SUBSTITUT DELETIONS, OR ADDITIONS TO THIS PLAN WITHOUT APPRO LANDSCAPE ARCHITECT.
- ALL CONSTRUCTION SHALL CONFORM TO CITY, COUNTY, 4. AND FEDERAL REQUIREMENTS. IT SHALL BE THE RESPONSIBILITY OF THE IRRIGATION CONTRACTOR TO ENSURE THAT ALL IRRIGATION EQUIPMENT MEETS GOVERNMENT REGULATIONS, CONTRACTOR SHALL ALSO BE RESPONSIBLE FOR OBTAINING ANY NECESSARY PERMITS OR APPROVALS.
- THIS PLAN IS SCHEMATIC AND DUE TO THE NATURE OF CONSTRUCTION SLIGHT FIELD MODIFICATIONS MAY BE NECESSARY TO IMPLEMENT PLAN.
- 6. IRRIGATION SYSTEMS CONNECTED TO POTABLE WATER SUPPLY, SHALL HAVE A BACKFLOW PREVENTER INSTALLED.
- 1. IRRIGATION LATERAL LINES, MAIN LINES AND EQUIPMENT MAY BE SHOWN OUTSIDE PROPERTY LINES ON THIS PLAN, ALL IRRIGATION LINES AND EQUIPMENT ARE TO BE WITHIN AND INSTALLED WITHIN THE LIMITS OF THE PROPERTY LINE.
- 8. ALL IRRIGATION SLEEVING TO BE THE RESPONSIBILITY OF THE IRRIGATION CONTRACTOR. ELECTRICAL WIRES FOR IRRIGATION VALVES AND IRRIGATION LINES ARE TO BE PLACED IN. SEPARATE SLEEVES. SEE SLEEVING DETAIL.
- 9. SUPPLY LINE AND WATER METER TO BE PROVIDED BY OWNERBACKFLOW PREVENTOR TO BE PROVIDED BY IRRIGATION CONTRACTOR. IRRIGATION CONTRACTOR'S POINT OF CONNECTION TO BEGIN DOWNSTREAM OF THE IRRIGATION WATER METER.

# IRRIGATION LEGEND

	MANUFACTURER
	NA
	HUNTER
CE. RPA	FEBCO (OR EQUA
	HUNTER ICV
	Sch 40 PVC
	HUNTER ICV
	Class 200 PVC
5	Class 200 PVC.
ine	Poluline
	RAINBIRD

I", Install Shut off valve. Smart Controller EQUAL) 1" / Freeze Protection Required

Irrigation maintenance shall be the responsibility of the Property Owner.

Water and Power source shall be the responsibility of the Developer/Builder.

SIZE / NOTES

I" Typ. 2 SIZES LARGER THAN PIPE TO BE SLEEVED. 3/4" SEE DETAIL

Size Equipment as Required for Flow Rate

— ONS, TO BEGINNING	IRRIGATION NOTES: Irrigation system maintenance and operation shall be the sole responsibility of the owner. It shall be the owners responsibility to ensure that fugitive water does not leave the site due to overwatering.
F ANY O BEGINNING	Irrigation shall be a complete underground system with Trees to receive 1 Netafim spiral (50' length) with 3 loops at a final radius of 4.5' from tree trunk, pinned in place. Netafim shall have emitters 12" o.c. with a flow of .6 gph. Shrubs to receive (2) 1.0 GPH Drip Emitters. Drip and Bubbler systems to be tied to 1/2" polypipe with flush caps at each end.
ROVAL OF THE	Point of connection for irrigation system is unknown at current time and will be coordinated in the field. Irrigation will be operated by automatic controller.
STATE,	Location of controller to be field determined and power source for controller

to be provided by others.

Mitchell Associates, inc

and an and the state of the second Landscape Architecture Danny Mitchell

danny@mitchellassociatesinc.com



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100 SUN AVENUE N.E., Ste 600 Albuquerque, New Mexico 87109 Phone (505) 338-1499 Fax (505) 338-1498

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ARCHITEC

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DATE: 24 May 2023 SCALE:	sheet-
PRQUECT TITLE	PROJECT MANAGER
STARBUCKS -	DEVIN NGUYEN
200 TRAMWAY BLVD. NE	SHEETTITLE
ALBUQUERQUE, NEW MEXICO	IRRIGATION PLAN
- TRAMW	JOB NO.
87123	SB-TRAMWAY
$\succ$	drawn e DTN

of-





SCALE: 1" = 20'-0"



<u>NOTE:</u> ATMOSPHERIC VACUUM BREAKERS SHOULD BE INSTALLED 6 - 12" ABOVE THE HIGHEST SPRINKLER HEAD WITHIN THE ZONE, OR, ACCORDING TO LOCAL CODE.

NOTES: 1. INSTALLATION TO BE COMPLETED IN ACCORDANCE WITH MANUFACTURER'S SPECIFICATIONS, 2. DO NOT SCALE DRAWINGS, 3. CONTRACTORS NOTE: FOR PRODUCT AND COMPANY INFORMATION VISIT www.CADdetails.com/info REFERENCE NUMBER 901-085n.







LID TO MATCH GRAVEL COLOR

1040

- FINISHED GRADE

-ELECTRIC VALVE

-LATERAL LINE -3" LAYER 3/4" CRUSHED ROCK

\_ MAINLINE

VALVE BOX



A. TREE TRUNK/ROOT CROWN B. 24" CIRCLE FROM TRUNK C. EMITTERS D. 1/8" DISTRIBUTION LINE E. PE DRIPLINE F. EMITTER PLACED WITHIN 6" OF PLANT STEM NOTE: PLACE EMITTER ABOVE PLANT ON SLOPE





N.T.S.

NOT TO SCALE



Alter States Landscape Architecture Danny Mitchell

danny@mitchellassociatesinc.com

505.639.9583







Scale : 1" = 40'

# PRIVATE DRAINAGE FACILITIES WITHIN CITY RIGHT-OF-WAY (SO-19)

1. BUILD SIDEWALK CULVERTS PER COA STD DWG 2236.

2. CONTACT STORM DRAIN MAINTENANCE AT (505) 857-8033 TO SCHEDULE A MEETING PRIOR TO

3. AN EXCAVATION PERMIT WILL BE REQUIRED BEFORE BEGINNING ANY WORK WITHIN CITY

4. ALL WORK ON THIS PROJECT SHALL BE PERFORMED IN ACCORDANCE WITH APPLICABLE FEDERAL, STATE AND LOCAL LAWS, RULES AND REGULATIONS CONCERNING CONSTRUCTION SAFETY AND

TWO WORKING DAYS PRIOR TO ANY EXCAVATION, THE CONTRACTOR MUST CONTACT NEW MEXICO ONE CALL, DIAL "811" [OR (505) 260-1990] FOR THE LOCATION OF EXISTING UTILITIES.

PRIOR TO CONSTRUCTION, THE CONTRACTOR SHALL EXCAVATE AND VERIFY THE LOCATIONS OF ALL OBSTRUCTIONS. SHOULD A CONFLICT EXIST, THE CONTRACTOR SHALL NOTIFY THE ENGINEER SO THAT THE CONFLICT CAN BE RESOLVED WITH A MINIMUM AMOUNT OF DELAY.

BACKFILL COMPACTION SHALL BE ACCORDING TO TRAFFIC/STREET USE.

MAINTENANCE OF THE FACILITY SHALL BE THE RESPONSIBILITY OF THE OWNER OF THE PROPERTY BEING SERVED.

9. WORK ON ARTERIAL STREETS MAY BE REQUIRED ON A 24-HOUR BASIS.

10. CONTRACTOR MUST CONTACT STORM DRAIN MAINTENANCE AT (505) 857-8033 TO SCHEDULE A CONSTRUCTION INSPECTION. FOR EXCAVATING AND BARRICADING INSPECTIONS, CONTACT CONSTRUCTION COORDINATION AT (505) 924-3416.

		-
	CURB & GUTTER	
	BOUNDARY LINE	
	RIGHT-OF-WAY	/ /
<u></u>	BUILDING	
	CONCRETE/SIDEWALK	
)	CONTOUR MAJOR	
1	CONTOUR MINOR	
25	SPOT ELEVATION	
-	FLOW ARROW	
	EXISTING CURB & GUTTER	
)— — —	EXISTING CONTOUR MAJOR	*
1	EXISTING CONTOUR MINOR	
8.25	EXISTING SPOT ELEVATION	
~~~~~	GRADE BREAK	
<del>,,,,,,,,,,,</del>	RETAINING WALL	

# SPOT ELEVATION LEGEND

ALL EXISTING UTILITIES/TOPOGRAPHY SHOWN WERE OBTAINED FROM RESEARCH, AS-BUILTS, SURVEYS OR INFORMATION PROVIDED BY OTHERS. IT SHALL BE THE SOLE RESPONSIBILITY OF THE CONTRACTOR TO CONDUCT ALL NECESSARY FIELD INVESTIGATIONS PRIOR TO AND INCLUDING ANY EXCAVATION, TO DETERMINE THE ACTUAL LOCATION OF UTILITIES AND OTHER IMPROVEMENTS, PRIOR TO STARTING THE WORK. ANY CHANGES FROM THIS PLAN SHALL BE COORDINATED WITH AND APPROVED BY THE



		EV DATE DESCRIPTION
GREAT BASIN ENGINEERING - SOUTH CONSULTING ENGINEERS and LAND SURVEYORS	Salt Lake City, Utah 84116	ince-izn(ino) xou pozi-ten(ino) waten ezen-izn(ino) fun tak tan 100
Preliminary Grading / Utility Plan	Smith's Food and Drug Stores	Salt Lake City, Utah 84104 Telephone (801) 974–1400
	HE WAL	2
22 O	FEELONIA CL. 200	to series

427GAS



# DPM Weighted E Method CH 6 Precipitation Zone 3 STARBUCKS ON TRAMWAY 5/24/2023 Date

<u>Equations:</u>

Weighted E = Ea\*Aa + Eb\*Ab + Ec\*Ac + Ed Volume = Weighted E \* Total Area Flow = Qa\*Aa + Qb\*Ab + Qc\*Ac + Qd\*Ad

HYDROLOGY CALCULATION TABLES

Basin Descriptions										100	)-Year, 6-H	r			
Basin	Troat	Area	Area	Area	Treatm	nent A	Treat	nent B	Treatm	nent C	Treatme	ent D	Weighted E	Volume	Flow
ID	Tract	(sf)	(acres)	(sq miles)	%	(acres)	%	(acres)	%	(acres)	%	(acres)	(in)	(ac-ft)	cfs
H1		195,800	4.49	0.00702	0%	0.000	0%	0.000	10%	0.449	90%	4.045	2.431	0.911	19.59
Total		195,800	4.49	0.00702		0.000		0.000		0.000		4.045		0.911	19.589

# **Proposed Conditions**

TWLLC

Basin Descriptions									100	-Year, 6-H	r				
Basin	Troat	Area	Area	Area	Treatme	ent A	Treatr	nent B	Treatm	ient C	Treatmer	nt D	Weighted E	Volume	Flow
ID	Tract	(sf)	(acres)	(sq miles)	%	(acres)	%	(acres)	%	(acres)	%	(acres)	(in)	(ac-ft)	cfs
H1		171,000	3.93	0.00613	0%	0.000	0%	0.000	10%	0.393	90%	3.533	2.431	0.795	17.11
D1		23,640	0.54	0.00085	0%	0.000	0%	0.000	30%	0.163	70%	0.3799	2.133	0.096	2.22
D2		3,135	0.07	0.00011	0%	0.000	0%	0.000	30%	0.022	70%	0.0504	2.133	0.013	0.29
Total		197,775	4.54	0.00709		0.000		0.000		0.577		3.963		0.905	19.624

SWQV-A POND VOLUME CALCULATIONS									
ELEVATION (ft)	AREA (sf)	VOLUME (cf)	CUMULATIVE VOLUME (cf)						
5650	199	0	0						
5651	525	362	362						

PROPOSED STORM WATER POND VOLUME TABLES

SWQV-B POND VOLUME CALCULATIONS								
ELEVATION (ft)	AREA (sf)	VOLUME (cf)	CUMULATIVE VOLUME (cf)					
5648	<mark>3</mark> 6	0	0					
5649	156	96	96					
5650	344	250	346					

SWOV & BOND VOLUME CALOUL ATIONS									
SWQV-C POND VOLUME CALCULATIONS									
ELEVATION (ft)	AREA (sf)	VOLUME (cf)	CUMULATIVE VOLUME (cf)						
5647.5	29	0	0						
5648	56	21.25	21						
5649	131	93.5	115						

-C	POND	VOL	UME	CAL	CUL	ATIONS	<i>i</i>

# REQUIRED STORM WATER QUALITY VOLUME CALCULATIONS

SCALE: 1"=60'

Basin D1 Storm Water Quality Volume								
Total Impervious Area =	ΣArea in "Treatment D"							
Retainage depth = 0.42" Per DPM Pg. 272	0.035	FT						
Retention Volume =	=0.035 x Area D	CF						
Area D (0.3799 acres)	16548	SF						
Required Retention Volume	579	CF						
Pond Volume Provided	708	CF						

Basin D2 Storm Water Quality Volume							
Total Impervious Area =	ΣArea in "Treatment D"						
Retainage depth = 0.42" Per DPM Pg. 272	0.035	FT					
Retention Volume =	=0.035 x Area D	CF					
Area D (0.0504 acres)	2195	SF					
Required Retention Volume	77	CF					
Pond Volume Provided	115	CF					



FIRM MAP:

35001C0354H

# EXISTING CONDITIONS

THE SUBJECT SITE IS LOCATED WITHIN A LARGER BASIN INDICATED AS AREA "B" IN THE DRAINAGE REPORT FOR SMITHS STORE #427 IN HYDRONUM FILE: L23D015. CURRENTLY THE SITE IS PAVED WITH PARKING SPACES AND THE STORM WATER DISCHARGE FREE FLOWS INTO VARIOUS STORM DRAIN INLETS ON THE SOUTHERN PART OF THE SMITHS LARGER TRACT AS SHOWN IN THE EXISTING CONDITIONS.

# PROPOSED CONDITIONS

A NEW COFFEE DRIVE THRU SHOP IS PROPOSED TO BE DEVELOPED AND WILL REPLACE EXISTING PARKING SPACES. THE DEVELOPED STORM WATER DISCHARGE SHALL BE ROUTED THROUGH STORM WATER QUALITY PONDS INSTALLED WITHIN PLANTERS PRIOR TO BEING DISCHARGED INTO THE EXISTING STORM DRAIN INLETS AS SHOWN IN THE PROPOSED CONDITIONS BASIN MAP.



Nyloplast 2' x 3' Steel Bar / MAG Grate Inlet Capacity Chart









# LEGEND

	CURB & GUTTER
	BOUNDARY LINE
	RIGHT-OF-WAY
	BUILDING
	CONCRETE/SIDEWALK
5010	CONTOUR MAJOR
5011	CONTOUR MINOR
x 5048.25	SPOT ELEVATION
	FLOW ARROW
	EXISTING CURB & GUTTER
5010	EXISTING CONTOUR MAJOR
— — — —5011– — — –	EXISTING CONTOUR MINOR
× 5048.25	EXISTING SPOT ELEVATION
	GRADE BREAK

# SPOT ELEVATION LEGEND

SW= TOP OF SIDEWALK TC= TOP OF CURB FL= FLOW LINE FF= FINISHED FLOOR

# SPOT ELEVATION NOTE:

ALL PROPOSED SPOT ELEVATIONS ARE FLOWLINE UNLESS OTHERWISE NOTED.

# CAUTION

ALL EXISTING UTILITIES/TOPOGRAPHY SHOWN WERE OBTAINED FROM RESEARCH, AS-BUILTS, SURVEYS OR INFORMATION PROVIDED BY OTHERS. IT SHALL BE THE SOLE RESPONSIBILITY OF THE CONTRACTOR TO CONDUCT ALL NECESSARY FIELD INVESTIGATIONS PRIOR TO AND INCLUDING ANY EXCAVATION, TO DETERMINE THE ACTUAL LOCATION OF UTILITIES AND OTHER IMPROVEMENTS, PRIOR TO STARTING THE WORK. ANY CHANGES FROM THIS PLAN SHALL BE COORDINATED WITH AND APPROVED BY THE ENGINEER.





(FOR R	ETAINING	PORT	ON ONLY)	
Н	В	Т	Y-BARS	X-BARS
ft.—in.	ft.—in.	in.		
2'-0"	2'-0"	9"	#4 @ 24" O.C.	



	CURB & GUTTER
	BOUNDARY LINE
	EASEMENT
	BUILDING
	SIDEWALK
	RETAINING WALL
	CURB AND GUTTER
	EXISTING CURB & GUTTER
	EXISTING BOUNDARY LINE
2	
0 ~	SINGLE CLEAN OUT
	DOUBLE CLEAN OUT
<u>(O</u> !	EXISTING SD MANHOLE
5	EXISTING SS MANHOLE
, , , ,	EXISTING FIRE HYDRANT
0	EXISTING WATER METER
0	EXISTING POWER POLE
ତ	EXISTING GAS VALVE
— — OH— — —	EXISTING OVERHEAD UTILITIES
SAS	EXISTING SANITARY SEWER LINE
— — — —EX. WL— — — —	EXISTING WATER LINE
—— · ——EX. RCP—— · ——	EXISTING STORM SEWER LINE
UU	PROPOSED TELEPHONE LINE
E	PROPOSED ELECTRIC LINE
G	PROPOSED GAS LINE
w	PROPOSED WATER LINE
SAS	PROPOSED SANITARY SEWER LINE
S	PROPOSED SAS MANHOLE
Ø	PROPOSED WATER METER

- CITY OF ALBUQUERQUE SPECIFICATIONS FOR PUBLIC UTILITY CONSTRUCTION,

- PRIOR TO DRB REVIEW. PNM'S STANDARD FOR PUBLIC UTILITY EASEMENTS IS

FIGURE 12

Date Submitted:

Date Site Plan Approved:

Date Preliminary Plat Approved:\_\_\_\_\_ Date Preliminary Plat Expires:\_\_\_\_

### INFRASTRUCTURE LIST

### (Rev. 2-16-18)

### EXHIBIT "A"

DHO Project No.: DHO Application No.:

TO SUBDIVISION IMPROVEMENTS AGREEMENT DEVELOPMENT HEARING OFFICER (DHO) REQUIRED INFRASTRUCTURE LIST

### PLAT FOR TRACTS J-1-A AND J-1-B, FOUR HILLS VILLAGE SHOPPING CENTER AND APARTMENT COMPLEX

### PROPOSED NAME OF PLAT

### TRACT J-1 REPL OF TRS F, H-1, J & UNPLATTED LAND FOUR HILLS VILLAGE SHOPPING CENTER & APT COMPLEX CONT 7.9444 AC +/- OR 346,058 SF +/-EXISTING LEGAL DESCRIPTION PRIOR TO PLATTING ACTION

Following is a summary of PUBLIC/PRIVATE Infrastructure required to be constructed or financially guaranteed for the above development. This Listing is not necessarily a complete listing. During the SIA process and/or in the review of the construction drawings, if the DRC Chair determines that appurtenant items and/or unforeseen items have not been included in the infrastructure listing, the DRC Chair may include those items in the listing and related financial guarantee. Likewise, if the DRC Chair determines that appurtenant or non-essential items can be deleted from the listing, those items may be deleted as well as the related portions of the financial guarantees. All such revisions require approval by the DRC Chair, the User Department and agent/owner. If such approvals are obtained, these revisions to the listing will be incorporated administratively. In addition, any unforeseen items which arise during construction which are necessary to complete the project and which normally are the Subdivider's responsibility will be required as a condition of project acceptance and close out by the City.

							Const	ruction Cert	ification
Financially	Constructed	Size	Type of Improvement	Location	From	То	Priva	ate	City Cnst
Guaranteed	Under						Inspector	P.E.	Engineer
DRC #	DRC #								
		4' Diameter	Public SAS Manhole	NE Corner of Tramway			/	1	/
<u> </u>				Plud and Wananah Ava					
				Bivu anu wenonan Ave.					
		Varies	Lengthen Median Turn Lane by 75'	Tramway Blvd	Central Ave	100 ft south of	/	/	/
		0'-12' wide	including Major collector paving	median		Central Ave			
			median curb and gutter.						
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PAGE \_\_\_\_\_ OF \_\_\_\_\_

Financially	Constructed						Caracter (	0
Suarantood	Linder	Sizo	Type of Improvement	Location	E rom	Ťe	Construction	Certification
DRC #		5126	Type of improvement	Location	From	10	Private	
DICC #	DRC #						Inspector P.E	Engine
			Engineer's Certification for Grading & Drainage is	required for release of Financial Gu	arantee		1 1	1
							1 1	1
					Approval of Creditable Items:		Approval of Credita	able Items:
					Impact Fee Admistrator Signa	ture Date	City User Dept. Si	gnature
				NOTES				
		If the site is	s located in a floodplain, then the finan	cial guarantee will not be rel	eased until the LOMR is approve	d by FEMA.		
			Street I	ights per City rquirements.				
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2 3	AGENT / OWNER			DEVELOPMENT RE	VIEW BOARD MEMBER APPROV	/ALS		
2 3	AGENT / OWNER	[ A D\$		DEVELOPMENT RE	VIEW BOARD MEMBER APPROV	/ALS		
2 3 Vincen	AGENT / OWNER	A, <u>P£</u>		DEVELOPMENT RE	VIEW BOARD MEMBER APPROV	/ALS		
2 3 Vincen	AGENT / OWNER TCARR (c i NAME (print)	A PE	PLANN	DEVELOPMENT RE	VIEW BOARD MEMBER APPROV	/ALS ECREATION - d	late	
2 3 Vincen Tierri	AGENT / OWNER TCARRICI NAME (print) 4 West L	A, PE LC	PLAN	DEVELOPMENT RE	VIEW BOARD MEMBER APPROV	/ALS ECREATION - d	date	
2 3 	AGENT / OWNER TCARRICI NAME (print) + West L FIRM-	<u> </u>	PLAN	DEVELOPMENT RE	VIEW BOARD MEMBER APPROV	/ALS ECREATION - d FCA - date	date	
2 3 Vincen Tierri	AGENT / OWNER TCARRICI NAME (print) 4 West L FIRM	<u> </u>	PLANN TRANSPORTATION	DEVELOPMENT RE	VIEW BOARD MEMBER APPROV	/ALS ECREATION - d FCA - date	date	
2 3 Vinceni Tierri	AGENT / OWNER TCARRICI NAME (print) 4 West L FIRM	<u>Pz</u> <u>LC</u> <u><u><u>9</u>-7-2</u></u>	PLANN TRANSPORTATION	DEVELOPMENT RE	VIEW BOARD MEMBER APPROV	/ALS ECREATION - d FCA - date	date	
2 3 Vincen TIERRI	AGENT / OWNER TCARRICI NAME (print) 4 West L FIRM SIGNATURE - date	<u><u><u></u></u> <u>LC</u> <u><u><u></u><u></u><u><u></u><u></u><u></u><u><u></u><u></u><u></u><u></u><u></u><u></u><u></u><u></u><u></u><u></u><u></u><u></u></u></u></u></u></u>	PLANN TRANSPORTATION	DEVELOPMENT RE IING- date DEVELOPMENT - date ELOPMENT - date	VIEW BOARD MEMBER APPROV	/ALS ECREATION - d FCA - date DRCEMENT - da	date	
2 3 Vincen Tisrri	AGENT / OWNER TCARRICI NAME (print) 4 West L FIRM SIGNATURE - date	<u>PE</u> <u>LC</u> <u><u>9-7-</u>2</u>	PLANN TRANSPORTATION	DEVELOPMENT RE IING- date DEVELOPMENT - date ELOPMENT - date	VIEW BOARD MEMBER APPROV PARKS & RE AMA CODE ENFO	/ALS ECREATION - d FCA - date DRCEMENT - da	date	

REVISION	DATE	DRC CHAIR	USER DEPARTMENT	AGENT /OWNER

PLANNING DEPARTMENT URBAN DESIGN & DEVELOPMENT DIVISION 600 2nd Street NW, 3rd Floor, Albuquerque, NM 87102 P.O. Box 1293, Albuquerque, NM 87103 Office (505) 924-3860 Fax (505) 924-3339



# OFFICIAL NOTIFICATION OF DECISION

July 20, 2023

SWI Real Estate LTD PTNS c/o Nickel & Co LLC 1014 Vine St. Floor 7th Cincinnati, OH 45202 Project # PR-2023-008767 SI-2023-001056- Site Plan - EPC, Major Amendment

# **LEGAL DESCRIPTION:**

Modulus Architects & Land Use Planning Inc. agent for SWI Real Estate PTNS c/o Nickle & Co LLC, requests a Site Plan-EPC Major Amendment, for all or a portion of Tract J-1, Replat of Tracts F, H-1, J and unplatted land, Four Hills Village Shopping Center and Apartment Complex, located south of Central Ave. SE, north of Wenonah Ave. SE, and between Tramway Blvd. SE and Four Hills Rd. SE (200 Tramway Blvd. SE), zoned MX-M, approximately 8.5 acres (L-22-Z)(L-23-Z) Staff Planner: Megan Jones

On July 20, 2023 the Environmental Planning Commission (EPC) voted to Approve Project # PR-2023-008767, SI-2023-001056 - Site Plan- EPC, Major Amendment based on the following Findings and subject to the following Conditions of Approval:

# FINDINGS - SI-2023-001056:

- 1. The request is for a Major Amendment for an approximately 8.5-acre site legally described all or a portion of Tract J-1, Replat of Tracts F, H-1, J and unplatted land, Four Hills Village Shopping Center and Apartment Complex, located at 200 Tramway Blvd. SE., south of Central Ave. SE, north of Wenonah Ave. SE, and between Tramway Blvd. SE and Four Hills Rd. SE (the "subject site").
- 2. The applicant wishes to amend the controlling Site Development Plan to allow a 1,310 SF restaurant with accessory drive-through use on the southwestern portion of the subject site. The amendment would facilitate development of the proposed Site Plan for a coffee shop associated with this request, which includes parking, landscaping, utilities, grading and drainage, which would be delegated to the DFT for final-sign off.
- 3. The proposed future Site Plan associated with this request is contingent upon the EPC's approval of the Major Amendment.
- 4. The subject site is zoned MX-M (Mixed-use Moderate Intensity), a zoning designation received upon adoption of IDO in May 2018. The subject site was formerly zoned SU-2/EG-CAC.

- 5. The amendment exceeds the thresholds found in IDO table 6-4-4: Allowable Minor Amendments, therefore it is classified as a Major Amendment pursuant to IDO section 14-16-6-4(Z)(1)(b). Pursuant to IDO Section 14-16-6-4(Q), the decision-making body may impose conditions necessary to ensure compliance with the development standards of this IDO via the Site Plan-EPC Review and Decision Criteria in IDO Section 14-16-6-6(J).
- 6. The subject site is located in an Area of Change, within the Four Hills Village Activity Center, within 660-feet of the Central Ave. Major Transit Corridor and Premium Transit Corridor as designated by the Comprehensive Plan.
- 7. The Albuquerque/Bernalillo County Comprehensive Plan and the Integrated Development Ordinance (IDO) are incorporated herein by reference and made part of the record for all purposes.
- 8. The request is generally consistent with the following Comprehensive Plan Goal from Chapter 4: Community Identity

<u>Goal 4.1.2– Character, Identity and Design</u>: Protect the identity and cohesiveness of neighborhoods by ensuring the appropriate scale and location of development, mix of uses, and character of building design.

This request would facilitate future development that is the appropriate scale and location in the existing Grocery Store Shopping Center, the Four Hills Village Activity Center, and a mixed-use zone district. Although, Activity Centers are intended to incorporate pedestrian friendly design and convenient day-to-day services at a neighborhoods scale within a walking or bike riding distance, the applicant has incorporated the required pedestrian design elements. The restaurant will be accessible by residents of the single-family neighborhood to the east and south, employees and shoppers in the Activity Center, and commuters along Tramway Blvd.

Future development will be subject to IDO requirements which will help to ensure appropriate character of building design and scale so that the surrounding neighborhood and Activity Center is not adversely affected. The new site plan will follow the Design Standards of the (IDO) MX-M Zone District dimensional standards (Table 5-1-2), buffer landscaping (14-16-5-6-(E)), and building design standards (14-16-5-11). Additionally, Use-specific standards for a drive-through will be required to be met.

- 9. The request is consistent with the following Comprehensive Plan Policies regarding Centers and Corridors and Efficient Development Patters from Chapter 5: Land Use
  - A. <u>Policy 5.1.6 Activity Centers:</u> Foster mixed-use centers of activity with a range of services and amenities that support healthy lifestyles and meet the needs of nearby residents and businesses.

The request would capture growth in Centers and Corridors. The subject site is located in the Four Hills Village Activity Center. Activity Centers are intended to incorporate a mix of neighborhood scale services within mixed use centers. They are intended to serve neighborhoods within a 20-minute walk or short bike ride. The development of a restaurant (coffee shop) with drive-through will provide a convenient service to the surrounding neighborhoods and retail in the area. It is within walking distance of the single-family residential development east of the subject site, and is accessible by transit.

Additionally, the subject site is located in an Area of Change which intends for development to benefit the surrounding community. Development of the subject site on a Major Transit Corridor and Activity Center will direct growth to the Activity Center as well as help to shape the built environment into a sustainable development pattern.

B. <u>Policy 5.3.1 Infill Development:</u> Support additional growth in areas with existing infrastructure and public facilities.

The subject site is within an existing Shopping Center (the controlling Site Development Plan), the Four Hills Village Activity Center, and within 600' of the Central Ave. Major Transit Corridor. The subject site is served by existing infrastructure and public facilities on the subject site, which future development will utilize. The development made possible by the request would support additional growth in an area with existing infrastructure and public facilities.

- 10. The request is generally consistent with the following Comprehensive Plan Goal and Policies regarding City Development Areas from Chapter 5: Land Use
  - A. <u>Goal 5.6 City Development Areas</u>: Encourage and direct growth to Areas of Change where it is expected and desired to ensure that development in and near Areas of Consistency reinforces the character and intensity of the surrounding area.

The subject site is located in an Area of Change where growth is expected and desired. The request would encourage, enable, and direct neighborhood scale growth to it.

B. <u>Policy 5.6.2 – Areas of Change:</u> Direct growth and more intense development to Centers, Corridors, industrial and business parks, and Metropolitan Redevelopment Areas where change is encouraged.

The amendment would facilitate development to an Area of Change within the Four Hills Village Activity Center and within 600' of a Major Transit Corridor. The amendment to the controlling SDP is consistent with this policy because its location is appropriate for small scale commercial uses and is accessible by all transportation modes. The request would facilitate small scale growth to a Center and Corridor where change is encouraged.

C. <u>Sub-Policy 5.6.2.h:</u> Encourage development in areas with a highly connected street grid and frequent transit service.

The request furthers this policy because the subject site is located in an area with a connected street grid in the SE quadrant of the City. It is within 600' of the Central Ave. Major Transit Corridor and Premium Transit Corridor, which both connect to transit service located on Tramway Blvd. The subject site is well served by transit; Juan Tabo Bus Route 1 and Bus Route 66 as well as the Green Line Rapid Ride Route 777. All three routes offer service Monday through Sunday with a peak frequency of 15-25 minutes. There are two Bus Stops at the NW corner of Wenonah Ave. SE and Tramway Blvd. SE.

 The request is generally consistent with the following Comprehensive Plan Policies from Chapter 8: Economic Development

<u>Policy 8.1.2 Resilient Economy</u>: Encourage economic development efforts that improve quality of life for new and existing residents and foster a robust, resilient, and diverse economy.

<u>Sub-Policy 8.1.2.c</u>: Prioritize local job creation, employer recruitment, and support for development projects that hire local residents.

The request could facilitate development that would bring economic growth opportunities to the Four Hills Village Activity Center. The restaurant would provide a variety jobs in a Major Transit Corridor which could help to improve the quality of life of those needing accessible job opportunities. The restaurant would be a new choice in the Activity Center and bring new neighborhood-scale commercial development to an established shopping center. The applicant intends on recruiting and retaining local employees.

- 12. The request meets the Site Plan-EPC Review & Decision Criteria in IDO Section 14-16-6-6(J)(3) as follows:
  - A. 14-16-6-6(J)(3)(a) As demonstrated by the policy analysis of the site plan, the request is consistent with applicable Comprehensive Goals and Policies.
  - B. 14-16-6-6(J)(3)(b) The subject site is zoned MX-M; therefore, this criterion does not apply.
  - C. 14-16-6-6(J)(3)(c) With the application of conditions of approval, the site plan major amendment would comply with all applicable provisions of the IDO.
  - D. <u>14-16-6-6(J)(3)(d)</u> The proposed future Site Plan request will be reviewed administratively via the Development Facilitation Team (DFT) process, which will address infrastructure and ensuring that infrastructure such as streets, trails, sidewalks, and drainage systems has sufficient capacity to serve a proposed development.
  - E. <u>14-16-6-6(J)(3)(e)</u> The future, proposed development will be required to comply with the decisions made through- the EPC and the DFT Process. The EPC's conditions of approval will improve compliance with the IDO, which contains regulations to mitigate site plan impacts to surrounding areas. The DFT review will ensure infrastructure is adequately addressed so that a proposed development will not burden the surrounding area.
  - F. 14-16-6-6(J)(3)(f) The subject site is not located within an approved Master Development Plan.
  - G. <u>14-16-6-6(J)(3)(g)</u> The subject property is not within the Railroad and Spur Area and no cumulative impact analysis is required. Therefore, the above criterion does not apply.
- 13. The applicant notified the Singing Arrow NA, the East Gateway coalition, and property owners within 100 feet of the subject site as required.

- 14. A pre-submittal neighborhood meeting was offered and held on May 25, 2023. Concerns about increased traffic were raised and addressed.
- 15. A letter was received from the Singing Arrow Neighborhood Association dated June 30, 2023 stating that the Board of Directors has no objection to the proposed Major Amendment, other than potential traffic flow issues.
- 16. Conditions of approval are needed to ensure that applicable IDO regulations are met and to provide clarification.

# **CONDITIONS OF APPROVAL:**

- 1. The EPC delegates final sign-off authority of this site development plan to the (Development Facilitation Team (DFT) administrative view process as per IDO Section 6-5(G)(1). The reviewer will be responsible for ensuring that the EPC Conditions have been satisfied and that other applicable City requirements have been met.
- 2. The applicant shall meet with the staff planner prior to submitting to the DFT to ensure that all conditions of approval have been satisfied.
- 3. Major Amendment: An amended Grading and Drainage Plan shall be provided prior to submitting the associated Site Plan to DFT.
- 4. Site Plan:
  - A. Provided bicycle parking total shall be added.
  - B. Light pole locations shall be added to the proposed site plan.
- 5. On-site walkways and crosswalks shall be identified to motorists and pedestrians pursuant to 5-3(D)(3)(c) Materials to Alert Motorists
- 6. Landscape plan: The amendments to the landscape plan shall be to scale and included in the Major Amendment Package and the "for reference only: label shall be removed.
- 7. Signage:

A. All signs shall be included on the elevations of the proposed building.

B. Dimensions, colors and materials shall be provided prior to submitting to the DFT.

- 8. Detail Sheets: Colors and materials shall be provided for all details.
- 9. Condition from Transportation Development Services:

The Traffic Impact Study is required per the Traffic Scoping Form and shall need to be reviewed and approved prior to site plan approval through DFT action.

10. Condition from the Solid Waste Management Department:

Any landscape that would overhang in the trash enclosure shall be relocated.

- 11. Conditions from the Public Service Company of New Mexico (PNM):
  - A. Any existing and/or new PNM easements and facilities shall be reflected on the Site Plan and any resulting Plat.
  - B. Typical electric utility easement widths vary depending on the type of facility. On-site transformers shall have a five-foot clear area on the sides and rear and ten-foot in front to allow for access and maintenance
  - C. Perimeter and interior landscape design shall abide by any easement restrictions and not impact PNM facilities.
- 12. Condition from Kirtland Airforce Base:

Dark skies are necessary to the mission operations of the Starfire Optical Range (SOR). Therefore, any lighting of the proposed use shall be shielded and focused downward in order to have the least effect on the base.

13. Elevations: The east and west elevation labels shall be updated to reflect the correct elevation façade.

<u>APPEAL</u>: If you wish to appeal this decision, you must do so within 15 days of the EPC's decision or by **August 4, 2023**. The date of the EPC's decision is not included in the 15-day period for filing an appeal, and if the 15<sup>th</sup> day falls on a Saturday, Sunday or Holiday, the next working day is considered as the deadline for filing the appeal.

For more information regarding the appeal process, please refer to Section 14-16-6-4(V) of the Integrated Development Ordinance (IDO), Administration and Enforcement. A Non-Refundable filing fee will be calculated at the Land Development Coordination Counter and is required at the time the appeal is filed. It is not possible to appeal an EPC Recommendation to the City Council since this is not a final decision.

You will receive notification if any person files an appeal. If there is no appeal, you can receive Building Permits at any time after the appeal deadline quoted above, provided all conditions imposed at the time of approval have been met. Successful applicants are reminded that other regulations of the IDO must be complied with, even after approval of the referenced application(s).

Sincerely,

Catalina Lehner

for Alan M. Varela, Planning Director

cc: Regina Okoye, Modulus Architects & Land Use Planning LLC, <u>Rokoye@madulusarchitects.com</u> Brooke Nowak-Neely, <u>Brooke@bnnlawfirm.com</u> Singing Arrow NA, Meg Beck, <u>123mbeck@gmail.com</u> Singing Arrow NA, Laurie Williams, <u>lwilliams751@gmail.com</u> East Gateway Coalition, Julie Dreike, <u>dreikeja@comcast.net</u> East Gateway Coalition, Michael Brasher, <u>eastgatewaycoalition@gmail.com</u> Legal, <u>dking@cabq.gov</u> EPC File



**Development Facilitation Team** City of Albuquerque Plaza Del Sol, 600 Second NW

Albuquerque, New Mexico 87102

# August 4, 2023

# RE: PR-2023-008767 – STARBUCKS – FINAL SIGN-OFF FOR MAJOR AMENDMENT TO A SITE DEVELOPMENT PLAN –EPC - 200 TRAMWAY BLVD SE

Members of the Development Facilitation Team Board,

Modulus Architects, Inc., hereafter referred to as "Agent" for the purpose of this request, represents Smith's Food & Drug Center Inc., hereafter referred to as "Applicant." We, "Agent" are requesting approval for a Final Sign-off for a Major Amendment to the Site Development plan through the EPC.

The EPC Site Plan Amendment to the Site Development Plan was approved on July 20, 2023. The request to added a new restaurant with a drive through use on the south west corner of the overall site. The approval was delegated to the DFT for final sign-off. Below is how each Condition of Approval was addressed.

# EPC Conditions of Approval –

- The EPC delegates final sign-off authority of this site development plan to the (Development Facilitation Team (DFT) administrative view process as per IDO Section 6-5(G)(1). The reviewer will be responsible for ensuring that the EPC Conditions have been satisfied and that other applicable City requirements have been met.
  - a. Applicant Response: The application has been submitted to the DFT for final sign-off to ensure that the EPC conditions have been satisfied.
- 2. The applicant shall meet with the staff planner prior to submitting to the DFT to ensure that all conditions of approval have been satisfied.
  - a. Applicant Response: The applicant has met with staff planner prior to submitted to the DFT. An approval letter has been obtained and is included in this application.
- 3. Major Amendment: An amended Grading and Drainage Plan shall be provided prior to submitting the associated Site Plan to DFT.
  - a. Applicant Response: Attached is the amended overall G&D Plan.
- 4. Site Plan:
  - a. Provided bicycle parking total shall be added.
    - i. Applicant Response: A total of (4) bicycle parking space was provided, please see sheet AS101; bottom right corner.
  - b. Light pole locations shall be added to the proposed site plan.
    - i. Applicant Response: Per sheet AS101, existing light poles are identified (keyed note #20). In addition, we're proposing (1) new light pole to the site (keyed note #21).

- 5. On-site walkways and crosswalks shall be identified to motorists and pedestrians pursuant to 5-3(D)(3)(c) Materials to Alert Motorists
  - a. Applicant Response: On-site walkways and crosswalks are identified on sheet AS101 with reference detail C5/A1.1, A3/A1.2, A5/A1.2; and C5/A1.2 with paving color change per 5-3(D)(3)(c) requirements.
- 6. Landscape plan: The amendments to the landscape plan shall be to scale and included in the Major Amendment Package and the "for reference only: label shall be removed.

## a. Applicant Response: Landscaping plan has been scaled and included in the Major Amendment Package and the "for reference only" label has been removed.

- 7. Signage:
  - a. All signs shall be included on the elevations of the proposed building.
    - i. Applicant Response: Refer to sheet A201-A and A201-B for signage locations.
  - b. Dimensions, colors and materials shall be provided prior to submitting to the DFT.
    - i. Applicant Response: Refer to sheet A201-A and A201-B for Signage Data Table for requested information. The following note have been added to the elevation plans "\*Signs will be permitted separately and the colors and material will be determined at the time of sign permit".
- 8. Detail Sheets: Colors and materials shall be provided for all details.
  - a. Applicant Response: Refer to Exterior Materials Table on sheet A201-A and A201-B, bottom right corner for building materials.
- Condition from Transportation Development Services: The Traffic Impact Study is required per the Traffic Scoping Form and shall need to be reviewed and approved prior to site plan approval through DFT action.
  - a. Applicant Response: The traffic study has been submitted and approved. Approval Letter include in submittal.
- 10. Condition from the Solid Waste Management Department: Any landscape that would overhang in the trash enclosure shall be relocated.
  - a. Applicant Response: There are no landscaping plants/trees that overhang onto the refuse enclosure.
- 11. Conditions from the Public Service Company of New Mexico (PNM):
  - a. Any existing and/or new PNM easements and facilities shall be reflected on the Site Plan and any resulting Plat.
    - i. Applicant Response: Noted. No easements are being provided at this time.
  - b. Typical electric utility easement widths vary depending on the type of facility. On-site transformers shall have a five-foot clear area on the sides and rear and ten-foot in front to allow for access and maintenance
    - i. Applicant Response: Noted. No easements are being provided at this time.
  - c. Perimeter and interior landscape design shall abide by any easement restrictions and not impact PNM facilities.
    - i. Applicant Response: Perimeter and interior landscape design abide by easement restrictions and do not impact PNM facilities.
- 12. Condition from Kirtland Air force Base:

Dark skies are necessary to the mission operations of the Starfire Optical Range (SOR). Therefore, any lighting of the proposed use shall be shielded and focused downward in order to have the least effect on the base.

- a. Applicant Response: Building surface-mount light fixtures will be shielded and downward focused.
- 13. Elevations: The east and west elevation labels shall be updated to reflect the correct elevation façade.
  - a. Applicant Response: Elevation 4/A201-B have been updated to reflect the correct elevation side (Eastern Elevation).

The responses and updated plan submittals should adequately address the comments provided.

Sincerely,

## **REGINA OKOYE, ENTITLEMENTS PROJECT MANAGER MODULUS ARCHITECTS & LAND USE PLANNING, INC.**

100 Sun Avenue NE, Suite 600 Albuquerque, NM 87109 Office 505.338.1499 (Ext. 1003) Mobile + Text 505.267.7686 Email: <u>rokoye@modulusarchitects.com</u> Website: <u>www.modulusarchitects.com</u> Join us on Facebook: <u>Modulus Architects on Facebook</u> **New Mexico | Texas | Arizona | Colorado | Oklahoma** 



# INTER-OFFICE MEMO

DATE:	September 11, 2023
TO:	Jolene Wolfley, DFT Chair Jay Rodenbeck, Planning Manager
FROM:	Megan Jones, MCRP - Senior Planner
RE:	Project #2023-008767/SI-2023-001056 Site Plan-EPC Major Amendment, 200 Tramway Blvd. SE

On July 20, 2023, the EPC voted to approve a Site Plan-EPC Major Amendment for an approximately 8.5-acre site known as Tract J-1, Replat of Tracts F, H-1, J and unplatted land, Four Hills Village Shopping Center and Apartment Complex, located south of Central Ave. SE, north of Wenonah Ave. SE, and between Tramway Blvd. SE and Four Hills Rd. SE. The Site Plan-EPC, Major Amendment is subject to meet Conditions of Approval prior to reeving final sign off by the DFT.

The applicant's agent reached out to Staff in August 2023 coordinate.

Note: Staff reviewed the DFT version of the Site Plan-EPC, Major Amendment against the EPC version of the Site Plan-EPC, Major Amendment which is the standard, required review.

Staff originally reviewed the Site Plan-EPC, Major Amendment Request, which was Conditioned by the EPC, and not the associated future Site Plan for the Coffee Shop. The request was for a major amendment and not for a Site Plan, which is why the Site Plan was not reviewed as part of this request.

Staff checked for incorporation of the Conditions of Approval and for unauthorized changes. The following is a list of conditions detailing how they were met, not met or partially met:

# EPC Conditions of Approval

- 1. <u>Condition 1 & 2:</u> The applicant has coordinated with the Staff Planner and provided a letter responding to the conditions off approval in the NOD dated July 20, 2023 for the Major Amendment request (see attached). The DFT Site Plan satisfies all, but three Conditions. It is the responsibility of the DFT reviewer to ensure that all other applicable City requirements are met for the associated Site Plan approval.
- 2. <u>Condition 3</u>: The applicant provided the amended Grading and Drainage plan.

- a. A note shall be added to sheet C1 stating that the EPC approved the requested amendments on 7-20-2023.
- b. The G&D Plans show exiting and proposed G& D as sheet C3.
- 3. <u>Condition 4, a:</u> The applicant shall update the Major Amendment sheet to reflect the provided bicycle spaces. This was only included on the Site Plan sheet associated with the request. All amendments shall be reflected on the Major Amendment Sheet.
- 4. Condition 4, b, 5-11: Satisfied.
- 5. <u>Condition 12.</u> The applicant shall provide a note on the Site Plan and light pole detail stating that all building surface mount light fixtured will be shielded and downward focused.
- 6. <u>Condition 13:</u> Satisfied.

# Unauthorized Changes

- 1. AS-101 Site Plan Sheet:
  - a. Keyed notes 17-21 have been added to reflect Conditions 5b & 5c.
  - b. General note E
- 2. New Grading and drainage sheet C3.
- 3. The master Utility Plan Sheet was renamed from sheet MU-1 to sheet C4.
  - a. General note 12 and keyed note 14 are new.

The conditions of approval have been mostly fulfilled. All conditions that have not been met shall be coordinated with DFT staff prior to receiving final sign-off. All unauthorized changes appear to have been provided to satisfy conditions off approval.

Staff did not review the associated Site Plan, therefore DFT staff shall review all applicable standards for the proposed restaurant with drive-through.

If you have any questions regarding this case, please call me at (505) 924-3352 or e-mail me at mdjones@cabq.gov. Thank you.



• KROGER CORPORATE REAL ESTATE • 1014 VINE STREET • CINCINNATI, OH 45202-1100

June 15, 2023

City of Albuquerque 600 2nd Street NW Albuquerque, NM 87102

### RE: AGENT AUTHORIZATION NOTICE - 200 TRAMWAY BLVD SE ALBUQUERQUE NM 87123

To Whom It May Concern,

Smith's Food & Drug Centers, Inc., hereby authorizes Modulus Architects & Land Use Planning, Inc., to perform as the Agent of Record with the City of Albuquerque. This Agent Authorization is for the property located at 200 TRAMWAY BLVD SE ALBUQUERQUE NM 87123 and legally described as:

### Lot: J1

TR J-1 REPL OF TRS F, H-1, J & UNPLATTED LAND FOUR HILLS VILLAGE SHOPPING CENTER & APT COMPLEX CONT 7.9444 AC +/- OR 346,058 SF +/-

This authorization is valid until further written notice from Smith's Food & Drug Centers, Inc. or Modulus Architects & Land Use Planning, Inc. (Agent). Please direct all correspondence and communication to our Agent for the purpose of this request for the Environmental Planning Commission and all supplements submittals.

Sincerely.

Rick J. Landrum Vice President Smith's Food & Drug Centers, Inc.



**Owner:** Smith's Food & Drug Centers, Inc. **Owner Address:** 1014 VINE ST FLOOR 7TH CINCINNATI OH 45202
#### FORM P: PRE-APPROVALS/SIGNATURES

# Legal Description & Location: \_200 TRAMWAY BLVD. SE, ALBUQUERQUE, NM 87123

TR J-1 REPL OF TRS F, H-1, J & UNPLATTED LAND FOUR HILLS VILLAGE SHOPPING CENTER & APT COMPLEX CONT 7.9444 AC +/- OR 346,058 SF +/-

NA NA NA \_\_ NA NA NA NA

#### □ <u>Hydrology:</u>

<ul> <li>Grading and Drainage Plan</li> <li>AMAFCA</li> <li>Bernalillo County</li> <li>NMDOT</li> <li>MRGCD</li> </ul>	XApproved Approved Approved Approved Approved	NA _XNA _xNA _xNA
Renée C. Brissette Hydrology Department <u>Transportation:</u>	07/28/23 Date	_
<ul> <li>Traffic Circulations Layout (TCL)</li> <li>Traffic Impact Study (TIS)</li> <li>Neighborhood Impact Analysis (N</li> <li>Bernalillo County</li> <li>MRCOG</li> <li>NMDOT</li> <li>MRGCD</li> </ul>	IA) X A X A A A A A A A	pproved
Einest Armijo	8/17/2023	

Transportation Department

8/17/2023 Date

#### □ Albuquerque Bernalillo County Water Utility Authority (ABCWUA):

•	Water/Sewer Availability Statement/Serviceability Letter	<u>x</u> Approved	NA
٠	ABCWUA Development Agreement	Approved	<u> </u>
•	ABCWUA Service Connection Agreement	Approved	x NA

Sarah Luckis ABCWUA

7/26/2023 Date

Infrastructure Improvements Agreement (IIA\*) \_\_\_\_\_ Approved NA Solid Waste Department Signature on the plan X Approved NA A Fire Marshall Signature on the plan NA X Approved

\* Prior to Final Site Plan approval submittals (include a copy of the recorded IIA)

# EXHIBIT 1 - HYDROLOGY APPROVAL CITY OF ALBUQUERQUE

Planning Department Alan Varela, Director



Mayor Timothy M. Keller

July 27, 2023

Ronald Bohannan, P.E. Tierra West, LLC 5571 Midway Park Place NE Albuquerque, NM 87109

#### RE: Starbucks - Tramway Grading and Drainage Plans Engineer's Stamp Date: 07/19/23 Hydrology File: L23D015A

Dear Mr. Bohannan:

Based upon the information provided in your submittal received 07/19/2023, the Grading & Drainage Plans are approved for Building Permit, Grading Permit, and for action by the Development Hearing Officer (DHO) on Preliminary/Final Plat. Please attach a copy of this approved plan in the construction sets for Building Permit processing along with a copy of this letter.

# PRIOR TO CERTIFICATE OF OCCUPANCY:

- NM 87103
   Engineer's Certification, per the DPM Part 6-14 (F): Engineer's Certification Checklist For Non-Subdivision is required.
- <sup>www.cabq.gov</sup>
  2. Please provide the executed paper Drainage Covenant (latest revision) printed on one-side only with Exhibit A and a check for \$25.00 made out to "Bernalillo County" for the stormwater quality ponds per Article 6-15(C) of the DPM to Hydrology for review at Plaza de Sol.

As a reminder, if the project total area of disturbance (including the staging area and any work within the adjacent Right-of-Way) is 1 acre or more, then an Erosion and Sediment Control (ESC) Plan and Owner's certified Notice of Intent (NOI) is required to be submitted to the Stormwater Quality Engineer (Doug Hughes, PE, <u>jhughes@cabq.gov</u>, 924-3420) 14 days prior to any earth disturbance.

# CITY OF ALBUQUERQUE

Planning Department Alan Varela, Director



Mayor Timothy M. Keller

If you have any questions, please contact me at 924-3995 or <u>rbrissette@cabq.gov</u>.

Sincerely,

Renée C. Brissette

Renée C. Brissette, P.E. CFM Senior Engineer, Hydrology Planning Department

PO Box 1293

Albuquerque

NM 87103

www.cabq.gov



**STARBUCKS ON TRAMWAY** 

Date

TWLLC

Volume = Weighted E \* Total Area Flow = Qa\*Aa + Qb\*Ab + Qc\*Ac + Qd\*Ad

HYDROLOGY CALCULATION TABLES

**Existing Conditions** 

				Basin Descriptions						
Basin	Treat	Area	Area	Area	Treatm	ent A	Treat	ment B	Treatn	nent C
ID	Tract	(sf)	(acres)	(sq miles)	%	(acres)	%	(acres)	%	(acre
H1		195,800	4.49	0.00702	0%	0.000	0%	0.000	10%	0.44
Total		195,800	4.49	0.00702		0.000		0.000		0.00

5/24/2023

# Proposed Conditions

	Basin Descriptions									
Basin	Treat	Area	Area	Area	Treatm	ent A	Treatr	nent B	Treatm	ent C
ID	Tract	(sf)	(acres)	(sq miles)	%	(acres)	%	(acres)	%	(acres)
H1		171,000	3.93	0.00613	0%	0.000	0%	0.000	10%	0.393
D1		23,640	0.54	0.00085	0%	0.000	0%	0.000	30%	0.163
D2		3,135	0.07	0.00011	0%	0.000	0%	0.000	30%	0.022
Total		197,775	4.54	0.00709		0.000		0.000		0.577

PROPOSED	STORM	WATER	POND	VOLUME	TABLES

SWQV-	A POND VC	LUME CALC	JLATIONS
ELEVATION (ft)	AREA (sf)	VOLUME (cf)	CUMULATIVE VOLUME (cf)
5650	199	0	0
5651	525	362	362
	1	·	

SWQV-	B POND VC	LUME CALCU	JLATIONS
ELEVATION (ft)	AREA (sf)	VOLUME (cf)	CUMULATIVE VOLUME (cf)
5648	36	0	0
5649	156	96	96
5650	344	250	346

ELEVATION (ft)	AREA (sf)	VOLUME (cf)	CUMULATIVE VOLUME (cf)
5647.5	29	0	0
5648	56	21.25	21
5649	131	93.5	115

Gh	APF	HC	SCA	LĽ
60	30		30	60

SCALE: 1"=60'

# REQUIRED STORM WATER QUALITY VOLUME CALCULATIONS

Basin D1 Storm W	ater Quality Volume	9
Total Impervious Area =	ΣArea in "Treatment	: D"
Retainage depth = 0.42" Per DPM Pg. 272	0.035	FT
Retention Volume =	=0.035 x Area D	CF
Area D (0.3799 acres)	16548	SF
Required Retention Volume	579	CF
Pond Volume Provided	708	CF

Basin D2 Storm Water Quality Volume				
Total Impervious Area =	ΣArea in "Treatment D"			
Retainage depth = 0.42" Per DPM Pg. 272	0.035	FT		
Retention Volume =	=0.035 x Area D	CF		
Area D (0.0504 acres)	2195	SF		
Required Retention Volume	77	CF		
Pond Volume Provided	115	CF		

Treatment D

Treatment D

%

90%

70%

70%

(acres)

4.045

4.045

(acres)

3.533

0.3799

0.0504

3.963

%

90%

(acres) 0.449

0.000

100-Year, 6-Hr

(ac-ft)

0.911

0.911

100-Year, 6-Hr

(ac-ft)

0.795

0.096

0.013

0.905

Weighted E | Volume |

(in)

2.431

2.133

2.133

Flow

cfs

19.59

19.589

Flow

cfs

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2.22

0.29

19.624

Weighted E | Volume |

(in)

2.431

Cit Plar Develop HYDRC <b>AP</b>
DATE: BY: HydroTrans #



JOB # 2023047

ALBUQUERQUE, NM 87109 (505) 858–3100

www.tierrawestllc.com

07/19/2023 RONALD R. BOHANNAN P.E. #7868







(FOR RETAINING PORTION ONLY)									
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# EXHIBIT 2 - TCL APPROVAL CITY OF ALBUQUERQUE

Planning Department Alan Varela, Director



Mayor Timothy M. Keller

August 17, 2023

Devin Nguyen, AAIA Modulus Architects 100 Sun Ave. NE Albuquerque, NM 87109

Re: Starbucks 200 Tramway Blvd. SE Traffic Circulation Layout Architect's Stamp 07-05-23 (L23-D015A)

Dear Mr. Nguyen,

The TCL submittal received 07-31-2023 is approved for Building Permit by Transportation. A copy of the stamped and signed plan will be needed for each of the building permit plans. Please keep the original to be used for certification of the site for final C.O. for Transportation.

<sup>PO Box 1293</sup> When the site construction is completed and an inspection for Certificate of Occupancy (C.O.) is requested, use the original City stamped approved TCL for certification. Redline any minor changes and adjustments that were made in the field. A NM registered architect or engineer must stamp, sign, and date the certification TCL along with indicating that the development was built in "substantial compliance" with the TCL. Submit this certification, the TCL, and a completed <u>Drainage and Transportation Information Sheet</u> to the <u>PLNDRS@cabq.gov</u> for log in and evaluation by Transportation. AN APPROVED PLAT IS A CONDITION OF RELEASING FINAL CO.

WWW.cabq.gov Once verification of certification is completed and approved, notification will be made to Building Safety to issue Final C.O. To confirm that a final C.O. has been issued, call Building Safety at 924-3690.

Sincerely,

Ernest Armijo, P.E. Principal Engineer, Planning Dept. Development Review Services

C: CO Clerk, File

ALBU DU	A
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MEX	

# City of Albuquerque

Planning Department Development & Building Services Division

DRAINAGE AND TRANSPORTATION INFORMATION SHEET (REV 6/2018)

Project Title: Starbucks Coffee Shop	Building Peri	mit #: <u>TBD</u>	Hydrology File #:
DRB#:	EPC#:		Work Order#:
TRACT J-1-A AND J-1-B, FOUR           Legal Description:         THE OFFICE OF THE COUNTY (	HILLS VILLAGE SHOP	PPING CENTER AND APAR LO COUNTY, NEW MEXICO	RTMENT COMPLEX, FILED IN D.
City Address: 200 TRAMWAY BLVD. SE, ALBUQU	ERQUE, NM 87123		
Applicant: Modulus Architects			Contact: Devin Nguyen
Address: 100 Sun Ave Suite 600, Albuque	erque NM 8710	09	
Phone#: 505-338-1499	Fax#:		E-mail: devinn@modulusarchitects.com
Other Contact:			Contact:
Address:			
Phone#:	Fax#:		E-mail:
TYPE OF DEVELOPMENT: PLAT	(# of lots)	RESIDENCE	DRB SITE $X$ ADMIN SITE
IS THIS A RESUBMITTAL? X Yes	No		
<b>DEPARTMENT</b> X TRANSPORTATION	HYD	ROLOGY/DRAINA	GE
Check all that Apply:		TYPE OF APPE	ROVAL/ACCEPTANCE SOUGHT:
TYPE OF SUBMITTAL:		<u> </u>	FPERMIT APPROVAL
ENGINEER/ARCHITECT CERTIFICATIO	DN		ATE OF OCCUPANCY
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GRADING PLAN		SITE PLAN	J FOR BLDG PERMIT APPROVAL
DRAINAGE REPORT		FINAL PL	AT APPROVAL
DRAINAGE MASTER PLAN			
FLOODPLAIN DEVELOPMENT PERMIT	APPLIC	SIA/ RELE	ASE OF FINANCIAL GUARANTEE
ELEVATION CERTIFICATE		FOUNDAT	ION PERMIT APPROVAL
CLOMR/LOMR		GRADING	PERMIT APPROVAL
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TRAFFIC IMPACT STUDY (TIS)		PAVING P	PERMIT APPROVAL
STREET LIGHT LAYOUT		GRADING	/ PAD CERTIFICATION
OTHER (SPECIFY)		WORK ORI	DER APPROVAL
PRE-DESIGN MEETING?		CLOMR/LO	OMR
		FLOODPL	AIN DEVELOPMENT PERMIT
		OTHER (S	PECIFY)
DATE SUBMITTED:	By: <u>07 Au</u>	ug 2023	<u> </u>
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# AN APPROVED PLAT IS A CONDITION OF RELEASING FINAL CO.









# EXHIBIT 3 - TIS APPROVAL CITY OF ALBUQUERQUE

*Planning Department* Brennon Williams, Director



Mayor Timothy M. Keller

August 16, 2023

Jonathon Kruse, P.E., PTOE Lee Engineering 8220 San Pedro Drive NE Suite 150 Albuquerque, NM 87113 Via email jkruse@lee-eng.com

#### Re: Tramway Starbucks, 200 Tramway Blvd. SE Traffic Impact Study final, dated August 2023 HT#L23D015A Engineering Seal date 8/15/2023

Dear Mr. Kruse,

Review of the Traffic Impact Study for Tramway Starbucks received August 15, 2023 has been completed by the City's Planning Transportation Development section. The City accepts and approves the TIS with the following recommendation for a minor infrastructure modification.

PO Box 1293

Albuquerque

• The existing southbound left turn lane serving the site at the west driveway be lengthened to meet DPM design specifications as closely as possible.

The Traffic Impact Study shall be valid for a period of three years. Should significant modifications to the approved development proposal occur, the approved study shall be revised to incorporate the changes.

If you have any questions, feel free to contact me at (505) 924-3362.

Sincerely,

NM 87103

MPM P.E.

Www.cabq.gov Matt Grush, P.E. Senior Engineer City of Albuquerque Planning Department Development Review Services

> via: email C: Applicant, File



# **City of Albuquerque**

Planning Department

Development & Building Services Division

### DRAINAGE AND TRANSPORTATION INFORMATION SHEET

Project Title: Tramway Starbucks Building	ng Permit #Hydrology File #
DRB#	EPC#
Legal Description: UPC: 1023056021345213	City Address OR Parcel 200 TRAMWAY BLVD. SE, ALBUQUERQUE, NM
Lee Engineering on Behalf	
Applicant/Agent: of Modulus Architects	Contact: Jonathon Kruse
Address: 8220 San Pedro Dr NE STE 150, Albuquerque NM 87113	Phone: 505-545-8459
Email: jkruse@lee-eng.com	
Applicant/Owner: Modulus Architects	Contact: Regina Okoye
Address:	Phone: 505-338-1499
Fmail rokove@modulusarchitects.co	I nonce
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ENGINEER/ARCHITECT CERTIFICATION	BUILDING PERMIT APPROVAL
PAD CERTIFICATION	CERTIFICATE OF OCCUPANCY
CONCEPTUAL G&D PLAN	CONCEPTUAL TCL DRB APPROVAL
GRADING PLAN	PRELIMINARY PLAT APPROVAL
DRAINAGE REPORT	SITE PLAN FOR SUB'D APPROVAL
DRAINAGE MASTER PLAN	SITE PLAN FOR BLDG PERMIT APPROVAL
FLOOD PLAN DEVELOPMENT PERMIT APP.	FINAL PLAT APPROVAL
ELEVATION CERTIFICATE	SIA/RELEASE OF FINANCIAL GUARANTEE
CLOMR/LOMR	FOUNDATION PERMIT APPROVAL
TRAFFIC CIRCULATION LAYOUT (TCL)	GRADING PERMIT APPROVAL
ADMINISTRATIVE	SO-19 APPROVAL
TRAFFIC CIRCULATION LAYOUT FOR DRB	PAVING PERMIT APPROVAL
APPROVAL	GRADING PAD CERTIFICATION
X TRAFFIC IMPACT STUDY (TIS)	WORK ORDER APPROVAL
STREET LIGHT LAYOUT	CLOMR/LOMR
OTHER (SPECIFY)	FLOOD PLAN DEVELOPMENT PERMIT
PRE-DESIGN MEETING?	X OTHER (SPECIFY) Final TIS
8/15/2023	

DATE SUBMITTED: 8/15/2023

# TRAFFIC IMPACT STUDY Tramway Starbucks

Final Report August 2023

Prepared for Modulus Architects

> 8220 San Pedro Drive NE, Suite 150 Albuquerque, NM 87113



(505) 338-0988



Ο

leeengineering.com

# Traffic Impact Analysis (TIA) for Tramway Starbucks

**Final Report** 



August 2023

Prepared for: Modulus Architects & Land Use Planning, Inc.

Prepared By:



# **EXECUTIVE SUMMARY**

The following contains a Traffic Impact Study (TIS) for a Starbucks coffee shop in Albuquerque, NM. Lee Engineering has completed this report for Modulus Architects & Land Use Planning, Inc (MODULUS). All analyses and items contained herein conform to scoping requirements set forth in a scoping meeting held with the City of Albuquerque on May 1<sup>st</sup>, 2023.

# BACKGROUND

The proposed development is to construct a Starbucks coffee shop on the parking lot of Smith's Superstore located along Tramway Blvd and Wenonah Ave. Nearby intersections include Wenonah Ave & 4 Hills Rd, Tramway Blvd & Wenonah Ave, West Entrance Driveway on Tramway Blvd, and East Entrance Driveway on Wenonah Ave.

The site is anticipated to generate 67 ingress and 66 egress trips during the AM peak hour, and 29 ingress and 28 egress during the PM peak hour. The number of vehicle trips generated by the proposed development was based on the trip generation rates and equations provided in the Trip Generation Manual, 10th Edition, by the Institute of Transportation Engineers (ITE) 937 – Coffee/Donut Shop with Drive-Through Window.

Site access is available from West Entrance Driveway along Tramway Blvd, and East Entrance Driveway along Wenonah Ave.

Study intersections include:

- 1. Wenonah Ave/ 4 Hills Rd
- 2. Tramway Blvd/ Wenonah Ave
- 3. West Entrance Driveway/ Tramway Blvd
- 4. East Entrance Driveway/ Wenonah Ave

Construction is anticipated to begin in 2023, with full completion of the Development in 2025. The Development is to be constructed in a single phase.

Analysis scenarios for this study include:

- Existing (2023) Field counted Existing traffic volumes
- Build-Out Year (2025) Background Existing traffic volumes with an applied annual growth rate.
- Build-Out Year (2025) Total Build-Out Year Background volumes plus Starbucks site-generated Direct and Pass-By trips.

Existing turning movement counts were collected on June 20, 2023, for all study intersections. These volumes were analyzed unaltered in the Existing portion of the Capacity Analysis section.

Site trips for the development site were generated based on ITE 937 –Coffee/Donut Shop with Drive-Through Window, Peak Hour Generator. Proposed development-generated trips were used to analyze Build-Out Total volumes.



# SUMMARY OF RECOMMENDATIONS

The following presents a summary of recommendations included in this report.

#### CONCLUSIONS

- All study intersections operate at an acceptable LOS throughout all study scenarios
- 95<sup>th</sup> % Queue Lengths do not exceed queue storage at study intersections for studied analysis scenarios
- HCS results do not suggest the need for capacity mitigation measures or street improvements related to the proposed development
- Proposed Drive-Through Queue Storage accommodates average and 88<sup>th</sup> percentile queues but fails to accommodate 95<sup>th</sup> percentile of vehicle queues as designed. However, the provided storage does meet requirements set forth in the City of Albuquerque's Integrated Development Ordinance.

#### RECOMMENDATIONS

It is recommended that the existing southbound left turn lane serving the site at the west driveway be lengthened to meet DPM design specifications as closely as possible.

Maintain sight distance at all driveways by keeping sight lines visibility free from any obstructions such as but not limited to parking, canopies, site displays, and landscaping.



# TABLE OF CONTENTS

Executive Summary	1
Background	1
Summary of Recommendations	2
Table of Figures	4
Table of Tables	4
List of Appendices	4
Introduction	5
Project Location & Site Plan	5
Study Area, Area Land Use, and Streets Narrative Summary	7
Study Area	7
Data Collection	8
Field Data Collection	8
Turning Movement Counts	8
Capacity analysis: Level of Service and Queuing	10
Analysis Volumes	10
Capacity Analysis	10
Build Year (2025) Analyses	13
Traffic Projections	13
Tramway Starbucks Site Trip Generation	13
Trip Distribution and Assignment	14
Traffic Volume Calculations	16
Traffic Analysis of Build-Out Background and Total	19
2025 Build-out Total Conditions	19
Site Related Capacity Mitigations and Street Improvements	22
Site Access Sight Distance	22
Auxiliary Lane Analysis	23
Drive-Thru Queuing Analysis	25
Drive-Thru Description	25
Queuing variables	25
Queuing Analysis	26
Crash Data Summary	27
Summary of Recommendations	29



# TABLE OF FIGURES

Figure 1: Site Plan	6
Figure 2: Vicinity Map	6
Figure 3: Existing Peak Hour Turning Movement Counts	9
Figure 4: Site Generated Direct Trips & Routing Percentages	15
Figure 5: Build-Out Year (2025) Background	17
Figure 6: Build-Out Year (2025) Total	18

# TABLE OF TABLES

Table 1: LOS Criteria and Descriptions	10
Table 2: HCS Result Summary for Existing (2023) Conditions	12
Table 3: Growth Rates	13
Table 4: Trip Generation	14
Table 5: HCS Result Summary for Build-Out Year (2025) Backgroud Conditions	20
Table 6: HCS Result Summary for Build-Out Year (2025) Total Conditions	21
Table 7: Site Distance Requirements	23
Table 8: Auxiliary Turn Lane Warrant	24
Table 9: Probability of "k" Vehicles in Queue	26
Table 10: Queuing Analysis Summary	26
Table 11: Crash Summary	27

# LIST OF APPENDICES

Appendix A	A – Scoping	Meeting	Notes
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- Appendix B Turning Movement Counts
- Appendix C Highway Software Analysis
- Appendix D Sight Distance Calculations

# INTRODUCTION

This report details the procedures and findings of a Traffic Impact Study (TIS) performed by Lee Engineering for Modulus Architects. This report and the analyses herein were performed for a Starbucks development to be constructed on the parking lot of Smith's Superstore located along Tramway Blvd and Wenonah Ave in Albuquerque, NM. This study examines the impacts of the proposed Development on surrounding traffic conditions and discusses the potential impacts of trips generated by the Development on the study intersections.

The scope of this report and the analyses performed were completed in agreement with the scoping requirements set forth by the CABQ. Scoping meeting notes from the scoping meeting held on May 1<sup>st</sup> 2023, are included in Appendix A. Analysis procedures, conclusions, and recommendations for this study were developed according to the *Highway Capacity Manual 6<sup>th</sup> Edition*.

Single-phase construction is anticipated to begin in 2023, with full completion of the Development in 2025. The proposed development site plan displayed in Figure 1 shows that the proposed development is a coffee/donut shop with a drive-through window. AM and PM peak hour volumes were analyzed for each scenario. Traffic generated by the site is anticipated to be 67 ingress and 66 egress trips during the AM peak hour, and 29 ingress and 28 egress trips during PM peak hour. Lee Engineering conducted an HCS Capacity Analysis for the following AM and PM peak hour scenarios:

#### Traffic Analysis

- Existing (2023) Field counted Existing traffic volumes
- Build-Out Year (2025) Background –Existing traffic volumes with an applied annual growth rate and the addition of traffic volumes generated by the nearby development of a Coffee/Donut Shop with Drive-Through Window ITE code 937.
- Build-Out Year (2025) Total Build-Out Year Background volumes plus Starbucks site-generated Direct Trips.

# PROJECT LOCATION & SITE PLAN

The development will be located on Tramway Blvd and Wenonah Ave, in the northeast quadrant, in the parking lot of the Smith's superstore in Albuquerque, NM. Figure 1 shows the proposed site plan, and Figure 2 shows the site location, study intersections, and the surrounding area. Nearby intersections include Wenonah Ave and 4 Hills Rd, and Tramway Blvd & Central Ave. Existing commercial businesses border the project area on to the west and south, and residential land use borders the development to the east and southwest.

The proposed development would convert approximately 0.56 acres of land into into a 1,310 square feet Starbucks with a driveway. The development would include 14 existing parking spaces, 24 new parking spaces with a lot of 21,253 square feet. Proposed access points include two existing shared access driveways located north and west to the development site.

The development Site Plan is presented in Figure 1, and Figure 2 shows the Vicinity Map, which includes the study area and intersections.



	1
PROPOSED STARBUCKS 1,310 SF DT = 21,253 SF B B B B B C C C C C C C C C C C C C	



Figure 1: Site Plan



Figure 2: Vicinity Map



# STUDY AREA, AREA LAND USE, AND STREETS NARRATIVE SUMMARY Study Area

The study area is defined as Wenonah Ave & 4 Hills Rd to West Driveway on Tramway Blvd. The following intersections were identified for analysis during the scoping meeting:

- 1. Wenonah Ave & 4 Hills Dr
- 2. Tramway Blvd & Wenonah Ave
- 3. West Entrance Driveway on Tramway Blvd
- 4. East Entrance Driveway on Wenonah Ave

#### AREA LAND USE

The Development will be located on the west side of the East Entrance Driveway on Wenonah Ave. Land uses adjacent to and surrounding consist of the following:

- Commercial: Existing commercial developments immediately surrounding the development site include markets, restaurants, gas stations, a travel center, and a hotel.
- Residential: Residential zones are located southwest and east of the development site.

#### STREETS

The following details the characteristics and features of streets included in the study area:

**4 Hills Rd** is a CABQ maintained roadway classified as a Major Collector, running north in Albuquerque, NM. The posted speed limit is 40 MPH. The roadway has 4 lanes that are 11 feet wide, and the road is divided by a 20-foot-wide raised median. The median narrows to accommodate northbound and southbound left turn lanes at 4 Hills Rd & Wenonah Ave, a northbound left turn lane at 4 Hills Rd, and a southbound left turn lane at 4 Hills Rd. In the southbound direction, there is a 6-foot bike lane that extends from Central Ave to Wenonah Ave. In the northbound firection there is a continuous sidewalk, and no bicycle facilities are present.

**Wenonah Ave** is CABQ maintained Major Collector running in an east-west direction, it is two-lane to the west of Tramway Blvd, and 3-lane to the west of Tramway Blvd. The posted speed limit is 25 MPH to the west of Tramway Blvd and 35 MPH to the east of Tramway Blvd. The roadway has 12-foot travel lanes with striping. To the west of Tramway Blvd, Wenonah Ave is undivided, and to the east of Tramway Blvd, it is divided bye a 12-ft two-way-two-lane median that extends from Tramway Blvd to 4 Hills Rd. There are 5-foot bike lanes in each direction to the east of Tramway Blvd, but no bicycle facilities to the west. Continuous sidewalk is present in both directions.

**Tramway Blvd** is a CABQ maintained Major Collector to the south of Central Ave & Tramway Blvd intersection, and a Principal Arterial to the north of Central Ave & Tramway Blvd intersection. The posted speed limit is 45 mph to the north of Central Ave & Tramway Blvd, and 35 MPH to the south of Central Ave & Tramway Blvd. The roadway has four lanes that are 12-ft wide, and the road is divided by a 18-ft-side raised median. The median narrows to accommodate northbound and southbound left turn lanes at Tramway Blvd. There are 5-foot bike lanes in each direction, and continouous sidewalk is present in both directions.

#### INTERSECTIONS

The following details the traffic control and characteristics of existing intersections in the study area:

**Wenonah Ave & 4 Hills Rd** is a 4-legged, stop-controlled intersection of major collectors. There are stop signs on the eastbound and westbound directions. The eastbound leg consists of one left-turn lane with approximately 128 ft of storage lane, one through-lane and one right-turn lane. The westbound leg consists of one left-turn lane and one through/right turn lane. The northbound and southbound legs consist of two



through-lanes and one left-turn lane with approximately 140 ft of storage. Curb cuts with ramps are present, but there are no painted crosswalks present for any leg of the intersection.

**Tramway Blvd and Wenonah Ave** is a 3-legged, stop-controlled intersection, both roadways are major collectors. There is a stop sign on Wenonah Ave, approaching eastbound traffic. The eastbound leg consists of one shared-lane. The northbound leg consists of one shared-lane, and the southbound leg consist of one through-lane and one through/right turn lane. Curb cuts with ramps are present, but there is no stripped crosswalk.

**West Entrance Driveway & Tramway Blvd** is a 4-leg intersection of a major collector and an unnamed business access road. For the purposes of this study the unnamed access road will be referred to as West Entrance Driveway. A stop sign is present on the eastbound approach. The eastbound and westbound legs consist of two lanes, and no stripping for any of the approaches. The northbound leg consist of one through-lane, one through/right turn lane, and one displaced left-turn lane separated by a painted 12-ft-wide lane. The southbound leg consists of one through-lane, one through/right turn lane with approximately 140 ft of storage. A painted crosswalk is present across Tramway Blvd.

**East Entrance Driveway & Wenonah Ave** is a 4-legged, stop-controlled intersection of a major collector and an unnamed business access road. For the purpose of this study the unnamed access road will be referred to as East Entrance Driveway. The northbound leg consists of one through/left turn lane and one right turn lane, and the southbound leg consists of one shared lane. The eastbound leg consists of one shared lane, and the westbound leg consists of one through-lane and one through/right turn lane. No stop signs are present for any approach. No crosswalks are present.

#### **BICYCLE FACILITIES**

An existing 5-foot-wide bike lane runs adjacent to the Proposed Starbucks development on Wenonah Ave and Tramway Blvd. This bike lane begins on the 4 Hills Rd & Wenonah Ave intersection and continues north to Central Ave. This bike lane in present for both eastbound and westbound approaches along Wenonah Ave.

# DATA COLLECTION

The following section details the data collection method used in subsequent analyses of this report. The data discussed below was collected via a combination of field observations and machine/video recordings.

# FIELD DATA COLLECTION

#### PEDESTRIANS AND BICYCLES

Pedestrian and bicycle volumes were collected at all study intersections with turning movement counts (see Turning Movement Counts section below).

#### TRANSIT

Based on the ABQRIDE System Map (February 2022) several transit routes serve Tramway Blvd through the study area. As such, there is one bus stop inside the study area.

### **TURNING MOVEMENT COUNTS**

Turning movement counts for the study intersections were collected for three separate three-hour periods: 6:00 AM to 9:00 AM, 11:00 AM to 2:00 PM, and 3:00 PM to 6:00 PM on June 20, 2023. Turning movement volumes collected at the study intersections show a typical commuter directionally biased distribution with observable AM and PM peak hour periods. AM and PM peak hour counts are shown in Figure 3 and complete turning movement counts can be found in Appendix B.





Figure 3: Existing Peak Hour Turning Movement Counts



# CAPACITY ANALYSIS: LEVEL OF SERVICE AND QUEUING

## **ANALYSIS VOLUMES**

#### EXISTING YEAR

For the Existing Year traffic volumes, video collected turning movement counts (TMCs) were used. AM and PM peak hours were analyzed for level of service, capacity, and queueing.

#### BUILD-OUT YEAR (2025) BACKGROUND

Existing TMCs were used with an applied annual growth rate developed from the MRCOG Metropolitan Transportation Plan (MTP) CUBE/2 Regional Model for the Build-Out Year Background volumes.

#### BUILD-OUT YEAR (2025) TOTAL

Site trips generated using the Institute of Transportation Engineers (ITE) Trip Generation Manual, 11th Edition, were added to the Build-Out Year Background volumes for analysis.

## CAPACITY ANALYSIS

Per the Highway Capacity Manual, LOS is presented as a letter grade (A through F) based on the calculated average delay for an intersection or movement. Delay is calculated as a function of several variables, including signal phasing operations, cycle length, traffic volumes, and opposing traffic volumes, but is a measurement of the average wait time a driver can expect when moving through an intersection. Factors such as total cycle time (for all movements), queueing restrictions, and vehicle volumes can affect measurements of delay, especially for lower volume movements and side streets. Generally, these factors are only realized when delays reach or exceed LOS E thresholds. In such cases, a narrative is offered in subsequent sections specific to the individual movement in question.

Table 1 below, reproduced from the Highway Capacity Manual, shows delay thresholds and the associated Level of Service assigned to delay ranges. Generally, a LOS of D or better is considered an acceptable level of service.

Level of Service	Average Control Delay (sec/vehicle)	General Description (Signalized Intersections)
А	≤10	Free flow
В	>10-20	Stable flow (slight delays)
С	>10 - 35	Stable flow (acceptable delays)
D	N2E EE	Approaching unstable flow (tolerable delay, occasionally wait
U	>33 - 33	through more than one signal cycle before proceeding)
E	>55 – 80	Unstable flow (intolerable delay)
F	>80	Forced flow (jammed)

#### Table 1: LOS Criteria and Descriptions

Queueing is reported in vehicles, with a base assumption of 20 feet queue length per vehicle, for Two-Way Stop Controlled intersections, including the proposed site access points. Queues are reported for queue measurements falling within the 95<sup>th</sup> percentile. It should be noted that 95<sup>th</sup> percentile queues are statistically expected to occur during only 5% of the peak hour's sign cycles. It is also noted that un-reported average queueing at an intersection would statistically be much shorter than 95<sup>th</sup> percentile queueing.

For the purposes of this analysis, acceptable levels of service (LOS) are defined to be a LOS D or better. Based on procedures outlined in the Highway Capacity Manual, intersection delay and level of service for stop-



controlled intersections are reported as the delay and level of service for the worst-case movement at each intersection. Detailed output sheets can be found in Appendix C.

#### HCS ANALYSIS

Highway Capacity Software was used to analyze the study intersections for Level of Service (LOS) and queueing conditions. All intersection approaches operate at a LOS of C or better during AM and PM peak hours under the Existing scenario. The results of the HCS analysis for the Existing conditions are shown in Table 2.



			Wenonah A	Ave & Four	Hills Rd		т	West	Entrance D	riveway & T	ramway Blv	/d	East Entrance Driveway & Wenonah Ave								
		Movement	Delay (s/veh)	LOS	Storage Length (veh)	95th% Length (veh)	Movement	Delay (s/veh)	LOS	Storage Length (veh)	95th% Length (veh)	Movement	Delay (s/veh)	LOS	Storage Length (veh)	95th% Length (veh)	Movement	Delay (s/veh)	LOS	Storage Length (veh)	95th% Length (veh)
		EBL	13.2	В	6.0	0.1	EBL	-	-	-	-	EBL	-	-	-	-	EBL	7.6	A	-	0.0
60	Чоц	EBR	9.1	A	-	0.5	EBR	-	-	-	-	EBR	-	-	-	-	EBR				
stin	ak H	EBT	15.2	С	-	0.1	EBT	11.7	В	-	0.8	EBT	13.9	В	-	0.1	EBT				
Exis	Pe	NBL	7.6	А	6.8	0.3	NBL	7.8	А	-	0.1	NBL	7.8	А	5.0	0.0	NBL	11.6	В	3.8	0.0
	AM	NBR					NBR					NBR					NBR	8.8	A	3.8	0.0
		NBT					NBT					NBT	-	-	-	-	NBT	-	-	-	-
		SBL	7.9	A	5.2	0.0	SBL					SBL	8.1	A	6.4	0.3	SBL	-	-	-	-
		SBR					SBR					SBR					SBR	-	-	-	-
		SBT					SBT					SBT					SBT	9.7	А	-	0.2
		WBL	14.8	В	4.2	0.0	WBL					WBL	-	-	-	-	WBL	7.6	A	-	0.0
		WBR	12.1	В	4.2	0.2	WBR					WBR	-	-	-	-	WBR				
		WBT	-	-	-	-	WBT					WBT	10.5	В	-	0.4	WBT				
			Wenonah A	Ave & Four	Hills Rd		Tramway Blvd & Wenonah Ave					West Entrance Driveway & Tramway Blvd					East Entrance Driveway & Wenonah Ave				e
		Movement	Delay (s/veh)	LOS	Storage Length (veh)	95th% Length (veh)	Movement	Delay (s/veh)	LOS	Storage Length (veh)	95th% Length (veh)	Movement	Delay (s/veh)	LOS	Storage Length (veh)	95th% Length (veh)	Movement	Delay (s/veh)	LOS	Storage Length (veh)	95th% Length (veh)
		EBL	14.7	В	6.0	0.7	EBL	-	-	-	-	EBL	-	-	-	-	EBL	7.6	A		0.1
	ur	EBR	11.6	В	-	2.0	EBR	-	-	-	-	EBR	-	-	-	-	EBR				
ng	HG	EBT	16.3	C	-	0.3	EBT	15.5	С	-	1.7	EBT	23.9	C	-	0.7	EBT				
(isti	eal	NBL	7.9	A	6.8	0.2	NBL	8.8	Α	-	0.2	NBL	8.6	A	5.0	0.0	NBL	17.0	С		0.0
ŵ	ΔÞ	NBR					NBR					NBR					NBR	9.6	A		0.0
	Ы	NBT					NBT					NBT	-	-	-	-	NBT	-	-	-	-
		SBL	7.8	A	5.2	0.0	SBL					SBL	8.8	A	6.4	0.8	SBL	-	-	-	-
		SBR					SBR					SBR					SBR	-	-	-	-
		SBT					SBT					SBT					SBT	12.4	В		1.1
		WBL	18.1	C	4.2	0.0	WBL					WBL	-	-	-	-	WBL	8.2	A	-	0.0
		WBR	12.3	В	4.2	0.1	WBR					WBR	-	-	-	-	WBR				
		WBT	-	-	-	-	WBT					WBT	12.6	В	-	1.4	WBT				

#### Table 2: HCS Result Summary for Existing (2023) Conditions



From the above table, the following conclusions are made from the Existing Year analysis:

• Under existing conditions, all approaches for all four stop-controlled intersections operate at an acceptable level of service (LOS) C or better during both the AM and PM peak hours. Queuing is accommodated by existing storage lengths during both AM and PM peak hours.

# BUILD YEAR (2025) ANALYSES

The following sections detail the methods and calculations used to obtain traffic volumes for Build-Out Year analysis scenarios. This process used the following tools as described below: Traffic Projections and Site Trip Distribution & Assignment. Figures at the end of this section show the resulting traffic volumes determined for the Build-Out Year (2025) analysis scenarios.

# TRAFFIC PROJECTIONS

Development construction is anticipated to begin in the current year (2023), with full completion expected in 2025. Build-Out Year (2025) volumes were forecast from existing traffic volumes using counted values from 2016 and the 2040 (updated) travel demand models provided by MRCOG. These models were then compared using AM and PM peak hour direction volumes (AMPH LOAD and PMPH LOAD) to calculate anticipated growth rates for individual roadways near the study area. Roadways calculated to have a yearly growth rate of 2%. Growth rates were then converted to growth factors for specific analysis scenarios. Values provided by MRCOG are reproduced verbatim in Table 3, in addition to the calculated growth rates used in the analysis. Growth rates were then applied to the 2023 existing volumes to forecast future volumes.

Roadway			MRCOG 2016 Model "Peak Hour Load"	MRCOG 2040 Model "Peak Hour Load"	Yearly Growth Rate	Average Yearly Growth	Growth Rate for Analysis	
Wenonah Ave & 4 Hills Pd	AM	PH	848	794	-0.27%			
	PM PH		575	601	0.18%	1 55%	2 00%	
Mononah Avo & Tramway	AM PH		199	528	4.15%	1.35%	2.00%	
Wentonan Ave & ITalliway	PM	PH	305	509	2.16%			

Table 3: Growth Rates

Projected turning movement volumes were used for the Build-Out Year Background scenario. Projected turning movement volumes plus the site-generated trips were used for the Build-Out Year Total scenario.

# TRAMWAY STARBUCKS SITE TRIP GENERATION

Trip generation for the Development was performed using the procedures and methodologies provided in the Institute of Transportation Engineers (ITE) Trip Generation Manual, 11th Edition. The land use category Coffee/Donut Shop with Drive-Through Window (ITE 937) was used to generate trips for the Development. Trips were calculated using rates for daily, AM peak hour, and PM peak hour generators. Trips generated by the proposed development are shown below in the tables. Site-generated trips were added to the Background traffic volumes to create the Total Build-Out traffic volumes. Table 4 below shows the trip generation and associated calculations.



#### Table 4: Trip Generation

			Trip Generation											
Use	Units			AM Peak		PM Peak								
			Rate	Enter/Exit % In Out Rate		Enter/Exit % In		Out						
ITE 937 - Coffee/Donut Shop with Drive- Through Window	1.31	SQ FT GFA	101.27	50/50	67	66	43.65	50/50	29	28				

## **TRIP DISTRIBUTION AND ASSIGNMENT**

Trip distribution was determined based on the analysis of existing intersection demand characteristics withing the study area. To facilitate a conservative capacity and queueing analysis, Pass-by trips were not considered in the trip generations above. It is noted that the ITE Trip Generation Manual, 11<sup>th</sup> Edition does provide a pass-by percentage of up to 50%.

The trips were routed withing the roadway network to and from the development based on the proportions of existing turning movement counts/demands. The routing was based on logical trip attractions and destinations. Figure 4 shows the trip distribution and routing percentages generated by the Development. When the applied distribution percentages did not result in whole vehicles or did not summate equal the total generated trips, rounding preference was assigned to the movement with the highest existing turning movement count volumes.





Figure 4: Site Generated Direct Trips & Routing Percentages



# TRAFFIC VOLUME CALCULATIONS

Traffic volumes used in the Build-Out Years analyses were calculated as follows:

- Build-Out Year (2025) Background Existing traffic volumes with an applied annual growth rate
- Build-Out Year (2025) Total Build-Out Year Background volumes plus Tramway Starbucks sitegenerated trips.

Figure 5 shows the Build-Out Year Background (2025) and Figure 6 shows Build-Out Year (2025) Total volumes.





Figure 5: Build-Out Year (2025) Background





Figure 6: Build-Out Year (2025) Total



# TRAFFIC ANALYSIS OF BUILD-OUT BACKGROUND AND TOTAL

As performed for Existing Background conditions, a Level of Service (LOS) and queueing analysis was performed for all Build-Out analysis scenarios using the same procedures, field data, and assumptions.

# **2025 Build-out Total Conditions**

Table 5 below summarizes the delay, level of service, and queueing under 2025 build-out background conditions, Table 6 summarizes delay, level of service, and queueing under 2025 build-out total conditions. Detailed capacity output sheets showing all individual movements can be found in Appendix C.



							10010 01 110	1001 (2020	) Bucky	Sindicions	,										
		Wenonah Ave & Four Hills Rd					Tramway Blvd & Wenonah Ave					West Entrance Driveway & Tramway Blvd					East Entrance Driveway & Wenonah Ave				
kground		Movement	Delay (s/veh)	LOS	Storage Length (veh)	95th% Length (veh)	Movement	Delay (s/veh)	LOS	Storage Length (veh)	95th% Length (veh)	Movement	Delay (s/veh)	LOS	Storage Length (veh)	95th% Length (veh)	Movement	Delay (s/veh)	LOS	Storage Length (veh)	95th% Length (veh)
		EBL	13.3	В	6.0	0.1	EBL	-	-	-	-	EBL	-	-	-	-	EBL	7.6	А	-	0.0
	Hoi	EBR	9.1	Α	-	0.5	EBR	-	-	-	-	EBR	-	-	-	-	EBR				
Bac	놑	EBT	15.5	С	-	0.1	EBT	11.9	В	-	0.9	EBT	14.1	В	-	0.1	EBT				
out	l Pe	NBL	7.6	A	6.8	0.3	NBL	7.8	А	-	0.1	NBL	7.8	Α	5.0	0.0	NBL	11.7	В	3.8	0.0
d-C	AM	NBR					NBR					NBR					NBR	8.8	Α	3.8	0.0
Buil		NBT					NBT					NBT			-		NBT	-	-	-	-
		SBL	7.9	A	5.2	0.0	SBL					SBL	8.2	A	6.4	0.3	SBL	-	-	-	-
		SBR					SBR					SBR					SBR	-	-	-	-
		SBT					SBT					SBT					SBT	9.7	Α	-	0.2
		WBL	15	В	4.2	0.0	WBL					WBL	-	-	-	-	WBL	7.6	Α	-	0.0
		WBR	12.3	В	4.2	0.2	WBR					WBR	-	-	-	-	WBR				
		WBT	-	-	-	-	WBT					WBT	10.6	В	-	0.5	WBT				
		Wenonah Ave & Four Hills Rd			Tramway Blvd & Wenonah Ave				West Entrance Driveway & Tramway Blvd					East Entrance Driveway & Wenonah Ave							
		Movement	Delay (s/veh)	LOS	Storage Length (veh)	95th% Length (veh)	Movement	Delay (s/veh)	LOS	Storage Length (veh)	95th% Length (veh)	Movement	Delay (s/veh)	LOS	Storage Length (veh)	95th% Length (veh)	Movement	Delay (s/veh)	LOS	Storage Length (veh)	95th% Length (veh)
pur		EBL	15	В	6.0	0.7	EBL	-	-	-	-	EBL	-	-	-	-	EBL	7.6	A	-	0.1
groi	our	EBR	11.8	В	-	2.1	EBR	-	-	-	-	EBR	-	-	-	-	EBR				
ack	Η¢	EBT	16.8	C	-	0.3	EBT	16.0	С	-	1.9	EBT	24.9	С	-	0.7	EBT				
t Ba	eal	NBL	7.9	A	6.8	0.2	NBL	8.9	A	-	0.2	NBL	8.7	A	5.0	0.0	NBL	17.5	С	3.8	0.0
no	2	NBR					NBR					NBR					NBR	9.6	A	3.8	0.0
Build-	Ы	NBT					NBT					NBT			-		NBT	-	-	-	-
		SBL	7.8	A	5.2	0.1	SBL					SBL	8.9	A	6.4	0.9	SBL	-	-	-	-
		SBR					SBR					SBR					SBR	-	-	-	-
		SBT					SBT					SBT					SBT	12.7	В	-	1.1
		WBL	18.9	C	4.2	0.0	WBL					WBL	-	-	-	-	WBL	8.3	Α	-	0.0
		WBR	12.5	В	4.2	0.1	WBR					WBR	-	-	-	-	WBR				
		WBT	-	-	-	-	WBT					WBT	12.8	В	-	1.4	WBT				

#### Table 5: HCS Result Summary for Build-Out Year (2025) Backgroud Conditions


		N N		Т	ramway Blv	d & Wenor	nah Ave		West	Entrance Dr	iveway & T	ramway Blv	ď	East Entrance Driveway & Wenonah Ave							
		Movement	Delay (s/veh)	LOS	Storage Length (veh)	95th% Length (veh)	Movement	Delay (s/veh)	LOS	Storage Length (veh)	95th% Length (veh)	Movement	Delay (s/veh)	LOS	Storage Length (veh)	95th% Length (veh)	Movement	Delay (s/veh)	LOS	Storage Length (veh)	95th% Length (veh)
		EBL	14.4	В	6.0	0.1	EBL	-	-	-	-	EBL	-	-	-	-	EBL	7.7	Α	-	0.0
tal	5	EBR	9.3	A	-	0.6	EBR	-	-	-	-	EBR	-	-	-	-	EBR				
To	Р	EBT	16.6	С	-	0.1	EBT	12.0	В	-	1	EBT	15.1	В	-	0.1	EBT				
ont	eak	NBL	7.7	A	6.8	0.3	NBL	7.8	Α	-	0.1	NBL	7.8	А	5.0	0.0	NBL	12.1	В	3.8	0.0
-bli	A Pe	NBR					NBR					NBR					NBR	8.8	Α	3.8	0.0
Bu	AN	NBT					NBT					NBT			-		NBT	-	-	-	-
		SBL	7.9	A	5.2	0.0	SBL					SBL	8.3	Α	6.4	0.4	SBL	-	-	-	-
		SBR					SBR					SBR					SBR	-	-	-	-
		SBT					SBT					SBT					SBT	10.8	В	-	0.4
		WBL	16.3	С	4.2	0.0	WBL					WBL	-	-	-	-	WBL	7.6	Α	-	0.0
		WBR	12.8	В	4.2	0.2	WBR					WBR	-	-	-	-	WBR				
		WBT	-	-	-	-	WBT					WBT	11.2	В	-	0.7	WBT				
		l l	Wenonah A	ve & Four H	Hills Rd		Т	'ramway Blv	d & Wenor	nah Ave		West I	Entrance Dr	iveway & T	ramway Blv	ď	East E	Intrance Dri	iveway & W	/enonah Av	e
		Movement	Delay (s/veh)	LOS	Storage Length (veh)	95th% Length (veh)	Movement	Delay (s/veh)	LOS	Storage Length (veh)	95th% Length (veh)	Movement	Delay (s/veh)	LOS	Storage Length (veh)	95th% Length (veh)	Movement	Delay (s/veh)	LOS	Storage Length (veh)	95th% Length (veh)
		EBL	15.4	С	6.0	0.8	EBL	-	-	-	-	EBL	-	-	-	-	EBL	7.6	A	-	0.1
tal	L.	EBR	11.9	В	-	2.2	EBR	-	-	-	-	EBR	-	-	-	-	EBR				
10 1	Р.	EBT	17.0	С	-	0.3	EBT	16.2	С	-	1.9	EBT	26.6	D	-	0.8	EBT				
- Po	eak	NBL	7.9	Α	6.8	0.2	NBL	8.9	Α	-	0.2	NBL	8.7	Α	5.0	0.00	NBL	17.7	С	3.8	0.0
-ild-	A P	NBR					NBR					NBR					NBR	9.6	Α	3.8	0.0
Bu	PA	NBT					NBT					NBT			-		NBT	-	-	-	-
		SBL	7.8	Α	5.2	0.1	SBL					SBL	8.9	Α	6.4	0.9	SBL	-	-	-	-
		SBR					SBR					SBR					SBR	-	-	-	-
		SBT					SBT					SBT					SBT	13.2	В	-	1.3
		WBL	19.4	С	4.2	0.0	WBL					WBL	-	-	-	-	WBL	8.3	Α	-	0.0
		WBR	12.6	В	4.2	0.2	WBR					WBR	-	-	-	-	WBR				
		W/BT	-	-	-	-	WBT					WBT	13.7	В	-	1.7	WBT				

#### Table 6: HCS Result Summary for Build-Out Year (2025) Total Conditions



From the above tables, the following conclusions are made from the Build-Out year analysis:

- Wenonah Ave & Four Hills Rd
  - Capacity Analysis
    - The intersection approaches are predicted to operate at LOS C or better, the westbound left turn changed from existing LOS B to C during the AM peak hour. And the eastbound left turn changed from LOS B to C during the PM peak hour.
  - Queueing Analysis
    - Where HCS results are present, all queue lengths are <1 vehicle (20 feet). Except for the eastbound right movement during the PM peak hour, with a queue of 2.2 vehicles.
- Wenonah Ave & Tramway Blvd
  - Capacity Analysis
    - The intersection approaches are predicted to operate at LOS C or better, and is unchanged from the existing operating LOS to build-out conditions.
  - Queueing Analysis
    - Where HCS results are present, all queue lengths for the northbound left during the AM peak hour and PM peak hour are <1 vehicle (20 feet). The eastbound through is 1 and 1.9 during the AM and PM peak hour, respectively.
- Tramway Blvd & West Entrance Dwy
  - o Capacity Analysis
    - The intersection approaches are predicted to operate at LOS D or better. The eastbound through changed from existing LOS C to D during the PM peak hour.
  - o Queueing Analysis
    - Where HCS results are present, all queue lengths are <1 vehicle (20 feet). Except for the westbound through movement during the PM peak hour, with a queue of 1.7 vehicles.
- Wenonah Ave & East Entrance Dwy
  - o Capacity Analysis
    - The intersection approaches are predicted to operate at LOS C or better. The southbound through changed from existing LOS A to B during the AM peak hour.
  - Queueing Analysis
    - Where HCS results are present, all queue lengths are <1 vehicle (20 feet). Except for the southbound through movement during the PM peak hour, with a queue of 1.3 vehicles.

# SITE RELATED CAPACITY MITIGATIONS AND STREET IMPROVEMENTS

The above section shows that capacity and queueing issues are not observed during the study peak hours. No capacity mitigations or street improvements are recommended based on the HCS Analysis results pertaining to the proposed site development.

## SITE ACCESS SIGHT DISTANCE

The following presents recommended intersection sight distance requirements for the access driveway serving the Development. Intersection sight distance requirements were calculated based on the CABP DPM Chapter 7-4 for the east driveway, and 2018 AASHTO "Green Book" chapter 9.5 for the west driveway as the DPM does not contain any cases for a 4 lane divided roadway. The design vehicle used was a passenger vehicle.

- Case B1 A stopped vehicle turning left turn from a minor street approach onto a major road.
- Case B2 A stopped vehicle turning right from a minor street approach onto a major road.



Intersection sight distances were calculated based on the following assumptions:

• Required intersection sight distance for Case B2 was calculated based on the design vehicle crossing into the first lane of the roadway.

Values shown below in Table 7 were rounded up to the nearest 5-foot increment. Formulas, values, and calculations used in the sight distance analysis can be found in the Appendix D.

Case	Roadway	Speed	Sight Distance Available	Sight Distance Required
Turning Left from East Dwy	Wenonah Ave	35 MPH	270 Ft	420 Ft
Turning Right from East Dwy	Wenonah Ave	35 MPH	480 Ft	340 Ft
Case B1 – Turning Left from West Dwy	Tramway Blvd	35 MPH	450 ft	465 Ft
Case B2 – Turning Right from West Dwy	Tramway Blvd	35 MPH	450 Ft	335 Ft

Table	7: 9	Site	Distance	Requirements
-------	------	------	----------	--------------

It is recommended that all development driveways adhere to the sight distance provisions detailed in the AASHTO "Green Book". An area bounded by the above sight distances with the decision point placed 14.5 feet back from the edge of the shoulder midway between the outbound driving lane should be maintained clear of any obstructions.

# AUXILIARY LANE ANALYSIS

CABQ DPM auxiliary lane warrants were reviewed for the site access driveways. DPM Table 7.4.67 was used to determine potential auxiliary lane needs for site access points and to guide recommendations. DPM Tables 7.4.68 and 7.4.70 were used to determine applicable deceleration lengths. 2025 Full Build-Out traffic volumes and direct trips were used in the analysis. The results of this analysis are shown in Table 8 and the narratives below.



Location	Access/Turn Type	Posted Speed Limit	Max Turning Volume per Hour	DPM Warrant Result	Recommendation
Tramway	Full Access (Right Turn)	35 MPH	31	Not Required	Existing auxiliary lane; no change recommended
Dwy	Full Access (Left Turn)	35 MPH	256	Required	Lengthen existing auxiliary lane to meet DPM criteria or geometric constraints
Wenonah & East	Full Access (Right Turn)	35 MPH	58	Required	No change recommended; see narrative below
Dwy	Full Access (Left Turn)	35 MPH	30	Not Required	Existing Two-Way Left-Turn Lane; no change recommended

#### Table 8: Auxiliary Turn Lane Warrant

For the intersection of Tramway and the West Access Driveway:

- A right turn lane was not warranted per DPM criteria, however a right turn lane exists of approximately 150ft with a 50ft transition taper. No changes are recommended for the existing auxiliary lane.
- A left turn lane is warranted per DPM criteria. A turn lane exist approximately 200 ft in length with a 50 ft transition taper. This lane as it exists does not meet DPM design specifications. It is recommended that this auxiliary lane be lengthened to meep DPM criteria or geometric constraints. DPM criteria for this auxiliary turn lane recommends a queueing length of 50 FT, and a deceleration length of 240-350 FT with a 150 FT reverse curve taper. However, geometric constraints may limit the total length available for the auxiliary lane.

For the intersection of Wenonah and the East Access Driveway:

A right turn lane is warranted per DPM criteria, however the construction of a new turn lane is not recommended. Existing traffic volumes, without the development, do not meet DPM criteria to warrant an auxiliary lane. While the addition of site trips does meet DPM criteria to warrant an auxiliary lane, only direct trips were analyzied in this report. The ITE Trip Generation Manual, 11<sup>th</sup> Edition provides a pass-by trip reduction of up to 50% which would potentially reduce the total number of new vehicles served by this intersection. Additionally, the right-in movement at this driveway is located approximately 300 feet from the stop controlled intersection of Four Hills Rd and Wenonah Ave where approaching traffic speeds are likely to be reduced. Furthermore, the site's location within an existing grocery store parking lot is likely to see a portion of its trips/sales through its "convenience" thereby reducing entering traffic. Therefore, construction of a right turn auxiliary lane at this driveway is not recommended.



• A left turn lane is not warranted per DPM criteria. However, a two-way left-turn lane exists as an auxiliary lane serving this access point. No changes are recommended for this movement.

Based on the information presented above, a turn lane is not recommended for right turns entering the site at Wenonah and the East Access Driveway. It is recommended that the existing left turn lane serving the site's west driveway be lengthened to meet DPM design specifications as closely as possible.

# DRIVE-THRU QUEUING ANALYSIS Drive-Thru Description

Based on the development site plan, the Tramway Starbucks will be located on a 21,253 square foot lot. 1,310 square feet will be developed for the building. The portion of the lot, designed for parking and vehicle access, contains an approximately 25-foot-wide parking lot travel lane and 14 parking spaces east of the building and drive-thru queue.

Per the site plan presented in Figure 1, queue storage is provided for 13 passenger vehicles. The queue processes counterclockwise around the building and exits into the existing parking lot travel lane adjacent to Wenonah Ave. Furthermore, there appears to be room within the parking lot travel lane to accommodate an additional three passenger cars without blocking the site's eastern entrance or spilling onto Wenonah Ave.

## **QUEUING VARIABLES**

Queue extension analysis is presented using the following variables:

- The Arrival Rate (λ) is measured in vehicles per hour (vph). This rate determines how many vehicles enter the system in an hour. The value used in this analysis for λ was the ITE Trip Generation AM peak hour ingress volume for land use Coffee/Donut Shop with Drive-Through Window- ITE 937 presented previously in Table 4.
  - The site includes interior dining facilities, and presumably, some portion of the total inbound traffic would be dining on-site rather than using the drive-thru. No data related to the dinein percentage was available. Thus, the conservative approach of routing 100% of ITE AM peak hour ingress traffic through the drive-thru was used.
- Average Time in System (E<sub>v</sub>) is measured in seconds and converted to hours for calculation purposes. The variable represents the average amount of time individual vehicles spend in the queue from entry to exit. It is used to calculate the service rate in conjunction with the arrival rate.
  - Lee Engineering used a national study conducted in 2021 to know the Average Time in System for the Tramway Starbucks. The Starbucks Average Time in System of 409 seconds was used for the pruposes of this analysis.
- The Service Rate (μ) is also measured in vph; this is the rate at which vehicles are processed through the drive-thru. The value for the Service Rate used in this analysis was calculated from the Average Time in System in conjunction with the Arrival Rate based on the Queuing Performance Equations for Random Arrival-Random Service Single Channel Systems presented in Traffic Flow Fundamentals by Adolf D. May.

$$\mu = \lambda + \frac{1}{E_v}$$

## **QUEUING ANALYSIS**

The queuing analysis assumed a single-channel queuing model where arrivals occur according to a Poisson process and service times have an exponential distribution (M/M/1 model). The following equation for M/M/1 queuing was used to determine a certain queue length's probability. The results for the probabilities of 0 through 25 vehicles are presented in Table 9.

Probability of More Than "k" Vehicles in Queue =	$\left(\frac{\lambda}{\mu}\right)^{k}$	k+1
--------------------------------------------------	----------------------------------------	-----

Number of Vehicles in Queue (k)	Probability of the Number of Vehicles	Cummulative Probability of the Number of Vehicles	Number of Vehicles in Queue (k)	Probability of the Number of Vehicles	Cummulative Probability of the Number of Vehicles
0	12%	12%	13	2%	83%
1	10%	22%	14	2%	85%
2	9%	31%	15	2%	87%
3	8%	40%	16	2%	88%
4	7%	47%	17	1%	90%
5	6%	53%	18	1%	91%
6	6%	59%	19	1%	92%
7	5%	64%	20	1%	93%
8	4%	68%	21	1%	94%
9	4%	72%	22	1%	94%
10	3%	75%	23	1%	95%
11	3%	78%	24	1%	96%
12	3%	81%	25	1%	96%

Table 0: Probability of "k" Vehicles in Queue

Table 9 shows that the 95<sup>th</sup> percentile probability event during the AM peak hour is equal to 23 vehicles in the queue, which fails to meet the threshold design of 13 vehicles. Although, there is potential for three additional vehicles, the design length does not appear to be enough to prevent conflict with the operations of the East driveway. The likelihood of the queue interfering with operations at the North Access Rd is low. This outcome is a 99<sup>th</sup>+ percentile event during PM peak hours. The results of the Queuing Analysis for the PM peak hour are further summarized in Table 10.

Table 10: Queuing Analysis Summary												
Probability of Exceeding Queue Storage	Probability of Queue Spillback to East Dwy	AM PH Average Number of Vehicles in Queue										
19%	9%	7										

Therefore, proposed Drive-Through Queue Storage accommodates average and 88th percentile queues but fails to accommodate 95th percentile of vehicle queues as designed. It is noted that the provided storage does meet requirements set forth in the City of Albuquerque's Integrated Development Ordinance.



# **CRASH DATA SUMMARY**

At the request of the NMDOT, a crash summary for the intersections within the study area has been completed. The purpose of this analysis is to highlight trends and observations from summarized crash data. Crash data was provided by NMDOT for the years 2015 to 2019 in aggregate form and is summarized in the table below.

	C rash S ummary	Wenonah Ave & Four Hills Rd	Tramway Blvd & Wenonah Ave
	Total Crashes	19	21
	2015	5	6
ear	2016	5	3
X	2017	3	3
Ð,	2018	3	6
	2019	3	3
	Fixed Object	0	4
	Left Blank	3	3
	Other Vehicle - Both Going Straight/Entering at Angle	7	1
	Other Vehicle - From Opposite Direction	0	1
بە	Other Vehicle - From Same Direction/Both Going Straight	1	2
[yp	Other Vehicle - From Same Direction/Rear End Collision	1	1
چ	Other Vehicle - From Same Direction/Sideswipe Collision	0	1
	Other Vehicle - One Left Turn/Entering At Angle	4	4
	Rollover	1	1
	Pedestrian Collision	1	0
	Other Vehicle - All Others/Entering At Angle	1	0
	Non-Collision - All Other/Not Stated	0	3
ing	Daylight	15	13
ght litio	Dawn/Dusk	2	0
on CLI	Dark	2	7
പ്റ	Invalid Code/Not Specified	0	1
Ę.	Fatal (K)	0	0
veri	Serious Injury (A)	0	0
Se	Visible Injury (B)	9	9
Βy	Complaint of Injury (C)	0	0
	Property Damage Only (O)	10	12
	Driver Inattention	4	6
	Passed Stop Sign	0	1
	Improper Overtaking	0	1
	Improper Lane Change	1	0
S	Pedestrian Error	1	0
ct c	Alcohol/Drug Involved	1	3
Fa	Avoid No Contact - Vehicle	1	0
ting	Collision with Motor Vehicle	0	0
ibut	Disregarded Traffic Signal	1	0
ntr	Excessive Speed	0	3
õ	Failed to Yield Right of Way	8	3
By	Following Too Closely	1	2
	Improper Backing	0	0
	Made Improper Turn	0	0
	Missing Data	1	0
	None	0	0
	Other	0	2

#### Table 11: Crash Summary



From the above table, the following observations are made:

- For the intersection of Wenonah Ave & Four Hills Rd:
  - Within the years 2015 to 2019, 19 crashes were reported.
  - The most common crash classification was Other Vehicle Both going straight/Entering at angle.
  - $\circ$   $\;$  The majority of crashes at this intersection occurred during daylight hours.
  - No fatal crashes were reported from 2015 to 2019. However, injuries were reported.
  - $\circ$   $\;$  The most common reported cause was Failed to Yield Right of Way.
- For the intersection of Tramway Blvd & Wenonah Ave:
  - $\circ$   $\;$  Within the years 2015 to 2019, 21 crashes were reported.
  - The most common crash classification was Fixed Object and Other Vehicle One Left Turn/Entering at Angle.
  - $\circ$   $\;$  The majority of crashes at this intersection occurred during daylight hours.
  - No fatal crashes were reported from 2015 to 2019. However, injuries were reported.
  - The most common reported cause was Driver Inattention.

# SUMMARY OF RECOMMENDATIONS

The following presents a summary of recommendations included in this report.

### CONCLUSIONS

- All study intersections operate at an acceptable LOS throughout all study scenarios
- 95<sup>th</sup> % Queue Lengths do not exceed queue storage at study intersections for studied analysis scenarios
- HCS results do not suggest the need for capacity mitigation measures or street improvements related to the proposed development
- Proposed Drive-Through Queue Storage accommodates average and 88<sup>th</sup> percentile queues but fails to accommodate 95<sup>th</sup> percentile of vehicle queues as designed. However, the provided storage does meet requirements set forth in the City of Albuquerque's Integrated Development Ordinance.

#### RECOMMENDATIONS

It is recommended that the existing southbound left turn lane serving the site at the west driveway be lengthened to meet DPM design specifications as closely as possible.

Maintain sight distance at all driveways by keeping sight lines visibility free from any obstructions such as but not limited to parking, canopies, site displays, and landscaping.



**APPENDIX A:** 

**SCOPING MEETING NOTES** 

## SCOPE OF TRAFFIC IMPACT STUDY (TIS)

TO: Terry Brown, P.E. Tierra West, LLC 5571 Midway Park PI. NE Albuquerque, NM 87109

**MEETING DATE:** Monday, May 1, 2023 (1:00 pm)

**ATTENDEES:** Matthew Grush, P.E. email comments (City of Albuquerque), Margaret Haynes<sup>\*</sup>, P.E. (NM DOT D3), Ronald R. Bohannan, P.E., Terry Brown, P.E., Amanda Herrera, P.E., and Derek Bohannan (Tierra West, LLC)

\* - Margaret Haynes sent an e-mail dated 05/01/2023 stating that the NM DOT will not be involved with this Traffic Impact Study.

**PROJECT:** Starbuck's Coffee (Wenonah Ave. / Tramway Blvd.)

**REQUESTED CITY ACTION:** Zone Change X Site Development Plan

\_\_\_\_Subdivision \_\_\_\_Building Permit \_\_\_\_Sector Plan \_\_\_\_Sector Plan

\_\_\_ Curb Cut Permit \_\_\_ Conditional Use \_\_\_ Annexation \_\_\_ Site Plan Amendment

**ASSOCIATED APPLICATION:** Coffee Shop w/ Drive- Thru Window (1,310 SF)

#### SCOPE OF REPORT:

The Traffic Impact Study should follow the standard report format, which is outlined in the DPM. The following supplemental information is provided for the preparation of this specific study.

1. Trip Generation - Use Trip Generation Manual, 11th Edition.

Local data may be used for certain land use types as determined by staff. Consultant to provide local traffic generation volumes (AM and PM Peak Hour) for the following existing Starbuck's and Dutch Brothers Coffee shops:

- 1) Starbuck's (Montgomery Blvd. & San Mateo)
- 2) Starbuck's (Paseo del Norte & Golf Course Rd.)
- 3) Starbuck's (Indian School Rd. / Juan Tabo Blvd.)
- 4) Dutch Brothers (Fortuna Rd. / Coors Blvd.)

(Compare data to ITE Trip Generation data (11<sup>th</sup> Edition) – Local data preferred if significant variations.

2. Appropriate study area: Signalized Intersections; N/A

Unsignalized Intersections;

- a. Wenonah Ave / Four Hills Rd
- b. Tramway / Wenonah Ave.

Driveway Intersections: Smith's Access driveways (2)

- a. West entrance Driveway on Tramway @ Smith's
- b. South entrance Driveway on Wenonah @ Smith's

3. Intersection turning movement counts

Study Time – 7-9 a.m. peak hour, 4-6 p.m. peak hour Consultant to provide for all intersections listed above. (Intersection turning movements counts to be correlated with TAQA data, if available)

- 4. Type of intersection progression and factors to be used. N/A
- 5. Boundaries of area to be used for trip distribution.

City Wide - residential, office or industrial; 2 mile radius – commercial; Interstate or to be determined by consultant - motel/hotel APS district boundary mapping for each school and bus routes

6. Basis for trip distribution.

Commercial - Use relationship based upon population. Use population data from 2040 Socioeconomic Forecasts, MRCOG – See MRCOG website for most current data.

Residential - Ts = (Tt)(Se/D)/(Se/D)Ts = Development to Individual Subarea Trips Tt = Total Trips Se = Subarea Employment D = Distance from Development to Subarea

Office/Industrial - Ts = (Tt) (Sp / D) / (Sp / D) Ts = Development to Individual Subarea Trips Tt = Total Trips Sp = Subarea Population

D = Distance from Development to Subarea

Commercial -Ts = (Tt) (Sp) / (Sp) Ts = Development to Individual Subarea Trips Tt = Total Trips Sp = Subarea Population

- 7. Traffic Assignment. Logical routing on the major street system.
- Proposed developments which have been approved but not constructed that are to be Included in the analyses. Projects in the area include:
  a. N/A
- Method of intersection capacity analysis planning or operational (see "2016 Highway Capacity Manual" or equivalent [i.e. HCS, Synchro, Teapac, etc.] as approved by staff). Must use latest version of design software and/or current edition of design manual. Implementation Year: 2025 Horizon Year: N/A
- 10. Traffic conditions for analysis:
  - a. Existing analysis \_\_\_yes <u>X</u> no year (N/A);

- b. Phase implementation year(s) without proposed development N/A
- c. Phase implementation year(s) with proposed development N/A
- d. Project completion year without proposed development 2025
- e. Project completion year with proposed development 2025
- f. Other -
- Background traffic growth. Method: use 10-year historical growth based on standard data from the MRCOG Traffic Flow Maps. Minimum growth rate to be used is 1/2%.
- Planned (programmed) traffic improvements. List planned CIP improvements in study area and projected project implementation year:
  - a. Project Location (Implementation Year) N/A
- 13. Items to be included in the study:
  - a. Intersection analysis. Yes
  - b. Signal progression An analysis is required if the driveway analysis indicates a traffic signal is possibly warranted. Analysis Method: N/A
  - c. Arterial LOS analysis; No
  - d. Recommended street, intersection and signal improvements. Yes
  - e. Site design features such as turning lanes, median cuts, queuing requirements and site circulation, including driveway signalization and visibility. Yes
  - f. Transportation system impacts. Yes
  - g. Other mitigating measures. Yes
  - Accident analyses <u>X</u> yes no; Location(s): Wenonah Ave / Four Hills Rd (5 years) Tramway / Wenonah Ave (5 years)
  - i. Weaving analyses \_\_\_\_yes \_X\_\_no; Location(s):
  - j. Other:

#### SUBMITTAL REQUIREMENTS:

- 1. Number of copies of report required
  - a. 1 paper copy
  - b. 1 digital copy
- 2. Submittal Fee \$1300 for up to 3 reviews (plus technology fee)

The Traffic Impact Study for this development proposal, project name, shall be performed in accordance with the above criteria. If there are any questions regarding the above items, please contact me at 924-3362.

MPM-P.E.

5/2/2023

Date

Matt Grush, P.E. Senior Engineer City of Albuquerque, Planning Transportation Development Section

via: email

C: TIS Task Force Attendees, file

**APPENDIX B:** 

**TURNING MOVEMENT COUNTS** 



## **Turning Movement Data**

			Tramway					Tramway			Wenonah Ave						
Chart Time			Southbound					Northbound					Eastbound				
Start Time	Right	Thru	U-Turn	Peds	App. Total	Thru	Left	U-Turn	Peds	App. Total	Right	Left	U-Turn	Peds	App. Total	Int. Total	
6:00 AM	5	5	0	4	10	4	3	0	0	7	2	14	0	0	16	33	
6:15 AM	9	8	0	0	17	11	4	0	0	15	3	16	0	0	19	51	
6:30 AM	8	15	0	4	23	13	3	0	0	16	4	23	0	0	27	66	
6:45 AM	5	14	0	0	19	13	6	0	0	19	1	19	0	1	20	58	
Hourly Total	27	42	0	8	69	41	16	0	0	57	10	72	0	1	82	208	
7:00 AM	13	16	0	0	29	19	3	0	0	22	3	14	0	1	17	68	
7:15 AM	13	17	0	0	30	19	5	0	0	24	7	22	1	0	30	84	
7:30 AM	20	27	0	0	47	21	8	0	0	29	4	35	0	2	39	115	
7:45 AM	8	25	0	0	33	22	6	0	0	28	5	27	0	1	32	93	
Hourly Total	54	85	0	0	139	81	22	0	0	103	19	98	1	4	118	360	
8:00 AM	13	36	0	1	49	27	11	0	0	38	4	22	0	1	26	113	
8:15 AM	15	41	0	1	56	32	5	0	0	37	3	33	0	1	36	129	
8:30 AM	15	31	0	2	46	23	5	0	0	28	3	33	0	2	36	110	
8:45 AM	19	42	0	0	61	28	6	0	0	34	11	29	0	2	40	135	
Hourly Total	62	150	0	4	212	110	27	0	0	137	21	117	0	6	138	487	
*** BREAK ***	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	
11:00 AM	14	48	0	3	62	29	5	0	0	34	6	14	0	4	20	116	
11:15 AM	11	49	0	3	60	35	4	0	0	39	5	17	0	8	22	121	
11:30 AM	20	46	0	2	66	33	5	0	0	38	14	19	0	1	33	137	
11:45 AM	18	49	0	0	67	24	5	0	0	29	9	18	0	3	27	123	
Hourly Total	63	192	0	8	255	121	19	0	0	140	34	68	0	16	102	497	
12:00 PM	11	51	0	1	62	53	13	0	2	66	12	27	0	2	39	167	
12:15 PM	19	63	0	0	82	38	10	0	3	48	8	22	0	1	30	160	
12:30 PM	20	59	0	1	79	42	6	0	0	48	7	21	0	1	28	155	
12:45 PM	19	64	0	2	83	37	16	0	1	53	7	17	0	0	24	160	
Hourly Total	69	237	0	4	306	170	45	0	6	215	34	87	0	4	121	642	
1:00 PM	16	61	0	1	77	30	9	0	0	39	10	15	0	1	25	141	
1:15 PM	18	43	0	0	61	42	6	0	0	48	10	12	0	1	22	131	
1:30 PM	14	65	1	1	80	37	10	0	0	47	8	19	0	3	27	154	
1:45 PM	25	53	0	1	78	26	6	0	0	32	7	26	0	0	33	143	
Hourly Total	73	222	1	3	296	135	31	0	0	166	35	72	0	5	107	569	
*** BREAK ***	-	-		-	-	-	-	-	-	-	-	-	-	-	-	-	
3:00 PM	20	65	0	0	85	34	9	0	0	43	9	24	0	1	33	161	
3:15 PM	29	66	1	1	96	30	7	0	0	37	9	19	0	3	28	161	
3:30 PM	18	54	0	1	72	29	9	0	0	38	6	32	0	7	38	148	
3:45 PM	24	78	0	3	102	29	10	0	0	39	8	19	0	2	27	168	

Hourly Total	91	263	1	5	355	122	35	0	0	157	32	94	0	13	126	638
4:00 PM	18	72	0	4	90	30	14	0	0	44	8	29	0	2	37	171
4:15 PM	26	87	0	0	113	46	11	0	0	57	9	22	0	0	31	201
4:30 PM	35	88	0	3	123	29	6	0	0	35	12	22	1	2	35	193
4:45 PM	31	89	0	3	120	30	10	0	0	40	19	23	0	4	42	202
Hourly Total	110	336	0	10	446	135	41	0	0	176	48	96	1	8	145	767
5:00 PM	27	69	0	0	96	24	20	0	1	44	19	34	0	1	53	193
5:15 PM	36	118	1	3	155	30	17	0	2	47	7	30	0	6	37	239
5:30 PM	33	77	0	4	110	33	12	0	3	45	14	31	0	2	45	200
5:45 PM	28	69	0	0	97	28	17	0	0	45	12	10	0	3	22	164
Hourly Total	124	333	1	7	458	115	66	0	6	181	52	105	0	12	157	796
Grand Total	673	1860	3	49	2536	1030	302	0	12	1332	285	809	2	69	1096	4964
Approach %	26.5	73.3	0.1	-	-	77.3	22.7	0.0	-	-	26.0	73.8	0.2	-	-	-
Total %	13.6	37.5	0.1	-	51.1	20.7	6.1	0.0	-	26.8	5.7	16.3	0.0	-	22.1	-
Lights	580	1839	3	-	2422	1003	288	0	-	1291	274	775	2	-	1051	4764
% Lights	86.2	98.9	100.0	-	95.5	97.4	95.4	-	-	96.9	96.1	95.8	100.0	-	95.9	96.0
Buses	63	1	0	-	64	0	0	0	-	0	0	1	0	-	1	65
% Buses	9.4	0.1	0.0	-	2.5	0.0	0.0	-	-	0.0	0.0	0.1	0.0	-	0.1	1.3
Trucks	12	20	0	-	32	24	5	0	-	29	2	13	0	-	15	76
% Trucks	1.8	1.1	0.0	-	1.3	2.3	1.7	-	-	2.2	0.7	1.6	0.0	-	1.4	1.5
Bicycles on Road	18	0	0	-	18	3	9	0	-	12	9	20	0	-	29	59
% Bicycles on Road	2.7	0.0	0.0	-	0.7	0.3	3.0	-	-	0.9	3.2	2.5	0.0	-	2.6	1.2
Bicycles on Crosswalk	-	-	-	0	-	-	-	-	2	-	-	-	-	5	-	-
% Bicycles on Crosswalk	-	-	-	0.0	-	-	-	-	16.7	-	-	-	-	7.2	-	-
Pedestrians	-	-	-	49	-	-	-	-	10	-	-	-	-	64	-	-
% Pedestrians	-	-	-	100.0	-	-	-	-	83.3	-	-	_	-	92.8	-	-





Turning Movement Data Plot



## Turning Movement Peak Hour Data (8:00 AM)

			Tramway			,		Tramway	-	-						
Chart Time			Southbound					Northbound					Eastbound			
Start Time	Right	Thru	U-Turn	Peds	App. Total	Thru	Left	U-Turn	Peds	App. Total	Right	Left	U-Turn	Peds	App. Total	Int. Total
8:00 AM	13	36	0	1	49	27	11	0	0	38	4	22	0	1	26	113
8:15 AM	15	41	0	1	56	32	5	0	0	37	3	33	0	1	36	129
8:30 AM	15	31	0	2	46	23	5	0	0	28	3	33	0	2	36	110
8:45 AM	19	42	0	0	61	28	6	0	0	34	11	29	0	2	40	135
Total	62	150	0	4	212	110	27	0	0	137	21	117	0	6	138	487
Approach %	29.2	70.8	0.0	-	-	80.3	19.7	0.0	-	-	15.2	84.8	0.0	-	-	-
Total %	12.7	30.8	0.0	-	43.5	22.6	5.5	0.0	-	28.1	4.3	24.0	0.0	-	28.3	-
PHF	0.816	0.893	0.000	-	0.869	0.859	0.614	0.000	-	0.901	0.477	0.886	0.000	-	0.863	0.902
Lights	52	141	0	-	193	107	25	0	-	132	20	115	0	-	135	460
% Lights	83.9	94.0	-	-	91.0	97.3	92.6	-	-	96.4	95.2	98.3	-	-	97.8	94.5
Buses	8	1	0	-	9	0	0	0	-	0	0	0	0	-	0	9
% Buses	12.9	0.7	-	-	4.2	0.0	0.0	-	-	0.0	0.0	0.0	-	-	0.0	1.8
Trucks	1	8	0	-	9	3	2	0	-	5	1	2	0	-	3	17
% Trucks	1.6	5.3	-	-	4.2	2.7	7.4	-	-	3.6	4.8	1.7	-	-	2.2	3.5
Bicycles on Road	1	0	0	-	1	0	0	0	-	0	0	0	0	-	0	1
% Bicycles on Road	1.6	0.0	-	-	0.5	0.0	0.0	-	-	0.0	0.0	0.0	-	-	0.0	0.2
Bicycles on Crosswalk	-	-	-	0	-	-	-	-	0	-	-	-	-	0	-	-
% Bicycles on Crosswalk	-	-	-	0.0	-	-	-	-	-	-	-	-	-	0.0	-	_
Pedestrians	-	-	-	4	-	-	-	-	0	-	-	-	-	6	-	-
% Pedestrians	-	-	-	100.0	-	-	-	-	-	-	-	-	-	100.0	-	-





Turning Movement Peak Hour Data Plot (8:00 AM)



## Turning Movement Peak Hour Data (4:45 PM)

			Tramway Southbound					Tramway Northbound	·							
Start Time	Right	Thru	U-Turn	Peds	App. Total	Thru	Left	U-Turn	Peds	App. Total	Right	Left	U-Turn	Peds	App. Total	Int. Total
4:45 PM	31	89	0	3	120	30	10	0	0	40	19	23	0	4	42	202
5:00 PM	27	69	0	0	96	24	20	0	1	44	19	34	0	1	53	193
5:15 PM	36	118	1	3	155	30	17	0	2	47	7	30	0	6	37	239
5:30 PM	33	77	0	4	110	33	12	0	3	45	14	31	0	2	45	200
Total	127	353	1	10	481	117	59	0	6	176	59	118	0	13	177	834
Approach %	26.4	73.4	0.2	-	-	66.5	33.5	0.0	-	-	33.3	66.7	0.0	-	-	-
Total %	15.2	42.3	0.1	-	57.7	14.0	7.1	0.0	-	21.1	7.1	14.1	0.0	-	21.2	-
PHF	0.882	0.748	0.250	-	0.776	0.886	0.738	0.000	-	0.936	0.776	0.868	0.000	-	0.835	0.872
Lights	119	352	1	-	472	111	57	0	-	168	56	110	0	-	166	806
% Lights	93.7	99.7	100.0	-	98.1	94.9	96.6	-	-	95.5	94.9	93.2	-	-	93.8	96.6
Buses	7	0	0	-	7	0	0	0	-	0	0	0	0	-	0	7
% Buses	5.5	0.0	0.0	-	1.5	0.0	0.0	-	-	0.0	0.0	0.0	-	-	0.0	0.8
Trucks	1	1	0	-	2	3	1	0	-	4	0	0	0	-	0	6
% Trucks	0.8	0.3	0.0	-	0.4	2.6	1.7	-	-	2.3	0.0	0.0	-	-	0.0	0.7
Bicycles on Road	0	0	0	-	0	3	1	0	-	4	3	8	0	-	11	15
% Bicycles on Road	0.0	0.0	0.0	-	0.0	2.6	1.7	-	-	2.3	5.1	6.8	-	-	6.2	1.8
Bicycles on Crosswalk	-	-	-	0	-	-	-	-	2	-	-	-	-	3	-	-
% Bicycles on Crosswalk	-	-	-	0.0	-	-	-	-	33.3	-	-	-	-	23.1	-	-
Pedestrians	-	-	-	10	-	-	-	-	4	-	-	-	-	10	-	-
% Pedestrians	-	-	-	100.0	-	-	-	-	66.7	-	-	-	-	76.9	-	-





Turning Movement Peak Hour Data Plot (4:45 PM)



## **Turning Movement Data**

										i uni	in ig i	10,01		Juiu											
			Driv Souti	/eway hbound					Weno Wes	nah Ave tbound					Driv Norti	veway hbound					Weno East	nah Ave tbound			
Start Time	Right	Thru	Left	U-Turn	Peds	App. Total	Right	Thru	Left	U-Turn	Peds	App. Total	Right	Thru	Left	U-Turn	Peds	App. Total	Right	Thru	Left	U-Turn	Peds	App. Total	Int. Total
6:00 AM	1	0	0	0	0	1	1	5	0	0	0	6	0	0	0	0	0	0	0	5	3	0	0	8	15
6:15 AM	3	0	0	0	2	3	2	12	0	1	0	15	0	0	0	0	0	0	0	10	2	0	0	12	30
6:30 AM	1	0	1	0	1	2	5	16	0	0	1	21	0	0	0	0	0	0	0	14	2	0	0	16	39
6:45 AM	2	0	2	0	0	4	4	19	0	0	0	23	0	0	0	0	1	0	1	16	0	0	0	17	44
Hourly Total	7	0	3	0	3	10	12	52	0	1	1	65	0	0	0	0	1	0	1	45	7	0	0	53	128
7:00 AM	5	0	0	0	0	5	6	15	0	0	0	21	0	0	0	0	0	0	3	16	0	0	0	19	45
7:15 AM	1	0	1	0	0	2	2	23	0	0	1	25	1	0	0	0	0	1	1	21	0	0	0	22	50
7:30 AM	3	0	2	0	0	5	7	27	0	0	1	34	0	0	0	0	0	0	1	24	3	0	0	28	67
7:45 AM	2	0	7	0	0	9	4	27	1	0	0	32	0	0	0	0	0	0	1	34	5	0	0	40	81
Hourly Total	11	0	10	0	0	21	19	92	1	0	2	112	1	0	0	0	0	1	6	95	8	0	0	109	243
8:00 AM	11	0	3	0	1	14	6	27	1	0	0	34	0	0	1	0	0	1	1	27	2	0	0	30	79
8:15 AM	4	0	5	0	0	9	8	33	0	0	0	41	0	0	2	0	0	2	0	43	2	0	0	45	97
8:30 AM	3	0	2	0	0	5	7	24	0	0	0	31	0	0	0	0	0	0	0	38	1	0	0	39	75
8:45 AM	8	0	3	0	0	11	3	26	0	0	0	29	0	0	0	0	0	0	1	44	5	0	0	50	90
Hourly Total	26	0	13	0	1	39	24	110	1	0	0	135	0	0	3	0	0	3	2	152	10	0	0	164	341
*** BREAK ***	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
11:00 AM	7	1	10	0	0	18	5	30	0	0	0	35	0	0	0	0	0	0	2	46	7	0	0	55	108
11:15 AM	11	0	11	0	1	22	8	25	3	0	0	36	1	0	0	0	0	1	1	48	5	0	1	54	113
11:30 AM	10	0	10	0	0	20	6	19	1	0	0	26	0	1	2	0	1	3	2	51	5	0	0	58	107
11:45 AM	9	1	10	0	0	20	3	20	0	0	0	23	0	0	2	0	0	2	2	43	7	0	0	52	97
Hourly Total	37	2	41	0	1	80	22	94	4	0	0	120	1	1	4	0	1	6	7	188	24	0	1	219	425
12:00 PM	18	1	6	0	0	25	4	46	1	0	0	51	2	0	2	0	0	4	2	53	8	0	0	63	143
12:15 PM	9	1	9	0	0	19	5	28	2	0	0	35	0	1	4	0	0	5	5	60	6	0	0	71	130
12:30 PM	12	1	7	0	0	20	4	38	0	0	0	42	0	1	1	0	1	2	2	58	4	0	0	64	128
12:45 PM	14	0	5	0	0	19	7	34	1	0	0	42	1	0	2	0	0	3	2	64	6	0	0	72	136
Hourly Total	53	3	27	0	0	83	20	146	4	0	0	170	3	2	9	0	1	14	11	235	24	0	0	270	537
1:00 PM	13	0	10	0	0	23	10	24	0	0	0	34	2	0	1	0	0	3	3	54	10	0	0	67	127
1:15 PM	12	0	11	0	0	23	9	28	0	0	0	37	0	0	0	0	0	0	3	38	6	0	1	47	107
1:30 PM	13	0	16	0	0	29	5	28	0	0	0	33	0	0	2	0	0	2	2	63	6	0	0	71	135
1:45 PM	9	0	14	0	0	23	5	23	0	0	0	28	0	1	0	0	0	1	3	52	6	0	1	61	113
Hourly Total	47	0	51	0	0	98	29	103	0	0	0	132	2	1	3	0	0	6	11	207	28	0	2	246	482
*** BREAK ***	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
3:00 PM	12	1	13	0	0	26	7	26	1	0	0	34	1	1	1	0	0	3	0	59	10	0	0	69	132
3:15 PM	14	0	18	0	0	32	11	20	0	0	0	31	0	0	2	0	0	2	1	67	5	0	0	73	138
3:30 PM	11	0	20	0	1	31	2	21	0	0	0	23	0	0	1	0	0	1	1	52	5	0	1	58	113

3:45 PM	15	0	11	0	0	26	12	18	0	0	0	30	1	0	0	0	0	1	2	75	4	0	0	81	138
Hourly Total	52	1	62	0	1	115	32	85	1	0	0	118	2	1	4	0	0	7	4	253	24	0	1	281	521
4:00 PM	13	0	24	0	0	37	7	26	0	0	0	33	0	1	3	0	0	4	0	71	8	0	0	79	153
4:15 PM	17	0	15	0	0	32	11	26	0	0	0	37	0	0	4	0	0	4	1	82	7	1	0	91	164
4:30 PM	10	0	15	0	1	25	6	22	0	0	0	28	0	0	0	0	0	0	0	92	11	0	0	103	156
4:45 PM	18	0	12	0	0	30	11	22	2	0	0	35	0	0	0	0	0	0	2	87	7	0	0	96	161
Hourly Total	58	0	66	0	1	124	35	96	2	0	0	133	0	1	7	0	0	8	3	332	33	1	0	369	634
5:00 PM	20	0	18	0	1	38	13	14	2	0	0	29	0	0	1	0	0	1	2	78	8	0	1	88	156
5:15 PM	24	0	24	0	0	48	8	22	0	0	0	30	1	0	1	0	0	2	0	113	5	0	0	118	198
5:30 PM	15	0	21	0	0	36	10	24	0	0	0	34	1	0	1	0	0	2	0	83	8	1	0	92	164
5:45 PM	20	0	13	0	0	33	6	18	2	0	0	26	0	0	1	0	0	1	2	65	8	0	0	75	135
Hourly Total	79	0	76	0	1	155	37	78	4	0	0	119	2	0	4	0	0	6	4	339	29	1	1	373	653
Grand Total	370	6	349	0	8	725	230	856	17	1	3	1104	11	6	34	0	3	51	49	1846	187	2	5	2084	3964
Approach %	51.0	0.8	48.1	0.0	-	-	20.8	77.5	1.5	0.1	-	-	21.6	11.8	66.7	0.0	-	-	2.4	88.6	9.0	0.1	-	-	-
Total %	9.3	0.2	8.8	0.0	-	18.3	5.8	21.6	0.4	0.0	-	27.9	0.3	0.2	0.9	0.0	-	1.3	1.2	46.6	4.7	0.1	-	52.6	-
Lights	348	6	348	0	-	702	226	833	17	1	-	1077	10	6	33	0	-	49	49	1823	180	2	-	2054	3882
% Lights	94.1	100.0	99.7	-	-	96.8	98.3	97.3	100.0	100.0	-	97.6	90.9	100.0	97.1	-	-	96.1	100.0	98.8	96.3	100.0	-	98.6	97.9
Buses	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0	1	0	0	-	1	1
% Buses	0.0	0.0	0.0	-	-	0.0	0.0	0.0	0.0	0.0	-	0.0	0.0	0.0	0.0	-	-	0.0	0.0	0.1	0.0	0.0	-	0.0	0.0
Trucks	21	0	1	0	-	22	4	11	0	0	-	15	1	0	0	0	-	1	0	13	7	0	-	20	58
% Trucks	5.7	0.0	0.3	-	-	3.0	1.7	1.3	0.0	0.0	-	1.4	9.1	0.0	0.0	-	-	2.0	0.0	0.7	3.7	0.0	-	1.0	1.5
Bicycles on Road	1	0	0	0	-	1	0	12	0	0	-	12	0	0	1	0	-	1	0	9	0	0	-	9	23
% Bicycles on Road	0.3	0.0	0.0	-	-	0.1	0.0	1.4	0.0	0.0	-	1.1	0.0	0.0	2.9	-	-	2.0	0.0	0.5	0.0	0.0	-	0.4	0.6
Bicycles on Crosswalk	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	-	-
% Bicycles on Crosswalk	-	-	-	-	0.0	-	-	-	-	-	0.0	-	-	-	-	-	0.0	-	-	-	-	-	0.0	-	-
Pedestrians	-	-	-	-	8	-	-	-	-	-	3	-	-	-	-	-	3	-	-	-	-	-	5	-	-
% Pedestrians	-	-	-	-	100.0	-	-	-	-	-	100.0	-	-	-	-	-	100.0	-	-	-	-	-	100.0	-	-
													-												





Turning Movement Data Plot



## Turning Movement Peak Hour Data (8:00 AM)

								1 011	in ig it	101011		oun	ioui	Duiu	10.00	,									
			Driv	eway					Wenor	nah Ave					Drive	eway					Wenor	ah Ave			
			South	nbound					West	bound					North	bound					East	ound			
Start Time	Right	Thru	Left	U-Turn	Peds	App. Total	Right	Thru	Left	U-Turn	Peds	App. Total	Right	Thru	Left	U-Turn	Peds	App. Total	Right	Thru	Left	U-Turn	Peds	App. Total	Int. Total
8:00 AM	11	0	3	0	1	14	6	27	1	0	0	34	0	0	1	0	0	1	1	27	2	0	0	30	79
8:15 AM	4	0	5	0	0	9	8	33	0	0	0	41	0	0	2	0	0	2	0	43	2	0	0	45	97
8:30 AM	3	0	2	0	0	5	7	24	0	0	0	31	0	0	0	0	0	0	0	38	1	0	0	39	75
8:45 AM	8	0	3	0	0	11	3	26	0	0	0	29	0	0	0	0	0	0	1	44	5	0	0	50	90
Total	26	0	13	0	1	39	24	110	1	0	0	135	0	0	3	0	0	3	2	152	10	0	0	164	341
Approach %	66.7	0.0	33.3	0.0	-	-	17.8	81.5	0.7	0.0	-	-	0.0	0.0	100.0	0.0	-	-	1.2	92.7	6.1	0.0	-	-	-
Total %	7.6	0.0	3.8	0.0	-	11.4	7.0	32.3	0.3	0.0	-	39.6	0.0	0.0	0.9	0.0	-	0.9	0.6	44.6	2.9	0.0	-	48.1	-
PHF	0.591	0.000	0.650	0.000	-	0.696	0.750	0.833	0.250	0.000	-	0.823	0.000	0.000	0.375	0.000	-	0.375	0.500	0.864	0.500	0.000	-	0.820	0.879
Lights	18	0	13	0	-	31	23	107	1	0	-	131	0	0	3	0	-	3	2	145	8	0	-	155	320
% Lights	69.2	-	100.0	-	-	79.5	95.8	97.3	100.0	-	-	97.0	-	-	100.0	-	-	100.0	100.0	95.4	80.0	-	-	94.5	93.8
Buses	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0
% Buses	0.0	-	0.0	-	-	0.0	0.0	0.0	0.0	-	-	0.0	-	-	0.0	-	-	0.0	0.0	0.0	0.0	-	-	0.0	0.0
Trucks	8	0	0	0	-	8	1	1	0	0	-	2	0	0	0	0	-	0	0	5	2	0	-	7	17
% Trucks	30.8	-	0.0	-	-	20.5	4.2	0.9	0.0	-	-	1.5	-	-	0.0	-	-	0.0	0.0	3.3	20.0	-	-	4.3	5.0
Bicycles on Road	0	0	0	0	-	0	0	2	0	0	-	2	0	0	0	0	-	0	0	2	0	0	-	2	4
% Bicycles on Road	0.0	-	0.0	-	-	0.0	0.0	1.8	0.0	-	-	1.5	-	-	0.0	-	-	0.0	0.0	1.3	0.0	-	-	1.2	1.2
Bicycles on Crosswalk	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	-	-
% Bicycles on Crosswalk	-	-	-	-	0.0	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Pedestrians	-	-	-	-	1	-	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	-	-
% Pedestrians	-	-	-	-	100.0	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-





Turning Movement Peak Hour Data Plot (8:00 AM)



## Turning Movement Peak Hour Data (4:45 PM)

								1 GIT	mig n	101011		oun	ioui	Duiu	( 1. 10	,									
			Driv	eway					Wenor	nah Ave					Drive	eway					Wenon	ah Ave			
			South	bound					West	bound					North	bound					East	ound			
Start Time	Right	Thru	Left	U-Turn	Peds	App. Total	Right	Thru	Left	U-Turn	Peds	App. Total	Right	Thru	Left	U-Turn	Peds	App. Total	Right	Thru	Left	U-Turn	Peds	App. Total	Int. Total
4:45 PM	18	0	12	0	0	30	11	22	2	0	0	35	0	0	0	0	0	0	2	87	7	0	0	96	161
5:00 PM	20	0	18	0	1	38	13	14	2	0	0	29	0	0	1	0	0	1	2	78	8	0	1	88	156
5:15 PM	24	0	24	0	0	48	8	22	0	0	0	30	1	0	1	0	0	2	0	113	5	0	0	118	198
5:30 PM	15	0	21	0	0	36	10	24	0	0	0	34	1	0	1	0	0	2	0	83	8	1	0	92	164
Total	77	0	75	0	1	152	42	82	4	0	0	128	2	0	3	0	0	5	4	361	28	1	1	394	679
Approach %	50.7	0.0	49.3	0.0	-	-	32.8	64.1	3.1	0.0	-	-	40.0	0.0	60.0	0.0	-	-	1.0	91.6	7.1	0.3	-	-	-
Total %	11.3	0.0	11.0	0.0	-	22.4	6.2	12.1	0.6	0.0	-	18.9	0.3	0.0	0.4	0.0	-	0.7	0.6	53.2	4.1	0.1	-	58.0	-
PHF	0.802	0.000	0.781	0.000	-	0.792	0.808	0.854	0.500	0.000	-	0.914	0.500	0.000	0.750	0.000	-	0.625	0.500	0.799	0.875	0.250	-	0.835	0.857
Lights	77	0	74	0	-	151	40	77	4	0	-	121	2	0	3	0	-	5	4	359	27	1	-	391	668
% Lights	100.0	-	98.7	-	-	99.3	95.2	93.9	100.0	-	-	94.5	100.0	-	100.0	-	-	100.0	100.0	99.4	96.4	100.0	-	99.2	98.4
Buses	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0
% Buses	0.0	-	0.0	-	-	0.0	0.0	0.0	0.0	-	-	0.0	0.0	-	0.0	-	-	0.0	0.0	0.0	0.0	0.0	-	0.0	0.0
Trucks	0	0	1	0	-	1	2	4	0	0	-	6	0	0	0	0	-	0	0	1	1	0	-	2	9
% Trucks	0.0	-	1.3	-	-	0.7	4.8	4.9	0.0	-	-	4.7	0.0	-	0.0	-	-	0.0	0.0	0.3	3.6	0.0	-	0.5	1.3
Bicycles on Road	0	0	0	0	-	0	0	1	0	0	-	1	0	0	0	0	-	0	0	1	0	0	-	1	2
% Bicycles on Road	0.0	-	0.0	-	-	0.0	0.0	1.2	0.0	-	-	0.8	0.0	_	0.0	-	-	0.0	0.0	0.3	0.0	0.0	-	0.3	0.3
Bicycles on Crosswalk	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	-	-
% Bicycles on Crosswalk	-	-	-	-	0.0	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	0.0	-	-
Pedestrians	-	-	-	-	1	-	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	1	-	-
% Pedestrians	-	-	-	-	100.0	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	100.0	-	-





Turning Movement Peak Hour Data Plot (4:45 PM)



## **Turning Movement Data**

										i uni	mig n	10,01		Juiu											
			4 Hi	ills Rd					Weno	nah Ave					4 H	lills Rd			-		Weno	nah Ave			
Ote at Time a			South	nbound					Wes	tbound					Nort	hbound			ļ		East	tbound			
Start Time	Right	Thru	Left	U-Turn	Peds	App. Total	Right	Thru	Left	U-Turn	Peds	App. Total	Right	Thru	Left	U-Turn	Peds	App. Total	Right	Thru	Left	U-Turn	Peds	App. Total	Int. Total
6:00 AM	1	2	0	0	0	3	1	0	0	0	0	1	0	23	6	0	0	29	2	1	0	0	0	3	36
6:15 AM	3	2	0	0	0	5	3	2	0	0	0	5	0	35	8	0	1	43	9	0	0	1	0	10	63
6:30 AM	2	5	0	0	0	7	1	3	0	0	0	4	0	39	15	0	0	54	15	1	1	0	1	17	82
6:45 AM	2	4	1	0	0	7	3	2	0	0	0	5	0	63	18	0	0	81	13	1	2	0	0	16	109
Hourly Total	8	13	1	0	0	22	8	7	0	0	0	15	0	160	47	0	1	207	39	3	3	1	1	46	290
7:00 AM	3	9	0	0	4	12	8	1	0	0	0	9	0	53	18	0	1	71	11	1	3	0	0	15	107
7:15 AM	5	16	0	0	0	21	5	3	0	0	0	8	0	75	18	0	0	93	21	0	3	0	0	24	146
7:30 AM	4	18	1	0	0	23	5	3	0	0	0	8	0	96	25	0	0	121	20	1	4	0	0	25	177
7:45 AM	2	16	0	0	0	18	8	1	2	0	0	11	0	75	26	0	0	101	29	3	3	0	0	35	165
Hourly Total	14	59	1	0	4	74	26	8	2	0	0	36	0	299	87	0	1	386	81	5	13	0	0	99	595
8:00 AM	4	17	0	0	0	21	1	3	0	0	0	4	0	68	27	0	0	95	32	3	2	0	0	37	157
8:15 AM	6	19	1	0	0	26	2	5	0	0	0	7	0	58	34	0	0	92	46	0	2	0	0	48	173
8:30 AM	4	16	0	0	0	20	4	3	0	0	0	7	0	76	24	0	0	100	30	1	0	0	0	31	158
8:45 AM	5	13	0	0	0	18	2	4	0	0	0	6	1	81	19	0	0	101	49	1	0	0	0	50	175
Hourly Total	19	65	1	0	0	85	9	15	0	0	0	24	1	283	104	0	0	388	157	5	4	0	0	166	663
*** BREAK ***	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
11:00 AM	2	25	0	0	0	27	1	0	0	0	1	1	0	56	30	0	0	86	48	1	4	0	0	53	167
11:15 AM	8	35	0	0	1	43	3	1	0	0	0	4	0	59	27	0	0	86	49	3	8	0	0	60	193
11:30 AM	9	29	2	0	0	40	0	1	0	0	0	1	0	50	18	0	0	68	50	3	10	0	0	63	172
11:45 AM	3	19	2	0	0	24	3	1	0	0	0	4	3	47	17	0	0	67	47	3	8	0	0	58	153
Hourly Total	22	108	4	0	1	134	7	3	0	0	1	10	3	212	92	0	0	307	194	10	30	0	0	234	685
12:00 PM	7	24	0	0	0	31	1	2	0	0	0	3	1	77	40	0	0	118	50	1	8	1	0	60	212
12:15 PM	6	31	1	1	0	39	2	1	0	0	0	3	2	48	29	0	0	79	60	1	9	0	0	70	191
12:30 PM	8	32	1	0	1	41	3	0	0	0	0	3	0	56	29	0	0	85	44	4	12	0	0	60	189
12:45 PM	9	34	3	0	0	46	4	1	0	0	0	5	0	56	35	0	0	91	62	2	5	0	0	69	211
Hourly Total	30	121	5	1	1	157	10	4	0	0	0	14	3	237	133	0	0	373	216	8	34	1	0	259	803
1:00 PM	9	36	1	0	0	46	2	2	0	0	0	4	0	53	20	0	0	73	54	4	7	0	0	65	188
1:15 PM	8	20	2	0	0	30	3	1	0	0	0	4	0	43	24	0	0	67	40	3	10	0	0	53	154
1:30 PM	5	19	2	0	0	26	2	1	0	0	0	3	0	41	22	0	1	63	52	8	14	0	0	74	166
1:45 PM	3	26	3	0	0	32	1	2	0	0	0	3	0	46	19	0	0	65	53	1	8	0	0	62	162
Hourly Total	25	101	8	0	0	134	8	6	0	0	0	14	0	183	85	0	1	268	199	16	39	0	0	254	670
*** BREAK ***	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
3:00 PM	5	29	3	0	1	37	0	1	0	0	0	1	0	62	30	0	0	92	67	2	8	0	0	77	207
3:15 PM	4	32	3	1	1	40	2	0	0	0	0	2	0	44	24	0	0	68	63	1	15	0	0	79	189
3:30 PM	0	31	3	0	0	34	1	1	0	0	1	2	1	39	23	0	0	63	52	2	16	0	0	70	169

3:45 PM	8	39	3	0	0	50	3	3	0	0	0	6	0	56	20	0	0	76	77	0	10	0	0	87	219
Hourly Total	17	131	12	1	2	161	6	5	0	0	1	11	1	201	97	0	0	299	259	5	49	0	0	313	784
4:00 PM	6	33	1	0	0	40	3	1	0	0	0	4	1	45	24	0	0	70	76	3	8	0	0	87	201
4:15 PM	7	38	3	1	0	49	2	2	0	0	0	4	1	44	30	0	0	75	80	7	12	0	0	99	227
4:30 PM	6	35	3	0	0	44	5	0	1	0	2	6	0	57	22	0	2	79	77	8	13	0	0	98	227
4:45 PM	9	46	2	0	0	57	4	3	0	0	0	7	0	45	23	0	0	68	89	6	15	0	0	110	242
Hourly Total	28	152	9	1	0	190	14	6	1	0	2	21	2	191	99	0	2	292	322	24	48	0	0	394	897
5:00 PM	7	43	3	0	0	53	0	3	0	0	0	3	0	58	20	0	0	78	66	5	12	0	0	83	217
5:15 PM	9	42	8	0	0	59	3	2	0	0	0	5	1	44	16	0	0	61	112	9	23	0	0	144	269
5:30 PM	8	44	6	0	0	58	5	2	0	0	0	7	0	65	24	1	0	90	77	7	27	0	0	111	266
5:45 PM	4	40	4	0	0	48	5	2	0	0	0	7	0	49	24	0	0	73	67	9	7	0	0	83	211
Hourly Total	28	169	21	0	0	218	13	9	0	0	0	22	1	216	84	1	0	302	322	30	69	0	0	421	963
Grand Total	191	919	62	3	8	1175	101	63	3	0	4	167	11	1982	828	1	5	2822	1789	106	289	2	1	2186	6350
Approach %	16.3	78.2	5.3	0.3	-	-	60.5	37.7	1.8	0.0	-	-	0.4	70.2	29.3	0.0	-	-	81.8	4.8	13.2	0.1	-	-	-
Total %	3.0	14.5	1.0	0.0	-	18.5	1.6	1.0	0.0	0.0	-	2.6	0.2	31.2	13.0	0.0	-	44.4	28.2	1.7	4.6	0.0	-	34.4	-
Lights	184	903	61	3	-	1151	101	61	3	0	-	165	7	1969	815	1	-	2792	1775	101	287	2	-	2165	6273
% Lights	96.3	98.3	98.4	100.0	-	98.0	100.0	96.8	100.0	-	-	98.8	63.6	99.3	98.4	100.0	-	98.9	99.2	95.3	99.3	100.0	-	99.0	98.8
Buses	0	0	0	0	-	0	0	0	0	0	-	0	0	2	1	0	-	3	1	0	0	0	-	1	4
% Buses	0.0	0.0	0.0	0.0	-	0.0	0.0	0.0	0.0	-	-	0.0	0.0	0.1	0.1	0.0	-	0.1	0.1	0.0	0.0	0.0	-	0.0	0.1
Trucks	3	16	1	0	-	20	0	1	0	0	-	1	2	10	10	0	-	22	13	0	2	0	-	15	58
% Trucks	1.6	1.7	1.6	0.0	-	1.7	0.0	1.6	0.0	-	-	0.6	18.2	0.5	1.2	0.0	-	0.8	0.7	0.0	0.7	0.0	-	0.7	0.9
Bicycles on Road	4	0	0	0	-	4	0	1	0	0	-	1	2	1	2	0	-	5	0	5	0	0	-	5	15
% Bicycles on Road	2.1	0.0	0.0	0.0	-	0.3	0.0	1.6	0.0	-	-	0.6	18.2	0.1	0.2	0.0	-	0.2	0.0	4.7	0.0	0.0	-	0.2	0.2
Bicycles on Crosswalk	-	-	-	-	1	-	-	-	-	-	1	-	-	-	-	-	0	-	-	-	-	-	0	-	-
% Bicycles on Crosswalk	-	-	-	-	12.5	-	-	-	-	-	25.0	-	-	-	-	-	0.0	-	-	-	-	-	0.0	-	-
Pedestrians	-	-	-	-	7	-	-	-	-	-	3	-	-	-	-	-	5	-	-		-	-	1	-	-
% Pedestrians	-	-	-	-	87.5	-	-	-	-	-	75.0	-	-	-	-	-	100.0	-	-	-	-	-	100.0	-	-





Turning Movement Data Plot



## Turning Movement Peak Hour Data (7:30 AM)

			4 Hi	lls Rd					Wenor	nah Ave					4 Hil	lls Rd					Wenon	ah Ave			
_			South	nbound					West	bound					North	bound					East	bound			
Start Time	Right	Thru	Left	U-Turn	Peds	App. Total	Right	Thru	Left	U-Turn	Peds	App. Total	Right	Thru	Left	U-Turn	Peds	App. Total	Right	Thru	Left	U-Turn	Peds	App. Total	Int. Total
7:30 AM	4	18	1	0	0	23	5	3	0	0	0	8	0	96	25	0	0	121	20	1	4	0	0	25	177
7:45 AM	2	16	0	0	0	18	8	1	2	0	0	11	0	75	26	0	0	101	29	3	3	0	0	35	165
8:00 AM	4	17	0	0	0	21	1	3	0	0	0	4	0	68	27	0	0	95	32	3	2	0	0	37	157
8:15 AM	6	19	1	0	0	26	2	5	0	0	0	7	0	58	34	0	0	92	46	0	2	0	0	48	173
Total	16	70	2	0	0	88	16	12	2	0	0	30	0	297	112	0	0	409	127	7	11	0	0	145	672
Approach %	18.2	79.5	2.3	0.0	-	-	53.3	40.0	6.7	0.0	-	-	0.0	72.6	27.4	0.0	-	-	87.6	4.8	7.6	0.0	-	-	-
Total %	2.4	10.4	0.3	0.0	-	13.1	2.4	1.8	0.3	0.0	-	4.5	0.0	44.2	16.7	0.0	-	60.9	18.9	1.0	1.6	0.0	-	21.6	-
PHF	0.667	0.921	0.500	0.000	-	0.846	0.500	0.600	0.250	0.000	-	0.682	0.000	0.773	0.824	0.000	-	0.845	0.690	0.583	0.688	0.000	-	0.755	0.949
Lights	16	66	2	0	-	84	16	12	2	0	-	30	0	296	112	0	-	408	123	7	11	0	-	141	663
% Lights	100.0	94.3	100.0	-	-	95.5	100.0	100.0	100.0	-	-	100.0	-	99.7	100.0	-	-	99.8	96.9	100.0	100.0	-	-	97.2	98.7
Buses	0	0	0	0	-	0	0	0	0	0	-	0	0	1	0	0	-	1	1	0	0	0	-	1	2
% Buses	0.0	0.0	0.0	-	-	0.0	0.0	0.0	0.0	-	-	0.0	-	0.3	0.0	-	-	0.2	0.8	0.0	0.0	-	-	0.7	0.3
Trucks	0	4	0	0	-	4	0	0	0	0	-	0	0	0	0	0	-	0	3	0	0	0	-	3	7
% Trucks	0.0	5.7	0.0	-	-	4.5	0.0	0.0	0.0	-	-	0.0	-	0.0	0.0	-	-	0.0	2.4	0.0	0.0	-	-	2.1	1.0
Bicycles on Road	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0
% Bicycles on Road	0.0	0.0	0.0	-	-	0.0	0.0	0.0	0.0	-	-	0.0	-	0.0	0.0	-	-	0.0	0.0	0.0	0.0	-	-	0.0	0.0
Bicycles on Crosswalk	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	-	-
% Bicycles on Crosswalk	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Pedestrians	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	-	
% Pedestrians	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-





Turning Movement Peak Hour Data Plot (7:30 AM)



## Turning Movement Peak Hour Data (4:45 PM)

			4 Hil	lls Rd					Wenor	nah Ave					4 Hil	ls Rd					Wenon	ah Ave			
Start Timo			South	ibound					West	bound					North	bound					East	ound			
Start Time	Right	Thru	Left	U-Turn	Peds	App. Total	Right	Thru	Left	U-Turn	Peds	App. Total	Right	Thru	Left	U-Turn	Peds	App. Total	Right	Thru	Left	U-Turn	Peds	App. Total	Int. Total
4:45 PM	9	46	2	0	0	57	4	3	0	0	0	7	0	45	23	0	0	68	89	6	15	0	0	110	242
5:00 PM	7	43	3	0	0	53	0	3	0	0	0	3	0	58	20	0	0	78	66	5	12	0	0	83	217
5:15 PM	9	42	8	0	0	59	3	2	0	0	0	5	1	44	16	0	0	61	112	9	23	0	0	144	269
5:30 PM	8	44	6	0	0	58	5	2	0	0	0	7	0	65	24	1	0	90	77	7	27	0	0	111	266
Total	33	175	19	0	0	227	12	10	0	0	0	22	1	212	83	1	0	297	344	27	77	0	0	448	994
Approach %	14.5	77.1	8.4	0.0	-	-	54.5	45.5	0.0	0.0	-	-	0.3	71.4	27.9	0.3	-	-	76.8	6.0	17.2	0.0	-	-	-
Total %	3.3	17.6	1.9	0.0	-	22.8	1.2	1.0	0.0	0.0	-	2.2	0.1	21.3	8.4	0.1	-	29.9	34.6	2.7	7.7	0.0	-	45.1	-
PHF	0.917	0.951	0.594	0.000	-	0.962	0.600	0.833	0.000	0.000	-	0.786	0.250	0.815	0.865	0.250	-	0.825	0.768	0.750	0.713	0.000	-	0.778	0.924
Lights	31	174	19	0	-	224	12	9	0	0	-	21	1	212	79	1	-	293	344	27	76	0	-	447	985
% Lights	93.9	99.4	100.0	-	-	98.7	100.0	90.0	-	-	-	95.5	100.0	100.0	95.2	100.0	-	98.7	100.0	100.0	98.7	-	-	99.8	99.1
Buses	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0
% Buses	0.0	0.0	0.0	-	-	0.0	0.0	0.0	-	-	-	0.0	0.0	0.0	0.0	0.0	-	0.0	0.0	0.0	0.0	-	-	0.0	0.0
Trucks	2	1	0	0	-	3	0	1	0	0	-	1	0	0	3	0	-	3	0	0	1	0	-	1	8
% Trucks	6.1	0.6	0.0	-	-	1.3	0.0	10.0	-	-	-	4.5	0.0	0.0	3.6	0.0	-	1.0	0.0	0.0	1.3	-	-	0.2	0.8
Bicycles on Road	0	0	0	0	-	0	0	0	0	0	-	0	0	0	1	0	-	1	0	0	0	0	-	0	1
% Bicycles on Road	0.0	0.0	0.0	-	-	0.0	0.0	0.0	-	-	-	0.0	0.0	0.0	1.2	0.0	-	0.3	0.0	0.0	0.0	-	-	0.0	0.1
Bicycles on Crosswalk	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	-	-
% Bicycles on Crosswalk	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Pedestrians	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0		
% Pedestrians	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-





Turning Movement Peak Hour Data Plot (4:45 PM)



## **Turning Movement Data**

										1 011	mig r	10101	i on r	Juiu											
			Trai South	mway nbound					Driv Wes	veway stbound	-				Tra Norti	imway hbound					Driv East	/eway bound			
Start Time	Right	Thru	Left	U-Turn	Peds	App. Total	Right	Thru	Left	U-Turn	Peds	App. Total	Right	Thru	Left	U-Turn	Peds	App. Total	Right	Thru	Left	U-Turn	Peds	App. Total	Int. Total
6:00 AM	7	12	8	2	0	29	6	2	0	0	0	8	2	17	0	0	0	19	0	0	0	0	2	0	56
6:15 AM	4	15	9	2	0	30	12	0	0	0	3	12	3	22	0	0	3	25	0	0	0	0	1	0	67
6:30 AM	2	21	12	1	1	36	7	2	1	0	1	10	1	35	2	0	0	38	0	0	1	0	1	1	85
6:45 AM	3	21	11	3	0	38	9	1	1	0	0	11	1	29	3	0	0	33	1	0	1	0	5	2	84
Hourly Total	16	69	40	8	1	133	34	5	2	0	4	41	7	103	5	0	3	115	1	0	2	0	9	3	292
7:00 AM	6	31	15	3	0	55	10	1	0	0	0	11	0	32	3	0	1	35	1	1	1	0	3	3	104
7:15 AM	5	31	17	4	0	57	18	1	3	0	1	22	6	35	3	0	. 1	44	0	0	2	0	1	2	125
7:30 AM	3	42	17	4	0	66	11	0	2	0	0	13	2	55	4	0	2	61	3	0	4	0	2	7	147
7:45 AM	2	41	19	1	0	63	17	1	3	0	1	21	3	44	2	0	7	49	1	0	0	0	8	1	134
Hourly Total	16	145	68	12	0	241	56	3	8	0	2	67	11	166	12	0	11	189	5	1	7	0	14	13	510
8:00 AM	3	43	17	3	0	66	10	3	0	0	1	13	2	38	5	0	1	45	2	0	1	0	1	3	127
8:15 AM	9	55	28	1	1	93	19	0	1	0	2	20	7	63	1	0	5	71	0	0	3	0	2	3	187
8:30 AM	7	47	26	0	0	80	24	1	1	0	0	26	6	52	2	0	6	60	0	0	4	0	3	4	170
8:45 AM	11	57	21	4	0	93	21	3	2	0	0	26	4	51	1	0	4	56	1	2	2	0	3	5	180
Hourly Total	30	202	92	8	1	332	74	7	4	0	3	85	19	204	9	0	16	232	3	2	10	0	9	15	664
*** BREAK ***	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
11:00 AM	13	67	43	0	0	123	20	4	1	0	1	25	4	58	1	0	2	63	3	1	4	0	10	8	219
11:15 AM	19	62	35	1	0	117	27	1	1	0	0	29	10	49	1	0	6	60	4	1	5	0	14	10	216
11:30 AM	8	58	38	3	0	107	34	3	4	0	0	41	7	62	2	0	3	71	2	0	5	0	0	7	226
11:45 AM	8	56	38	2	0	104	43	1	1	0	3	45	13	32	2	0	4	47	2	2	4	1	1	9	205
Hourly Total	48	243	154	6	0	451	124	9	7	0	4	140	34	201	6	0	15	241	11	4	18	1	25	34	866
12:00 PM	13	68	40	1	1	122	36	2	0	0	1	38	5	72	3	0	5	80	1	0	1	0	1	2	242
12:15 PM	12	69	38	0	0	119	28	5	4	0	1	37	4	56	1	0	4	61	4	4	5	0	3	13	230
12:30 PM	15	76	40	2	1	133	38	4	1	0	1	43	4	69	0	0	1	73	5	1	3	0	0	9	258
12:45 PM	15	77	36	1	0	129	32	0	2	0	1	34	5	61	0	0	1	66	2	1	9	0	2	12	241
Hourly Total	55	290	154	4	2	503	134	11	7	0	4	152	18	258	4	0	11	280	12	6	18	0	6	36	971
1:00 PM	16	70	51	3	0	140	31	6	3	0	0	40	2	44	4	0	0	50	5	4	6	0	2	15	245
1:15 PM	11	54	40	2	0	107	31	4	2	0	0	37	7	52	0	0	. 1	59	4	1	3	0	3	8	211
1:30 PM	15	74	48	3	0	140	42	0	0	0	1	42	2	57	3	0	3	62	4	2	4	0	1	10	254
1:45 PM	15	73	31	1	0	120	30	3	4	0	2	37	9	52	1	0	6	62	3	0	5	0	2	8	227
Hourly Total	57	271	170	9	0	507	134	13	9	0	3	156	20	205	8	0	10	233	16	7	18	0	8	41	937
*** BREAK ***	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
3:00 PM	16	78	59	4	1	157	33	1	0	0	0	34	4	59	4	0	0	67	4	1	4	0	2	9	267
3:15 PM	15	94	52	5	0	166	52	3	1	0	0	56	7	40	3	0	1	50	4	0	5	0	7	9	281
3:30 PM	15	60	47	6	0	128	41	1	1	0	0	43	2	63	2	0	1	67	7	2	2	0	5	11	249
3:45 PM	15	88	47	2	1	152	44	4	2	0	2	50	8	48	1	0	5	57	4	2	4	0	1	10	269
----------------------------	------	------	------	------	-------	------	------	-------	------	-----	------	------	------	------	------	-----	------	------	------	-------	------	-------	------	------	------
Hourly Total	61	320	205	17	2	603	170	9	4	0	2	183	21	210	10	0	7	241	19	5	15	0	15	39	1066
4:00 PM	14	83	53	3	0	153	49	0	1	0	0	50	8	57	1	0	14	66	6	3	4	0	4	13	282
4:15 PM	19	105	52	2	0	178	39	2	0	0	1	41	8	68	2	0	8	78	4	0	4	0	3	8	305
4:30 PM	10	118	56	4	0	188	39	1	2	0	1	42	3	51	2	0	3	56	6	1	3	0	2	10	296
4:45 PM	17	112	63	3	0	195	39	0	0	0	1	39	8	56	2	0	2	66	3	2	1	0	7	6	306
Hourly Total	60	418	224	12	0	714	166	3	3	0	3	172	27	232	7	0	27	266	19	6	12	0	16	37	1189
5:00 PM	15	96	54	1	0	166	58	2	3	0	1	63	9	63	3	0	2	75	4	2	6	0	2	12	316
5:15 PM	13	132	59	3	0	207	47	1	1	0	0	49	9	60	3	0	3	72	10	2	6	0	5	18	346
5:30 PM	14	105	58	5	0	182	47	1	4	0	0	52	1	63	2	0	4	66	5	1	0	0	6	6	306
5:45 PM	9	84	56	2	1	151	52	2	0	0	1	54	6	45	1	0	4	52	7	2	4	0	6	13	270
Hourly Total	51	417	227	11	1	706	204	6	8	0	2	218	25	231	9	0	13	265	26	7	16	0	19	49	1238
Grand Total	394	2375	1334	87	7	4190	1096	66	52	0	27	1214	182	1810	70	0	113	2062	112	38	116	1	121	267	7733
Approach %	9.4	56.7	31.8	2.1	-	-	90.3	5.4	4.3	0.0	-	-	8.8	87.8	3.4	0.0	-	-	41.9	14.2	43.4	0.4	-	-	-
Total %	5.1	30.7	17.3	1.1	-	54.2	14.2	0.9	0.7	0.0	-	15.7	2.4	23.4	0.9	0.0	-	26.7	1.4	0.5	1.5	0.0	-	3.5	-
Lights	389	2263	1321	85	-	4058	1088	66	46	0	-	1200	181	1748	68	0	-	1997	111	38	111	1	-	261	7516
% Lights	98.7	95.3	99.0	97.7	-	96.8	99.3	100.0	88.5	-	-	98.8	99.5	96.6	97.1	-	-	96.8	99.1	100.0	95.7	100.0	-	97.8	97.2
Buses	1	65	0	0	-	66	2	0	0	0	-	2	0	1	0	0	-	1	0	0	0	0	-	0	69
% Buses	0.3	2.7	0.0	0.0	-	1.6	0.2	0.0	0.0	-	-	0.2	0.0	0.1	0.0	-	-	0.0	0.0	0.0	0.0	0.0	-	0.0	0.9
Trucks	2	31	11	1	-	45	6	0	0	0	-	6	1	38	2	0	-	41	1	0	5	0	-	6	98
% Trucks	0.5	1.3	0.8	1.1	-	1.1	0.5	0.0	0.0	-	-	0.5	0.5	2.1	2.9	-	-	2.0	0.9	0.0	4.3	0.0	-	2.2	1.3
Bicycles on Road	2	16	2	1	-	21	0	0	6	0	-	6	0	23	0	0	-	23	0	0	0	0	-	0	50
% Bicycles on Road	0.5	0.7	0.1	1.1	-	0.5	0.0	0.0	11.5	-	-	0.5	0.0	1.3	0.0	-	-	1.1	0.0	0.0	0.0	0.0	-	0.0	0.6
Bicycles on Crosswalk	-	-	-	-	0	-	-	-	-	-	9	-	-	-	-	-	4	-	-	-	-	-	1	-	-
% Bicycles on Crosswalk	-	-	-	-	0.0	-	-	-	-	-	33.3	-	-	-	-	-	3.5	-	-	-	-	-	0.8	-	-
Pedestrians	-	-	-	-	7	-	-	-	-	-	18	-	-	-	-	-	109	-	-	-	-	-	120	-	-
% Pedestrians	-	-	-	-	100.0	-	-	-	-	-	66.7	-	-	-	-	-	96.5	-	-	-	-	-	99.2	-	-





Turning Movement Data Plot



# Turning Movement Peak Hour Data (8:00 AM)

			Trar	mway					Driv	eway					Tran	nway					Drive	eway			
Ctart Time			South	bound					West	bound					North	bound					East	bound			
Start Time	Right	Thru	Left	U-Turn	Peds	App. Total	Right	Thru	Left	U-Turn	Peds	App. Total	Right	Thru	Left	U-Turn	Peds	App. Total	Right	Thru	Left	U-Turn	Peds	App. Total	Int. Total
8:00 AM	3	43	17	3	0	66	10	3	0	0	1	13	2	38	5	0	1	45	2	0	1	0	1	3	127
8:15 AM	9	55	28	1	1	93	19	0	1	0	2	20	7	63	1	0	5	71	0	0	3	0	2	3	187
8:30 AM	7	47	26	0	0	80	24	1	1	0	0	26	6	52	2	0	6	60	0	0	4	0	3	4	170
8:45 AM	11	57	21	4	0	93	21	3	2	0	0	26	4	51	1	0	4	56	1	2	2	0	3	5	180
Total	30	202	92	8	1	332	74	7	4	0	3	85	19	204	9	0	16	232	3	2	10	0	9	15	664
Approach %	9.0	60.8	27.7	2.4	-	-	87.1	8.2	4.7	0.0	-	-	8.2	87.9	3.9	0.0	-	-	20.0	13.3	66.7	0.0	-	-	-
Total %	4.5	30.4	13.9	1.2	-	50.0	11.1	1.1	0.6	0.0	-	12.8	2.9	30.7	1.4	0.0	-	34.9	0.5	0.3	1.5	0.0	-	2.3	-
PHF	0.682	0.886	0.821	0.500	-	0.892	0.771	0.583	0.500	0.000	-	0.817	0.679	0.810	0.450	0.000	-	0.817	0.375	0.250	0.625	0.000	-	0.750	0.888
Lights	30	187	90	8	-	315	72	7	4	0	-	83	19	191	9	0	-	219	3	2	10	0	-	15	632
% Lights	100.0	92.6	97.8	100.0	-	94.9	97.3	100.0	100.0	-	-	97.6	100.0	93.6	100.0	-	-	94.4	100.0	100.0	100.0	-	-	100.0	95.2
Buses	0	6	0	0	-	6	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	6
% Buses	0.0	3.0	0.0	0.0	-	1.8	0.0	0.0	0.0	-	-	0.0	0.0	0.0	0.0	-	-	0.0	0.0	0.0	0.0	-	-	0.0	0.9
Trucks	0	8	2	0	-	10	2	0	0	0	-	2	0	9	0	0	-	9	0	0	0	0	-	0	21
% Trucks	0.0	4.0	2.2	0.0	-	3.0	2.7	0.0	0.0	-	-	2.4	0.0	4.4	0.0	-	-	3.9	0.0	0.0	0.0	-	-	0.0	3.2
Bicycles on Road	0	1	0	0	-	1	0	0	0	0	-	0	0	4	0	0	-	4	0	0	0	0	-	0	5
% Bicycles on Road	0.0	0.5	0.0	0.0	-	0.3	0.0	0.0	0.0	-	-	0.0	0.0	2.0	0.0	-	-	1.7	0.0	0.0	0.0	-	-	0.0	0.8
Bicycles on Crosswalk	-	-	-	-	0	-	-	-	-	-	1	-	-	-	-	-	1	-	-	-	-	-	0	-	-
% Bicycles on Crosswalk	-	-	-	-	0.0	-	-	-	-	-	33.3	-	-	-	-	-	6.3	-	-	-	-	-	0.0	-	-
Pedestrians	-	-	-	-	1	-	-	-	-	-	2	-	-	-	-	-	15	-	-	-	-	-	9	-	
% Pedestrians	-	-	-	-	100.0	-	-	-	-	-	66.7	-	-	-	-	-	93.8	-	-	-	-	-	100.0	-	-





Turning Movement Peak Hour Data Plot (8:00 AM)



# Turning Movement Peak Hour Data (4:45 PM)

			Trar	nway					Driv	reway					Tran	nway					Drive	eway			
Start Time	Right	Thru	Left	U-Turn	Peds	App. Total	Right	Thru	Left	U-Turn	Peds	App. Total	Right	Thru	Left	U-Turn	Peds	App. Total	Right	Thru	Left	U-Turn	Peds	App. Total	Int. Total
4:45 PM	17	112	63	3	0	195	39	0	0	0	1	39	8	56	2	0	2	66	3	2	1	0	7	6	306
5:00 PM	15	96	54	1	0	166	58	2	3	0	1	63	9	63	3	0	2	75	4	2	6	0	2	12	316
5:15 PM	13	132	59	3	0	207	47	1	1	0	0	49	9	60	3	0	3	72	10	2	6	0	5	18	346
5:30 PM	14	105	58	5	0	182	47	1	4	0	0	52	1	63	2	0	4	66	5	1	0	0	6	6	306
Total	59	445	234	12	0	750	191	4	8	0	2	203	27	242	10	0	11	279	22	7	13	0	20	42	1274
Approach %	7.9	59.3	31.2	1.6	-	-	94.1	2.0	3.9	0.0	-	-	9.7	86.7	3.6	0.0	-	-	52.4	16.7	31.0	0.0	-	-	-
Total %	4.6	34.9	18.4	0.9	-	58.9	15.0	0.3	0.6	0.0	-	15.9	2.1	19.0	0.8	0.0	-	21.9	1.7	0.5	1.0	0.0	-	3.3	-
PHF	0.868	0.843	0.929	0.600	-	0.906	0.823	0.500	0.500	0.000	-	0.806	0.750	0.960	0.833	0.000	-	0.930	0.550	0.875	0.542	0.000	-	0.583	0.921
Lights	58	435	233	12	-	738	190	4	8	0	-	202	27	233	9	0	-	269	22	7	13	0	-	42	1251
% Lights	98.3	97.8	99.6	100.0	-	98.4	99.5	100.0	100.0	-	-	99.5	100.0	96.3	90.0	-	-	96.4	100.0	100.0	100.0	-	-	100.0	98.2
Buses	0	8	0	0	-	8	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	8
% Buses	0.0	1.8	0.0	0.0	-	1.1	0.0	0.0	0.0	-	-	0.0	0.0	0.0	0.0	-	-	0.0	0.0	0.0	0.0	-	-	0.0	0.6
Trucks	0	1	0	0	-	1	1	0	0	0	-	1	0	2	1	0	-	3	0	0	0	0	-	0	5
% Trucks	0.0	0.2	0.0	0.0	-	0.1	0.5	0.0	0.0	-	-	0.5	0.0	0.8	10.0	-	-	1.1	0.0	0.0	0.0	-	-	0.0	0.4
Bicycles on Road	1	1	1	0	-	3	0	0	0	0	-	0	0	7	0	0	-	7	0	0	0	0	-	0	10
% Bicycles on Road	1.7	0.2	0.4	0.0	-	0.4	0.0	0.0	0.0	-	-	0.0	0.0	2.9	0.0	-	-	2.5	0.0	0.0	0.0	-	-	0.0	0.8
Bicycles on Crosswalk	-	-	-	-	0	-	-	-	-	-	1	-	-	-	-	-	0	-	-	-	-	-	0	-	-
% Bicycles on Crosswalk	-	-	-	-	-	-	-	-	-	-	50.0	-	-	-	-	-	0.0	-	-	-	-	-	0.0	-	-
Pedestrians	-	-	-	-	0	-	-	-	-	-	1	-	-	-	-	-	11	-	-	-	-	-	20	-	-
% Pedestrians	-	-	_	-	-	-	-	-	-		50.0		-	-	-	-	100.0	-	-	-	-	-	100.0	-	-





Turning Movement Peak Hour Data Plot (4:45 PM)

**APPENDIX C:** 

HIGHWAY CAPACITY SOFTWARE ANALYSIS

	HCS Two-Way Stop	-Control Report	
General Information		Site Information	
Analyst	PD	Intersection	Wenonah Ave & 4 Hills Ave
Agency/Co.	Lee Engineering	Jurisdiction	CABQ
Date Performed	6/20/2023	East/West Street	Wenonah Ave
Analysis Year	2023	North/South Street	4 Hills Rd
Time Analyzed		Peak Hour Factor	0.95
Intersection Orientation	North-South	Analysis Time Period (hrs)	0.25
Project Description	Existing 2023 AM		
Lanes			



Vehicle Volumes and Adju	istme	nts														
Approach		Eastb	ound			West	oound			North	bound			South	bound	
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R
Priority		10	11	12		7	8	9	1U	1	2	3	4U	4	5	6
Number of Lanes		1	1	1		1	1	0	0	1	2	0	0	1	2	0
Configuration		L	Т	R		L		TR		L	Т	TR		L	Т	TR
Volume (veh/h)		11	7	127		2	12	16	0	112	297	0	0	2	70	16
Percent Heavy Vehicles (%)		3	3	3		3	3	3	3	3			3	3		
Proportion Time Blocked																
Percent Grade (%)		(	C			(	C									
Right Turn Channelized		N	lo													
Median Type   Storage				Left	Only								1			
Critical and Follow-up He	adwa	ys														
Base Critical Headway (sec)		7.5	6.5	6.9		7.5	6.5	6.9		4.1				4.1		
Critical Headway (sec)		7.56	6.56	6.96		7.56	6.56	6.96		4.16				4.16		
Base Follow-Up Headway (sec)		3.5	4.0	3.3		3.5	4.0	3.3		2.2				2.2		
Follow-Up Headway (sec)		3.53	4.03	3.33		3.53	4.03	3.33		2.23				2.23		
Delay, Queue Length, and	Leve	l of Se	ervice													
Flow Rate, v (veh/h)		12	7	134		2		29		118				2		
Capacity, c (veh/h)		453	361	1011		371		536		1495				1237		
v/c Ratio		0.03	0.02	0.13		0.01		0.06		0.08				0.00		
95% Queue Length, Q <sub>95</sub> (veh)		0.1	0.1	0.5		0.0		0.2		0.3				0.0		
Control Delay (s/veh)		13.2	15.2	9.1		14.8		12.1		7.6				7.9		
Level of Service (LOS)		В	С	А		В		В		А				А		
Approach Delay (s/veh)		9	.7			12	2.3			2	.1			0	.2	
Approach LOS		ļ	4			I	3			ļ	Ą			ļ	Ą	

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	HCS Two-Way Stop	-Control Report	
General Information		Site Information	
Analyst	PD	Intersection	Wenonah Ave & 4 Hills Ave
Agency/Co.	Lee Engineering	Jurisdiction	CABQ
Date Performed	6/20/2023	East/West Street	Wenonah Ave
Analysis Year	2023	North/South Street	4 Hills Rd
Time Analyzed		Peak Hour Factor	0.92
Intersection Orientation	North-South	Analysis Time Period (hrs)	0.25
Project Description	Existing 2023 AM		
Lanes			



### **Vehicle Volumes and Adjustments** Approach Eastbound Westbound Northbound Southbound U U U L т R L т R U L Т R L R Movement Т 7 Priority 10 11 12 8 9 1U 1 2 3 4U 4 5 6 2 Number of Lanes 1 1 1 1 1 0 0 1 0 0 1 2 0 Configuration Т R L TR L Т TR L Т TR L Volume (veh/h) 344 0 12 83 212 175 77 27 10 0 1 0 19 33 3 Percent Heavy Vehicles (%) 3 3 3 3 3 3 3 3 3 **Proportion Time Blocked** 0 0 Percent Grade (%) **Right Turn Channelized** No Median Type | Storage Left Only 1 Critical and Follow-up Headways Base Critical Headway (sec) 7.5 6.5 6.9 7.5 6.5 6.9 4.1 4.1 Critical Headway (sec) 7.56 6.56 6.96 7.56 6.56 6.96 4.16 4.16 3.3 3.5 4.0 3.3 3.5 4.0 2.2 2.2 Base Follow-Up Headway (sec) Follow-Up Headway (sec) 3.53 4.03 3.33 3.53 4.03 3.33 2.23 2.23 Delay, Queue Length, and Level of Service Flow Rate, v (veh/h) 29 374 24 90 21 84 0 Capacity, c (veh/h) 456 348 915 274 516 1332 1326 v/c Ratio 0.18 0.08 0.41 0.00 0.05 0.07 0.02 0.7 0.3 2.0 0.0 0.1 0.2 0.0 95% Queue Length, Q<sub>95</sub> (veh) Control Delay (s/veh) 14.7 16.3 18.1 12.3 7.9 11.6 7.8 С С Level of Service (LOS) В В В А А Approach Delay (s/veh) 12.4 12.3 2.2 0.6 Approach LOS В В А А

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	HCS Two-Way Stop	-Control Report	
General Information		Site Information	
Analyst	PD	Intersection	Wenonah Ave & 4 Hills Ave
Agency/Co.	Lee Engineering	Jurisdiction	CABQ
Date Performed	6/20/2023	East/West Street	Wenonah Ave
Analysis Year	2023	North/South Street	4 Hills Rd
Time Analyzed		Peak Hour Factor	0.95
Intersection Orientation	North-South	Analysis Time Period (hrs)	0.25
Project Description	Background 2025 AM		
Lanes			



Vehicle Volumes and Adju	istme	nts														
Approach		Eastb	ound			West	oound			North	oound			South	bound	
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R
Priority		10	11	12		7	8	9	1U	1	2	3	4U	4	5	6
Number of Lanes		1	1	1		1	1	0	0	1	2	0	0	1	2	0
Configuration		L	Т	R		L		TR		L	Т	TR		L	Т	TR
Volume (veh/h)		11	7	131		2	12	16	0	115	306	0	0	2	72	16
Percent Heavy Vehicles (%)		3	3	3		3	3	3	3	3			3	3		
Proportion Time Blocked																
Percent Grade (%)		(	)			(	C									
Right Turn Channelized		N	0													
Median Type   Storage				Left	Only								1			
Critical and Follow-up He	adwa	ys														
Base Critical Headway (sec)		7.5	6.5	6.9		7.5	6.5	6.9		4.1				4.1		
Critical Headway (sec)		7.56	6.56	6.96		7.56	6.56	6.96		4.16				4.16		
Base Follow-Up Headway (sec)		3.5	4.0	3.3		3.5	4.0	3.3		2.2				2.2		
Follow-Up Headway (sec)		3.53	4.03	3.33		3.53	4.03	3.33		2.23				2.23		
Delay, Queue Length, and	Leve	l of Se	ervice													
Flow Rate, v (veh/h)		12	7	138		2		29		121				2		
Capacity, c (veh/h)		444	352	1010		361		525		1492				1227		
v/c Ratio		0.03	0.02	0.14		0.01		0.06		0.08				0.00		
95% Queue Length, $Q_{95}$ (veh)		0.1	0.1	0.5		0.0		0.2		0.3				0.0		
Control Delay (s/veh)		13.3	15.5	9.1		15.0		12.3		7.6				7.9		
Level of Service (LOS)		В	С	А		С		В		А				А		
Approach Delay (s/veh)		9	.7			12	2.4			2	.1			0.	.2	
Approach LOS		ļ	4			I	3			A	4			A	4	

	HCS Two-Way Stop	-Control Report	
General Information		Site Information	
Analyst	PD	Intersection	Wenonah Ave & 4 Hills Ave
Agency/Co.	Lee Engineering	Jurisdiction	CABQ
Date Performed	6/20/2023	East/West Street	Wenonah Ave
Analysis Year	2023	North/South Street	4 Hills Rd
Time Analyzed		Peak Hour Factor	0.92
Intersection Orientation	North-South	Analysis Time Period (hrs)	0.25
Project Description	Background 2023 PM		
Lanes			



Vehicle Volumes and Adju	istme	nts														
Approach		Eastb	ound			West	oound			North	bound			South	bound	
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R
Priority		10	11	12		7	8	9	1U	1	2	3	4U	4	5	6
Number of Lanes		1	1	1		1	1	0	0	1	2	0	0	1	2	0
Configuration		L	Т	R		L		TR		L	Т	TR		L	Т	TR
Volume (veh/h)		79	28	354		0	10	12	0	86	218	1	0	20	180	34
Percent Heavy Vehicles (%)		3	3	3		3	3	3	3	3			3	3		
Proportion Time Blocked																
Percent Grade (%)		(	)			(	C									
Right Turn Channelized		N	0													
Median Type   Storage				Left	Only								1			
Critical and Follow-up He	adwa	ys														
Base Critical Headway (sec)		7.5	6.5	6.9		7.5	6.5	6.9		4.1				4.1		
Critical Headway (sec)		7.56	6.56	6.96		7.56	6.56	6.96		4.16				4.16		
Base Follow-Up Headway (sec)		3.5	4.0	3.3		3.5	4.0	3.3		2.2				2.2		
Follow-Up Headway (sec)		3.53	4.03	3.33		3.53	4.03	3.33		2.23				2.23		
Delay, Queue Length, and	Leve	l of Se	ervice													
Flow Rate, v (veh/h)		86	30	385		0		24		93				22		
Capacity, c (veh/h)		446	337	911		259		504		1325				1319		
v/c Ratio		0.19	0.09	0.42		0.00		0.05		0.07				0.02		
95% Queue Length, $Q_{95}$ (veh)		0.7	0.3	2.1		0.0		0.1		0.2				0.1		
Control Delay (s/veh)		15.0	16.8	11.8		18.9		12.5		7.9				7.8		
Level of Service (LOS)		В	С	В		С		В		А				А		
Approach Delay (s/veh)		12	7			12	2.5			2	.2			0	7	
Approach LOS		E	3			I	3			ŀ	4			ļ	Ą	

	HCS Two-Way Stop	-Control Report	
General Information		Site Information	
Analyst	PD	Intersection	Wenonah Ave & 4 Hills Ave
Agency/Co.	Lee Engineering	Jurisdiction	CABQ
Date Performed	6/20/2023	East/West Street	Wenonah Ave
Analysis Year	2023	North/South Street	4 Hills Rd
Time Analyzed		Peak Hour Factor	0.95
Intersection Orientation	North-South	Analysis Time Period (hrs)	0.25
Project Description	Full Build-Out 2025 AM		
Lanes			



Vehicle Volumes and Adju	istme	nts														
Approach		Eastb	ound			West	oound			North	bound			South	bound	
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R
Priority		10	11	12		7	8	9	1U	1	2	3	4U	4	5	6
Number of Lanes		1	1	1		1	1	0	0	1	2	0	0	1	2	0
Configuration		L	Т	R		L		TR		L	Т	TR		L	Т	TR
Volume (veh/h)		18	7	157		2	12	16	0	142	306	0	0	2	72	23
Percent Heavy Vehicles (%)		3	3	3		3	3	3	3	3			3	3		
Proportion Time Blocked																
Percent Grade (%)		(	C			(	0									
Right Turn Channelized		N	lo													
Median Type   Storage				Left	Only								1			
Critical and Follow-up He	adwa	ys														
Base Critical Headway (sec)		7.5	6.5	6.9		7.5	6.5	6.9		4.1				4.1		
Critical Headway (sec)		7.56	6.56	6.96		7.56	6.56	6.96		4.16				4.16		
Base Follow-Up Headway (sec)		3.5	4.0	3.3		3.5	4.0	3.3		2.2				2.2		
Follow-Up Headway (sec)		3.53	4.03	3.33		3.53	4.03	3.33		2.23				2.23		
Delay, Queue Length, and	l Leve	l of Se	ervice													
Flow Rate, v (veh/h)		19	7	165		2		29		149				2		
Capacity, c (veh/h)		401	317	1004		322		490		1483				1227		
v/c Ratio		0.05	0.02	0.16		0.01		0.06		0.10				0.00		
95% Queue Length, $Q_{95}$ (veh)		0.1	0.1	0.6		0.0		0.2		0.3				0.0		
Control Delay (s/veh)		14.4	16.6	9.3		16.3		12.8		7.7				7.9		
Level of Service (LOS)		В	С	А		С		В		А				А		
Approach Delay (s/veh)		10	).1			13	3.1			2	.4			0	.2	
Approach LOS		I	3			I	В			1	4			/	Ą	

	HCS Two-Way Stop	-Control Report	
General Information		Site Information	
Analyst	PD	Intersection	Wenonah Ave & 4 Hills Ave
Agency/Co.	Lee Engineering	Jurisdiction	CABQ
Date Performed	6/20/2023	East/West Street	Wenonah Ave
Analysis Year	2023	North/South Street	4 Hills Rd
Time Analyzed		Peak Hour Factor	0.92
Intersection Orientation	North-South	Analysis Time Period (hrs)	0.25
Project Description	Full Build-Out 2025 PM		
Lanes			



Vehicle Volumes and Adju	istme	nts														
Approach		Eastb	ound			West	ound			North	bound			South	oound	
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R
Priority		10	11	12		7	8	9	1U	1	2	3	4U	4	5	6
Number of Lanes		1	1	1		1	1	0	0	1	2	0	0	1	2	0
Configuration		L	Т	R		L		TR		L	Т	TR		L	Т	TR
Volume (veh/h)		84	28	360		0	10	12	0	91	218	1	0	20	180	38
Percent Heavy Vehicles (%)		3	3	3		3	3	3	3	3			3	3		
Proportion Time Blocked																
Percent Grade (%)		(	C			(	)									
Right Turn Channelized		N	lo													
Median Type   Storage				Left	Only								1			
Critical and Follow-up He	adwa	ys														
Base Critical Headway (sec)		7.5	6.5	6.9		7.5	6.5	6.9		4.1				4.1		
Critical Headway (sec)		7.56	6.56	6.96		7.56	6.56	6.96		4.16				4.16		
Base Follow-Up Headway (sec)		3.5	4.0	3.3		3.5	4.0	3.3		2.2				2.2		
Follow-Up Headway (sec)		3.53	4.03	3.33		3.53	4.03	3.33		2.23				2.23		
Delay, Queue Length, and	Leve	l of Se	ervice													
Flow Rate, v (veh/h)		91	30	391		0		24		99				22		
Capacity, c (veh/h)		437	329	908		249		495		1320				1319		
v/c Ratio		0.21	0.09	0.43		0.00		0.05		0.07				0.02		
95% Queue Length, Q <sub>95</sub> (veh)		0.8	0.3	2.2		0.0		0.2		0.2				0.1		
Control Delay (s/veh)		15.4	17.0	11.9		19.4		12.6		7.9				7.8		
Level of Service (LOS)		С	С	В		С		В		А				А		
Approach Delay (s/veh)		12	2.9			12	6			2	.3			0.	7	
Approach LOS	B B									ļ	4			ŀ	A	

		ŀ	ICS -	Гwo-	Way	Stop	-Cor	ntrol	Repo	ort						
General Information							Site	Inforr	natio	n						_
Analyst	PD						Inters	ection			Wend	onah Ave	e & East	Drivew		
Agency/Co.	Lee E	ngineeri	ng				Jurisc	liction			САВС	2				
Date Performed	6/20/	/2023					East/	West Stre	eet		Wend	onah Ave	<u>,</u>			
Analysis Year	2023						North	n/South S	Street		East E	Entrance	Drivewa	y		
Time Analyzed							Peak	Hour Fac	ctor		0.88			-		
Intersection Orientation	East-	West					Analy	sis Time	Period (	hrs)	0.25					
Project Description	Existi	ng 2023	AM													
Lanes																
				24 1 2 4 1 2 4 P 1	h f Maj	م م م م الم م الم م م م م م م م م م م م	t t t	4 1 74 4 7 1 4								
Vehicle Volumes and Adj	Istments Eastbound Westbound Northbound Southbound															
Approach	<u> </u>	Eastbound Westbound Northbound Southbound														
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R
Priority	10	1	2	3	4U	4	5	6		7	8	9		10	11	12
Number of Lanes	0	0	2	0	0	0	1	0		0	1	1		0	1	0
Configuration		LT		TR			LTR			LT		R			LTR	
Volume (veh/h)		10	152	2		1	110	24		3	0	0		13	0	26
Percent Heavy Vehicles (%)		3				3				3	3	3		3	3	3
Proportion Time Blocked																
Percent Grade (%)											0				0	
Right Turn Channelized										Ν	10					
Median Type   Storage				Undi	vided											
Critical and Follow-up He	eadwa	ys														
Base Critical Headway (sec)		4.1				4.1				7.5	6.5	6.9		7.5	6.5	6.2
Critical Headway (sec)		4.16				4.16				7.56	6.56	6.96		7.56	6.56	6.26
Base Follow-Up Headway (sec)		2.2				2.2				3.5	4.0	3.3		3.5	4.0	3.3
Follow-Up Headway (sec)		2.23				2.23				3.53	4.03	3.33		3.53	4.03	3.33
Delay, Queue Length, and	d Leve	l of Se	ervice							<u>.</u>	<u>.</u>		<u>.</u>			
Flow Rate, v (veh/h)		11				1				3		0			44	
Capacity, c (veh/h)		1419				1392				552		950			812	
v/c Ratio		0.01				0.00				0.01		0.00			0.05	
95% Queue Length, Q <sub>95</sub> (veh)		0.0				0.0				0.0		0.0			0.2	
Control Delay (s/veh)		7.6	0.1			7.6	0.0	0.0		11.6		8.8			9.7	
Level of Service (LOS)		А	A			A	A	A		В		A			A	
Approach Delay (s/veh)		0	.5			0	.1			1	1.6			9	.7	
Approach LOS		A A												1	4	

		ŀ	HCS <sup>-</sup>	Гwo-	Way	Stop	-Cor	ntrol	Repo	ort						
General Information							Site	Inforr	natio	n						
Analyst	PD						Inters	ection			Wend	onah Ave	e & East	Drivew		
Agency/Co.	Lee E	ngineeri	ng				Jurisc	liction			САВС	2				
Date Performed	6/20/	2023	5				East/	West Str	eet		Wend	onah Ave	<u>,</u>			
Analysis Year	2023						North	n/South :	Street		East E	Intrance	Drivewa	у		
Time Analyzed							Peak	Hour Fac	ctor		0.86			-		
Intersection Orientation	East-	West					Analy	sis Time	Period	(hrs)	0.25					
Project Description	Existi	ng 2023	PM													
Lanes		-														
				J 4 1 1 4 4 6 0		۲ ۲ ۲ ۲ ۲ ۲ ۲	st-West	1 1 4 4 Y 4 P C D								
Vehicle Volumes and Adju	Istments Eastbound Westbound Northbound Southbound															
Approach	Eastbound Westbound Northbound Southbound															
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12
Number of Lanes	0	0	2	0	0	0	1	0		0	1	1		0	1	0
Configuration		LT		TR			LTR			LT		R			LTR	
Volume (veh/h)		28	361	4		4	82	42		3	0	2		75	0	77
Percent Heavy Vehicles (%)		3				3				3	3	3		3	3	3
Proportion Time Blocked																
Percent Grade (%)											0			(	)	
Right Turn Channelized										Ν	lo					
Median Type   Storage				Undi	vided											
Critical and Follow-up He	adwa	ys														
Base Critical Headway (sec)		4.1				4.1				7.5	6.5	6.9		7.5	6.5	6.2
Critical Headway (sec)		4.16				4.16				7.56	6.56	6.96		7.56	6.56	6.26
Base Follow-Up Headway (sec)		2.2				2.2				3.5	4.0	3.3		3.5	4.0	3.3
Follow-Up Headway (sec)		2.23				2.23				3.53	4.03	3.33		3.53	4.03	3.33
Delay, Queue Length, and	l Leve	l of Se	ervice	I												
Flow Rate, v (veh/h)		33				5				3		2			177	
Capacity, c (veh/h)		1429				1124				303		790			660	
v/c Ratio		0.02				0.00				0.01		0.00			0.27	
95% Queue Length, Q <sub>95</sub> (veh)		0.1				0.0				0.0		0.0			1.1	
Control Delay (s/veh)		7.6	0.2			8.2	0.0	0.0		17.0		9.6			12.4	
Level of Service (LOS)		A	A			A	A	A		С		A			В	
Approach Delay (s/veh)		0	.7			0	.3			14	4.0			12	2.4	
Approach LOS	0.7         0.3         14.0         12.4           A         A         B         B												3			

		ŀ	ICS <sup>-</sup>	Гwo-	Way	Stop	-Cor	ntrol	Repo	ort						
General Information		_					Site	Inforr	natio	n						_
Analyst	PD						Inters	ection			Wend	onah Ave	e & East	Drivew		
Agency/Co.	Lee E	ngineeri	ng				Jurisc	liction			САВС	2				
Date Performed	6/20/	/2023					East/	West Stre	eet		Wend	onah Ave	<u>,</u>			
Analysis Year	2023						North	n/South S	Street		East E	Entrance	Drivewa	y		
Time Analyzed							Peak	Hour Fac	ctor		0.88			-		
Intersection Orientation	East-	West					Analy	sis Time	Period (	(hrs)	0.25					
Project Description	Back	ground 2	023 AM													
Lanes																
				244444 44	۲ ۲ Maj	م الم معالم المعالم الم for Street: Ea	t t C st-West	4 1 7 4 4 7 1								
Vehicle Volumes and Adj	Istments Eastbound Westbound Northbound Southbound															
Approach		Eastbound Westbound Northbound Southbound														
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R
Priority	10	1	2	3	4U	4	5	6		7	8	9		10	11	12
Number of Lanes	0	0	2	0	0	0	1	0		0	1	1		0	1	0
Configuration		LT		TR			LTR			LT		R			LTR	
Volume (veh/h)		10	157	2		1	113	25		3	0	0		13	0	27
Percent Heavy Vehicles (%)		3				3				3	3	3		3	3	3
Proportion Time Blocked																
Percent Grade (%)											0				0	
Right Turn Channelized										Ν	10					
Median Type   Storage				Undi	vided											
Critical and Follow-up He	eadwa	ys														
Base Critical Headway (sec)		4.1				4.1				7.5	6.5	6.9		7.5	6.5	6.2
Critical Headway (sec)		4.16				4.16				7.56	6.56	6.96		7.56	6.56	6.26
Base Follow-Up Headway (sec)		2.2				2.2				3.5	4.0	3.3		3.5	4.0	3.3
Follow-Up Headway (sec)		2.23				2.23				3.53	4.03	3.33		3.53	4.03	3.33
Delay, Queue Length, and	d Leve	l of Se	ervice								<u>.</u>		<u>.</u>			
Flow Rate, v (veh/h)		11				1				3		0			45	
Capacity, c (veh/h)		1413				1385				542		946			808	
v/c Ratio		0.01				0.00				0.01		0.00			0.06	
95% Queue Length, Q <sub>95</sub> (veh)		0.0				0.0				0.0		0.0			0.2	
Control Delay (s/veh)		7.6	0.1			7.6	0.0	0.0		11.7		8.8			9.7	
Level of Service (LOS)		А	А			А	A	А		В		А			А	
Approach Delay (s/veh)		0	.5			0	0.1			1	1.7			9	.7	
Approach LOS	A         A         B         A											4				

		ŀ	ICS <sup>-</sup>	Гwo-	Way	Stop	o-Cor	ntrol	Repo	ort						
General Information							Site	Inforr	natio	n						
Analyst	PD						Inters	ection			Wend	onah Ave	e & East	Drivew		
Agency/Co.	Lee E	ngineeri	ng				Jurisc	liction			САВС	2				
Date Performed	6/20/	/2023					East/	West Stre	eet		Wend	onah Ave				
Analysis Year	2023						North	n/South S	Street		East E	Entrance	Drivewa	v		
Time Analyzed							Peak	Hour Fac	ctor		0.86			, 		
Intersection Orientation	East-	West					Analy	sis Time	Period (	hrs)	0.25					
Project Description	Backo	ground 2	025 PM				,			. ,						
Lanes																
						1 r 1 r or Street: Ea	t to st-West	4 1 7 4 7 1 P								
Vehicle Volumes and Adj	Istments Eastbound Westbound Northbound Southbound															
Approach		Eastbound Westbound Northbound Southbound														
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12
Number of Lanes	0	0	2	0	0	0	1	0		0	1	1		0	1	0
Configuration		LT		TR			LTR			LT		R			LTR	
Volume (veh/h)		29	372	4		4	84	43		3	0	2		77	0	79
Percent Heavy Vehicles (%)		3				3				3	3	3		3	3	3
Proportion Time Blocked																
Percent Grade (%)											0				0	
Right Turn Channelized										Ν	10					
Median Type   Storage				Undi	vided											
Critical and Follow-up He	eadwa	ys														
Base Critical Headway (sec)		4.1				4.1				7.5	6.5	6.9		7.5	6.5	6.2
Critical Headway (sec)		4.16				4.16				7.56	6.56	6.96		7.56	6.56	6.26
Base Follow-Up Headway (sec)		2.2				2.2				3.5	4.0	3.3		3.5	4.0	3.3
Follow-Up Headway (sec)		2.23				2.23				3.53	4.03	3.33		3.53	4.03	3.33
Delay, Queue Length, and	d Leve	l of Se	ervice													
Flow Rate, v (veh/h)		34				5				3		2			181	
Capacity, c (veh/h)		1424				1112				292		782			650	
v/c Ratio		0.02				0.00				0.01		0.00			0.28	
95% Queue Length, Q <sub>95</sub> (veh)		0.1				0.0				0.0		0.0			1.1	
Control Delay (s/veh)		7.6	0.2			8.3	0.0	0.0		17.5		9.6			12.7	
Level of Service (LOS)		А	A			А	A	A		С		A			В	
Approach Delay (s/veh)		0	.7			0	0.3			14	4.3			12	2.7	
Approach LOS	A         A         B         B															

		ŀ	ICS <sup>-</sup>	Гwo-	Way	Stop	-Cor	ntrol	Repo	ort						
General Information							Site	Inforr	natio	n						
Analyst	PD			_			Inters	ection	_		Wend	onah Ave	e & East	Drivew		
Agency/Co.	Lee E	ngineeri	ng				Jurisc	liction			САВС	2				
Date Performed	6/20/	2023					East/	West Stre	eet		Wend	onah Ave	<u>.</u>			
Analysis Year	2023						North	n/South S	Street		East E	Intrance	Drivewa	у		
Time Analyzed							Peak	Hour Fac	ctor		0.88					
Intersection Orientation	East-	West					Analy	sis Time	Period (	(hrs)	0.25					
Project Description	Full B	Uild-Ou	t 2025 A	M												
Lanes																
				$J \neq J + J + J + J + J + J + J + J + J + $		م Street: Ea	t t T st-West	1114 4 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1								
Vehicle Volumes and Adju	Ijustments Eastbound Westbound Northbound Southbound															
Approach	Eastbound Westbound Northbound Southbound															
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12
Number of Lanes	0	0	2	0	0	0	1	0		0	1	1		0	1	0
Configuration		LT		TR			LTR			LT		R			LTR	
Volume (veh/h)		14	157	2		1	113	58		3	0	0		46	0	30
Percent Heavy Vehicles (%)		3				3				3	3	3		3	3	3
Proportion Time Blocked																
Percent Grade (%)											0				0	
Right Turn Channelized										Ν	10					
Median Type   Storage				Undi	vided											
Critical and Follow-up He	eadwa	ys														
Base Critical Headway (sec)		4.1				4.1				7.5	6.5	6.9		7.5	6.5	6.2
Critical Headway (sec)		4.16				4.16				7.56	6.56	6.96		7.56	6.56	6.26
Base Follow-Up Headway (sec)		2.2				2.2				3.5	4.0	3.3		3.5	4.0	3.3
Follow-Up Headway (sec)		2.23				2.23				3.53	4.03	3.33		3.53	4.03	3.33
Delay, Queue Length, and	d Leve	l of Se	ervice													
Flow Rate, v (veh/h)		16				1				3		0			86	
Capacity, c (veh/h)		1369				1385				512		946			712	
v/c Ratio		0.01				0.00				0.01		0.00			0.12	
95% Queue Length, Q <sub>95</sub> (veh)		0.0				0.0				0.0		0.0			0.4	
Control Delay (s/veh)		7.7	0.1			7.6	0.0	0.0		12.1		8.8			10.8	
Level of Service (LOS)		А	А			A	А	A		В		А			В	
Approach Delay (s/veh)		0	.7			0	.1			1	2.1			1(	).8	
Approach LOS			4				A				В			I	В	

		ŀ	ICS -	Гwo-	Way	Stop	-Cor	ntrol	Repo	ort						
General Information			_	_	_		Site	Inforr	natio	n						_
Analyst	PD						Inters	ection			Wend	onah Ave	e & East	Drivew		
Agency/Co.	Lee E	ngineeri	ng				Jurisc	liction			САВС	2				
Date Performed	6/20/	/2023					East/	West Stre	eet		Wend	onah Ave	9			
Analysis Year	2023						North	n/South S	Street		East E	Entrance	Drivewa	ıy		
Time Analyzed							Peak	Hour Fac	ctor		0.86			-		
Intersection Orientation	East-	West					Analy	sis Time	Period (	(hrs)	0.25					
Project Description	Full B	Build-Out	: 2025 P	M												
Lanes																
						م من کند من کند و	t t o st-West	4 1 7 4 4 7 1 4								
Vehicle Volumes and Adj	Lastbound Westbound Northbound Southbound															
Approach		Eastbound Westbound Northbound Southbound														
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R
Priority	10	1	2	3	4U	4	5	6		7	8	9		10	11	12
Number of Lanes	0	0	2	0	0	0	1	0		0	1	1		0	1	0
Configuration		LT		TR			LTR			LT		R			LTR	
Volume (veh/h)		30	372	4		4	84	53		3	0	2		87	0	81
Percent Heavy Vehicles (%)		3				3				3	3	3		3	3	3
Proportion Time Blocked																
Percent Grade (%)											0				0	
Right Turn Channelized										Ν	10					
Median Type   Storage				Undi	vided											
Critical and Follow-up H	eadwa	ys														
Base Critical Headway (sec)		4.1				4.1				7.5	6.5	6.9		7.5	6.5	6.2
Critical Headway (sec)		4.16				4.16				7.56	6.56	6.96		7.56	6.56	6.26
Base Follow-Up Headway (sec)		2.2				2.2				3.5	4.0	3.3		3.5	4.0	3.3
Follow-Up Headway (sec)		2.23				2.23				3.53	4.03	3.33		3.53	4.03	3.33
Delay, Queue Length, and	d Leve	l of Se	ervice													
Flow Rate, v (veh/h)	T	35				5				3		2			195	
Capacity, c (veh/h)		1410				1112				286		782			633	
v/c Ratio		0.02				0.00				0.01		0.00			0.31	
95% Queue Length, Q <sub>95</sub> (veh)		0.1				0.0				0.0		0.0			1.3	
Control Delay (s/veh)		7.6	0.2			8.3	0.0	0.0		17.7		9.6			13.2	
Level of Service (LOS)		А	A			A	A	A		С		A			В	
Approach Delay (s/veh)		0	.7			0	.3			14	4.5			13	3.2	
Approach LOS			Ą		0.7 0.3 14.5 13.2 A A B B											

		ŀ	ICS 1	Гwo-	Way	Stop	o-Cor	ntrol	Repo	ort						
General Information							Site	Infor	matio	n						
Analyst	PD						Inters	section			Wenc	onah & T	ramway			
Agency/Co.	Lee E	ngineeri	ng				Jurisc	diction			CABC	)	,			
Date Performed	6/20/	2023					East/	West Str	eet		Wenc	onah				
Analysis Year	2023						North	n/South	Street		Tram	way Blvd				
Time Analyzed							Peak	Hour Fa	ctor		0.90	-				
Intersection Orientation	North	n-South					Analy	/sis Time	Period (	(hrs)	0.25					
Project Description	Existi	ng 2023	AM													
Lanes																
Vehicle Volumes and Adiu	ustme	nts			۹. Majo	۲ ۲ ۲ Street: No	th-South	4 1 7 4 1 7								
Approach		Facth	ound			Wost	bound		1	North	bound			South	bound	
Movement			т	R		1	т	R			т	R		J	т	R
Priority	Ū	10	11	12		7	8	9	111	1	2	3	411	4	5	6
Number of Lanes		0	1	0		0	0	0	0	0	1	0	0	0	2	1
Configuration			LR							LT					T	R
Volume (veh/h)		117		21						27	110				150	62
Percent Heavy Vehicles (%)		3		3					-	3						
Proportion Time Blocked																
Percent Grade (%)			0													
Right Turn Channelized														Ν	lo	
Median Type   Storage				Left	Only								1			
Critical and Follow-up He	adwa	ys			-											
Base Critical Headway (sec)		7.5		6.9						4.1						
Critical Headway (sec)		6.86		6.96						4.16						
Base Follow-Up Headway (sec)		3.5		3.3						2.2						
Follow-Up Headway (sec)		3.53		3.33						2.23						
Delay, Queue Length, and	l Leve	l of Se	ervice													
Flow Rate, v (veh/h)			153							30						
Capacity, c (veh/h)			689							1322						
v/c Ratio			0.22							0.02						
95% Queue Length, Q <sub>95</sub> (veh)			0.8							0.1						
Control Delay (s/veh)			11.7							7.8	0.2					
Level of Service (LOS)			В							А	А					
Approach Delay (s/veh)		1	1.7						1	.7						

В

Approach LOS

		ŀ	HCS <sup>-</sup>	Гwo-'	Way	Stop	-Cor	ntrol	Repo	ort						
General Information							Site	Inforr	natio	n						
Analyst	PD						Inters	ection			Wenc	nah & T	ramway			
Agency/Co.	Lee Ei	ngineeri	ng				Jurisc	liction			САВС	2				
Date Performed	6/20/	2023	<u> </u>				East/	West Stre	eet		Wenc	nah				
Analysis Year	2023						North	n/South S	Street		Tram	way Blvd				
Time Analyzed							Peak	Hour Fac	ctor		0.87	-				
Intersection Orientation	North	-South					Analy	sis Time	Period (	hrs)	0.25					
Project Description	Existir	ng 2023	AM													
Lanes																
Vohicle Volumes and Adi	istmo	nte			Major	↑ ↑ ↑ ↑ ↑ Street: Nor	<b>↑ ↑ ↑</b> th-South	1 4 4 4 4								
	Istme	nts			1											
Approach	<u> </u>	Eastb	bound			West	oound			North	bound	_		South	bound	
Driverity	0	L 10	11	R 12	U	L	1	R	111	L		R		L		R
Number of Lance	<u> </u>	10	1	12		/	0	9	10		2	5	40	4	2	0
		0		0		0	0		0			0	0	0	2 T	
Volume (veh/h)		118		59						59	117				353	127
Porcont Honyy Vahiclos (%)		2		2					<u> </u>	2	117				555	127
Proportion Time Blocked		5		5						5						
Porcent Grade (%)			0						<u> </u>							
Pight Turn Chappelized			0											N		
Median Type   Storage				Left	Only								1			
Critical and Follow-up He	adwa	vs		Len	Only				<u> </u>							
Base Critical Headway (sec)		75		69					1	41						1
Critical Headway (sec)		6.86		6.96						4 16						
Base Follow-Up Headway (sec)		3.5		33						22						
Follow-Up Headway (sec)		3 53		3 33						2.23						
Delay, Queue Length, and	Leve	l of Se	ervice	0.00						2120						
Elow Bate y (vah/h)			202							69						1
Flow Rate, v (veh/h)			205				<u> </u>			1007						
v/c Ratio			0.37							0.07					_	
95% Oueue Length O (veh)			17							0.07						
Control Delay (c/yeh)			1.7							0.2						
			15 5							88	06					
Level of Service (LOS)			15.5 C							8.8 A	0.6 A					

С

Approach LOS

		ŀ	ICS -	Гwo-	Way	Stop	o-Cor	ntrol	Repo	ort						
General Information							Site	Infor	matio	n						
Analyst	PD						Inters	section			Wenc	onah & T	ramway			
Agency/Co.	Lee E	naineeri	ng				Jurisc	diction			CABC	)	,			
Date Performed	6/20/	2023					East/	West Str	eet		Wenc	onah				
Analysis Year	2023						North	n/South	Street		Tram	way Blvd				
Time Analyzed							Peak	Hour Fa	ctor		0.90	-				
Intersection Orientation	North	n-South					Analy	sis Time	Period (	(hrs)	0.25					
Project Description	Backg	ground 2	025 AM													
Lanes																
Vakielo Valumes and Adi					٩٦ Major	₹ ¶ ¶ ∳ Y r Street: No	th-South	14474								
Vehicle Volumes and Adju	istme	nts														
Approach		Eastb	ound			West	bound		<u> </u>	North	bound			South	bound	
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R
Priority		10	11	12		7	8	9	10	1	2	3	40	4	5	6
Number of Lanes		0	1	0		0	0	0	0	0	1	0	0	0	2	1
Configuration			LR							LT					Т	R
Volume (veh/h)		121		22						28	113				155	64
Percent Heavy Vehicles (%)		3		3						3					<u> </u>	
Proportion Time Blocked																
Percent Grade (%)			0													
Right Turn Channelized														Ν	io	
Median Type   Storage				Left	Only								1			
Critical and Follow-up He	adwa	ys														
Base Critical Headway (sec)		7.5		6.9						4.1						
Critical Headway (sec)		6.86		6.96						4.16						
Base Follow-Up Headway (sec)		3.5		3.3						2.2						
Follow-Up Headway (sec)		3.53		3.33						2.23						
Delay, Queue Length, and	l Leve	l of Se	ervice													
Flow Rate, v (veh/h)			159							31						
Capacity, c (veh/h)			683							1313						
v/c Ratio			0.23							0.02						
95% Queue Length, Q <sub>95</sub> (veh)			0.9							0.1						
Control Delay (s/veh)			11.9							7.8	0.2					
Level of Service (LOS)			В							A	A					
Approach Delay (s/veh)		- 1 <sup>-</sup>	1.9	-		-	-			1	.7	-		-		-

В

Approach LOS

								_		_						
		ŀ	HCS <sup>-</sup>	Гwo-	Way	Stop	o-Cor	ntrol	Repo	ort						
General Information	_	_	_	_	_	_	Site	Infor	matio	n	_	_	_	_		_
Analyst	PD				_		Inters	section			Wenc	onah & T	ramway	_		
Agency/Co.	Lee E	ngineeri	ng				Juriso	diction			САВС	2				
Date Performed	6/20/	2023					East/	West Str	eet		Wenc	onah				
Analysis Year	2023						North	n/South	Street		Tram	way Blvd				
Time Analyzed							Peak	Hour Fa	ctor		0.87					
Intersection Orientation	North	n-South					Analy	/sis Time	Period (	(hrs)	0.25					
Project Description	Backg	ground 2	2025 PM													
Lanes																
	_	_		$J \neq \downarrow \downarrow \downarrow \downarrow \downarrow \downarrow \downarrow \downarrow \downarrow \downarrow$	A 'n Major	1 1 4 Y r Street: No	th-South	14 4 7 4 8 6								
Vehicle Volumes and Adjustments																
Approach		Eastb	ound			West	bound			North	bound			South	bound	
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R
Priority		10	11	12		7	8	9	1U	1	2	3	4U	4	5	6
Number of Lanes		0	1	0		0	0	0	0	0	1	0	0	0	2	1
Configuration			LR							LT					Т	R
Volume (veh/h)		122		61						61	121				364	131
Percent Heavy Vehicles (%)		3		3						3						
Proportion Time Blocked																
Percent Grade (%)			0													
Right Turn Channelized														Ν	lo	
Median Type   Storage				Left	Only								1			
Critical and Follow-up He	eadwa	ys														
Base Critical Headway (sec)		7.5		6.9						4.1						
Critical Headway (sec)		6.86		6.96						4.16						
Base Follow-Up Headway (sec)		3.5		3.3						2.2						
Follow-Up Headway (sec)		3.53		3.33						2.23						
Delay, Queue Length, and	d Leve	l of S	ervice		<u>.</u>	<u>.</u>	<u> </u>						<u>.</u>	<u>.</u>		
Flow Rate, v (veh/h)	<u> </u>		210							70						
Capacity, c (veh/h)			535							993						
v/c Ratio			0.39							0.07						
95% Queue Length, Q <sub>95</sub> (veh)			1.9							0.2						
Control Delay (s/veh)			16.0							8.9	0.7					
Level of Service (LOS)			С							А	A					
Approach Delay (s/yeh)		1	5.0							3	.4					

С

Approach LOS

		ŀ	ICS 1	Гwo-	Way	Stop	-Cor	ntrol	Repo	ort						
General Information							Site	Inforr	natio	n						
Analyst	PD						Inters	section			Wend	onah & T	ramway			
Agency/Co.	Lee E	ngineeri	ng				Juriso	liction			САВС	2				
Date Performed	6/20/	2023	5				East/	West Str	eet		Wend	onah				
Analysis Year	2023						North	n/South :	Street		Tram	way Blvd				
Time Analyzed							Peak	Hour Fac	ctor		0.90					
Intersection Orientation	North	n-South	Peak Hour Factor     0.90       South     Analysis Time Period (hrs)     0.25       Jild-Out 2025 AM     Image: Comparison of the second													
Project Description	Full B	Site Information         Intersection       Wenonah & Tramway         cABQ         2023       CABQ         2023       East/West Street       Wenonah & Tramway Blvd         2023       Venonah         2023       CABQ         South Street       Venonah         North/South Street       0.90         South Colspan="4">Venonah         South Street       0.90         South Street       0.90         South Street       0.90         South Colspan="4">Venthout Street       0.90         South Street       Onthout Street         South Street       South Street         Venthout Street       South Street         Westbould Street North-South         Merit North-South Street       South Street         Merit North-South         South Street North-South       South Street         Merit North-South       South Street														
Lanes																
Vohicle Volumes and Adi	istmo	ntc		14 4 4 4 4 4 4 4	٩٦ Major	۲ ۲ Street: Nor	th-South	7412485								
venicle volumes and Adju	Jstme	nts							1							
Approach		Eastb	ound			West	bound			North	bound	-		South	bound	-
Movement	U	L	T	R	U	L	T	R	U	L	T	R	U	L	T	R
Priority		10	11	12		7	8	9	10	1	2	3	40	4	5	6
Number of Lanes		0	1	0		0	0	0	0	0	1	0	0	0	2	1
		407	LK				<u> </u>				442	<u> </u>				R
Volume (veh/h)		127		25						31	113				155	70
Percent Heavy Vehicles (%)		3		3			<u> </u>			3					<u> </u>	
Proportion Time Blocked																
Percent Grade (%)			0													
Right Turn Channelized														N	0	
Median Type   Storage				Left	Only								1			
Critical and Follow-up He	adwa	ys														
Base Critical Headway (sec)		7.5		6.9						4.1						
Critical Headway (sec)		6.86		6.96						4.16						
Base Follow-Up Headway (sec)		3.5		3.3						2.2						
Follow-Up Headway (sec)		3.53		3.33						2.23						
Delay, Queue Length, and	l Leve	l of Se	ervice													
Flow Rate, v (veh/h)			169							34						
Capacity, c (veh/h)			680							1305						
v/c Ratio			0.25							0.03						
95% Queue Length, Q <sub>95</sub> (veh)			1.0							0.1						
Control Delay (s/veh)			12.0							7.8	0.2					
Level of Service (LOS)			В							A	A					
Approach Delay (s/veh)		12	2.0							1	.9					

В

Approach LOS

		ŀ	ICS -	Гwo-'	Way	Stop	-Cor	ntrol	Repo	ort						
General Information							Site	Inforr	natio	n						
Analyst	PD						Inters	section			Wend	onah & T	ramway			
Agency/Co.	Lee E	ngineeri	ng				Jurisc	liction			САВС	2				
Date Performed	6/20/	2023	-				East/	West Str	eet		Wend	onah				
Analysis Year	2023						North	n/South :	Street		Tram	way Blvd				
Time Analyzed							Peak	Hour Fac	ctor		0.87					
Intersection Orientation	North	n-South					Analy	vsis Time	Period (	hrs)	0.25					
Project Description	Full B	uild-Out	: 2025 PI	M												
Lanes																
Vehicle Volumes and Adiu	ustme	nts			۹. Majo	۲ ۲ ۲ Street: Nor	th-South	4 1 2 4 4 1 2								
Approach		Easth	ound			Wost	bound			North	bound		1	South	bound	
Movement		Eastbound         Westbound         Northbound         Southbound           U         L         T         R         U         L         T         R         U         L         T         R         V         L         T         R         V         L         T         R         V         L         T         R         V         L         T         R         V         L         T         R         V         L         T         R         V         L         T         R         V         L         T         R         V         L         T         R         V         L         T         R         V         L         T         R         V         L         T         R         V         L         T         R         V         L         T         R         V         L         T         R         V         L         T         R         V         L         T         R         V         L         T         R         V         L         T         R         V         L         T         R         V         L         T         R         V         L </td <td>D</td>													D	
Priority		L 10	11	۲ 12	0	7	8	R Q	111	L 1	2	л З	411		5	R 6
Number of Lanes		0	1	0		0	0	0	0	0	1	0	-+0	0	2	1
		0	IR	0			0	0	Ŭ	IT		0		Ŭ	T	R
Volume (veh/h)		124		62						62	121				364	134
Percent Heavy Vehicles (%)		2		3						3	121				304	134
Proportion Time Blocked															<u> </u>	
Percent Grade (%)			0													
Right Turn Channelized														Ν	lo	
Median Type   Storage				Left	l Onlv								1			
Critical and Follow-up He	adwa	vs			<u> </u>								-			
Base Critical Headway (sec)		7.5		6.9						4.1						
Critical Headway (sec)		6.86		6.96						4.16						
Base Follow-Up Headway (sec)		3.5		3.3						2.2					<u> </u>	
Follow-Up Headway (sec)		3.53		3.33						2.23					<u> </u>	
Delay, Queue Length, and	l Leve	l of Se	ervice		<u> </u>	<u> </u>	<u> </u>	<u> </u>	<u> </u>	<u> </u>	<u> </u>	<u> </u>	<u> </u>	<u> </u>		<u> </u>
Flow Rate, v (veh/h)	<u> </u>		214							71					_	
Capacity, c (veh/h)			534							990						
v/c Ratio			0.40							0.07						
95% Queue Length, Qas (veh)			1.9							0.2						
Control Delay (s/veh)			16.2							8.9	0.7					
Level of Service (LOS)			С							A	A					
Approach Delay (s/veh)		16	5.2							3	.5					

С

Approach LOS

	HCS Two-Way Stop	-Control Report	
General Information		Site Information	
Analyst	PD	Intersection	Tramway & West Dwy
Agency/Co.	Lee Engineering	Jurisdiction	CABQ
Date Performed	6/20/2023	East/West Street	West Dwy
Analysis Year	2023	North/South Street	Tramway Blvd
Time Analyzed		Peak Hour Factor	0.88
Intersection Orientation	North-South	Analysis Time Period (hrs)	0.25
Project Description	Existing 2023 AM		
Lanes			



Approach Eastbound Westbound Northbound Southbound																		
Approach		Eastb	ound			West	oound			North	bound			South	bound			
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R		
Priority		10	11	12		7	8	9	1U	1	2	3	4U	4	5	6		
Number of Lanes		0	1	0		0	1	0	0	1	2	1	0	1	2	1		
Configuration			LTR				LTR			L	Т	R		L	Т	R		
Volume (veh/h)		10	2	3		4	7	74	0	9	204	19	8	92	202	30		
Percent Heavy Vehicles (%)		3	3	3		3	3	3	3	3			3	3				
Proportion Time Blocked																		
Percent Grade (%)		(	C			(	0											
Right Turn Channelized										Ν	No No							
Median Type   Storage		Left Only											1					
Critical and Follow-up He	adwa	dways																
Base Critical Headway (sec)		7.5	6.5	6.9		7.5	6.5	6.9		4.1			6.4	4.1				
Critical Headway (sec)		7.56	6.56	6.96		7.56	6.56	6.96		4.16			6.46	4.16				
Base Follow-Up Headway (sec)		3.5	4.0	3.3		3.5	4.0	3.3		2.2			2.5	2.2				
Follow-Up Headway (sec)		3.53	4.03	3.33		3.53	4.03	3.33		2.23			2.53	2.23				
Delay, Queue Length, and	l Leve	l of Se	ervice															
Flow Rate, v (veh/h)			17				97			10				114				
Capacity, c (veh/h)			422				750			1290				1260				
v/c Ratio			0.04				0.13			0.01				0.09				
95% Queue Length, Q <sub>95</sub> (veh)			0.1				0.4			0.0				0.3				
Control Delay (s/veh)			13.9				10.5			7.8				8.1				
Level of Service (LOS)	B B B								AAA									
Approach Delay (s/veh)		13	3.9			1(	).5			0	.3			2	.5			
Approach LOS		I	3			I	В			/	4			/	4			

	HCS Two-Way Stop	-Control Report	
General Information		Site Information	
Analyst	PD	Intersection	Tramway & West Dwy
Agency/Co.	Lee Engineering	Jurisdiction	CABQ
Date Performed	6/20/2023	East/West Street	West Dwy
Analysis Year	2023	North/South Street	Tramway Blvd
Time Analyzed		Peak Hour Factor	0.92
Intersection Orientation	North-South	Analysis Time Period (hrs)	0.25
Project Description	Existing 2023 PM		
Lanes			



Vehicle Volumes and Adju	istme	nts															
Approach		Eastb	ound			Westk	oound			North	bound			South	bound		
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R	
Priority		10	11	12		7	8	9	1U	1	2	3	4U	4	5	6	
Number of Lanes		0	1	0		0	1	0	0	1	2	1	0	1	2	1	
Configuration			LTR				LTR			L	Т	R		L	Т	R	
Volume (veh/h)		13	7	22		8	4	191	0	10	242	27	12	234	445	59	
Percent Heavy Vehicles (%)		3	3	3		3	3	3	3	3			3	3			
Proportion Time Blocked																	
Percent Grade (%)		(	0			(	C										
Right Turn Channelized										N	lo			N	0		
Median Type   Storage				Left	Only				1								
Critical and Follow-up He	adwa	dways															
Base Critical Headway (sec)		7.5	6.5	6.9		7.5	6.5	6.9		4.1			6.4	4.1			
Critical Headway (sec)		7.56	6.56	6.96		7.56	6.56	6.96		4.16			6.46	4.16			
Base Follow-Up Headway (sec)		3.5	4.0	3.3		3.5	4.0	3.3		2.2			2.5	2.2			
Follow-Up Headway (sec)		3.53	4.03	3.33		3.53	4.03	3.33		2.23			2.53	2.23			
Delay, Queue Length, and	Leve	l of Se	ervice														
Flow Rate, v (veh/h)			46				221			11				267			
Capacity, c (veh/h)			236				695			1011				1218			
v/c Ratio			0.19				0.32			0.01				0.22			
95% Queue Length, Q <sub>95</sub> (veh)			0.7				1.4			0.0				0.8			
Control Delay (s/veh)			23.9				12.6			8.6				8.8			
Level of Service (LOS)			С				В			А				А			
Approach Delay (s/veh)	23.9 12.6									0	.3	2.9					
Approach LOS		(	с			F	В			ŀ	4				4		

	HCS Two-Way Stop	-Control Report	
General Information		Site Information	
Analyst	PD	Intersection	Tramway & West Dwy
Agency/Co.	Lee Engineering	Jurisdiction	CABQ
Date Performed	6/20/2023	East/West Street	West Dwy
Analysis Year	2023	North/South Street	Tramway Blvd
Time Analyzed		Peak Hour Factor	0.88
Intersection Orientation	North-South	Analysis Time Period (hrs)	0.25
Project Description	Background 2023 AM		
Lanes			



Vehicle Volumes and Adjustments           Approach         Eastbound         Westbound         Northbound         Southbound																			
Approach		Eastb	ound			West	oound			North	bound			South	bound				
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R			
Priority		10	11	12		7	8	9	1U	1	2	3	4U	4	5	6			
Number of Lanes		0	1	0		0	1	0	0	1	2	1	0	1	2	1			
Configuration			LTR				LTR			L	Т	R		L	Т	R			
Volume (veh/h)		10	2	3		4	7	76	0	9	210	20	8	95	208	31			
Percent Heavy Vehicles (%)		3	3	3		3	3	3	3	3			3	3					
Proportion Time Blocked																			
Percent Grade (%)		(	C			(	C												
Right Turn Channelized										N	lo		No						
Median Type   Storage		Left Only								1									
Critical and Follow-up He	adwa	dways																	
Base Critical Headway (sec)		7.5	6.5	6.9		7.5	6.5	6.9		4.1			6.4	4.1					
Critical Headway (sec)		7.56	6.56	6.96		7.56	6.56	6.96		4.16			6.46	4.16					
Base Follow-Up Headway (sec)		3.5	4.0	3.3		3.5	4.0	3.3		2.2			2.5	2.2					
Follow-Up Headway (sec)		3.53	4.03	3.33		3.53	4.03	3.33		2.23			2.53	2.23					
Delay, Queue Length, and	l Leve	l of Se	ervice																
Flow Rate, v (veh/h)			17				99			10				117					
Capacity, c (veh/h)			411				745			1282				1252					
v/c Ratio			0.04				0.13			0.01				0.09					
95% Queue Length, Q <sub>95</sub> (veh)			0.1				0.5			0.0				0.3					
Control Delay (s/veh)			14.1				10.6			7.8				8.2					
Level of Service (LOS)	B B B									A A A									
Approach Delay (s/veh)		14	4.1			1(	).6			0	.3			2	.5				
Approach LOS		I	3			I	3			/	4			A	Ą				

	HCS Two-Way Stop	-Control Report	
General Information		Site Information	
Analyst	PD	Intersection	Tramway & West Dwy
Agency/Co.	Lee Engineering	Jurisdiction	CABQ
Date Performed	6/20/2023	East/West Street	West Dwy
Analysis Year	2023	North/South Street	Tramway Blvd
Time Analyzed		Peak Hour Factor	0.92
Intersection Orientation	North-South	Analysis Time Period (hrs)	0.25
Project Description	Background 2023 PM		
Lanes			



Vehicle Volumes and Adjustments           Approach         Eastbound         Westbound         Northbound         Southbound																				
Approach		Eastb	ound			West	oound			North	bound			South	bound					
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R				
Priority		10	11	12		7	8	9	1U	1	2	3	4U	4	5	6				
Number of Lanes		0	1	0		0	1	0	0	1	2	1	0	1	2	1				
Configuration			LTR				LTR			L	Т	R		L	Т	R				
Volume (veh/h)		13	7	23		8	4	197	0	10	249	28	12	241	458	61				
Percent Heavy Vehicles (%)		3	3	3		3	3	3	3	3			3	3						
Proportion Time Blocked																				
Percent Grade (%)		(	C			(	0													
Right Turn Channelized										Ν	lo			No						
Median Type   Storage		Left Only								1										
Critical and Follow-up He	adwa	dways																		
Base Critical Headway (sec)		7.5	6.5	6.9		7.5	6.5	6.9		4.1			6.4	4.1						
Critical Headway (sec)		7.56	6.56	6.96		7.56	6.56	6.96		4.16			6.46	4.16						
Base Follow-Up Headway (sec)		3.5	4.0	3.3		3.5	4.0	3.3		2.2			2.5	2.2						
Follow-Up Headway (sec)		3.53	4.03	3.33		3.53	4.03	3.33		2.23			2.53	2.23						
Delay, Queue Length, and	Leve	l of Se	ervice																	
Flow Rate, v (veh/h)			47				227			11				275						
Capacity, c (veh/h)			227				686			997				1209						
v/c Ratio			0.21				0.33			0.01				0.23						
95% Queue Length, Q <sub>95</sub> (veh)			0.8				1.4			0.0				0.9						
Control Delay (s/veh)			24.9				12.8			8.7				8.9						
Level of Service (LOS)	C B									А			A							
Approach Delay (s/veh)		24	1.9			12	2.8			0	.3			2	.9					
Approach LOS		(	2			I	В			/	4			/	Ą					

	HCS Two-Way Stop	-Control Report	
General Information		Site Information	
Analyst	PD	Intersection	Tramway & West Dwy
Agency/Co.	Lee Engineering	Jurisdiction	CABQ
Date Performed	6/20/2023	East/West Street	West Dwy
Analysis Year	2023	North/South Street	Tramway Blvd
Time Analyzed		Peak Hour Factor	0.88
Intersection Orientation	North-South	Analysis Time Period (hrs)	0.25
Project Description	Full Build-Out 2025 AM		
Lanes			



ehicle Volumes and Adjustments Approach Eastbound Westbound Northbound Southbound																	
Approach		Eastb	ound			West	bound			North	bound			South	bound		
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R	
Priority		10	11	12		7	8	9	1U	1	2	3	4U	4	5	6	
Number of Lanes		0	1	0		0	1	0	0	1	2	1	0	1	2	1	
Configuration			LTR				LTR			L	Т	R		L	Т	R	
Volume (veh/h)		10	2	3		11	7	99	0	9	210	26	8	118	208	31	
Percent Heavy Vehicles (%)		3	3	3		3	3	3	3	3			3	3			
Proportion Time Blocked																	
Percent Grade (%)		(	0				0										
Right Turn Channelized										Ν	lo			N	lo		
Median Type   Storage				Left	Only				1								
Critical and Follow-up He	adwa	adways															
Base Critical Headway (sec)		7.5	6.5	6.9		7.5	6.5	6.9		4.1			6.4	4.1			
Critical Headway (sec)		7.56	6.56	6.96		7.56	6.56	6.96		4.16			6.46	4.16			
Base Follow-Up Headway (sec)		3.5	4.0	3.3		3.5	4.0	3.3		2.2			2.5	2.2			
Follow-Up Headway (sec)		3.53	4.03	3.33		3.53	4.03	3.33		2.23			2.53	2.23			
Delay, Queue Length, and	l Leve	l of Se	ervice														
Flow Rate, v (veh/h)			17				133			10				143			
Capacity, c (veh/h)			372				717			1282				1249			
v/c Ratio			0.05				0.19			0.01				0.11			
95% Queue Length, Q <sub>95</sub> (veh)			0.1				0.7			0.0				0.4			
Control Delay (s/veh)	15.1 11.2									7.8				8.3			
Level of Service (LOS)	C B B									А				А			
Approach Delay (s/veh)	15.1 11.2									0	.3			2	.8		
Approach LOS		(	C			l	В			1	4			1	4		

HCS Two-Way Stop-Control Report								
General Information		Site Information						
Analyst	PD	Intersection	Tramway & West Dwy					
Agency/Co.	Lee Engineering	Jurisdiction	CABQ					
Date Performed	6/20/2023	East/West Street	West Dwy					
Analysis Year	2023	North/South Street	Tramway Blvd					
Time Analyzed		Peak Hour Factor	0.92					
Intersection Orientation	North-South	Analysis Time Period (hrs)	0.25					
Project Description	Full Build-Out 2025 PM							
Lanes								



venicle volumes and Adju	Istme	nτs														
Approach		Eastb	ound			West	ound			North	bound			South	bound	
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R
Priority		10	11	12		7	8	9	1U	1	2	3	4U	4	5	6
Number of Lanes		0	1	0		0	1	0	0	1	2	1	0	1	2	1
Configuration			LTR				LTR			L	Т	R		L	Т	R
Volume (veh/h)		13	7	23		11	4	211	0	10	249	31	12	256	458	61
Percent Heavy Vehicles (%)		3	3	3		3	3	3	3	3			3	3		
Proportion Time Blocked																
Percent Grade (%)	0				0											
Right Turn Channelized									No				No			
Median Type   Storage				Left	Only				1							
Critical and Follow-up He	adwa	ys														
Base Critical Headway (sec)		7.5	6.5	6.9		7.5	6.5	6.9		4.1			6.4	4.1		
Critical Headway (sec)		7.56	6.56	6.96		7.56	6.56	6.96		4.16			6.46	4.16		
Base Follow-Up Headway (sec)		3.5	4.0	3.3		3.5	4.0	3.3		2.2			2.5	2.2		
Follow-Up Headway (sec)		3.53	4.03	3.33		3.53	4.03	3.33		2.23			2.53	2.23		
Delay, Queue Length, and	Leve	l of Se	ervice													
Flow Rate, v (veh/h)			47				246			11				291		
Capacity, c (veh/h)			213				657			997				1206		
v/c Ratio			0.22				0.37			0.01				0.24		
95% Queue Length, $Q_{95}$ (veh)			0.8				1.7			0.0				0.9		
Control Delay (s/veh)			26.6				13.7			8.7				8.9		
Level of Service (LOS)			D				В			А				А		
Approach Delay (s/veh)		26	5.6		13.7			0.3			3.0					
Approach LOS		[	)			I	3			1	Ą			ŀ	Ą	

**APPENDIX D:** 

SIGHT DISTANCE CALCULATIONS

# Intersection Sight Distance Calculations and Tables

TABLE 7.	4.65 Minii	mum Inte	rsection Si	ght Distanc	е		
Speed	Minimu	m Intersect	tion Sight Dis	tance			
Limit (MPH)	2 Lane U	ndivided	3 Lane Und Lane Divid Median	livided or 2 ed w/ 12 ft.	4 Lane Undivided		
	Left Turn	Right Turn	Left Turn	Right Turn	Left Turn	Right Turn	
20	230 ft.	200 ft.	240 ft.	200 ft.	250 ft.	200 ft.	
25	280 ft.	240 ft.	300 ft.	240 ft.	320 ft.	240 ft.	
30	340 ft.	290 ft.	360 ft.	290 ft.	380 ft.	290 ft.	
35	390 ft.	340 ft.	420 ft.	340 ft.	440 ft.	340 ft.	
40	450 ft.	390 ft.	480 ft.	390 ft.	500 ft.	390 ft.	
45	500 ft.	430 ft.	530 ft.	430 ft.	570 ft.	430 ft.	
50	560 ft.	480 ft.	590 ft.	480 ft.	630 ft.	480 ft.	

Reference: City of Albuquerque Development Process Manual chapter 7-4

Reference: 2018 AASHTO "Green Book" chapter 9.5 Design Vehicle: Pasenger Car Major Road Lanes: 2 NB, 2 SB divided by a 15 ft raised median Case B1: A stopped vehicle turning left from a minor street approach onto a major road Case B2: A stopped vehicle turning right from a minor street approach onto a major road

## FORMULA:

ISD= 1.47\*V<sub>major</sub>,\*t<sub>g</sub>

Units: ISD (ft), V<sub>major</sub>(MPH), and t<sub>g</sub>(seconds) Speed(V<sub>major</sub>): 35 MPH Time Gaps (t<sub>g</sub>): 7.5 sec (for passenger car crossing one lane of traffic) 1.5 sec (for extra lane of traffic crossed)

## CASE B1 (LEFT TURN):

Time Gap ( $t_g$ )= 7.5s + 1.5s = 9.0 s ISD = 1.47\*35\*9 = 463.05 ~ **465 ft** 

### CASE B2 (RIGHT TURN):

Assumption: Design vehicle is turning right into the first lane of major roadway. Time Gap ( $t_g$ ): 6.5s ISD = 1.47\*35\*6.5 = 334.43 ~ **335 ft** 

	t, Valu	es		
		Passenger	Single-Unit	Combination
	CASE	Car	Truck	Truck
B1	Left Turn from the Minor Road	7.5	9.5	11.5
<b>B</b> 2	Right Turn from the Minor Road	65	0.5	10.5
<b>B</b> 3	Crossing Maneuver from the Minor Road	0.5	0.5	10.5
F	Left Turn from the Major Road	5.5	6.5	7.5
each	additional lane, from the left, in excess of one	e, to be crosse	a by the turnin	ig venicle.
he app SE B	or road approach grades: 0.2 seconds for each percent grade proach grade is an upgrade that exceeds 3 per 2 + B3 - For a stopped vehicle to turn right and grades 3 percent or less	cent. It onto or cre	oss a <u>2-lane hi</u>	ghway with <u>1</u>
t mino + he app ASE B dian t cross + +	or road approach grades: 0.2 seconds for each percent grade proach grade is an upgrade that exceeds 3 per 2 + B3 - For a stopped vehicle to turn right and grades 3 percent or less sing a major road with more than 2 lanes: 0.5 seconds for passenger cars 0.7 seconds for trucks	cent. It onto or cro	oss a <u>2-lane hi</u>	<u>ghway</u> with <u>1</u>
he app ASE B dian cross + each	or road approach grades: 0.2 seconds for each percent grade proach grade is an upgrade that exceeds 3 per 2 + B3 - For a stopped vehicle to turn right and grades 3 percent or less sing a major road with more than 2 lanes: 0.5 seconds for passenger cars 0.7 seconds for trucks additional lane to be crossed and narrow med	cent. It onto or cro lians that can	oss a <u>2-lane hi</u> not store the de	<u>ghway</u> with <u>i</u> sign vehicle.
t cross + + each t minc + + each	or road approach grades: 0.2 seconds for each percent grade proach grade is an upgrade that exceeds 3 per <b>2 + B3 - For a stopped vehicle to turn right</b> and grades 3 percent or less sing a major road with more than 2 lanes: 0.5 seconds for passenger cars 0.7 seconds for trucks additional lane to be crossed and narrow med or road approach grades: 0.1 seconds for each percent grade proach grade is an upgrade that exceeds 3 per	cent. It onto or cro lians that cam cent.	oss a <u>2-lane hi</u> not store the de	<u>ghway</u> with <u>i</u> sign vehicle.
t mino + he app ASE B dian t cross + + each t mino + he app	or road approach grades: 0.2 seconds for each percent grade proach grade is an upgrade that exceeds 3 per <b>2 + B3 - For a stopped vehicle to turn right</b> and grades 3 percent or less sing a major road with more than 2 lanes: 0.5 seconds for passenger cars 0.7 seconds for trucks additional lane to be crossed and narrow med or road approach grades: 0.1 seconds for each percent grade proach grade is an upgrade that exceeds 3 per <b>- For a stopped vehicle to turn across <u>one</u></b>	cent. It onto or cro lians that cam cent. lane of oppo	oss a <u>2-lane hi</u> not store the de osing traffic	ghway with <u>i</u> sign vehicle.
t cross + + + + + + + + + + + + +	or road approach grades: 0.2 seconds for each percent grade proach grade is an upgrade that exceeds 3 per <b>2 + B3 - For a stopped vehicle to turn right</b> and grades 3 percent or less sing a major road with more than 2 lanes: 0.5 seconds for passenger cars 0.7 seconds for trucks additional lane to be crossed and narrow med or road approach grades: 0.1 seconds for each percent grade proach grade is an upgrade that exceeds 3 per <b>- For a stopped vehicle to turn across <u>one</u> urning vehicles that cross more than 1 oppose</b>	cent. It onto or cro lians that can cent. lane of oppo ing lane:	oss a <u>2-lane hi</u> not store the de	ghway with g
t cross + + + + + + + + + + + + +	or road approach grades: 0.2 seconds for each percent grade broach grade is an upgrade that exceeds 3 per <b>2 + B3 - For a stopped vehicle to turn right</b> and grades 3 percent or less sing a major road with more than 2 lanes: 0.5 seconds for passenger cars 0.7 seconds for trucks additional lane to be crossed and narrow med or road approach grades: 0.1 seconds for each percent grade broach grade is an upgrade that exceeds 3 per <b>- For a stopped vehicle to turn across</b> <u>one</u> turning vehicles that cross more than 1 oppose 0.5 seconds for passenger cars	cent. It onto or cro lians that cam cent. <u>lane of oppo</u> ing lane:	oss a <u>2-lane hi</u> not store the de	ghway with g
t cross + + + + + + + + + + + + +	or road approach grades: 0.2 seconds for each percent grade proach grade is an upgrade that exceeds 3 per <b>2 + B3 - For a stopped vehicle to turn right</b> and grades 3 percent or less sing a major road with more than 2 lanes: 0.5 seconds for passenger cars 0.7 seconds for passenger cars 0.7 seconds for trucks additional lane to be crossed and narrow med or road approach grades: 0.1 seconds for each percent grade proach grade is an upgrade that exceeds 3 per <b>- For a stopped vehicle to turn across <u>one</u> turning vehicles that cross more than 1 oppose 0.5 seconds for passenger cars 0.7 seconds for passenger cars 0.7 seconds for passenger cars 0.7 seconds for passenger cars 0.7 seconds for passenger cars</b>	cent. It onto or cro lians that cam cent. <u>lane of oppo</u> ing lane:	oss a <u>2-lane hi</u> not store the de	<u>ghway</u> with sign vehicle.

**EXHIBIT 4 - Water Availability Statement** 



Albuquerque Bernalillo County Water Utility Authority

PO Box 568 Albuquerque, NM 87103 www.abcwua.org

July 14, 2023

<u>Chair</u> Eric C. Olivas County of Bernalillo Commissioner, District 5

Vice Chair

Tammy Fiebelkorn City of Albuquerque Councilor, District 7

Barbara Baca County of Bernalillo Commissioner, District 1

Adriann Barboa County of Bernalillo Commissioner, District 3

Pat Davis City of Albuquerque Councilor, District 6

Trudy E. Jones City of Albuquerque Councilor, District 8

Timothy M. Keller City of Albuquerque Mayor

*Ex-Officio Member* Gilbert Benavides Village of Los Ranchos Board Trustee

Executive Director Mark S. Sanchez

Website www.abcwua.org Luis Noriega Tierra West LLC 5571 Midway Park Place NE, Albuquerque, NM 87109

RE: Water and Sanitary Sewer Availability Statement #230609 Project Name: Starbucks - Tramway Project Address: N/A Legal Description: Tract J-1, Replat of Tracts F, H-1, J & Unplatted Land Four Hills Village Shopping Center & Apartment Complex UPC: 102305602134521316 Zone Atlas Map: L-23-Z

Dear Mr. Noriega:

**Project Description:** The subject site is located north-east of the Tramway Boulevard and Wenonah Avenue intersection within the City of Albuquerque. The proposed development consists of approximately 0.49 acres and the property is currently zoned MX-M for moderate intensity mixed-use. The property lies within the Pressure Zone 6E in the Freeway Trunk.

The request for availability indicates plans for commercial development to develop a 1,468 square foot commercial food service business. Additionally, a concurrent platting action is taking place that will subdivide existing Tract J-1 into two Tracts: J-1-A and J-1-B. Existing businesses located on proposed Tract J-1-A already have service with the Water Authority. The aforementioned proposed food service business will be developed and located on proposed Tract J-1-B.

## **Existing Conditions:**

Water infrastructure in the area consists of the following:

- 12-inch PVC distribution line (project #26-3340-90) along Wenonah Avenue.
- 12-inch capped PVC distribution line (project #26-3340-90) connected from the preceding distribution line and terminating at the project site.
- 12-inch PVC distribution line (project #26-2459-86) along Tramway Boulevard

Sanitary sewer infrastructure in the area consists of the following:

- Public Eight-inch PVC sanitary sewer collector (project #26-4361.90-93) extending along Wenonah Drive and terminating near the property boundary of currently platted Tract J-1 within public right-of-way.
- Private eight-inch PVC sanitary sewer collector on currently platted J-1 which extends from the existing public eight-inch PVC sanitary sewer collector described above.

**Water Service:** The existing development on proposed Tract J-1-A already receives water service. New metered water service to the proposed Tract J-1-B can be provided via routine connection to the existing distribution lines described in the preceding
Existing Conditions section. The engineer is responsible for determining pressure losses and sizing of the service line(s) downstream of the public water line to serve the proposed development.

Service is also contingent upon compliance with the Fire Marshal's instantaneous fire flow requirements. Water service will not be sold without adequate fire protection. Water service will only be sold in conjunction with sanitary sewer service. Each legally platted property shall have individual independent water services. No property shall share a water service with any other property. Existing service lines and fire lines that will not be utilized are to be removed by shutting the valve near the distribution main. For fire lines, the line shall be capped near the public valve and valve access shall be grouted and collar removed.

**Non-Potable Water Service:** Currently, there is no non-potable infrastructure available to serve the subject property.

**Sanitary Sewer Service:** The existing development on proposed Tract J-1-A already receives sanitary sewer service. However, since the proposed subdivision will cause a portion of the existing private sanitary sewer service serving proposed Tract J-1-A to be located within proposed Tract J-1-B, Tract J-1-B will be required to grant a private sanitary sewer easement for this portion of the private sanitary sewer service. This can be addressed with the platting action or separate document.

New sanitary sewer service to proposed Tract J-1-B can be provided contingent on the developer funded construction of a new manhole at the terminus of the existing eightinch public sanitary sewer within public right-of-way. Tract J-1-B shall connect downstream of the proposed manhole for its routine service connection. No property shall share a private sewer service with any other property. The engineer is responsible for sizing the service line(s) upstream of the public sanitary sewer line to serve the proposed development. All food service establishments must install a grease trap upstream of the domestic private sewer connection prior to discharge into the public sanitary sewer lines.

**Fire Protection:** From the Fire Marshal's requirements, the instantaneous fire flow requirements for the project are 1,750 gallons-per-minute. One fire hydrant is required. There is one existing public hydrant available and zero new public hydrants are proposed with this project. As modeled using InfoWater™ computer software, the fire flow can be met by applying the required fire flow to the system as shown in the information provided by the requestor. Analysis was performed by simulating the required fire flow at existing public fire hydrant #200 southwest of proposed Tract J-1-B. The existing private hydrant located on proposed Tract J-1-A shall not be used for Tract J-1-B's fire protection. Any changes to the proposed connection points shall be coordinated through Utility Development. All new required hydrants as well as their exact locations must be determined through the City of Albuquerque Fire Marshal's Office and verified through the Utility Development Office prior to sale of service. The engineer is responsible for determining pressure losses and sizing of the fire line(s) downstream of the public water line to serve the proposed fire hydrants and/or fire suppression system.

**Cross Connection Prevention:** Per the Cross Connection Prevention and Control Ordinance, all new non-residential premises must have a reduced pressure principal backflow prevention assembly approved by the Water Authority installed at each

domestic service connection at a location accessible to the Water Authority. No tees, branches, possible connection fittings or openings are allowed between the reduced principal backflow prevention assembly and the service connection, unless protected by a backflow prevention assembly. These requirements also apply to all remodeled non-residential premises when the work area of the building undergoing repairs, alterations or rehabilitation, as defined in the International Existing Building Code, exceeds 50 percent of the aggregate area of the building regardless of the costs of repairs, alteration.

All non-residential irrigation water systems connected to the public water system shall have a pressure vacuum breaker, spill-resistant pressure vacuum breaker or a reduced pressure principal backflow prevention assembly installed after the service connection. Such devices shall be approved by the Water Authority. No tees, branches, or possible connection fittings or openings are allowed between the containment backflow prevention assembly and the service connection.

All non-residential customers connected via piping to an alternative water source or an auxiliary water supply and the public water system shall install a containment reduced pressure principal backflow prevention assembly approved by the Water Authority after the potable service connection.

All new services to private fire protection systems shall be equipped with a containment reduced pressure principal backflow prevention assembly approved by the Water Authority and Fire Marshal having jurisdiction installed after the service connection. No tees, branches, possible connection fittings or openings are allowed between the containment backflow prevention assembly and the service connection. A double check valve assembly approved by the Water Authority and Fire Marshal having jurisdiction may be installed instead of a reduced pressure backflow prevention assembly provided the private fire protection system meets or exceeds ANSI/NSF Standard 60 61 throughout the entire private fire protection system, the fire sprinkler drain discharges into atmosphere, there are no reservoirs, fire department connections nor connections from auxiliary water supplies.

The Water Authority recommends that all backflow (containment) devices be located above ground just outside the easement or road right-of-way, the containment backflow device can be installed within the building if there are no tees, branches, possible connection fittings or openings between the reduced principal backflow prevention assembly, and the service connection unless protected by another reduced pressure backflow prevention assembly device. Contact Cross Connection at (505) 289-3465 for more information.

**Pretreatment – Fats, Oils and Greases:** The development is for commercial use and has the potential to discharge Fats, Oils, Grease and/or Solids (FOGS) to the sanitary sewer and/or falls under one of the applicable users in the SUO:

FOGS Applicability SUO Section 3-3-2 A.:

Users "...such as food service establishments, commercial food processors, automotive shops, auto wash racks, car washes, vehicle fueling stations, septic tank pumpers, grease rendering facilities, breweries/distilleries, bottling plants, commercial and industrial laundries, slaughterhouses and meat packing establishments (fish, fowl, meat, curing, hide curing), oil tank firms and transporters..." Such Users must comply with all FOGS discharge requirements defined in SUO Section 3-3-2 and FOGS Policy including but not limited to:

1. Installation of an adequately sized Grease Interceptor (GI) approved by the appropriate code enforcement authority (City of Albuquerque, and/or Bernalillo County)

a. Interceptors and/or Separators are required for outdoor pools, dumpsters pads and outdoor washdown areas that have the potential to discharge grease, sand, solids, flammable liquids to the sanitary sewer. Pad shall be installed at an elevation higher than surrounding grade. It is not required for dumpster pads to have a sewer connection.

b. Placement of Interceptors in drive-thru or traffic lanes is not allowed.

- 2. All FOGS source within the facility are plumbed to the GI as required by the appropriate plumbing code.
- 3. Long term Best Management Practices (BMP), and GI maintenance such as pumping and manifest requirements.
- 4. Unobstructed access to inspections of the facility and records.

A copy of the Sewer Use and Wastewater Control Ordinance and FOGS Policy can be found on the Pretreatment page of the Water Authority Website: <u>https://www.abcwua.org/sewer-system-industrial-pretreatment-overview/</u>

Contact the Industrial Pretreatment Engineer, Travis Peacock, at (505) 289-3439 or pretreatment@abcwua.org for coordination or clarification of any of the above requirements.

**Easements and Property:** Exclusive public water and sanitary sewer easements are required for all public lines that are to be constructed outside of any dedicated Rightsof-Way. A minimum width easement of 20 feet is required for a single utility and 25 feet for water and sewer both within the same easement. Easements for standard sized water meters need to be five feet by five feet and include the length of the water service if located on private property. For larger meters that require a meter vault, a 35 feet by 35 feet easement is required. Actual easement widths may vary depending on the depth of the lines to be installed. Acceptable easements must be documented prior to approval of service. A Warranty Deed shall be required when a property will be transferred to the Water Authority for the installation of Water Authority owned facilities such as pump stations, reservoirs, wells, lift stations, or any other facility.

Required public water and/or sanitary sewer easements shall be for the construction, installation, maintenance, repair, modification, replacement and operation of public water and sanitary service lines, equipment and facilities reasonably necessary to provide service together with free access on and over the easement, the right to remove trees, shrubs, undergrowth and any other obstacles, modifications, or structures which interfere with use of the easement.

**Pro Rata:** Pro Rata is not owed and the property can utilize the services available upon completion of the requirements of this statement to connect to water and sanitary sewer.

**Design and Construction:** Design and construction of all required improvements will be at the developer/property owner's expense. Improvements must be coordinated through the Water Authority Connection Permit process. Construction must be performed by a licensed and bonded public utility contractor.

**Utility Expansion Charge (UEC):** In addition to installation and construction costs, any new metered water services will be subject to both water and sanitary sewer Utility Expansion Charges (UEC) payable at the time of service application. All charges and rates collected will be based on the ordinances and policies in effect at the time service is actually requested and authorized. Per the Rate Ordinance, each customer classification on the same premise requires a separate meter. Contact Customer Service at (505) 842-9287 (option 3) for more information regarding UECs.

**Water Use:** All new commercial developments shall be subject to the requirements for water usage and water conservation requirements as defined by the Water Authority, particularly the Water Waste Reduction Ordinance. Where available, outdoor water usage shall utilize reclaimed water.

**Closure:** This availability statement provides a commitment from the Water Authority to provide services to the development as long as identified conditions are met. It will remain in effect for a period of one year from the date of issue and applies only to the development identified herein. Its validity is in part, contingent upon the continuing accuracy of the information supplied by the developer. Changes in the proposed development may require reevaluation of availability and should be brought to the attention of the Utility Development Section of the Water Authority as soon as possible.

Please feel free to contact Mr. Kristopher Cadena in our Utility Development Section at (505) 289-3301 or email at <u>kcadena@abcwua.org</u> if you have questions regarding the information presented herein or need additional information.

Sincerely,

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Mark S. Sanchez Executive Director

Enclosures: Infrastructure Maps

f/ Availability Statement #230609





## EXHIBIT 5 - SOLID WASTE APPROVAL





## **EXHIBIT 6 - FIRE 1 APPROVAL**



## CAUTION

CONTRACTOR TO CONDUCT ALL NECESSARY FIELD AND APPROVED BY THE ENGINEER.

