

**MEMORANDUM****DATE:** December 13, 2016Courtyard I  
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Albuquerque, NM  
87109-4335  
[www.bhinc.com](http://www.bhinc.com)**TO:** Gina Ross, PE, City of Albuquerquevoice: 505.823.1000  
facsimile: 505.798.7988  
toll free: 800.877.5332**FROM:** Rifka Wine, PE *RW***SUBJECT:** South Broadway Hydraulic Analysis**Background**

This technical memorandum summarizes the hydraulic modeling analysis conducted by Bohannan Huston, Inc. (BHI) of the storm drain system between the South Broadway Detention Basin and the Commercial at Bell Pump Station in Albuquerque, New Mexico. The City of Albuquerque (City) has experienced flooding in the neighborhood to the south of the South Broadway Detention Basin. This project included a records research and field survey of the existing storm drains in the subject area. This technical memorandum then summarizes recommended improvements to the system to alleviate flooding in the area.

The storm drain system in this section of the city conveys stormwater generated on the streets above that enters the pipe network via a series of inlets which drains to the Bell Pump Station. The flow from the Bell Pump Station is then pumped to a 54-inch storm drain which discharges to the Rio Grande at Bridge Blvd, see Figure 1 - Existing System.

The South Broadway Detention Basin was modified in 2015, and with this reconstruction, the headwall at the southern edge of the pond (just above the pond's outlet) was raised by 1 foot to increase the storage capacity of the pond. Since that reconstruction, city personnel stated that they had received reports of street flooding during heavy rainfall events in subject area and believe the increased head on the pond was responsible for the flooding.

**Hydrology**

The hydrology used for this study was taken from the South Broadway DMP by URS, Inc. All flows are based on the 100-year, 24-hour storm event and assume fully developed conditions.

**Hydraulic Analysis**

As part of the modeling effort, BHI performed a field survey of the existing storm drain infrastructure in the project area, which included collecting rim and invert elevations for all applicable storm drain inlets and manholes along with sizes of inlets and pipes. BHI survey data was entered into a Civil 3D drawing, and this file was the basis for constructing the storm drain network for the area between Bell Pump Station and the South Broadway Detention Basin. The Civil 3D pipe network was then exported to Autodesk Storm and Sanitary Analysis 2016, which runs the EPA SWMM 5 engine, in order to perform the storm drain hydraulic modeling.

The survey data indicated the presence of adverse pipe slopes within the network, and as such the model needed to be run under dynamic wave settings (full use of the St. Venant flow equations) which can account for backflow and reverse flow conditions within the pipes. Initially the SWMM model was run using a constant flow (no inflow hydrographs) with the water level in the South Broadway Pond set at full depth. However, after subsequent analysis and additional discussion with the City, it was decided to use inflow hydrographs for both the flow into the South Broadway Pond (from other areas) and from the subject area that flows by gravity into the Bell Pump Station.

### **Existing Conditions Model Runs**

Since the URS model did not supply sufficient detail in the subject area, BHI delineated contributing basins and flow paths for the area between Broadway Blvd. to the east, the South Broadway Detention Basin to the north, the railroad fence on the west boundary at Commercial Blvd, and Bell Ave. to the south. BHI used the Modified Rational Method in the COA DPM to compute 100-year hydrographs for import into SWMM. The Modified Rational Method calculations are included in Appendix A. The results of the SWMM model showed flooding occurred in a number of locations throughout the subject area (see Appendix C for the model results of the existing system). At locations where the hydraulic grade line came out of the ground, the model includes street links to route the flows. The locations where the hydraulic grade line came out of the ground (causing manhole covers to pop causing street and flooding outside of street) are shown in Figure 1 and summarized below:

- Manhole No. K14954 – Intersection of Commercial and Pacific
- Manhole No. K14945 – Intersection of Pacific and William
- Manhole No. K14952 – Intersection of Cromwell and William
- Manhole No. L14052 – William between Cromwell and Garfield
- Manhole No. L14053 – John between Cromwell and Garfield
- Manhole No. L14041 – Intersection of Garfield and Commercial

The inflow hydrograph from the existing South Broadway DMP SWMM model exceeded the stage-storage-discharge capacity of the existing pond and primary outlet. The emergency spillway was added to allow flow in excess of the pond capacity to be removed from the system via the rectangular, concrete-lined spillway. This emergency spillway flow causes some local flooding on Commercial, and mitigating this flooding is not included in the scope of this project.

### **Recommended Improvements**

Drainage infrastructure improvements first attempted to alleviate flooding by increasing pipe sizes in the model as summarized in Table 1 below. However, after performing a number of iterations of varying pipe sizes, it became clear that pipe size adjustments alone could not eliminate all flooding areas.

**Table 1: SWMM Model Scenarios**

Model Scenario	Flap Gates?	Manholes Bolted?	Description	Junctions Flooded
Scenario #1	No	No	SD inlets added to MH K14944 and MH K14951.	K14954, DI #2046, K14941, K14952, L14052, L14053, L14042
Scenario #2	Yes	No	Flap gates added to DI #2046 and DI #2024.	K14954, DI #2046, K14941, K14952, L14052, L14053, L14042
Scenario #3	Yes	Yes	Same as Scenario #2 with K14954, K14945, K14944, K14951 and K14953 bolted down.	DI #2046, L14052, L14053, L14042, L14141, and L14143
Upsize #1	No	No	Same as Scenario #1 with links SDP11, SDP12, SDP13, SDP19, SDP25, SDP24, SDP7, SDP8 upsized based on COA PDF.	DI #2046, L14042, L14051, L14052, and L14053
Upsize #2	No	No	Same as Upsize #1 with links SDP3, SDP4, and SDP5 upsized based on COA PDF.	DI #2046, L14042, L14051, L14052, and L14053
Upsize #3	No	No	Same as Option #1 with links SDP4, SDP5, SDP6, SDP7, and SDP8 upsized to 48".	L14042, L14052, and L14053
Upsize #4	No	No	Same as Upsize #3 with a 24" link along Cromwell connecting Commercial and William and link SDP27 upsized to 24".	L14042, L14052, and L14053

A number of additional scenarios were run that included additional ponding and additional storm drains. The final recommended solution is comprised of additional ponding and replacing some of the existing storm drain pipe as described below:

#### **Proposed Detention Pond at South Broadway Park**

An additional 3 ac-ft of storage was added to the system at South Broadway Park with an 18-inch storm drain outleting to the existing 36-inch storm drain at William and Lewis.

#### **South Broadway Emergency Spillway Flow**

This flow will pond behind an existing speed bump until the water surface elevation in the Detention Basin drops, and then flow can reenter the Detention Basin through a drop inlet and pipe which has a flap gate.

### **Storm Drain Pipe Upgrades**

In addition, in order to remove all the flooding from the system, a small section of 18-inch existing storm drain at the intersection of Commercial and Garfield will need to be replaced with 24-inch storm drain, and the existing 12-inch John St will need to be replaced with 24-inch storm drain and lowered vertically between Cromwell and Garfield.

The recommended improvements are shown in Figure 2, and the SWMM model for the recommended improvements is included in Appendix C. A secondary outlet to the Detention Basin was initially added down John St, connecting to the existing 12-inch storm drain at John and Cromwell. A number of sizes were tried; however, this secondary outlet is not recommended. The improvements described above remove flooding from the streets downstream of the South Broadway Detention Basin. Additionally, if the Bell Pump Station failed, a significantly larger amount of water from the South Broadway Detention Basin would flood the project area.

### **Design Considerations**

The following summarizes the invert elevations were modeled for new connections and provide guidance when the improvements are designed:

- Replacement 24-inch @ John and Cromwell: 4943.86 (DP1)
- Replacement 24-inch @ John and Garfield: 4942.28 (DP2)
- South Broadway Park Storage Bottom: 4941.5 (DP3)
- New 18-inch outlet @ South Broadway Park: 4941.5 (DP4)
- New 18-inch outlet @ William and Lewis: 4939.08 (DP5)

### **Conclusion**

The improvements described above will remove flooding from the streets downstream of the South Broadway Detention Basin. These improvements do not mitigate flooding caused by overtopping of the South Broadway Detention Basin. Over two hours, 9 ac-ft of water overtops this facility.

REW/AQC/le

Attachments

Figure 1 – Existing System

Figure 2 – Recommended Improvements

Appendix A – Modified Rational Method Calculations

Appendix B – SWMM Model Input Data

Appendix C – SWMM Models



Coordinate System: NAD 1983 StatePlane New Mexico Central FIPS 3002 Feet  
 Projection: Transverse Mercator  
 Datum: North American 1983

**Figure 1: Existing System**



0 75 150 300  
 1 in = 150 ft  
 Feet



Coordinate System: NAD 1983 StatePlane New Mexico Central FIPS 3002 Feet  
Projection: Transverse Mercator

## **Figure 2: Recommended Improvements**



A scale bar at the bottom of the map indicates distances up to 300 feet. It features a horizontal line with tick marks at 0, 75, 150, and 300. Below the line, the text "1 in = 150 ft" is written.

**APPENDIX A**  
**MODIFIED RATIONAL METHOD CALCULATIONS**



Coordinate System: NAD 1983 StatePlane New Mexico Central FIPS 3002 Feet  
 Projection: Transverse Mercator  
 Datum: North American 1983

## Basin Map



0 75 150 300  
1 in = 150 ft  
Feet

**South Broadway SWMM Model**

**Proposed Developed Conditions Basin Data Table**

This table is based on the DPM Section 22.2, Zone: 2

Basin ID	Area (SQ. FT)	Area (AC.)	Land Treatment Percentages				Q(100yr) (cfs/ac.)	Q(100yr) (CFS)	V(100yr) (inches)	V <sub>(100yr-6hr)</sub> (CF)	V <sub>(100yr-24hr)</sub> (CF)	Q(2yr) (cfs/ac.)	Q(2yr) (CFS)	WT E (inches)	V <sub>(2yr-6hr)</sub> (CF)	V <sub>(2yr-24hr)</sub> (CF)
			A	B	C	D										
<b>CURRENT ONSITE BASINS</b>																
B1	101391	2.33	0.0%	0.0%	20.0%	80.0%	4.39	10.21	1.92	16239	18943	1.61	3.74	0.66	5593	6767
B2	238638	5.48	0.0%	0.0%	50.0%	50.0%	3.92	21.48	1.63	32316	36293	1.23	6.74	0.47	9347	11073
B3	74360	1.71	0.0%	0.0%	60.0%	40.0%	3.76	6.43	1.53	9456	10448	1.10	1.88	0.41	2516	2946
B4	80904	1.86	0.0%	20.0%	30.0%	50.0%	3.75	6.96	1.56	10484	11832	1.13	2.09	0.44	2993	3579
B5	166316	3.82	0.0%	10.0%	40.0%	50.0%	3.83	14.64	1.59	22037	24809	1.18	4.50	0.46	6334	7537
B6	133318	3.06	0.0%	10.0%	40.0%	50.0%	3.83	11.73	1.59	17665	19887	1.18	3.61	0.46	5077	6042
B7	103914	2.39	0.0%	10.0%	40.0%	50.0%	3.83	9.15	1.59	13769	15501	1.18	2.81	0.46	3957	4709
B8	98935	2.27	0.0%	10.0%	40.0%	50.0%	3.83	8.71	1.59	13109	14758	1.18	2.68	0.46	3768	4483
B9	128439	2.95	0.0%	20.0%	30.0%	50.0%	3.75	11.05	1.56	16644	18784	1.13	3.32	0.44	4752	5681
B10	57119	1.31	0.0%	10.0%	40.0%	50.0%	3.83	5.03	1.59	7568	8520	1.18	1.54	0.46	2175	2588
B11	231926	5.32	0.0%	30.0%	40.0%	30.0%	3.35	17.84	1.32	25551	27870	0.82	4.38	0.30	5856	6863
B12	83856	1.93	0.0%	10.0%	50.0%	40.0%	3.68	7.08	1.49	10419	11537	1.05	2.03	0.39	2746	3232
B13	301213	6.91	0.0%	10.0%	40.0%	50.0%	3.83	26.51	1.59	39911	44931	1.18	8.15	0.46	11471	13650
<b>TOTAL</b>	<b>1800330</b>	<b>41.33</b>	-	-	-	-	<b>156.81</b>	-		<b>235166</b>	<b>264112</b>	-	<b>47.46</b>	-	<b>66587</b>	<b>79149</b>
<b>CURRENT OFFSITE BASINS</b>																
OS1	0	0.00	0.0%	0.0%	0.0%	0.0%	0.00	0.00	0.00	0	0	0.00	0.00	0.00	0	0
OS2	0	0.00	0.0%	0.0%	0.0%	0.0%	0.00	0.00	0.00	0	0	0.00	0.00	0.00	0	0
<b>TOTAL</b>	<b>0</b>	<b>0.00</b>	-	-	-	-	<b>0.00</b>	-		<b>0</b>	<b>0</b>	-	<b>0.00</b>	-	<b>0</b>	<b>0</b>

Basin ID	Area (ac)	Area Land Treatment D (ac)	$Q_p$ (cfs)	$t_c$ (hr)	Excess Precipitation (in.)	$t_p$ (hr)	$t_p + 0.25 * A_D / A_T$ (hr)	$t_B$ (hr)
B1	2.33	1.86	10.21	0.2	1.92	0.21	0.41	0.72
B2	5.48	2.74	21.48	0.2	1.63	0.23	0.36	0.75
B3	1.71	0.68	6.43	0.2	1.53	0.24	0.34	0.75
B4	1.86	0.93	6.96	0.2	1.56	0.23	0.36	0.75
B5	3.82	1.91	14.64	0.2	1.59	0.23	0.36	0.75
B6	3.06	1.53	11.73	0.2	1.59	0.23	0.36	0.75
B7	2.39	1.19	9.15	0.2	1.59	0.23	0.36	0.75
B8	2.27	1.14	8.71	0.2	1.59	0.23	0.36	0.75
B9	2.95	1.47	11.05	0.2	1.56	0.23	0.36	0.75
B10	1.31	0.66	5.03	0.2	1.59	0.23	0.36	0.75
B11	5.32	1.60	17.84	0.2	1.32	0.25	0.32	0.76
B12	1.93	0.77	7.08	0.2	1.49	0.24	0.34	0.75
B13	6.91	3.46	26.51	0.2	1.59	0.23	0.36	0.75

Values from Sheet 2 "COA\_DPM\_100yr, 6-hr"

Auto-calculated

Assumes minimum  $t_c = 12$  min. (0.2 hr)

**APPENDIX B**  
**SWMM MODEL INPUT DATA**

Project Title: COA South Broadway Drainage Analysis

BHI Project #: 20160354

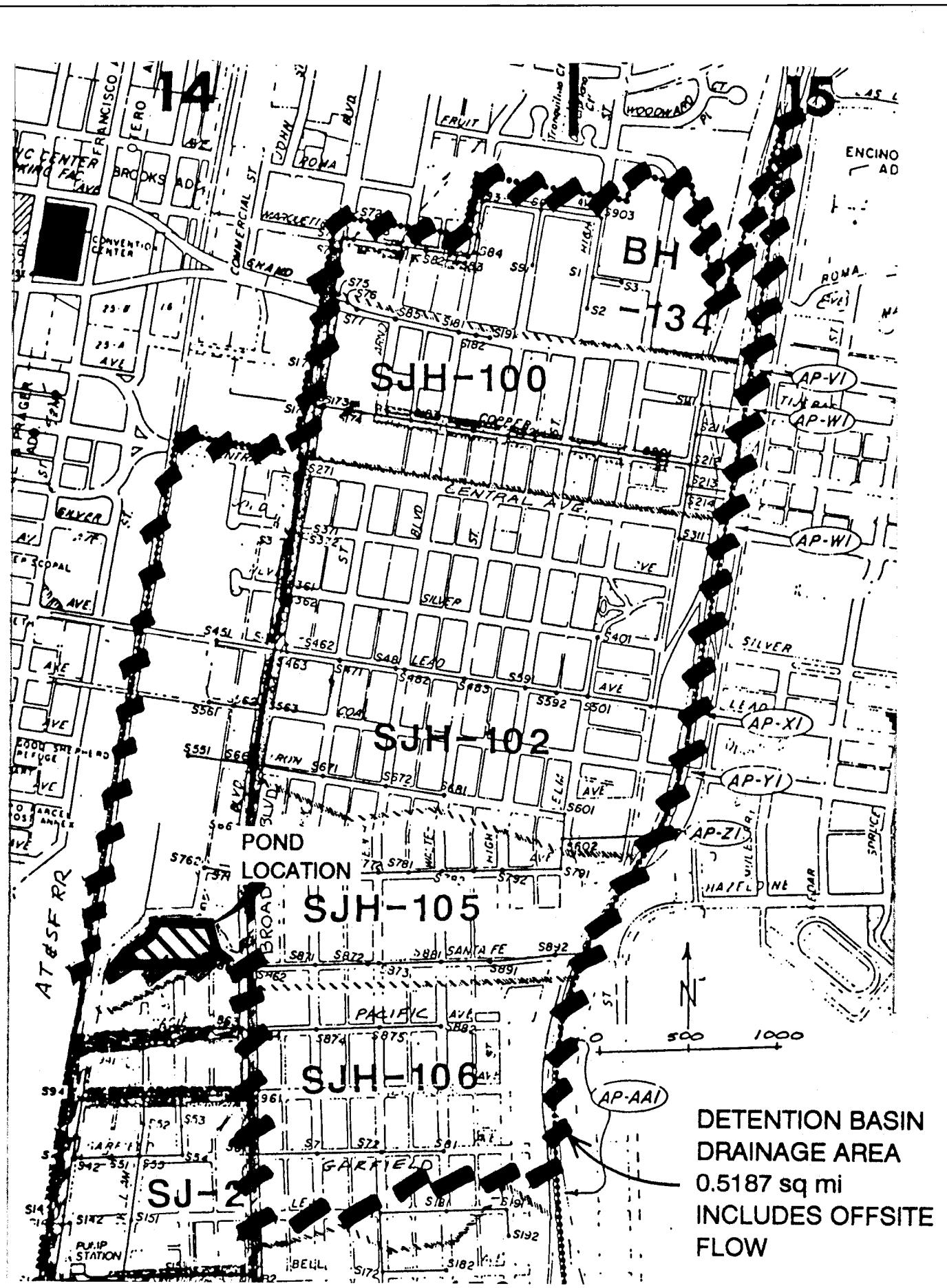
Document: Notes on General Assumptions for Pipe Network Layout

Date: 3-3-2016

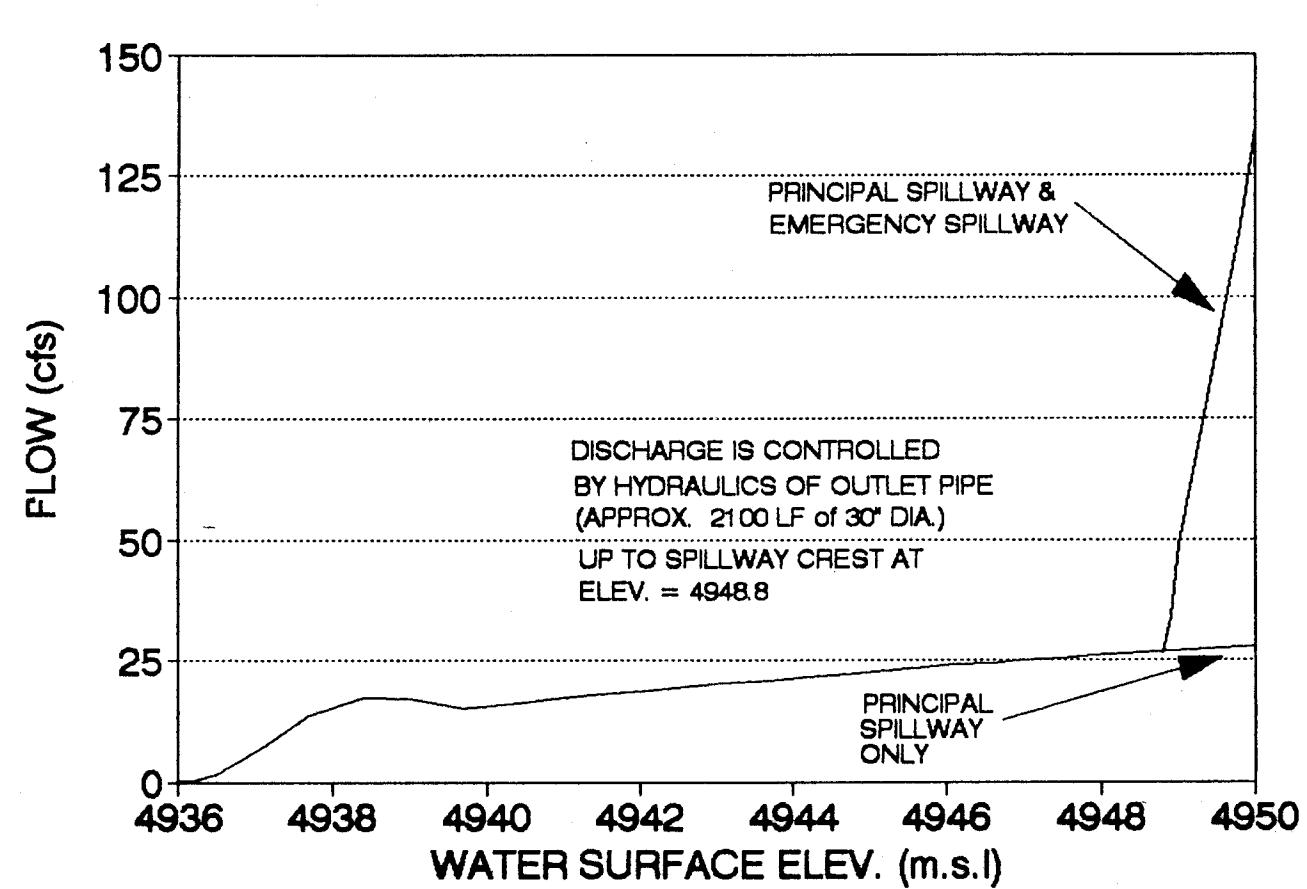
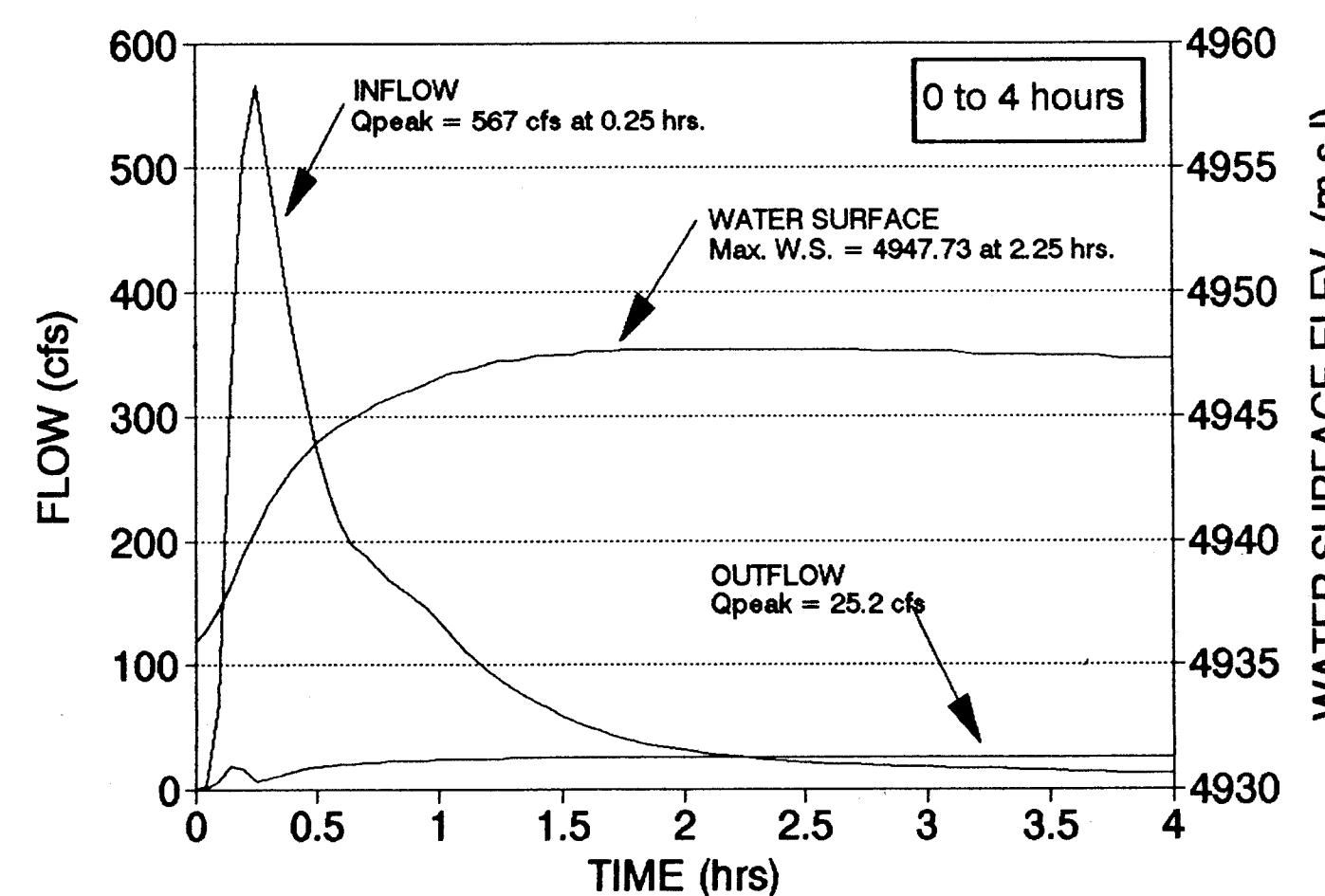
Design Engineer: Cameron Herrington

Assumptions Made:

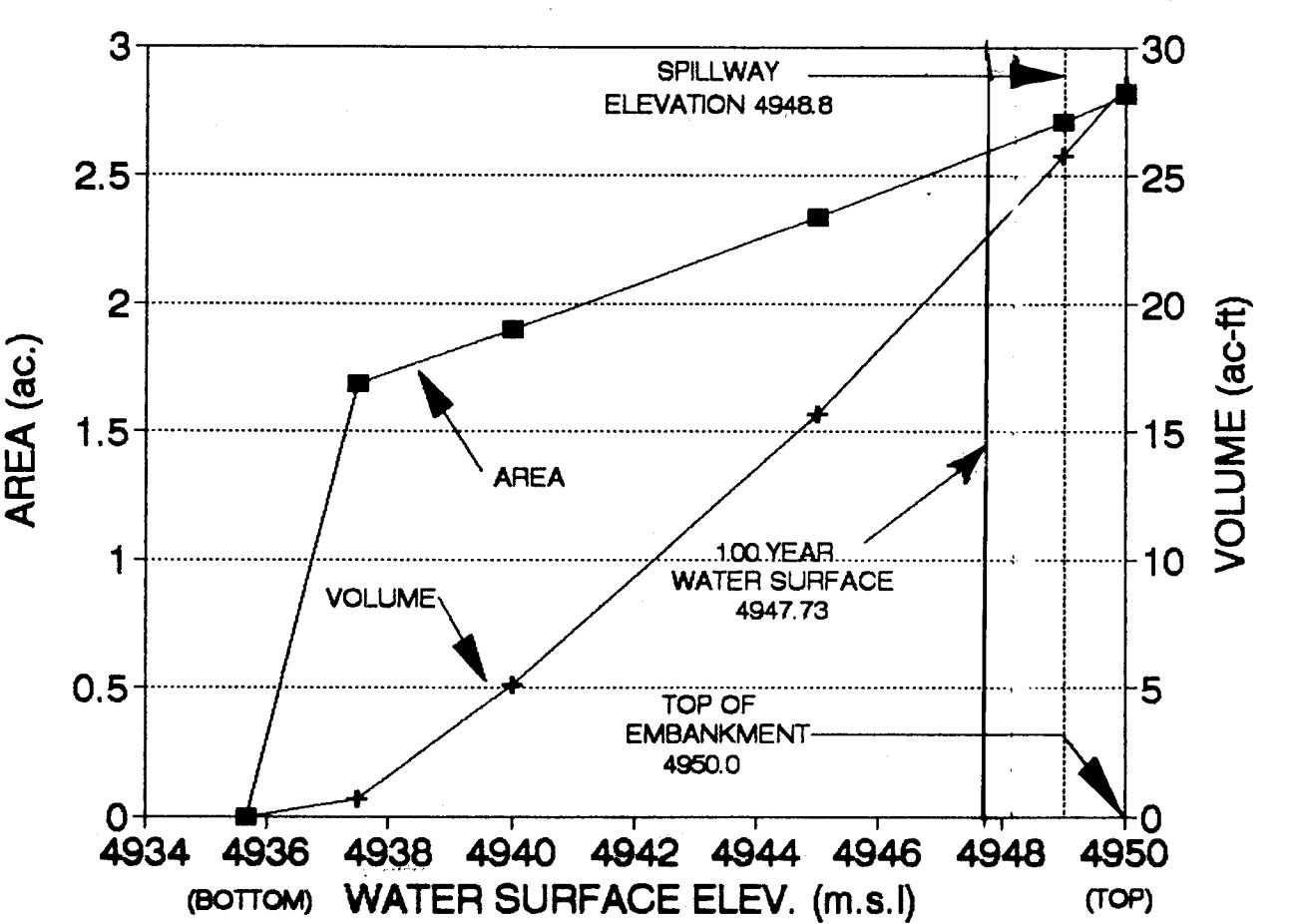
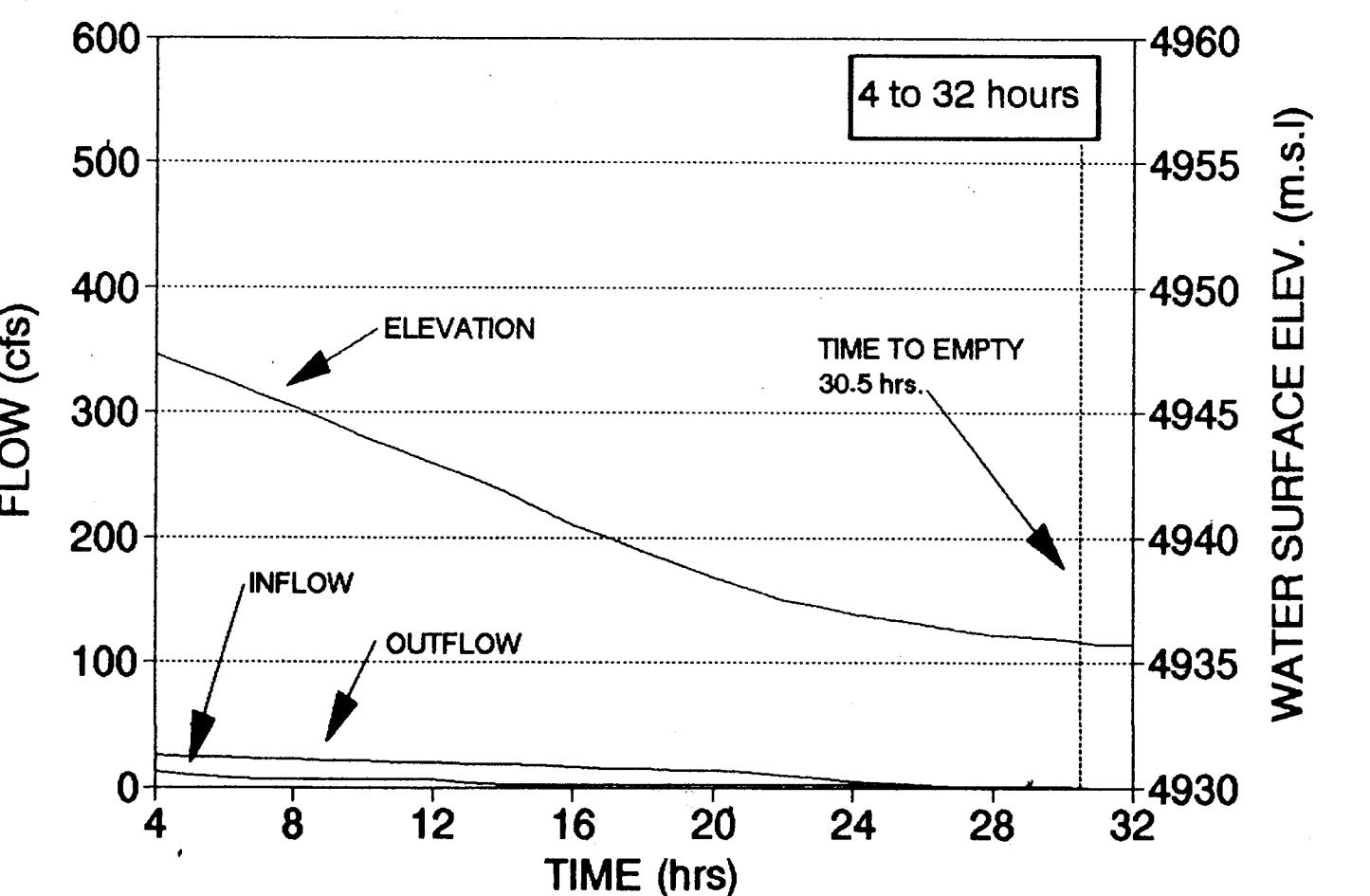
1. **Pipe SDP24** from SDMH42 to SDMH41 – Survey was unable to reach the pipe and did not record the invert at SDMH41. Solution was to obtain the pipe's slope ( $S=0.0012$ ) from the record drawing (COA #08-541-54, Sheet 9) and to calculate the invert based on the centerline distance between the two manholes.
2. **Pipe SDP27** from SDMH943 to SDMH942 – Survey was unable to obtain the pipe invert at SDMH942. Solution was to obtain the pipe's slope ( $S=0.004$ ) from the record drawing (COA #26-4165.90-93, Sheet 11) and to calculate the invert based on the centerline distance between the two manholes.
3. **Pipe SDP26** from SDMH146 to SDMH142 – Survey was unable to obtain the pipe invert at SDMH142 and the pipe slope was not shown in COA record drawings. Solution was to assume the pipe carried the same slope as SDP24 ( $S=0.0012$ ; Item #1 above) since both manholes are located adjacent to Commercial Street and near each other, and to calculate the invert based on the centerline distance between the two manholes on each of its ends.
4. **Pipe SDP25** from SDMH42 to SDMH51 – Survey was unable to reach the pipe and did not record the invert at SDMH42. Solution was to obtain the pipe's slope ( $S=0.0012$ ) from the record drawing (COA #08-541-54, Sheet 9) and to calculate the invert based on the centerline distance between the two manholes.
5. **Pipe SDP19** from SDMH51 to SDMH55 – Survey was unable to reach the pipe and did not record the invert at SDMH55. Solution was to obtain the pipe's slope ( $S=0.0012$ ) from the record drawing (COA #08-541-54, Sheet 9) and to calculate the invert based on the centerline distance between the two manholes.
6. **Pipe SDP22** from SDMH54 to SDMH55 – Survey was unable to reach the pipe and did not record the invert at SDMH55. Solution was to obtain the pipe's slope ( $S=0.0012$ ) from the record drawing (COA #08-541-54, Sheet 9) and to calculate the invert based on the centerline distance between the two manholes.



**100-YR 24-HR STORM**  
INFLOW, OUTFLOW, AND WATER SURFACE



**100-YR 24-HR STORM**  
INFLOW, OUTFLOW, AND WATER SURFACE



FILE-B:SBNOTES LAST REVISION=11/14/91

GENERAL NOTES

- ALL WORK DETAILED ON THESE PLANS TO BE PERFORMED UNDER CONTRACT SHALL, EXCEPT AS OTHERWISE STATED OR PROVIDED FOR HEREIN, BE CONSTRUCTED IN ACCORDANCE WITH THE CITY OF ALBUQUERQUE STANDARD SPECIFICATIONS FOR PUBLIC WORKS CONSTRUCTION (1986) INCLUDING UPDATES. FOR SHORT, THESE MAY ALSO BE REFERRED TO AS THE SPECIFICATIONS OR THE STANDARD SPECIFICATIONS.
  - IF THE CONTRACTOR IS UNCLEAR AS TO THE INTENT OF THE PLANS OR SPECIFICATIONS HE SHALL PROMPTLY DISCUSS THEM WITH THE ENGINEER.
  - TIMING OF WORK: SEE THE SUPPLEMENTAL GENERAL CONDITIONS AND SPECIFICATIONS FOR REQUIREMENTS FOR THE TIMING OF CONSTRUCTION.
  - MUCH OF THE WORK OF THIS PROJECT OCCURS IN OR NEAR ESTABLISHED RESIDENTIAL AREAS. THE CONTRACTOR SHALL CONDUCT HIS WORK IN SUCH A WAY AS TO AVOID UNNECESSARY DISRUPTION, INCLUDING BUT NOT LIMITED TO DISRUPTIONS CAUSED BY TRAFFIC, NOISE, AND DUST.
  - THE UTILITY LOCATIONS SHOWN ON THE PLANS ARE BASED ON RECORD DRAWINGS AND FIELD SPOTTINGS AND MUST BE CONSIDERED APPROXIMATE. TWO (2) WORKING DAYS PRIOR TO ANY EXCAVATION, CONTRACTOR MUST CONTACT NEW MEXICO ONE-CALL, 260-1990, FOR LOCATION OF EXISTING UTILITIES. THE ONE-CALL TICKET NUMBER FOR DESIGN OF THIS PROJECT WAS 910517 1429 0174.
  - ALL UTILITIES SHOWN AND ANY OTHER UTILITIES THAT ARE NOT SHOWN ON THE PLANS BUT ARE WITHIN THE CONSTRUCTION AREA SHALL BE LOCATED, IDENTIFIED AND PROTECTED BY THE CONTRACTOR AT ALL TIMES DURING CONSTRUCTION. THE CONTRACTOR SHALL VERIFY ALL DIMENSIONS AND CONDITIONS SHOWN ON THE DRAWINGS, INCLUDING THE HORIZONTAL AND VERTICAL LOCATION OF ALL OBSTRUCTIONS, PRIOR TO STARTING WORK AND SHALL NOTIFY THE ENGINEER OF ANY DISCREPANCIES OR INCONSISTENCIES SO THAT THE CONFLICT CAN BE RESOLVED WITH A MINIMUM AMOUNT OF DELAY.
  - UTILITIES COORDINATION: THE CONTRACTOR SHALL COORDINATE WITH THE UTILITIES INVOLVED BEFORE AND DURING WORK ON THIS PROJECT, AND SHALL INFORM THE ENGINEER OF ANY PROBLEMS AFFECTING THE DESIGN OR EXECUTION OF THE PROJECT. THE UTILITY CONTACT PERSON FOR THIS PROJECT ARE:
- | NAME OF CONTACT                      | PHONE           |
|--------------------------------------|-----------------|
| PUBLIC SERVICE COMPANY OF NEW MEXICO | 646-3470        |
| BRENT SWITZER                        | 761-7775        |
| TIM CYNOVA                           | 245-6410        |
| U.S. WEST COMMUNICATIONS             | 761-6220        |
| JONES INTERCABLE                     | ISHMAEL PANTOJA |
| (AT&T)                               | 642-2605        |

- THE GAS CO. SHALL MOVE OR REMOVE ITS LINES AS REQUIRED IN ADVANCE OF CONSTRUCTION.
- HIGH PRESSURE AND VERY HIGH PRESSURE GAS LINES EXIST CLOSE TO THE WORK IN SEVERAL PLACES, PARTICULARLY ALONG COMMERCIAL STREET.
- THE CONTRACTOR SHALL VERIFY THAT THE SANITARY SEWER LINE IN BROADWAY BLVD. SHOWN ON THE PLANS AS BEING ABANDONED ACTUALLY IS ABANDONED BEFORE REMOVING OR DISTURBING IT.
- ALL UTILITIES WITHIN THIS PROJECT SHALL REMAIN IN OPERATION UNLESS OTHERWISE INDICATED ON THE PLANS.
- DESIGN NOTE: THE CATCH BASINS ON THE WEST SIDE OF BROADWAY BLVD. SOUTH OF SANTA FE AVE. WILL CONTINUE TO BE CONNECTED TO THE EXISTING 72-INCH STORM DRAIN. CATCH BASINS ON THE EAST SIDE WILL CONNECT TO NEW STORM DRAINS AS SHOWN ON THE PLANS.
- THREE (3) WORKING DAYS PRIOR TO BEGINNING CONSTRUCTION THE CONTRACTOR SHALL SUBMIT TO THE CONSTRUCTION CO-ORDINATION DIVISION A DETAILED CONSTRUCTION SCHEDULE. TWO (2) WORKING DAYS PRIOR TO CONSTRUCTION THE CONTRACTOR SHALL OBTAIN A BARRICAADING PERMIT FROM THE CONSTRUCTION CO-ORDINATION DIVISION. THE CONTRACTOR SHALL NOTIFY THE BARRICADE ENGINEER (768-2551) PRIOR TO OCCUPYING AN INTERSECTION. SEE SECTION 19 OF THE SPECIFICATIONS.
- THE CONTRACTOR SHALL COORDINATE TRAFFIC CONTROL WITH THE CONSTRUCTION COORDINATOR OF THE CITY OF ALBUQUERQUE PUBLIC WORKS DEPT.
- THE CONTRACTOR SHALL MAINTAIN THE TRAFFIC CONTROL SIGNAL (STOP LIGHT) AT BROADWAY BLVD. AND PACIFIC AVE. IN OPERATION AT ALL TIMES UNLESS OTHERWISE DIRECTED BY THE ENGINEER. THE CONTRACTOR SHALL REPLACE ALL SIGNAL LOOPS TO THEIR ORIGINAL LOCATION, KIND, AND OPERATION. AT ANY TIME WHEN A STOP SIGN HAS BEEN REMOVED OR OBSCURED (E.G. SANTA FE AVE WEST OF BROADWAY BLVD) THE CONTRACTOR SHALL PROVIDE AN APPROPRIATE TEMPORARY STOP SIGN UNLESS OTHERWISE DIRECTED BY THE ENGINEER.
- ALL PAVEMENT STRIPING WITHIN THIS PROJECT SHALL BE REPLACED BY THE CONTRACTOR IN LOCATION AND IN KIND AS EXISTING, OR AS INDICATED BY THIS PLAN SET AS DIRECTED BY THE ENGINEER.

- SURVEY MONUMENTS: THE CONTRACTOR SHALL NOTIFY THE ENGINEER NOT LESS THAN SEVEN (7) DAYS PRIOR TO STARTING WORK IN ORDER THAT THE ENGINEER MAY TAKE NECESSARY MEASURES TO INSURE THE PRESERVATION OF SURVEY MONUMENTS. CONTRACTOR SHALL NOT DISTURB PERMANENT SURVEY MONUMENTS WITHOUT THE CONSENT OF THE ENGINEER AND SHALL NOTIFY THE ENGINEER AND BEAR THE EXPENSE OF REPLACING ANY THAT MAY BE DISTURBED WITHOUT PERMISSION. REPLACEMENT SHALL BE DONE ONLY BY THE ENGINEER. WHEN A CHANGE IS MADE IN THE FINISHED ELEVATION OF THE PAVEMENT OF ANY ROADWAY IN WHICH A PERMANENT SURVEY MONUMENT IS LOCATED, CONTRACTOR SHALL, AT HIS OWN EXPENSE, ADJUST THE MONUMENT COVER TO THE NEW GRADE UNLESS OTHERWISE SPECIFIED. REFER TO SECTION 4.4 OF THE SPECIFICATIONS.
- BEARINGS & DISTANCES: THE DETENTION BASIN IS DESIGNED TO BE LAID OUT USING PROJECT BEARINGS AND DISTANCES AS SHOWN ON THE PLANS. LINE 1 ON SHEET 5 IS THE MAIN LAYOUT LINE FOR THE PROJECT. THE PLAT WAS CREATED USING DIFFERENT C.O.A. MONUMENTATION. SHEET 5 CONTAINS BOTH PLAT AND PROJECT DESCRIPTIONS OF LINE 1. THE PROJECT LIES WITHIN THE PLAT BOUNDARIES.
- LEFT AND RIGHT MEAN LEFT AND RIGHT WHEN LOOKING IN THE DIRECTION OF INCREASING STATIONING.
- PROFILE GRADE IS GIVEN AT RIM ELEVATION OF EXISTING STORM DRAIN MANHOLES IN BROADWAY, WILLIAM, PACIFIC, AND COMMERCIAL. THE CONTRACTOR SHALL DOCUMENT EXISTING PAVEMENT GRADES PRIOR TO CONSTRUCTION AND SHALL RESTORE EXISTING GRADES WHEN REPAVING UNLESS OTHERWISE SHOWN ON THE PLANS OR OTHERWISE DIRECTED BY THE ENGINEER.
- CONTROL OF DUST: THE CONTRACTOR SHALL BE FAMILIAR WITH THE RELEVANT REGULATIONS OF THE ALBUQUERQUE/BERNALILLO COUNTY AIR QUALITY CONTROL BOARD. AT A MINIMUM, THE CONTRACTOR SHALL MAKE A VOLUNTARY WORK STOPPAGE WHEN WIND AND SOIL CONDITIONS ARE SUCH THAT HE CANNOT CONFINE THE DUST TO THE SITE. SOIL CONDITIONS MAY REQUIRE THAT ALL EXCAVATION LIFTS BE PRE-WETTED, PARTICULARLY IF PADDLE SCRAPPERS ARE USED. ALL TRUCKS SHALL BE PROPERLY MAINTAINED AND EQUIPPED AND ALL LOADS ENTERING AND LEAVING THE SITE SHALL BE COVERED. LEAVE ALL DISTURBED AREAS IN STABLE CONDITION AT THE END OF EACH WORK DAY. THE CONTRACTOR SHALL CLEAN UP ADJACENT PAVED STREETS DIRTYED BY HIS OPERATIONS, DAILY OR AS DIRECTED BY THE ENGINEER, BY WET BROOMING OR OTHER APPROVED MEANS. DRY BROOMING WILL NOT BE PERMITTED.
- CONTROL OF WATER: THE CONTRACTOR SHALL CONTROL STORM AND NUISANCE WATER WITHIN THE PROJECT AREA. IN PARTICULAR, THE CONTRACTOR SHALL ENSURE THAT WATER FROM STORM DRAINAGE SYSTEMS WITHIN THE PROJECT AREA (PARTICULARLY THE BROADWAY STORM DRAINAGE SYSTEM) ARE DIRECTED SAFELY AWAY FROM THE WORK AREA. SEE THE CONTRACT DOCUMENTS INCLUDING SUPPLEMENTAL GENERAL CONDITIONS AND TECHNICAL SPECIFICATIONS, SPECIFICALLY STS SECTION 625.

- TRENCHING: SEE THE SPECIFICATIONS AND SUPPLEMENTAL SPECIFICATIONS, INCLUDING SECTION 701.6.5. THE DEPTH OF NEW PIPING, THE NEARNESS OF OTHER UTILITIES AND STRUCTURES WHICH THE CONTRACTOR IS REQUIRED TO PROTECT, PAY LIMITS, AND TRAFFIC CONCERN INDICATE THAT TRENCH BOXES, SHORING, OR OTHER SUPPORT MAY BE APPROPRIATE IN MUCH OF THIS PROJECT. BACKFILL BETWEEN A TRENCH SHIELD (TRENCH BOX), IF USED, AND THE TRENCH WALL IS REQUIRED IN SOME LOCATIONS.
- VIBRATORY COMPACTION: SEE THE SUPPLEMENTAL SPECIFICATIONS FOR LIMITATIONS.
- PIPE MATERIALS AND INSTALLATION: SEE THE PLANS AND SPECIFICATIONS FOR ADDITIONAL REQUIREMENTS. ALL CONCRETE PIPE IS CLASS III REINFORCED CONCRETE PIPE EXCEPT AS OTHERWISE NOTED ON THE PLANS.
- LEAN FILL: LEAN FILL CONSTRUCTION PER STS SEC.207 IS PERMITTED AT THE CONTRACTOR'S OPTION UPON AUTHORIZATION BY THE ENGINEER.
- PIPE CROSSINGS: ALL PIPING AND OTHER UTILITIES THAT CROSS WITH LESS THAN EIGHT (8) INCHES OF CLEARANCE SHALL HAVE CONCRETE PLACED BETWEEN THEM FOR A DISTANCE OF 2.5 FEET ON EITHER SIDE OF THE CROSSING SO AS TO PROVIDE FIRM AND UNIFORM SUPPORT EXCEPT AS OTHERWISE DIRECTED BY THE ENGINEER.
- PIPE ABANDONMENT: ALL LATERALS AND OTHER PIPE THAT IS TO BE ABANDONED IN PLACE SHALL HAVE ALL OPEN ENDS SECURELY PLUGGED WITH CONCRETE. ANY PIPE THAT IS DAMAGED, AND ANY OTHER PIPE DESIGNATED BY THE ENGINEER, SHALL BE REMOVED AND THE REMAINING OPEN ENDS SHALL BE PLUGGED.
- SALVAGE: SALVAGE ALL MANHOLE RINGS AND COVERS, CATCH BASIN GRATES, AND ANY WATERLINE OR STORM DRAIN COMPONENTS AS DIRECTED BY THE ENGINEER, TO THE PINO AVENUE CITY YARDS. THIS WORK IS INCIDENTAL TO CONSTRUCTION.
- TEMPORARY PAVING: TEMPORARY PAVING AS NEEDED SHALL BE PROVIDED PER THE DETAIL SHOWN ON SHEET 13 AND THE STANDARD SPECIFICATIONS. APPROXIMATELY 80 SQ FT OF TEMPORARY PAVING MAY BE NEEDED AT BROADWAY BLVD. AND SANTA FE AVE. TEMPORARY PAVING SHALL BE INCIDENTAL TO CONSTRUCTION AND NO SEPARATE PAYMENT WILL BE MADE THEREFORE.

LEGEND		AS BUILT INFORMATION	
DASHED SYMBOLS DENOTE EXISTING ITEMS		CONTRACTOR APPROVALS DATE	
--W--	EXISTING WATER LINE	STAKED BY COA	DATE 12/24/91
-W-	PROPOSED WATER LINE	FIELD SURVEY BY COA	DATE 12/24/91
/ \	EXISTING CONTOURS	DRAWINGS BY COA	DATE 12/24/91
/ \	PROPOSED CONTOURS	DRAWINGS BY COA	DATE 12/24/91
57.50	EXISTING ELEVATION	DRAWINGS BY COA	DATE 12/24/91
50.00	PROPOSED ELEVATION	DRAWINGS BY COA	DATE 12/24/91
←	FLOW DIRECTION	DRAWINGS BY COA	DATE 12/24/91
▼	SLOPE INDICATOR	DRAWINGS BY COA	DATE 12/24/91
○	EXISTING FIRE HYDRANT	DRAWINGS BY COA	DATE 12/24/91
○ PP	POWER POLE	DRAWINGS BY COA	DATE 12/24/91
■■■	STORM DRAIN INLET	DRAWINGS BY COA	DATE 12/24/91
=====	CURB AND GUTTER	DRAWINGS BY COA	DATE 12/24/91
○	MANHOLE	DRAWINGS BY COA	DATE 12/24/91
***	FENCE	DRAWINGS BY COA	DATE 12/24/91
—OE—	OVERHEAD POWER LINE	DRAWINGS BY COA	DATE 12/24/91
—UT—	UNDRGRND. TELEPHONE	DRAWINGS BY COA	DATE 12/24/91
—I—	IRRIGATION LINE	DRAWINGS BY COA	DATE 12/24/91
—G—	GAS LINE	DRAWINGS BY COA	DATE 12/24/91
—PL—	PROPERTY LINE	DRAWINGS BY COA	DATE 12/24/91
—CL—	CENTERLINE	DRAWINGS BY COA	DATE 12/24/91
... .	EXTENSION OF PL	DRAWINGS BY COA	DATE 12/24/91
■■■■	ASPHALT PAVEMENT	DRAWINGS BY COA	DATE 12/24/91
■■■■■	EARTH	DRAWINGS BY COA	DATE 12/24/91
■■■■■■	CONCRETE	DRAWINGS BY COA	DATE 12/24/91
△	TRANSITION AREA	DRAWINGS BY COA	DATE 12/24/91

BENCH MARKS

AN ACS CONTROL CAP 9-L14 LOCATED AT THE INTERSECTION OF BROADWAY AND STADIUM

NO. BY DATE

1 RESOURCE 4/91 X=381.804.60 E=1,480,772.92

TECHNOLOGY, INC 7/91 AN ACS BENCHMARK K14-K15 LOCATED AT THE INTERSECTION OF BROADWAY AND PACIFIC

NO. BY DATE

1 BENCH 10/91

NO. BY DATE

1 BENCH 11/91

AS BUILT INFORMATION

CONTRACTOR APPROVALS DATE

NO. BY DATE

1 BENCH 12/24/91

NO. BY DATE

1 BENCH 12/24/91

NO. BY DATE

1 BENCH 12/24/91

NO. BY DATE

ABBREVIATIONS

T.C. TOP OF CURB  
T.O.C. TOP OF CONCRETE  
T.O.W. TOP OF WALL  
E.O.P. EDGE OF PAVEMENT  
END OF PROJECT

B.O.P. BEGINNING OF PROJECT  
F.F. FINISHED FLOOR  
F.G. FINISHED GRADE  
P.G. PROPERTY GRADE  
C. CENTERLINE

V.P.T. (VERTICAL) POINT OF TANGENT  
P.I. POINT OF INTERSECTION  
F.C. FACE OF CURB  
T.G. TOP OF GRADE  
S.D. (COA) STANDARD DRAWING  
M.H. MANHOLE  
REM. REMOVE  
DISP. DISPOSE  
L.F. LINEAR FEET  
EXIST. EXISTING  
S.A.S. SANITARY SEWER  
R/W. RIGHT OF WAY  
F.L. FLOW LINE  
F.C. FACE OF CURB  
H.G.I. HYDRAULIC GRADE LINE  
R.C.P. REINFORCED CONCRETE PIPE  
S.D. STORM DRAIN

(V.P.I.) (VERTICAL) POINT OF INTERSECTION  
(V.P.C.) (VERTICAL) POINT CURVATURE  
O.C. ON CENTER

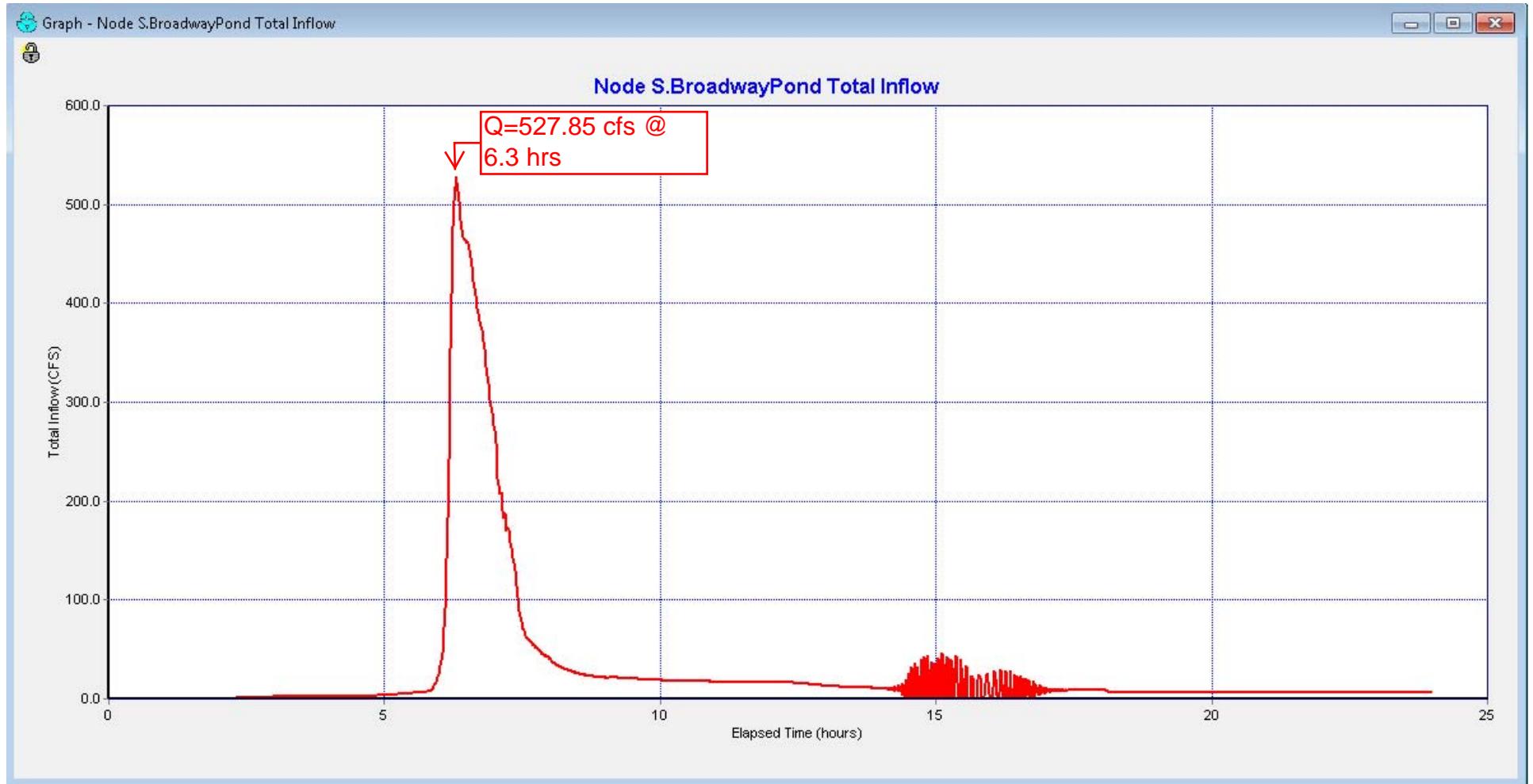
DESIGNED BY T.H.G. DATE 10/91  
DRAWN BY B.G. DATE 10/91  
CHECKED BY E.V.D. DATE 11/91

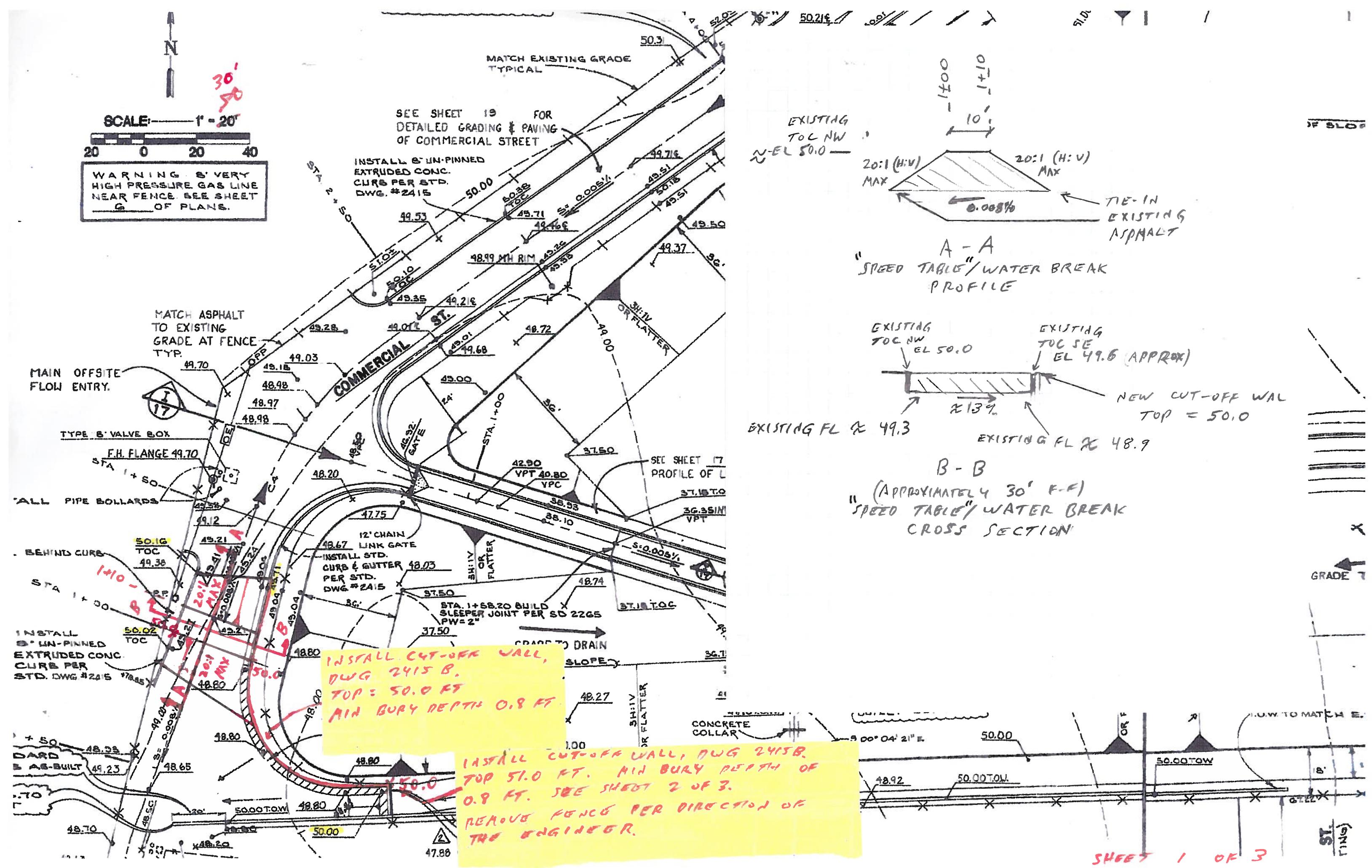
**Resource Technology, Inc.**  
ENGINEERS & ENVIRONMENTAL SCIENTISTS  
2128 OSUWA NE - SUITE 200, ALBUQUERQUE, NEW MEXICO 87113  
TELEPHONE: (505) 345-3718

CITY OF ALBUQUERQUE  
PUBLIC WORKS DEPARTMENT  
ENGINEERING GROUP

**TITLE:  
BASIN LOCATION AND HYDROGRAPHS**

APPROVALS ENGINEER DATE APPROVALS ENGINEER DATE  
DRC CHAIRMAN Roger H. 2-5-92 WATER  
TRANSPORT. B. B. 1-22-92 WASTE WATER  
HYDROLOGY B. B. 1-17-92

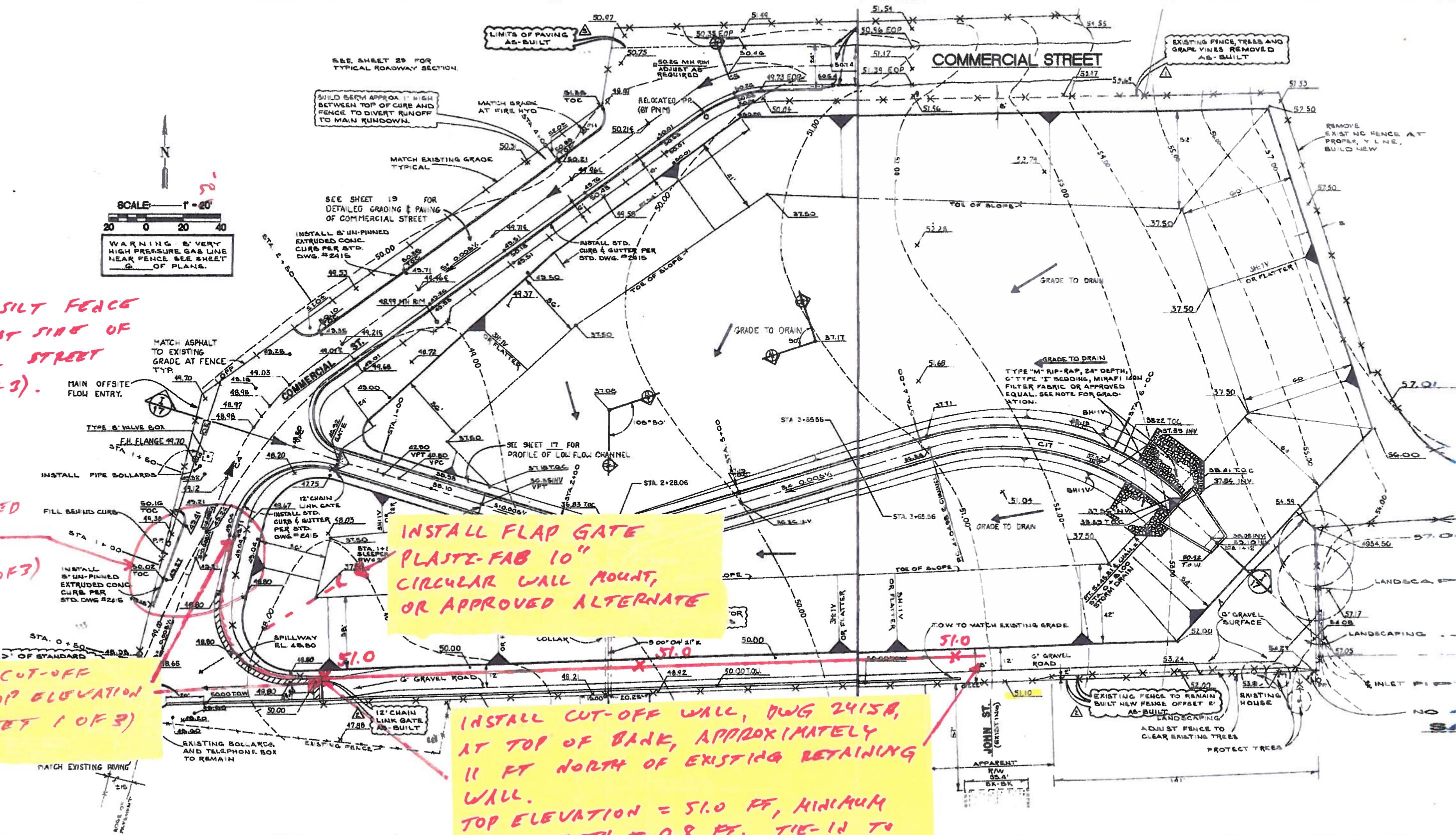




- INSTALL SILT FENCE  
ALONG WEST SIDE OF  
COMMERCIAL STREET  
(SHEET 3 OF 3). MAIN OFF SITE  
FLOW ENTRY.

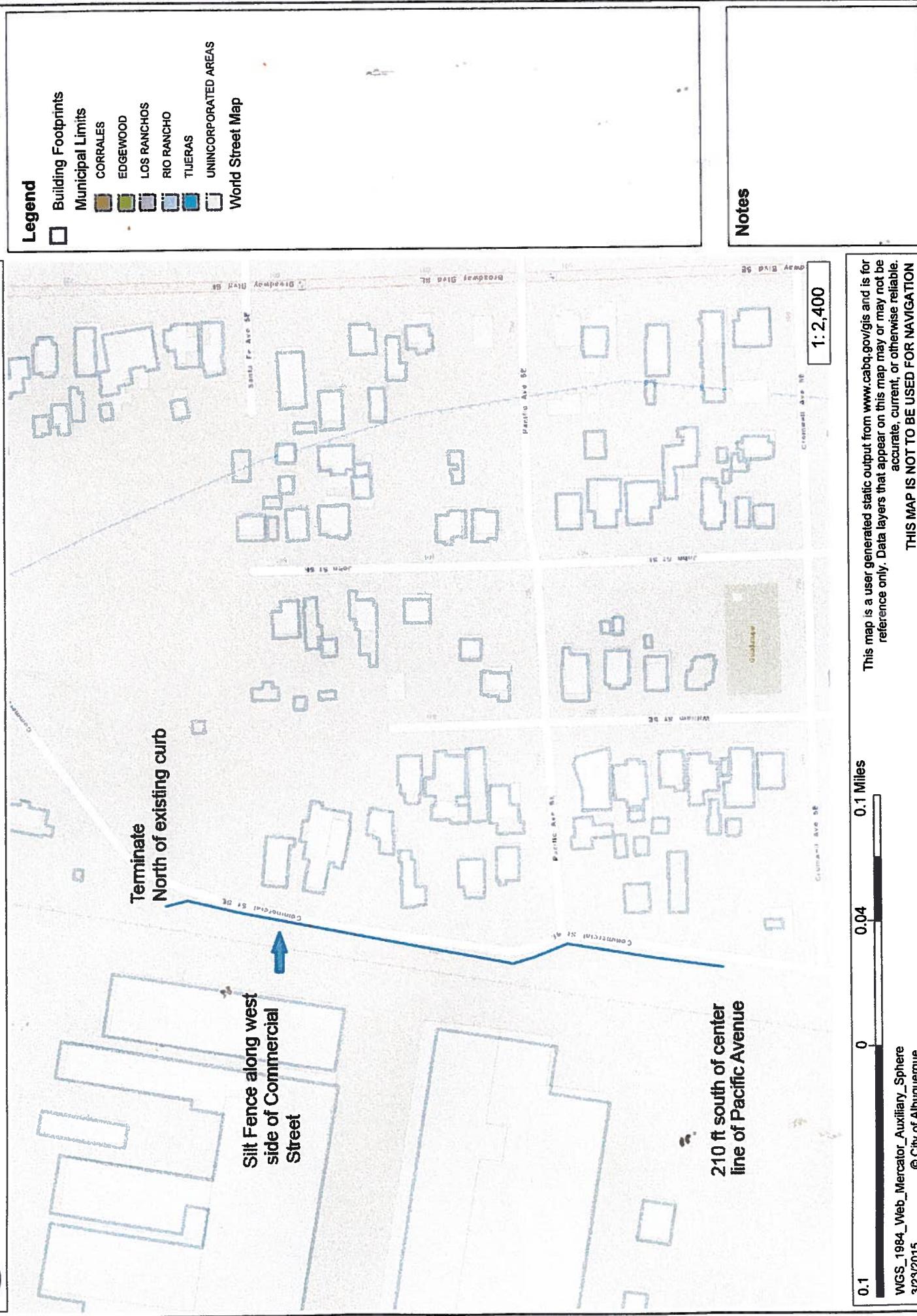
PROPOSED  
WATER  
BREAK —  
(SHOOT 1 OF 3)

PROPOSED CUT-OFF  
WALL, TOP ELEVATION  
50.0, (SUITE 1 OF 3)



15.000 - 20.250  
INSTALL CUT-OFF WALL, DWG 2415A,  
AT TOP OF BANK, APPROXIMATELY  
11 FT NORTH OF EXISTING RETAINING  
WALL.  
TOP ELEVATION = 51.0 FT, MINIMUM  
BURY DEPTH = 0.8 FT. TIE-ID TO  
EXISTING GROUND AT EAST END.

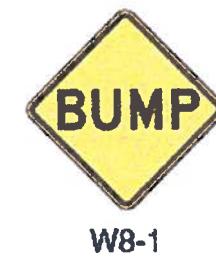
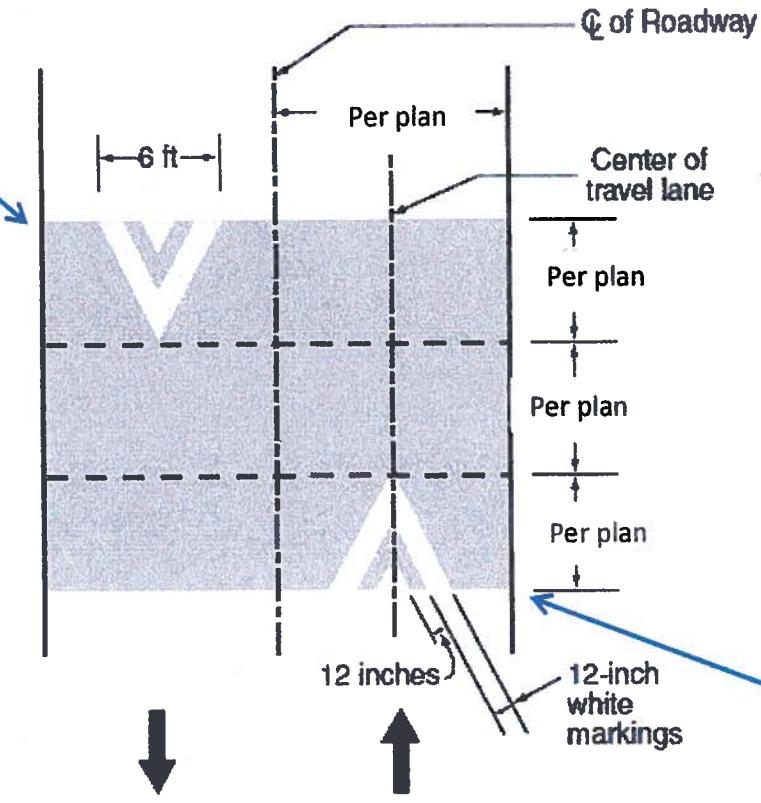
## City of Albuquerque



## SIGNING AND STRIPING FOR WATER BREAK



W8-1



W8-1

Located on Commercial Street 100 feet north and south of water break per the direction of the Engineer:



W8-1



W16-9P

part of a retrofit in 2002. Previously, they were in service as activated sludge pumps at the City of Albuquerque's Southside Water Reclamation Facility.

At maximum capacity, the pumps are capable of delivering between 13,900 and 32,000 gpm depending on the total dynamic head (TDH) required. The flow rate depends on the wet well's water level, which causes static lift to range from approximately 16 feet to 35 feet. A larger static lift corresponds to a smaller flow rate as the pumps must work harder to lift the water. The lift pumps may be operated in different configurations to suit variable pumping requirements. Pump No. 1 should not be operated by itself, but rather as an auxiliary to the two (2) Flygt pumps when more capacity is needed. This is because the required flow rate and TDH may be outside the manufacturer's recommended operating range under expected conditions. Refer to Appendix C for the manufacturer's pump curve and data. Also included in Appendix C are an additional set of curves showing approximate system hydraulics and possible pump operating combinations.

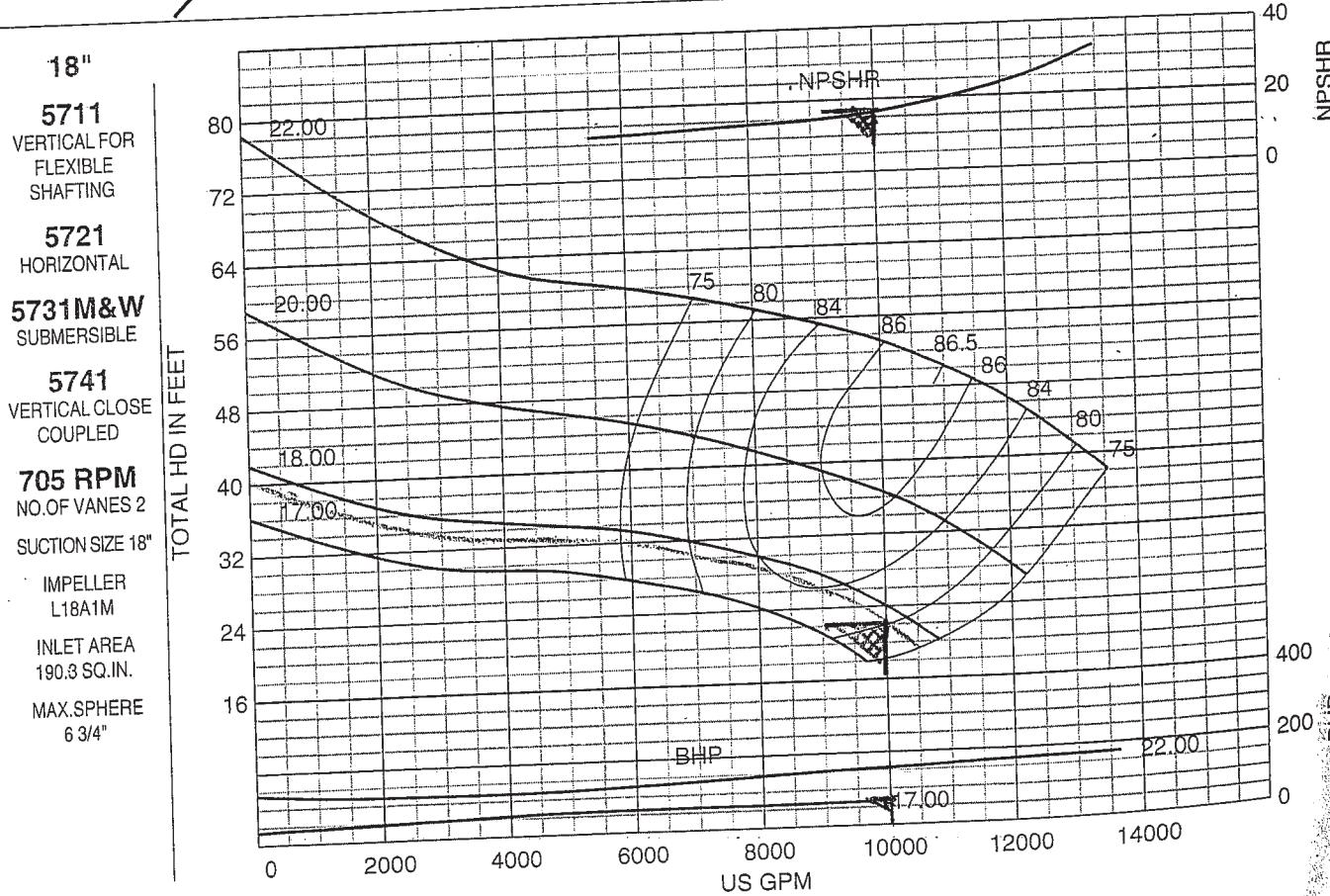
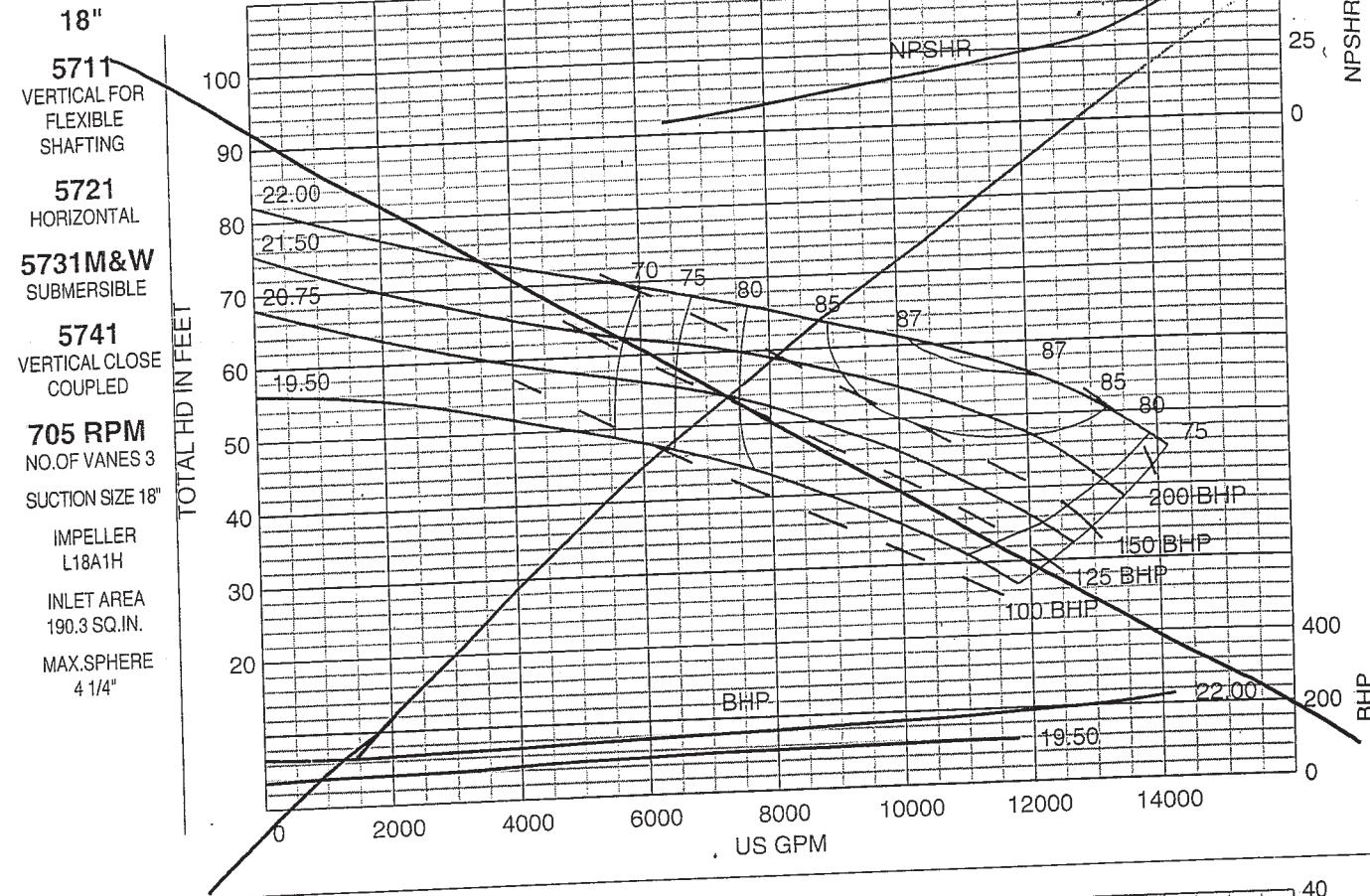
The manufacturer's pump curves for the FM 5710-18 and Flygt CT3500 indicate that the pumps require 18 feet and 16 feet of net positive suction head (NPSH) to prevent cavitation, respectively. When cavitation occurs, the pump runs noisily and sounds as if it were pumping marbles. Prolonged cavitation will result in pitting of the impeller and volute. Stormwater periods tend to be brief and some cavitation is tolerable over the life of the pump.

At the station's site elevation, the quantity of available suction head is greater than the required quantity. Therefore, the pumps are capable of pulling between 4.5 (FM 5710) and 8.5 (Flygt CT3500) feet of suction lift. That is, the pumps could pull water through suction piping from a wet well at a lower elevation with a water level approximately 4.5 to 8.5 feet below the elevation of the impeller. The station is designed such that the pumps pull less suction lift than this amount.

The Flygt pumps are installed in Flygt's "T" configuration, which indicates it is installed vertically in a dry-pit with permanent (flanged) suction and discharge piping.

412

Pump I

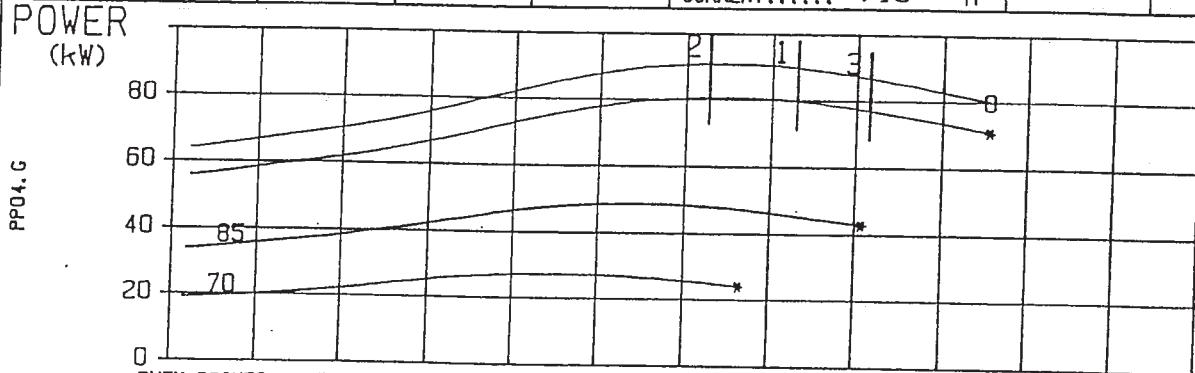




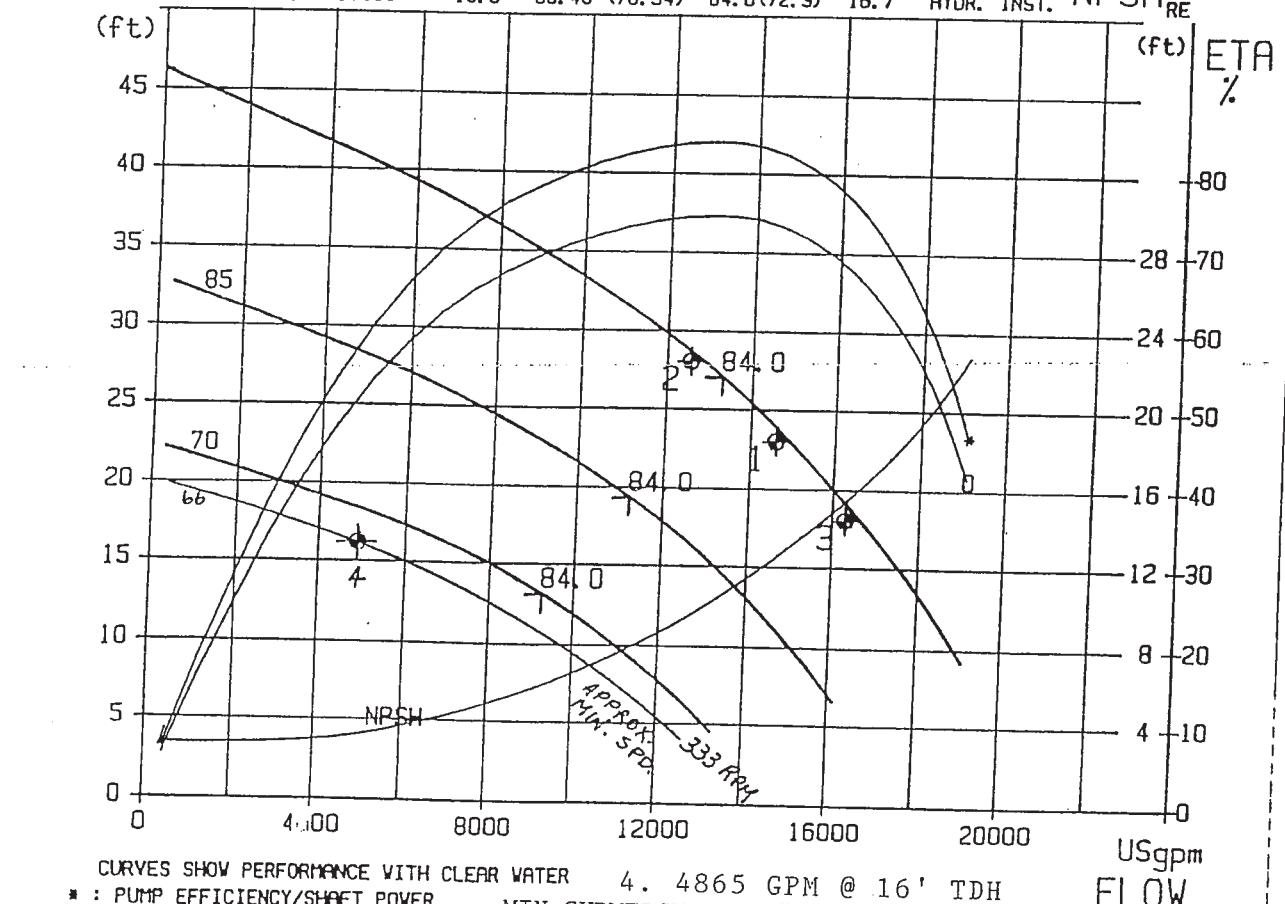
# PERFORMANCE CURVE

Pump 2 #3

DATE 1990-11-08	PROJECT C. D. M. ALBQ. WWTP - R. A. S. PUMPS	ISSUE 5	PROD 3500
NO. OF BLADES..... 3	TOT. MOM. OF INERTIA..... 12.99 KGM <sup>2</sup>	POLES 14	FREQ 60 HZ
IMPELLER THROUGHLET... 110*250 RECTA.	RATED SPEED..... 505 RPM	VOLTAGE..... 460 V	MOTOR SHAFT POWER..... 100 KW
		STARTING TORQUE..... 1715 NM	IMPELLER DIAMETER 595 MM
	1/1-LOAD MOTOR COS FI 0.63	3/4-LOAD MOTOR EFFICIENCY 88.5%	HOTORTYPE 51-56-14AA/3
	1/2-LOAD GEAR EFFICIENCY 86.5%	RATED CURRENT..... 225 A	GEARTYPE RATIO
		STARTING CURRENT..... 715 A	



DUTY-POINTS	FLOW (USgpm)	HEAD (ft)	POWER (kW)	EFF. (%)	NPSH (ft)	GURANTY
1.	14600	23.0	89.64 (79.48)	70.7 (79.7)	13.4	HYDR. INST.
2.	12540	28.0	91.08 (80.79)	72.7 (82.0)	10.2	HYDR. INST.
3.	16300	18.0	86.48 (76.54)	64.0 (72.3)	16.7	HYDR. INST.



CURVES SHOW PERFORMANCE WITH CLEAR WATER

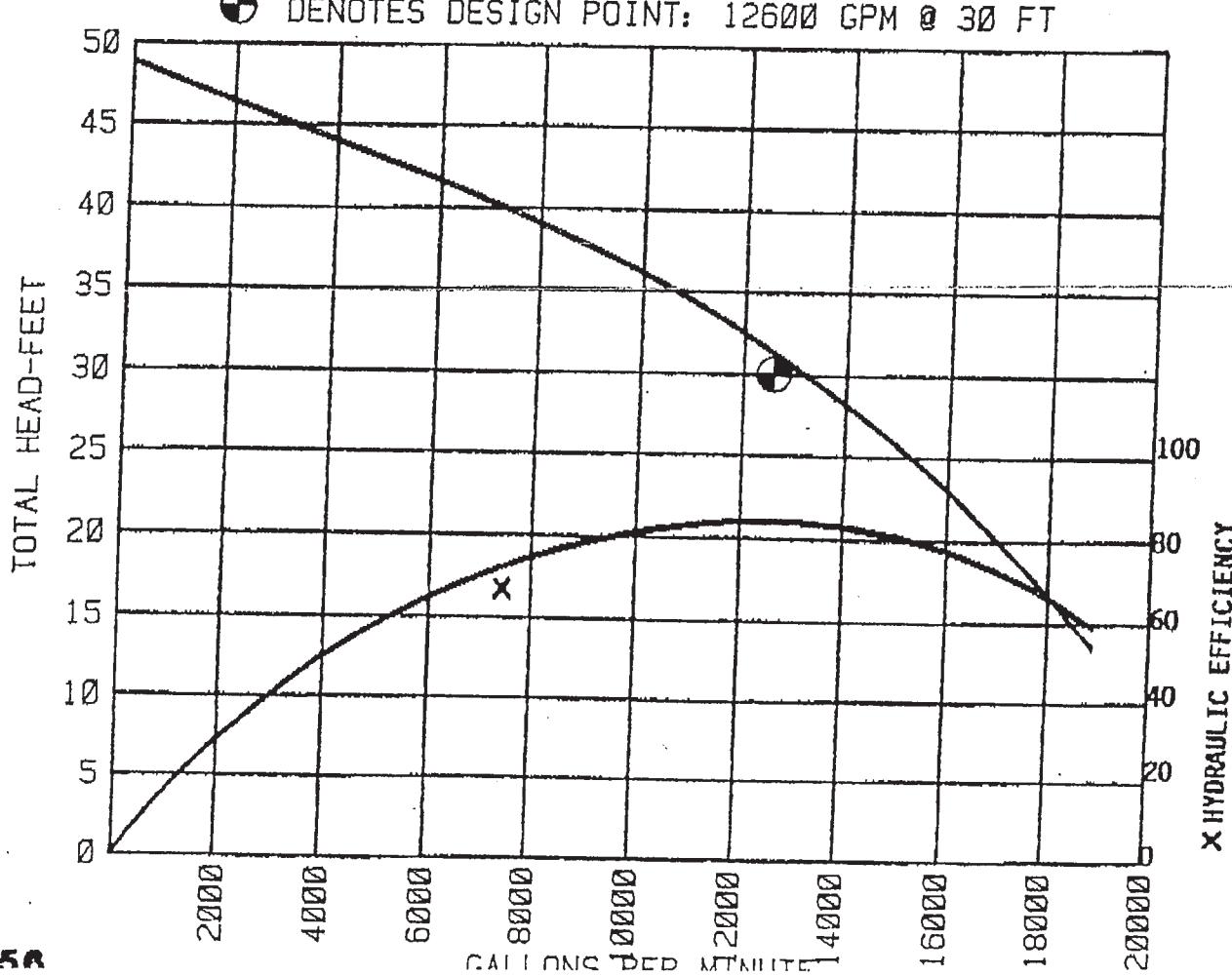
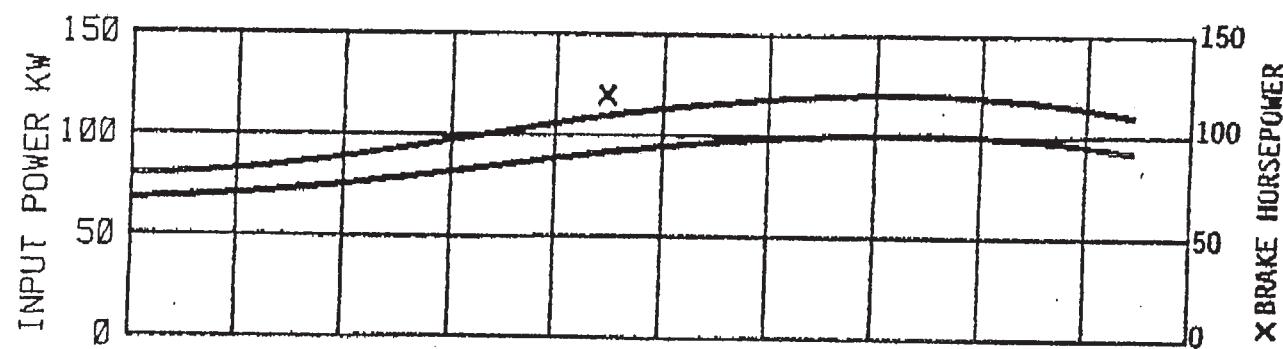
#### PUMP EFFICIENCY/SHEET POWER

Pump 2 & 3

SIMILAR UNIT TEST DATA

FLYGT
CUSTOMER REF. NO.
James Cooke
Squaw Peak Pkwy
Phoenix, AZ
REP.
James Cooke

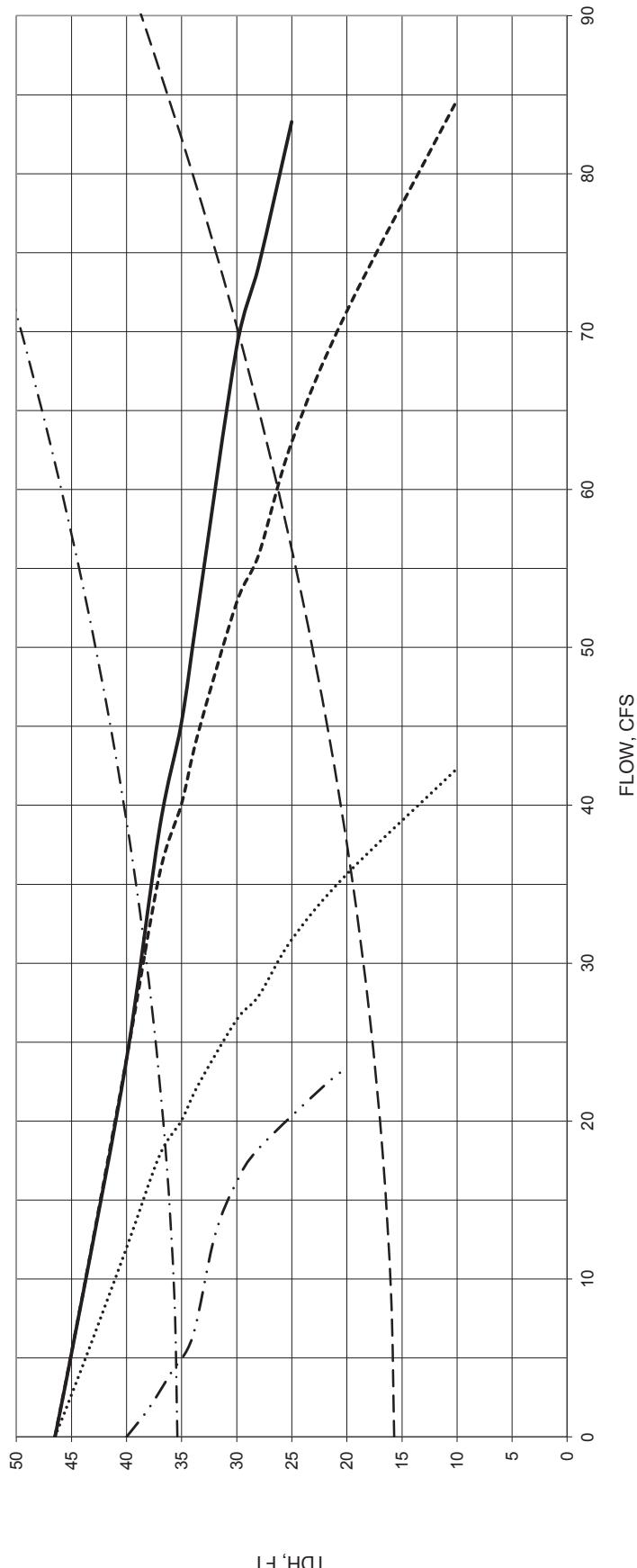
CERTIFIED TEST CURVE NO. 5950			
SER. NO. 880-8850643	KW 112	FLYGT REF. NO. C - 21868A	
HP 134	PH 3	VOLT. 480	PUMP MODEL CP-3500X
RTD. RPM 505	IMPLR. 1430	SIZE 20"	
CERTIFICATION THIS TEST WAS CONDUCTED AT A FLYGT CORPORATION TEST FACILITY USING CLEAN WATER AT AMBIENT TEMP. (60-80°F). FLOW, HEAD AND POWER READINGS WERE TAKEN FROM ELECTRONIC METERING EQUIPMENT. ACCURACY OF THE TEST EQUIPMENT CONFIRMED BY PERIODIC CALIBRATION.			
T.A.# 6662	TEST BY Lindas	PREP. BY A. Hannis	WITNESSED BY
	DATE 6/29/88	DATE 7/29/88	DATE



# LIFT PUMPS, 36" Pipe

Two Exist. CT3500/860, 1430, 595 mm 134 HP  
One Exist VT SN 796118, 75 HP

— · System Head Lo Well ..... One 134 HP Pump  
— — System Head Hi Well - - - Two 134 HP Pumps  
· · · One 75 HP Pump — All Three Pumps



## **APPENDIX C**

## **SWMM MODELS**

## SWMM Model Input - Existing System

### [TITLE]

```
[OPTIONS]
FLOW_UNITS          CFS
INFILTRATION       HORTON
FLOW_ROUTING        DYNWAVE
START_DATE          05/06/2016
START_TIME          00:00:00
REPORT_START_DATE   05/06/2016
REPORT_START_TIME   00:00:00
END_DATE            05/06/2016
END_TIME             06:00:00
SWEEP_START         01/01
SWEEP_END           12/31
DRY_DAYS             0
REPORT_STEP          00:00:15
WET_STEP              00:05:00
DRY_STEP              01:00:00
ROUTING_STEP         0:00:15
ALLOW_PONDING        NO
INERTIAL_DAMPING    PARTIAL
VARIABLE_STEP        0.75
LENGTHENING_STEP     0
MIN_SURFAREA        12.557
NORMAL_FLOW_LIMITED BOTH
SKIP_STEADY_STATE   NO
FORCE_MAIN_EQUATION  H-W
LINK_OFFSETS         DEPTH
MIN_SLOPE            0.001
```

### [EVAPORATION]

```
; ;Type      Parameters
; ;----- -----
CONSTANT            0.0
DRY_ONLY            NO
```

### [JUNCTIONS]

	Invert ; ;Name Elev.	Max. Depth	Init. Depth	Surcharge Depth	Ponded Area
DI2024	4945.64	3.87	0	0	0
;					
DI2046	4945.39	3.5	0	0	0
;					
K14941	4937.35	11.75	0	0	0
;					
K14942	4936.82	13.35	0	0	0
K14943	4946.02	3.75	0	0	0
K14944	4937.6	11.85	0	0	0
;					
K14945	4937.94	12.2	0	0	0
;					
K14951	4945.21	4.6	0	0	0
;					
K14952	4943.04	6.05	0	0	0
;					
K14953	4945.46	5.57	0	0	0
;					
K14954	4937.94	11.81	0	0	0
;					
L14041	4936.6	11.17	0	0	0
L14042	4943.53	3.7	0	0	0
L14051	4944.35	2.95	0	0	0
;					
L14052	4941.40	7.23	0	0	0
;					
L14053	4944.98	5.39	0	0	0
;					
L14054	4944.59	7.27	0	0	0
;					
L14055	4939.88	9.56	0	0	0
L14141	4936.08	12.24	0	0	0
;					
L14142	4935.24	13.23	0	0	0
;					
L14143	4935.66	12.35	0	0	0
L14146	4943.06	5.41	0	0	0

;MH rim elevation adj. to lowest connecting inlet grate elevation (DI #2210 - 4947.80) plus 8" curb height.  
 L14151 4939.08 9.82 0 0 0  
 ;MH rim elevation adj. to lowest connecting inlet grate elevation (DI #2180 - 4947.83) plus 8" curb height.  
 L14152 4937.58 11.05 0 0 0  
 L14999 4937.3 11.28 0 0 0  
 L14JB145 4934.73 13.55 0 0 0

**[OUTFALLS]**  
 ;  
 ;:Name Invert Elev. Outfall Type Stage/Table Time Series Tide Gate  
 ;-----  
 L14JB2 4952.21 FREE NO

**[STORAGE]**  
 ;  
 ;:Name Invert Elev. Max. Depth Init. Depth Storage Curve Curve Params Ponded Area Evap. Frac. Infiltration Paramet  
 ;-----  
 L14PS1 4926.99 22.67 0 TABULAR L14PS1 0 0  
 SBroadwayPond 4938.42 15.25 15.25 TABULAR SB.DepthArea 0 0

**[CONDUITS]**  
 ;  
 ;:Name Inlet Node Outlet Node Length Manning N Inlet Offset Outlet Offset Init. Flow Max. Flow  
 ;-----  
 Bell\_A L14JB145 L14143 86 0.017 12.88 11.44 0 0  
 Bell\_B L14999 L14JB145 261 0.017 10.53 12.88 0 0  
 Bell\_C L14152 L14999 35 0.017 10.25 10.53 0 0  
 Commercial\_1 K14942 L14041 393 0.017 12.68 10.29 0 0  
 Commercial\_2 L14142 L14041 350 0.017 12.47 10.29 0 0  
 Commercial\_Sag K14941 DI2046 92.157 0.017 10.86 2.56 0 0  
 Cromwell\_Junct K14942 K14943 48 0.017 12.68 3.01 0 0  
 E.Cromwell K14953 K14952 170 0.017 4.71 5.28 0 0  
 E.Garfield L14054 L14055 244 0.017 6.6 8.71 0 0  
 Lewis\_Junct L14142 L14146 35 0.017 12.47 4.65 0 0  
 Link-13 DI2024 K14951 18.41 0.013 0 0.35 0 0  
 Link-14 DI2046 K14944 46.09 0.013 0 0.15 0 0  
 N.Commercial K14942 K14941 234 0.017 12.68 10.86 0 0  
 N.John K14953 L14053 191 0.017 4.71 4.91 0 0  
 N.William DI2024 K14952 355 0.017 3.02 5.28 0 0  
 Pacific K14945 DI2046 268 0.017 11.53 2.56 0 0  
 Pacific\_William K14954 K14951 34 0.017 11.14 3.45 0 0  
 S.Commercial\_A L14142 L14141 330 0.017 12.47 11.57 0 0  
 S.Commercial\_B L14141 L14143 36 0.017 11.57 11.44 0 0  
 S.John L14054 L14053 200 0.017 6.6 4.91 0 0  
 S.William L14152 L14151 370 0.017 10.25 8.72 0 0  
 SDP1 SBroadwayPond K14954 410.95811861 0.013 0 0.1 0 0  
 SDP10 L14143 L14JB145 85.40790915 0.013 0 0.42 0 0  
 SDP11 K14951 K14952 354.71353104 0.013 0 0.08000000000000001 0 0  
 SDP12 K14952 L14052 158.63026916 0.013 0 0.4 0 0  
 SDP13 L14052 L14055 223.09104012 0.013 0 0.5 0 0  
 SDP14 L14055 L14151 339.09603202 0.013 0 0.37 0 0  
 SDP15 L14151 L14152 369.53315621 0.013 0 0.85 0 0  
 SDP16 L14152 L14999 34.22873553 0.013 0 0.77 0 0  
 SDP17 L14999 L14JB145 260.75662555 0.013 0 0.7 0 0  
 SDP19 L14055 L14051 183.12768235 0.013 4.69 0 0 0  
 SDP2 K14954 K14945 43.12332141 0.013 0 0.0500000000000003 0 0  
 SDP20 L14053 L14054 199.67810886 0.013 0 0 0 0  
 SDP21 K14953 L14053 191.14310008 0.013 0 0.0800000000000001 0 0  
 SDP22 L14054 L14055 244.301283 0.013 0.070000000000003 4.487 0 0  
 SDP24 L14042 L14041 19.24659539 0.013 0 6.907 0 0  
 SDP25 L14051 L14042 180.4767637 0.013 0 0.6 0 0  
 SDP26 L14146 L14142 35.18528675 0.013 0 7.778 0 0  
 SDP27 K14943 K14942 47.70588865 0.013 0 9 0 0  
 SDP28 L14JB145 L14PS1 53.05839922 0.013 0 6.45 0 0  
 SDP3 K14945 K14944 267.90742539 0.013 0 0.15 0 0  
 SDP4 K14944 K14941 92.15714564 0.013 0 0.15 0 0  
 SDP5 K14941 K14942 233.62246284 0.013 0 0.2 0 0  
 SDP6 K14942 L14041 392.68031286 0.013 0 0.12 0 0  
 SDP7 L14041 L14142 349.68993313 0.013 0 0.0999999999999996 0 0  
 SDP8 L14142 L14141 330.32958193 0.013 0 0.0800000000000001 0 0  
 SDP9 L14141 L14143 35.81241274 0.013 0 1.22 0 0  
 W.Garfield\_A L14051 L14042 181 0.017 2.32 2.77 0 0  
 W.Garfield\_B L14055 L14051 183 0.017 8.71 2.32 0 0  
 ;Initial\_invert\_4943.53\_to\_4936.6\_switched\_direction  
 W.Garfield\_Junct L14041 L14042 19 0.017 10.29 2.77 0 0  
 William\_1A K14952 L14052 158 0.017 5.28 6.38 0 0  
 William\_1B L14055 L14052 223 0.017 8.71 6.38 0 0  
 William\_2 L14055 L14151 339 0.017 8.71 8.72 0 0  
 William\_Pacific K14945 K14954 43 0.017 11.53 11.14 0 0

**[PUMPS]**  
 ;  
 SWMM 5.1 Inlet Outlet Pump Init. Startup Shutoff

<i>; ;Name</i>	<i>Node</i>	<i>Node</i>	<i>Curve</i>	<i>Status</i>	<i>Depth</i>	<i>Depth</i>
<i>; ;</i>			<i>PumpStation</i>	<i>OFF</i>	<i>3.5</i>	<i>0</i>
<b>[XSECTIONS]</b>						
<i>; ;Link</i>	<i>Shape</i>	<i>Geom1</i>	<i>Geom2</i>	<i>Geom3</i>	<i>Geom4</i>	<i>Barrels</i>
<i>; ;</i>						
Bell_A	IRREGULAR	Standard	0	0	0	1
Bell_B	IRREGULAR	Standard	0	0	0	1
Bell_C	IRREGULAR	Standard	0	0	0	1
Commercial_1	IRREGULAR	Standard	0	0	0	1
Commercial_2	IRREGULAR	Standard	0	0	0	1
Commercial_Sag	IRREGULAR	Standard	0	0	0	1
Cromwell_Junct	IRREGULAR	Standard	0	0	0	1
E.Cromwell	IRREGULAR	Standard	0	0	0	1
E.Garfield	IRREGULAR	Standard	0	0	0	1
Lewis_Junct	IRREGULAR	Standard	0	0	0	1
Link-13	CIRCULAR	1.5	0	1	1	1
Link-14	CIRCULAR	1.5	0	1	1	1
N.Commercial	IRREGULAR	Standard	0	0	0	1
N.John	IRREGULAR	Standard	0	0	0	1
N.William	IRREGULAR	Standard	0	0	0	1
Pacific	IRREGULAR	Standard	0	0	0	1
Pacific_William	IRREGULAR	Standard	0	0	0	1
S.Commercial_A	IRREGULAR	Standard	0	0	0	1
S.Commercial_B	IRREGULAR	Standard	0	0	0	1
S.John	IRREGULAR	Standard	0	0	0	1
S.William	IRREGULAR	Standard	0	0	0	1
SDP1	CIRCULAR	2.5	0	1	1	1
SDP10	CIRCULAR	4	0	1	1	1
SDP11	CIRCULAR	2	0	1	1	1
SDP12	CIRCULAR	2	0	1	1	1
SDP13	CIRCULAR	2.5	0	1	1	1
SDP14	CIRCULAR	3	0	1	1	1
SDP15	CIRCULAR	3	0	1	1	1
SDP16	CIRCULAR	4	0	1	1	1
SDP17	CIRCULAR	4	0	1	1	1
SDP19	CIRCULAR	1	0	1	1	1
SDP2	CIRCULAR	2.5	0	1	1	1
SDP20	CIRCULAR	1	0	1	1	1
SDP21	CIRCULAR	1	0	1	1	1
SDP22	CIRCULAR	1	0	1	1	1
SDP24	CIRCULAR	1.5	0	1	1	1
SDP25	CIRCULAR	1	0	1	1	1
SDP26	CIRCULAR	1.5	0	1	1	1
SDP27	CIRCULAR	1.5	0	1	1	1
SDP28	CIRCULAR	4	0	1	1	1
SDP3	CIRCULAR	2.5	0	1	1	1
SDP4	CIRCULAR	2.5	0	1	1	1
SDP5	CIRCULAR	2.5	0	1	1	1
SDP6	CIRCULAR	2.5	0	1	1	1
SDP7	CIRCULAR	2.5	0	1	1	1
SDP8	CIRCULAR	2.5	0	1	1	1
SDP9	CIRCULAR	4	0	1	1	1
W.Garfield_A	IRREGULAR	Standard	0	0	0	1
W.Garfield_B	IRREGULAR	Standard	0	0	0	1
W.Garfield_Junct	IRREGULAR	Standard	0	0	0	1
William_1A	IRREGULAR	Standard	0	0	0	1
William_1B	IRREGULAR	Standard	0	0	0	1
William_2	IRREGULAR	Standard	0	0	0	1
William_Pacific	IRREGULAR	Standard	0	0	0	1

**[TRANSECTS]**

*;Standard roadway section for the project with 2% cross slope, 32' full street section and 8" curb and gutter.*

NC	0	0	0.017			
X1	Standard	5	0.0	0.0	0.0	0.0
GR	0.67	0	0.1	0.32	16	0
					31.9	0.67
						32

**[LOSSES]**

<i>; ;Link</i>	<i>Inlet</i>	<i>Outlet</i>	<i>Average</i>	<i>Flap Gate</i>
<i>; ;</i>				
Link-13	0.5	0.6	0	NO
Link-14	0.5	0.6	0	NO
SDP1	0.025	0.025	0	NO
SDP10	.025	.025	0	NO
SDP11	.025	.025	0	NO
SDP12	.025	.025	0	NO
SDP13	.025	.025	0	NO
SDP14	0.025	0.025	0	NO
SDP15	.025	.025	0	NO
SDP16	.025	.025	0	NO

SDP17	0.025	.025	0	NO
SDP19	.025	.025	0	NO
SDP2	0.025	0.025	0	NO
SDP20	.025	.025	0	NO
SDP21	0.025	.025	0	NO
SDP22	.025	.025	0	NO
SDP24	0.025	0.025	0	NO
SDP25	.025	.025	0	NO
SDP26	0.025	0.025	0	NO
SDP27	0.025	0.025	0	NO
SDP28	.025	.025	0	NO
SDP3	.025	0.025	0	NO
SDP4	0.025	0.025	0	NO
SDP5	0.025	0.025	0	NO
SDP6	0.025	0.025	0	NO
SDP7	.025	.025	0	NO
SDP8	0.025	.025	0	NO
SDP9	.025	.025	0	NO

[INFLows]

;;Node	Parameter	Time Series	Param Type	Units Factor	Scale Factor	Baseline Value	Baseline Pattern
DI2024	FLOW	UH-B2	FLOW	1.0	1.0		
DI2046	FLOW	UH-B1	FLOW	1.0	1.0		
K14941	FLOW	UH-B3	FLOW	1.0	1.0		
K14952	FLOW	UH-B4	FLOW	1.0	1.0		
K14953	FLOW	UH-B5	FLOW	1.0	1.0		
L14042	FLOW	UH-B6	FLOW	1.0	1.0		
L14052	FLOW	UH-B7	FLOW	1.0	1.0		
L14053	FLOW	UH-B8	FLOW	1.0	1.0		
L14055	FLOW	UH-B9	FLOW	1.0	1.0		
L14142	FLOW	UH-B10	FLOW	1.0	1.0		
L14143	FLOW	UH-B12	FLOW	1.0	1.0		
L14151	FLOW	UH-B11	FLOW	1.0	1.0		
L14152	FLOW	UH-B13	FLOW	1.0	1.0		

[CURVES]

;;Name	Type	X-Value	Y-Value
PumpStation	Pump3	26.5	80
PumpStation		28	75
PumpStation		29.5	70
PumpStation		31.5	65
PumpStation		32	60
PumpStation		33	55
PumpStation		33.5	50
PumpStation		35	45
PumpStation		37	40
PumpStation		47	0
L14PS1	Storage	0	0
L14PS1		25	496
SB.DepthArea	Storage	0	0
SB.DepthArea		3.94	21780
SB.DepthArea		4.42	43560
SB.DepthArea		5.19	65340
SB.DepthArea		8.74	87120
SB.DepthArea		14.36	108900
SB.DepthArea		16.54	121968

[TIMESERIES]

;;Name	Date	Time	Value
;			
;Unit hydrograph for sub-basin #1 developed using COA DPM Modified Rational Method.			
UH-B1	5/6/2016	0:00	0
UH-B1	5/6/2016	00:12	10.21
UH-B1	5/6/2016	00:24	10.21
UH-B1	5/6/2016	00:43	0
;			
;Unit hydrograph for sub-basin #10 using COA DPM Modified Rational Method.			
UH-B10	5/6/2016	0:00	0
UH-B10	5/6/2016	0:14	5.03
UH-B10	5/6/2016	0:21	5.03
UH-B10	5/6/2016	0:45	0
;			
;Unit hydrograph for sub-basin #11 using COA DPM Modified Rational Method.			
UH-B11	5/6/2016	0:00	0
UH-B11	5/6/2016	0:15	17.84
UH-B11	5/6/2016	0:19	17.84
UH-B11	5/6/2016	0:45	0

```

;Unit hydrograph for sub-basin 12 using COA DPM Modified Rational Method.
UH-B12      5/6/2016  0:00      0
UH-B12      5/6/2016  0:14     7.08
UH-B12      5/6/2016  0:20     7.08
UH-B12      5/6/2016  0:45      0

;Unit hydrograph for sub-basin 13 using COA DPM Modified Rational Method.
UH-B13      5/6/2016  0:00      0
UH-B13      5/6/2016  0:14    26.51
UH-B13      5/6/2016  0:21    26.51
UH-B13      5/6/2016  0:45      0

;Unit hydrograph for sub-basin #2 using COA DPM Modified Rational Method.
UH-B2      5/6/2016  0:00      0
UH-B2      5/6/2016  0:14    21.48
UH-B2      5/6/2016  0:21    21.48
UH-B2      5/6/2016  0:45      0

;Unit hydrograph for sub-basin #3 using COA DPM Modified Rational Method.
UH-B3      5/6/2016  0:00      0
UH-B3      5/6/2016  0:14    6.43
UH-B3      5/6/2016  0:20    6.43
UH-B3      5/6/2016  0:45      0

;Unit hydrograph for sub-basin #4 using COA DPM Modified Rational Method.
UH-B4      5/6/2016  0:00      0
UH-B4      5/6/2016  0:14    6.96
UH-B4      5/6/2016  0:21    6.96
UH-B4      5/6/2016  0:45      0

;Unit hydrograph for sub-basin #5 using COA DPM Modified Rational Method
UH-B5      5/6/2016  0:00      0
UH-B5      5/6/2016  0:14   14.64
UH-B5      5/6/2016  0:21   14.64
UH-B5      5/6/2016  0:45      0

;Unit hydrograph for sub-basin #6 using COA DPM Modified Rational Method.
UH-B6      5/6/2016  0:00      0
UH-B6      5/6/2016  0:14   11.73
UH-B6      5/6/2016  0:21   11.73
UH-B6      5/6/2016  0:45      0

;Unit hydrograph for sub-basin #7 using COA DPM Modified Rational Method.
UH-B7      5/6/2016  0:00      0
UH-B7      5/6/2016  0:14    9.15
UH-B7      5/6/2016  0:21    9.15
UH-B7      5/6/2016  0:45      0

;Unit hydrograph for sub-basin #8 using COA DPM Modified Rational Method.
UH-B8      5/6/2016  0:00      0
UH-B8      5/6/2016  0:14   8.71
UH-B8      5/6/2016  0:21   8.71
UH-B8      5/6/2016  0:45      0

;Unit hydrograph for sub-basin #9 using COA DPM Modified Rational Method.
UH-B9      5/6/2016  0:00      0
UH-B9      5/6/2016  0:14  11.05
UH-B9      5/6/2016  0:21  11.05
UH-B9      5/6/2016  0:45      0

```

[REPORT]  
 INPUT YES  
 CONTROLS YES  
 SUBCATCHMENTS ALL  
 NODES ALL  
 LINKS ALL

[TAGS]

[MAP]  
 DIMENSIONS 1520848.930 1481094.560 1521757.510 1483178.350  
 Units Feet

[COORDINATES]  
 ;Node X-Coord Y-Coord  
 ;-----  
 DI2024 1521464.400 1482627.250  
 DI2046 1521132.640 1482625.370  
 K14941 1521127.100 1482521.880  
 K14942 1521091.600 1482287.960  
 K14943 1521139.220 1482285.170

K14944	1521142.930	1482609.670
K14945	1521410.070	1482629.980
K14951	1521435.400	1482633.430
K14952	1521444.180	1482278.820
K14953	1521650.770	1482287.060
K14954	1521432.240	1482666.960
L14041	1521030.200	1481900.110
L14042	1521049.290	1481897.720
L14051	1521229.760	1481896.040
L14052	1521448.090	1482120.240
L14053	1521654.920	1482095.970
L14054	1521657.120	1481896.300
L14055	1521412.850	1481899.950
L14141	1520921.980	1481228.760
L14142	1520974.200	1481554.940
L14143	1520918.320	1481193.130
L14146	1521008.940	1481549.410
L14151	1521357.360	1481565.420
L14152	1521298.170	1481200.660
L14999	1521264.330	1481195.520
L14JB145	1521003.650	1481189.280
L14JB2	1520997.600	1481244.150
L14PS1	1520997.730	1481242.010
SBroadwayPond	1521439.080	1483083.630

[VERTICES]

;	Link	X-Coord	Y-Coord
;	Bell_A	1520962.580	1481217.120
;	Bell_B	1521135.350	1481250.800
;	Bell_C	1521271.320	1481233.960
;	Commercial_1	1521006.170	1482100.160
;	Commercial_2	1520943.120	1481714.300
;	Commercial_Sag	1521091.070	1482563.310
;	Cromwell_Junct	1521113.770	1482303.590
;	E.Cromwell	1521561.320	1482327.700
;	E.Garfield	1521540.070	1481972.680
;	Lewis_Junct	1521002.710	1481579.910
;	N.Commercial	1521030.160	1482398.290
;	N.John	1521705.920	1482179.230
;	N.William	1521487.300	1482435.300
;	Pacific	1521246.240	1482667.330
;	Pacific_William	1521465.120	1482651.400
;	S.Commercial_A	1520895.830	1481387.380
;	S.Commercial_B	1520890.230	1481220.240
;	S.John	1521716.210	1482043.270
;	S.William	1521398.890	1481381.210
;	W.Garfield_A	1521152.260	1481980.280
;	W.Garfield_B	1521238.510	1481985.020
;	W.Garfield_Junct	1521043.840	1481915.030
;	William_1A	1521513.220	1482191.700
;	William_1B	1521505.120	1482095.360
;	William_2	1521448.920	1481693.740
;	William_Pacific	1521394.650	1482667.620

[PROFILES]

;	Name	Links
;	"SouthBroadwaytoPump"	SDP1 SDP2 SDP3 SDP4 SDP5
;	"SouthBroadwaytoPump"	SDP6 SDP7 SDP8 SDP9 SDP10
;	"SouthBroadwaytoPump"	SDP28
;	"SouthBroadway Pond to Pump Station through Road"	SDP1 William_Pacific Pacific_Commercial SDP4 SDP5
;	"SouthBroadway Pond to Pump Station through Road"	SDP6 SDP7 SDP8 SDP9 SDP10
;	"SouthBroadway Pond to Pump Station through Road"	SDP28
;	"	SDP1 Williams_Pacific SDP4 SDP5 SDP6
;	"	SDP7 SDP8 SDP9 SDP10 SDP28
;	"SouthBroadway Street Profile"	SDP1 Williams_Pacific SDP4 SDP5 SDP6
;	"SouthBroadway Street Profile"	SDP7 SDP8 SDP9 SDP10 SDP28
;	"FullStreet"	SDP1 Williams_Pacific Commercial SDP5 SDP6
;	"FullStreet"	SDP7 SDP8 SDP9 SDP10 SDP28

## SWMM Model Output - Existing System

EPA STORM WATER MANAGEMENT MODEL - VERSION 5.0 (Build 5.0.022)

\*\*\*\*\*  
NOTE: The summary statistics displayed in this report are  
based on results found at every computational time step,  
not just on results from each reporting time step.  
\*\*\*\*\*

\*\*\*\*\*

### Analysis Options

\*\*\*\*\*

Flow Units ..... CFS

#### Process Models:

Rainfall/Runoff ..... NO

Snowmelt ..... NO

Groundwater ..... NO

Flow Routing ..... YES

Ponding Allowed ..... NO

Water Quality ..... NO

Flow Routing Method ..... DYNWAVE

Starting Date ..... MAY-06-2016 00:00:00

Ending Date ..... MAY-06-2016 06:00:00

Antecedent Dry Days ..... 0.0

Report Time Step ..... 00:00:15

Routing Time Step ..... 15.00 sec

WARNING 04: minimum elevation drop used for Conduit Bell\_C

WARNING 04: minimum elevation drop used for Conduit Lewis\_Junct

WARNING 02: maximum depth increased for Node L14051

WARNING 02: maximum depth increased for Node L14053

\*\*\*\*\*

### Element Count

\*\*\*\*\*

Number of rain gages ..... 0

Number of subcatchments ... 0

Number of nodes ..... 29

Number of links ..... 55

Number of pollutants ..... 0

Number of land uses ..... 0

\*\*\*\*\*

### Node Summary

\*\*\*\*\*

Name	Type	Invert Elev.	Max. Depth	Ponded Area	External Inflow
DI2024	JUNCTION	4945.64	3.87	0.0	Yes
DI2046	JUNCTION	4945.39	3.50	0.0	Yes
K14941	JUNCTION	4937.35	11.75	0.0	Yes
K14942	JUNCTION	4936.82	13.35	0.0	
K14943	JUNCTION	4946.02	3.75	0.0	
K14944	JUNCTION	4937.60	11.85	0.0	
K14945	JUNCTION	4937.94	12.20	0.0	
K14951	JUNCTION	4945.21	4.60	0.0	
K14952	JUNCTION	4943.04	6.05	0.0	Yes
K14953	JUNCTION	4945.46	5.57	0.0	Yes
K14954	JUNCTION	4937.94	11.81	0.0	
L14041	JUNCTION	4936.60	11.17	0.0	
L14042	JUNCTION	4943.53	3.70	0.0	Yes
L14051	JUNCTION	4944.35	2.99	0.0	
L14052	JUNCTION	4941.40	7.23	0.0	Yes
L14053	JUNCTION	4944.98	5.58	0.0	Yes
L14054	JUNCTION	4944.59	7.27	0.0	
L14055	JUNCTION	4939.88	9.56	0.0	Yes
L14141	JUNCTION	4936.08	12.24	0.0	
L14142	JUNCTION	4935.24	13.23	0.0	Yes
L14143	JUNCTION	4935.66	12.35	0.0	Yes
L14146	JUNCTION	4943.06	5.41	0.0	
L14151	JUNCTION	4939.08	9.82	0.0	Yes
L14152	JUNCTION	4937.58	11.05	0.0	Yes
L14999	JUNCTION	4937.30	11.28	0.0	
L14JB145	JUNCTION	4934.73	13.55	0.0	
L14JB2	OUTFALL	4952.21	0.00	0.0	

L14PS1	STORAGE	4926.99	22.67	0.0
SBroadwayPond	STORAGE	4938.42	15.25	0.0

\*\*\*\*\*  
Link Summary  
\*\*\*\*\*

Name	From Node	To Node	Type	Length	%Slope	Roughness
Bell_A	L14JB145	L14143	CONDUIT	86.0	0.5930	0.0170
Bell_B	L14999	L14JB145	CONDUIT	261.0	0.0843	0.0170
Bell_C	L14152	L14999	CONDUIT	35.0	0.0029	0.0170
Commercial_1	K14942	L14041	CONDUIT	393.0	0.6641	0.0170
Commercial_2	L14142	L14041	CONDUIT	350.0	0.2343	0.0170
Commercial_Sag	K14941	DI2046	CONDUIT	92.2	0.2821	0.0170
Cromwell_Junct	K14942	K14943	CONDUIT	48.0	0.9792	0.0170
E.Cromwell	K14953	K14952	CONDUIT	170.0	1.0883	0.0170
E.Garfield	L14054	L14055	CONDUIT	244.0	1.0656	0.0170
Lewis_Junct	L14142	L14146	CONDUIT	35.0	0.0029	0.0170
Link-13	DI2024	K14951	CONDUIT	18.4	0.4346	0.0130
Link-14	DI2046	K14944	CONDUIT	46.1	16.8088	0.0130
N.Commercial	K14942	K14941	CONDUIT	234.0	0.5513	0.0170
N.John	K14953	L14053	CONDUIT	191.0	0.1466	0.0170
N.William	DI2024	K14952	CONDUIT	355.0	0.0958	0.0170
Pacific	K14945	DI2046	CONDUIT	268.0	0.5672	0.0170
Pacific_William	K14954	K14951	CONDUIT	34.0	1.2354	0.0170
S.Commercial_A	L14142	L14141	CONDUIT	330.0	0.0182	0.0170
S.Commercial_B	L14141	L14143	CONDUIT	36.0	1.5280	0.0170
S.John	L14054	L14053	CONDUIT	200.0	0.6500	0.0170
S.William	L14152	L14151	CONDUIT	370.0	0.0081	0.0170
SDP1	SBroadwayPond	K14954	CONDUIT	411.0	0.0925	0.0130
SDP10	L14143	L14JB145	CONDUIT	85.4	0.5971	0.0130
SDP11	K14951	K14952	CONDUIT	354.7	0.5892	0.0130
SDP12	K14952	L14052	CONDUIT	158.6	0.7817	0.0130
SDP13	L14052	L14055	CONDUIT	223.1	0.4572	0.0130
SDP14	L14055	L14151	CONDUIT	339.1	0.1268	0.0130
SDP15	L14151	L14152	CONDUIT	369.5	0.1759	0.0130
SDP16	L14999	L14152	CONDUIT	34.2	1.4317	0.0130
SDP17	L14999	L14JB145	CONDUIT	260.8	0.7172	0.0130
SDP19	L14055	L14051	CONDUIT	183.1	0.1201	0.0130
SDP2	K14945	K14954	CONDUIT	43.1	0.1159	0.0130
SDP20	L14053	L14054	CONDUIT	199.7	0.1953	0.0130
SDP21	K14953	L14053	CONDUIT	191.1	0.2093	0.0130
SDP22	L14054	L14055	CONDUIT	244.3	0.1199	0.0130
SDP24	L14042	L14041	CONDUIT	19.2	0.1195	0.0130
SDP25	L14051	L14042	CONDUIT	180.5	0.1219	0.0130
SDP26	L14146	L14142	CONDUIT	35.2	0.1194	0.0130
SDP27	K14943	K14942	CONDUIT	47.7	0.4192	0.0130
SDP28	L14JB145	L14PS1	CONDUIT	53.1	2.4320	0.0130
SDP3	K14945	K14944	CONDUIT	267.9	0.0709	0.0130
SDP4	K14944	K14941	CONDUIT	92.2	0.1085	0.0130
SDP5	K14941	K14942	CONDUIT	233.6	0.1413	0.0130
SDP6	K14942	L14041	CONDUIT	392.7	0.0255	0.0130
SDP7	L14041	L14142	CONDUIT	349.7	0.3603	0.0130
SDP8	L14141	L14142	CONDUIT	330.3	0.2785	0.0130
SDP9	L14143	L14141	CONDUIT	35.8	2.2344	0.0130
W.Garfield_A	L14051	L14042	CONDUIT	181.0	0.2044	0.0170
W.Garfield_B	L14055	L14051	CONDUIT	183.0	1.0492	0.0170
W.Garfield_Junct	L14041	L14042	CONDUIT	19.0	3.1068	0.0170
William_1A	K14952	L14052	CONDUIT	158.0	0.3418	0.0170
William_1B	L14055	L14052	CONDUIT	223.0	0.3632	0.0170
William_2	L14055	L14151	CONDUIT	339.0	0.2330	0.0170
William_Pacific	K14945	K14954	CONDUIT	43.0	0.9070	0.0170
PUMP_CT-3500-135_02L14PS1		L14JB2	TYPE3 PUMP			

\*\*\*\*\*  
Cross Section Summary  
\*\*\*\*\*

Conduit	Shape	Full Depth	Full Area	Hyd. Rad.	Max. Width	No. of Barrels	Full Flow
Bell_A	Standard	0.67	16.28	0.49	32.00	1	68.05
Bell_B	Standard	0.67	16.28	0.49	32.00	1	25.66
Bell_C	Standard	0.67	16.28	0.49	32.00	1	4.72
Commercial_1	Standard	0.67	16.28	0.49	32.00	1	72.02
Commercial_2	Standard	0.67	16.28	0.49	32.00	1	42.77
Commercial_Sag	Standard	0.67	16.28	0.49	32.00	1	46.94
Cromwell_Junct	Standard	0.67	16.28	0.49	32.00	1	87.44
E.Cromwell	Standard	0.67	16.28	0.49	32.00	1	92.19
E.Garfield	Standard	0.67	16.28	0.49	32.00	1	91.22
Lewis_Junct	Standard	0.67	16.28	0.49	32.00	1	4.72

Link-13	CIRCULAR	1.50	1.77	0.38	1.50	1	6.92
Link-14	CIRCULAR	1.50	1.77	0.38	1.50	1	43.07
N.Commercial	Standard	0.67	16.28	0.49	32.00	1	65.61
N.John	Standard	0.67	16.28	0.49	32.00	1	33.83
N.William	Standard	0.67	16.28	0.49	32.00	1	27.35
Pacific	Standard	0.67	16.28	0.49	32.00	1	66.55
Pacific_William	Standard	0.67	16.28	0.49	32.00	1	98.22
S.Commercial_A	Standard	0.67	16.28	0.49	32.00	1	11.92
S.Commercial_B	Standard	0.67	16.28	0.49	32.00	1	109.23
S.John	Standard	0.67	16.28	0.49	32.00	1	71.25
S.William	Standard	0.67	16.28	0.49	32.00	1	7.96
SDP1	CIRCULAR	2.50	4.91	0.63	2.50	1	12.47
SDP10	CIRCULAR	4.00	12.57	1.00	4.00	1	111.00
SDP11	CIRCULAR	2.00	3.14	0.50	2.00	1	17.37
SDP12	CIRCULAR	2.00	3.14	0.50	2.00	1	20.00
SDP13	CIRCULAR	2.50	4.91	0.63	2.50	1	27.73
SDP14	CIRCULAR	3.00	7.07	0.75	3.00	1	23.75
SDP15	CIRCULAR	3.00	7.07	0.75	3.00	1	27.97
SDP16	CIRCULAR	4.00	12.57	1.00	4.00	1	171.87
SDP17	CIRCULAR	4.00	12.57	1.00	4.00	1	121.64
SDP19	CIRCULAR	1.00	0.79	0.25	1.00	1	1.23
SDP2	CIRCULAR	2.50	4.91	0.63	2.50	1	13.97
SDP20	CIRCULAR	1.00	0.79	0.25	1.00	1	1.57
SDP21	CIRCULAR	1.00	0.79	0.25	1.00	1	1.63
SDP22	CIRCULAR	1.00	0.79	0.25	1.00	1	1.23
SDP24	CIRCULAR	1.50	1.77	0.38	1.50	1	3.63
SDP25	CIRCULAR	1.00	0.79	0.25	1.00	1	1.24
SDP26	CIRCULAR	1.50	1.77	0.38	1.50	1	3.63
SDP27	CIRCULAR	1.50	1.77	0.38	1.50	1	6.80
SDP28	CIRCULAR	4.00	12.57	1.00	4.00	1	224.01
SDP3	CIRCULAR	2.50	4.91	0.63	2.50	1	10.92
SDP4	CIRCULAR	2.50	4.91	0.63	2.50	1	13.51
SDP5	CIRCULAR	2.50	4.91	0.63	2.50	1	15.42
SDP6	CIRCULAR	2.50	4.91	0.63	2.50	1	6.55
SDP7	CIRCULAR	2.50	4.91	0.63	2.50	1	24.62
SDP8	CIRCULAR	2.50	4.91	0.63	2.50	1	21.65
SDP9	CIRCULAR	4.00	12.57	1.00	4.00	1	214.72
W.Garfield_A	Standard	0.67	16.28	0.49	32.00	1	39.95
W.Garfield_B	Standard	0.67	16.28	0.49	32.00	1	90.52
W.Garfield_Junct	Standard	0.67	16.28	0.49	32.00	1	155.76
William_1A	Standard	0.67	16.28	0.49	32.00	1	51.66
William_1B	Standard	0.67	16.28	0.49	32.00	1	53.26
William_2	Standard	0.67	16.28	0.49	32.00	1	42.66
William_Pacific	Standard	0.67	16.28	0.49	32.00	1	84.16

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Transect Summary

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Transect Standard

Area:

0.0005	0.0022	0.0049	0.0088	0.0137
0.0198	0.0269	0.0352	0.0445	0.0550
0.0665	0.0791	0.0929	0.1077	0.1236
0.1407	0.1588	0.1780	0.1984	0.2198
0.2423	0.2660	0.2907	0.3165	0.3428
0.3690	0.3953	0.4215	0.4478	0.4740
0.5003	0.5266	0.5528	0.5791	0.6054
0.6317	0.6580	0.6843	0.7106	0.7369
0.7632	0.7895	0.8158	0.8421	0.8684
0.8947	0.9210	0.9473	0.9737	1.0000

Hrad:

0.0134	0.0268	0.0402	0.0536	0.0670
0.0805	0.0939	0.1073	0.1207	0.1341
0.1475	0.1609	0.1743	0.1877	0.2011
0.2145	0.2279	0.2414	0.2548	0.2682
0.2816	0.2950	0.3084	0.3234	0.3499
0.3764	0.4028	0.4292	0.4556	0.4819
0.5082	0.5344	0.5606	0.5868	0.6129
0.6390	0.6650	0.6910	0.7170	0.7429
0.7688	0.7947	0.8205	0.8462	0.8719
0.8976	0.9233	0.9489	0.9745	1.0000

Width:

0.0417	0.0835	0.1252	0.1670	0.2087
0.2504	0.2922	0.3339	0.3756	0.4174
0.4591	0.5009	0.5426	0.5843	0.6261
0.6678	0.7096	0.7513	0.7930	0.8348
0.8765	0.9182	0.9600	0.9968	0.9969
0.9970	0.9971	0.9972	0.9974	0.9975
0.9976	0.9977	0.9979	0.9980	0.9981

0.9983	0.9984	0.9985	0.9986	0.9988
0.9989	0.9990	0.9991	0.9992	0.9994
0.9995	0.9996	0.9998	0.9999	1.0000

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Control Actions Taken  
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Flow Routing Continuity	Volume acre-feet	Volume $10^6$ gal
Dry Weather Inflow .....	0.000	0.000
Wet Weather Inflow .....	0.000	0.000
Groundwater Inflow .....	0.000	0.000
RDII Inflow .....	0.000	0.000
External Inflow .....	5.591	1.822
External Outflow .....	18.610	6.064
Internal Outflow .....	2.144	0.699
Storage Losses .....	0.000	0.000
Initial Stored Volume ....	23.467	7.647
Final Stored Volume .....	8.203	2.673
Continuity Error (%) .....	0.348	

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Highest Continuity Errors  
\*\*\*\*\*  
Node L14051 (-4.50%)

Time-Step Critical Elements
Link SDP9 (86.90%)
Link SDP24 (3.01%)
Link William_Pacific (2.56%)
Link Link-13 (2.50%)
Link W.Garfield_Junct (2.24%)

Highest Flow Instability Indexes
Link SDP9 (5)
Link SDP10 (4)
Link SDP28 (3)
Link SDP11 (2)
Link Link-13 (1)

Routing Time Step Summary
Minimum Time Step : 0.50 sec
Average Time Step : 2.16 sec
Maximum Time Step : 15.00 sec
Percent in Steady State : 0.00
Average Iterations per Step : 2.14

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Node Depth Summary  
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Node	Type	Average Depth Feet	Maximum Depth Feet	Maximum HGL Feet	Time of Max Occurrence days hr:min
DI2024	JUNCTION	0.46	3.84	4949.48	0 00:21
DI2046	JUNCTION	0.93	3.50	4948.89	0 00:20
K14941	JUNCTION	8.16	11.55	4948.90	0 00:21
K14942	JUNCTION	7.42	11.45	4948.27	0 00:21
K14943	JUNCTION	0.15	2.25	4948.27	0 00:21
K14944	JUNCTION	8.42	11.32	4948.92	0 00:21
K14945	JUNCTION	9.46	11.62	4949.56	0 00:50
K14951	JUNCTION	0.56	4.53	4949.74	0 00:16
K14952	JUNCTION	0.60	6.05	4949.09	0 00:16
K14953	JUNCTION	0.58	5.07	4950.53	0 00:20
K14954	JUNCTION	9.75	11.81	4949.75	0 00:16

L14041	JUNCTION	5.58	10.66	4947.26	0	00:20
L14042	JUNCTION	0.31	3.70	4947.23	0	00:19
L14051	JUNCTION	0.20	2.95	4947.30	0	00:21
L14052	JUNCTION	0.66	7.23	4948.63	0	00:16
L14053	JUNCTION	0.62	5.58	4950.56	0	00:17
L14054	JUNCTION	0.55	5.02	4949.61	0	00:23
L14055	JUNCTION	0.79	8.56	4948.44	0	00:21
L14141	JUNCTION	2.49	10.58	4946.66	0	00:21
L14142	JUNCTION	5.07	11.84	4947.08	0	00:20
L14143	JUNCTION	2.02	11.00	4946.66	0	00:21
L14146	JUNCTION	0.18	4.02	4947.08	0	00:20
L14151	JUNCTION	0.78	9.02	4948.10	0	00:24
L14152	JUNCTION	1.15	9.58	4947.16	0	00:21
L14999	JUNCTION	0.72	9.79	4947.09	0	00:21
L14JB145	JUNCTION	1.76	11.91	4946.64	0	00:21
L14JB2	OUTFALL	0.00	0.00	4952.21	0	00:00
L14PS1	STORAGE	1.73	19.45	4946.44	0	00:21
SBroadwayPond	STORAGE	11.76	15.25	4953.67	0	00:00

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Node Inflow Summary
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Node	Type	Maximum Lateral Inflow CFS	Maximum Total Inflow CFS	Time of Max Occurrence days hr:min	Lateral Inflow Volume 10^6 gal	Total Inflow Volume 10^6 gal
DI2024	JUNCTION	21.48	28.31	0 00:16	0.250	0.385
DI2046	JUNCTION	10.21	12.95	0 00:14	0.126	0.154
K14941	JUNCTION	6.43	47.59	0 00:03	0.074	4.691
K14942	JUNCTION	0.00	44.21	0 00:02	0.000	4.673
K14943	JUNCTION	0.00	0.57	0 00:12	0.000	0.001
K14944	JUNCTION	0.00	46.18	0 00:03	0.000	4.527
K14945	JUNCTION	0.00	65.25	0 00:00	0.000	5.171
K14951	JUNCTION	0.00	25.21	0 00:51	0.000	0.466
K14952	JUNCTION	6.96	67.70	0 00:16	0.081	0.952
K14953	JUNCTION	14.64	20.21	0 00:18	0.171	0.216
K14954	JUNCTION	0.00	79.95	0 00:17	0.000	5.709
L14041	JUNCTION	0.00	38.63	0 00:10	0.000	4.752
L14042	JUNCTION	11.73	41.84	0 00:20	0.137	0.262
L14051	JUNCTION	0.00	11.61	0 00:18	0.000	0.043
L14052	JUNCTION	9.15	68.86	0 00:20	0.107	1.049
L14053	JUNCTION	8.71	11.77	0 00:13	0.102	0.121
L14054	JUNCTION	0.00	4.50	0 00:06	0.000	0.074
L14055	JUNCTION	11.05	46.99	0 00:13	0.129	0.831
L14141	JUNCTION	0.00	40.79	0 00:10	0.000	4.688
L14142	JUNCTION	5.03	41.43	0 00:10	0.059	4.705
L14143	JUNCTION	7.08	45.44	0 00:58	0.081	4.764
L14146	JUNCTION	0.00	1.77	0 00:13	0.000	0.001
L14151	JUNCTION	17.84	58.37	0 00:12	0.196	0.998
L14152	JUNCTION	26.51	79.53	0 00:12	0.309	1.303
L14999	JUNCTION	0.00	76.46	0 00:12	0.000	1.303
L14JB145	JUNCTION	0.00	108.70	0 00:10	0.000	6.061
L14JB2	OUTFALL	0.00	80.00	0 00:02	0.000	6.064
L14PS1	STORAGE	0.00	99.95	0 00:09	0.000	6.063
SBroadwayPond	STORAGE	0.00	0.00	0 00:00	0.000	7.639

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Node Surcharge Summary
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Surcharging occurs when water rises above the top of the highest conduit.

Node	Type	Hours Surcharged	Max. Height Above Crown	Min. Depth Below Rim
			Feet	Feet
DI2024	JUNCTION	0.22	0.147	0.033
DI2046	JUNCTION	0.39	0.270	0.000
K14941	JUNCTION	0.04	0.025	0.195
K14944	JUNCTION	5.98	8.670	0.530
K14951	JUNCTION	0.63	0.412	0.068
K14952	JUNCTION	0.22	0.100	0.000
K14954	JUNCTION	0.48	0.000	0.000
L14042	JUNCTION	0.48	0.260	0.000
L14052	JUNCTION	0.45	0.180	0.000
L14053	JUNCTION	0.08	0.000	0.000
L14PS1	STORAGE	0.82	9.004	3.216

SBroadwayPond                    STORAGE                    6.00                    12.750                    0.000

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Node Flooding Summary  
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Flooding refers to all water that overflows a node, whether it ponds or not.

Node	Hours Flooded	Maximum Rate CFS	Time of Max Occurrence days hr:min	Total Flood Volume 10^6 gal	Maximum Ponded Depth Feet
DI2046	0.11	5.81	0 00:21	0.006	3.50
K14952	0.14	10.94	0 00:16	0.014	6.05
K14954	0.47	13.41	0 00:21	0.104	11.81
L14042	0.28	41.72	0 00:20	0.139	3.70
L14052	0.42	56.56	0 00:21	0.434	7.23
L14053	0.08	0.70	0 00:20	0.001	5.58

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Storage Volume Summary  
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Storage Unit	Average Volume 1000 ft3	Avg Pcnt	E&I Pcnt	Maximum Volume 1000 ft3	Max Pcnt	Time of Max Occurrence days hr:min	Maximum Outflow CFS
L14PS1	0.268	5	0	3.754	74	0 00:21	80.00
SBroadwayPond	659.176	65	0	1021.242	100	0 00:00	69.44

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Outfall Loading Summary  
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Outfall Node	Flow Freq. Pcnt.	Avg. Flow CFS	Max. Flow CFS	Total Volume 10^6 gal
L14JB2	99.24	36.57	80.00	6.064
System	99.24	36.57	80.00	6.064

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Link Flow Summary  
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Link	Type	Maximum  Flow  CFS	Time of Max Occurrence days hr:min	Maximum  Veloc  ft/sec	Max/ Full Flow	Max/ Full Depth
Bell_A	CHANNEL	0.00	0 00:00	0.00	0.00	0.00
Bell_B	CHANNEL	0.00	0 00:00	0.00	0.00	0.00
Bell_C	CHANNEL	0.00	0 00:00	0.00	0.00	0.00
Commercial_1	CHANNEL	0.00	0 00:00	0.00	0.00	0.28
Commercial_2	CHANNEL	0.00	0 00:00	0.00	0.00	0.28
Commercial_Sag	CHANNEL	11.22	0 00:16	0.89	0.24	1.00
Cromwell_Junct	CHANNEL	0.00	0 00:00	0.00	0.00	0.00
E.Cromwell	CHANNEL	20.19	0 00:20	2.36	0.22	0.77
E.Garfield	CHANNEL	0.00	0 00:00	0.00	0.00	0.00
Lewis_Junct	CHANNEL	0.00	0 00:00	0.00	0.00	0.00
Link-13	CONDUIT	10.55	0 00:07	5.97	1.52	1.00
Link-14	CONDUIT	5.30	0 00:06	3.19	0.12	1.00
N.Commercial	CHANNEL	0.00	0 00:00	0.00	0.00	0.50
N.John	CHANNEL	5.35	0 00:23	0.68	0.16	0.77
N.William	CHANNEL	30.82	0 00:16	1.93	1.13	1.00
Pacific	CHANNEL	0.34	0 00:50	0.11	0.01	0.50
Pacific_William	CHANNEL	25.21	0 00:51	2.86	0.26	1.00
S.Commercial_A	CHANNEL	0.00	0 00:00	0.00	0.00	0.00
S.Commercial_B	CHANNEL	0.00	0 00:00	0.00	0.00	0.00
S.John	CHANNEL	0.00	0 00:00	0.00	0.00	0.50
S.William	CHANNEL	0.78	0 00:24	0.38	0.10	0.30
SDP1	CONDUIT	69.44	0 00:00	14.15	5.57	1.00
SDP10	CONDUIT	47.96	0 00:58	8.20	0.43	1.00
SDP11	CONDUIT	21.03	0 00:55	6.69	1.21	1.00

SDP12	CONDUIT	27.81	0	00:50	8.85	1.39	1.00
SDP13	CONDUIT	36.41	0	00:46	7.42	1.31	1.00
SDP14	CONDUIT	43.54	0	00:12	6.16	1.83	1.00
SDP15	CONDUIT	56.30	0	00:12	8.00	2.01	1.00
SDP16	CONDUIT	76.46	0	00:12	10.14	0.44	1.00
SDP17	CONDUIT	71.14	0	00:11	9.48	0.58	1.00
SDP19	CONDUIT	3.17	0	00:49	4.33	2.57	1.00
SDP2	CONDUIT	65.25	0	00:00	13.29	4.67	1.00
SDP20	CONDUIT	4.50	0	00:06	5.73	2.86	1.00
SDP21	CONDUIT	3.25	0	00:04	4.14	1.99	1.00
SDP22	CONDUIT	3.94	0	00:49	5.19	3.19	1.00
SDP24	CONDUIT	9.99	0	00:13	5.82	2.75	1.00
SDP25	CONDUIT	1.64	0	00:13	2.14	1.32	1.00
SDP26	CONDUIT	1.77	0	00:13	2.31	0.49	1.00
SDP27	CONDUIT	0.57	0	00:12	1.00	0.08	1.00
SDP28	CONDUIT	99.95	0	00:09	16.69	0.45	1.00
SDP3	CONDUIT	43.72	0	00:01	8.91	4.00	1.00
SDP4	CONDUIT	46.18	0	00:03	9.41	3.42	1.00
SDP5	CONDUIT	44.21	0	00:02	9.01	2.87	1.00
SDP6	CONDUIT	34.60	0	00:04	7.06	5.29	1.00
SDP7	CONDUIT	37.53	0	00:10	7.65	1.52	1.00
SDP8	CONDUIT	40.79	0	00:10	8.48	1.88	1.00
SDP9	CONDUIT	45.44	0	00:58	8.02	0.21	1.00
W.Garfield_A	CHANNEL	9.03	0	00:22	0.71	0.23	0.97
W.Garfield_B	CHANNEL	0.00	0	00:00	0.00	0.00	0.47
W.Garfield_Junct	CHANNEL	27.43	0	00:20	2.50	0.18	0.78
William_1A	CHANNEL	47.68	0	00:25	2.93	0.92	1.00
William_1B	CHANNEL	0.07	0	00:34	0.01	0.00	0.53
William_2	CHANNEL	0.00	0	00:00	0.00	0.00	0.22
William_Pacific	CHANNEL	42.26	0	00:27	3.32	0.50	0.87
PUMP_CT-3500-135_02	PUMP	80.00	0	00:02		1.00	

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**Flow Classification Summary**  
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Conduit	Adjusted /Actual Length	Fraction of Time in Flow Class						Avg. Froude Number	Avg. Flow Change
		Dry	Up Dry	Down Dry	Sub Crit	Sup Crit	Up Crit		
Bell_A	1.00	1.00	0.00	0.00	0.00	0.00	0.00	0.00	0.0000
Bell_B	1.00	1.00	0.00	0.00	0.00	0.00	0.00	0.00	0.0000
Bell_C	1.00	1.00	0.00	0.00	0.00	0.00	0.00	0.00	0.0000
Commercial_1	1.00	0.95	0.05	0.00	0.00	0.00	0.00	0.00	0.0000
Commercial_2	1.00	0.95	0.05	0.00	0.00	0.00	0.00	0.00	0.0000
Commercial_Sag	1.00	0.88	0.02	0.00	0.05	0.00	0.05	0.00	0.0001
Cromwell_Junct	1.00	1.00	0.00	0.00	0.00	0.00	0.00	0.00	0.0000
E.Cromwell	1.00	0.90	0.00	0.00	0.05	0.00	0.00	0.04	0.06
E.Garfield	1.00	1.00	0.00	0.00	0.00	0.00	0.00	0.00	0.0000
Lewis_Junct	1.00	1.00	0.00	0.00	0.00	0.00	0.00	0.00	0.0000
Link-13	1.00	0.00	0.00	0.00	0.22	0.00	0.00	0.78	0.05
Link-14	1.00	0.00	0.31	0.00	0.69	0.00	0.00	0.00	0.0001
N.Commercial	1.00	0.94	0.06	0.00	0.00	0.00	0.00	0.00	0.0000
N.John	1.00	0.90	0.00	0.00	0.09	0.00	0.00	0.01	0.02
N.William	1.00	0.89	0.00	0.00	0.05	0.00	0.00	0.06	0.04
Pacific	1.00	0.88	0.12	0.00	0.00	0.00	0.00	0.00	0.0000
Pacific_William	1.00	0.75	0.00	0.00	0.07	0.00	0.00	0.17	0.22
S.Commercial_A	1.00	1.00	0.00	0.00	0.00	0.00	0.00	0.00	0.0000
S.Commercial_B	1.00	1.00	0.00	0.00	0.00	0.00	0.00	0.00	0.0000
S.John	1.00	0.91	0.09	0.00	0.00	0.00	0.00	0.00	0.0000
S.William	1.00	0.98	0.00	0.00	0.00	0.00	0.02	0.00	0.0000
SDP1	1.00	0.00	0.00	0.00	1.00	0.00	0.00	0.00	0.0009
SDP10	1.00	0.00	0.00	0.00	0.11	0.00	0.00	0.89	1.14
SDP11	1.00	0.00	0.00	0.00	0.12	0.00	0.00	0.88	0.28
SDP12	1.00	0.00	0.00	0.00	0.10	0.00	0.00	0.90	0.44
SDP13	1.00	0.00	0.00	0.00	0.13	0.00	0.00	0.87	0.37
SDP14	1.00	0.00	0.00	0.00	0.13	0.00	0.00	0.87	0.35
SDP15	1.00	0.00	0.00	0.00	0.09	0.00	0.00	0.91	0.45
SDP16	1.00	0.00	0.00	0.00	0.09	0.00	0.91	0.00	0.14
SDP17	1.00	0.00	0.00	0.00	0.88	0.02	0.00	0.09	0.23
SDP19	1.00	0.02	0.87	0.00	0.06	0.00	0.05	0.00	0.011
SDP2	1.00	0.00	0.00	0.00	1.00	0.00	0.00	0.00	0.0019
SDP20	1.00	0.00	0.00	0.00	1.00	0.00	0.00	0.00	0.0007
SDP21	1.00	0.00	0.00	0.00	0.17	0.00	0.00	0.83	0.16
SDP22	1.00	0.00	0.00	0.00	0.05	0.00	0.00	0.94	0.34
SDP24	1.00	0.00	0.00	0.00	0.09	0.00	0.00	0.91	0.33
SDP25	1.00	0.01	0.01	0.00	0.12	0.00	0.00	0.86	0.26
SDP26	1.00	0.06	0.00	0.00	0.07	0.00	0.00	0.88	0.07
SDP27	1.00	0.03	0.00	0.00	0.11	0.00	0.00	0.85	0.06
SDP28	1.00	0.00	0.00	0.00	0.11	0.00	0.00	0.89	2.27

SDP3	1.00	0.00	0.00	0.00	1.00	0.00	0.00	0.00	0.00	0.00	0.0011
SDP4	1.00	0.00	0.00	0.00	1.00	0.00	0.00	0.00	0.00	0.00	0.0010
SDP5	1.00	0.00	0.00	0.00	0.99	0.00	0.00	0.01	0.01	0.0007	
SDP6	1.00	0.00	0.00	0.00	0.99	0.00	0.00	0.01	0.01	0.0012	
SDP7	1.00	0.00	0.00	0.00	0.99	0.00	0.00	0.01	0.01	0.0005	
SDP8	1.00	0.00	0.01	0.00	0.99	0.00	0.00	0.00	0.59	0.0005	
SDP9	1.00	0.01	0.00	0.00	0.10	0.00	0.89	0.00	1.02	0.0001	
W.Garfield_A	1.00	0.95	0.00	0.00	0.05	0.00	0.00	0.00	0.00	0.0002	
W.Garfield_B	1.00	0.95	0.05	0.00	0.00	0.00	0.00	0.00	0.00	0.0000	
W.Garfield_Junct	1.00	0.95	0.01	0.00	0.04	0.00	0.00	0.00	0.01	0.0002	
William_1A	1.00	0.94	0.00	0.00	0.05	0.00	0.00	0.01	0.04	0.0002	
William_1B	1.00	0.95	0.01	0.00	0.00	0.00	0.04	0.00	0.00	0.0000	
William_2	1.00	0.98	0.02	0.00	0.00	0.00	0.00	0.00	0.00	0.0000	
William_Pacific	1.00	0.75	0.19	0.00	0.00	0.05	0.00	0.05	0.05	0.0001	

\*\*\*\*\*
Conduit Surcharge Summary
\*\*\*\*\*

Conduit	Hours Full			Above Normal	Capacity Limited
	Both Ends	Upstream	Dnstream		
Commercial_Sag	0.04	0.04	0.04	0.01	0.01
Link-13	0.85	0.85	0.85	0.54	0.14
Link-14	1.79	1.79	1.79	0.01	0.01
N.William	0.18	0.18	0.18	0.11	0.10
Pacific_William	0.47	0.47	0.48	0.01	0.01
SDP1	6.00	6.00	6.00	6.00	6.00
SDP10	0.75	0.75	0.75	0.01	0.01
SDP11	0.79	0.79	0.79	0.12	0.09
SDP12	0.76	0.76	0.76	0.26	0.19
SDP13	0.75	0.75	0.75	0.35	0.34
SDP14	0.73	0.73	0.73	0.64	0.54
SDP15	0.67	0.67	0.67	0.82	0.67
SDP16	0.64	0.64	0.64	0.01	0.01
SDP17	0.67	0.67	0.67	0.01	0.01
SDP19	0.59	0.59	0.59	0.52	0.39
SDP2	5.99	5.99	5.99	6.00	0.01
SDP20	0.88	0.88	0.88	0.86	0.83
SDP21	0.88	0.88	0.88	0.07	0.07
SDP22	0.60	0.60	0.60	0.92	0.60
SDP24	0.67	0.67	0.67	0.40	0.29
SDP25	0.64	0.64	0.64	0.01	0.01
SDP26	0.58	0.58	0.58	0.01	0.01
SDP27	0.59	0.59	0.59	0.01	0.01
SDP28	0.78	0.78	0.78	0.01	0.01
SDP3	5.98	5.98	5.98	5.67	5.66
SDP4	5.97	5.97	5.97	5.48	5.47
SDP5	5.95	5.95	5.95	5.97	5.95
SDP6	5.93	5.93	5.93	5.96	5.93
SDP7	5.93	5.93	5.93	5.53	5.52
SDP8	0.79	0.79	0.79	5.62	0.01
SDP9	0.68	0.68	0.68	0.01	0.01
William_1A	0.22	0.22	0.22	0.01	0.01

\*\*\*\*\*
Pumping Summary
\*\*\*\*\*

Pump	Percent Utilized	Number of Start-Ups	Min Flow	Avg Flow	Max Flow	Total Volume	Power Usage	% Time Off Pump Curve
			CFS	CFS	CFS	10^6 gal	Kw-hr	Low High
PUMP_CT-3500-135_02	99.27	1	0.00	36.57	80.00	6.064	386.07	0.0 100.0

Analysis begun on: Tue Dec 13 10:26:44 2016  
Analysis ended on: Tue Dec 13 10:26:45 2016  
Total elapsed time: 00:00:01

## SWMM Model Input - Recommended Improvements

### [TITLE]

### [OPTIONS]

FLOW\_UNITS CFS  
 INFILTRATION HORTON  
 FLOW\_ROUTING DYNWAVE  
 START\_DATE 05/06/2016  
 START\_TIME 00:00:00  
 REPORT\_START\_DATE 05/06/2016  
 REPORT\_START\_TIME 00:00:00  
 END\_DATE 05/07/2016  
 END\_TIME 00:00:00  
 SWEEP\_START 01/01  
 SWEEP\_END 12/31  
 DRY\_DAYS 0  
 REPORT\_STEP 00:00:15  
 WET\_STEP 00:05:00  
 DRY\_STEP 01:00:00  
 ROUTING\_STEP 0:00:15  
 ALLOW\_PONDING NO  
 INERTIAL\_DAMPING PARTIAL  
 VARIABLE\_STEP 0.75  
 LENGTHENING\_STEP 0  
 MIN\_SURFAREA 12.557  
 NORMAL\_FLOW\_LIMITED BOTH  
 SKIP\_STEADY\_STATE NO  
 FORCE\_MAIN\_EQUATION H-W  
 LINK\_OFFSETS DEPTH  
 MIN\_SLOPE 0.001

### [EVAPORATION]

<i>;Type</i>	<i>Parameters</i>
<i>;-----</i>	<i>-----</i>
CONSTANT	0.0
DRY_ONLY	NO

### [JUNCTIONS]

<i>;</i>	Invert	Max.	Init.	Surcharge	Ponded
<i>;</i> <i>Name</i>	Elev.	Depth	Depth	Depth	Area
<i>;</i> <i>-----</i>	<i>-----</i>	<i>-----</i>	<i>-----</i>	<i>-----</i>	<i>-----</i>
DI2024	4945.64	3.87	0	0	0
<i>;Max/rim_elev._is_top_of_curb_elevation.</i>					
DI2046	4945.39	3.5	0	0	0
<i>;Rim elevation was raised to match grate elevation plus 8" curb height.</i>					
K14941	4937.35	11.75	0	0	0
<i>;MH rim elevation changed to match lowest connecting inlet grate elevation (DI #2079 - 4949.03) plus 8" curb height.</i>					
K14942	4936.82	13.35	0	0	0
K14943	4946.02	3.75	0	0	0
K14944	4937.6	11.85	0	0	0
<i>;Manhole rim elevation has been adjusted to the flowline elevation at the curb plus the curb height (the manhole</i>					
<i>;is located 11 feet from the curb at a 2% cross slope).</i>					
K14945	4937.94	12.2	0	0	0
<i>;MH rim elevation adjusted to lowest connecting inlet grate elevation (DI #2024 - 4948.66) plus 8" curb height.</i>					
K14951	4945.21	4.6	0	0	0
<i>;MH rim elevation changed to reflect lowest connecting inlet grate elevation (DI #2327 - 4948.32) plus 8" curb he</i>					
K14952	4943.04	6.05	0	0	0
<i>;MH rim elevation adj. to lowest connecting inlet grate elevation (DI #2336 - 4950.17) plus 8" curb height.</i>					
K14953	4943.86	7.17	0	0	0
<i>;Changed rim elevation to reflect the location of the sd inlet in the middle of the street. William Street has an</i>					
<i>;crown with a 2% cross slope with the sd inlet grate elevation being lower than the curb flowline. Curb elevation</i>					
<i>;determined to be at 4949.24 ft. multiplied by 2% slope results in an inlet grate elevation of 4949.04 ft. The ma</i>					
<i>;was then adjusted back to the curb height elevation by adding 8" to the grate elevation.</i>					
K14954	4937.94	11.81	0	0	0
<i>;MH rim elevation adj. to lowest connecting inlet grate elevation (DI #2009 - 4946.30) plus 8" curb height.</i>					
L14041	4936.6	11.17	0	0	0
L14042	4943.53	3.7	0	0	0
L14051	4944.35	2.95	0	0	0
<i>;MH rim elevation adj. to lowest connecting grate elevation (DI #2303 - 4947.78) plus 8" curb height.</i>					
L14052	4941.40	7.23	0	0	0
<i>;MH rim elevation adj. to lowest connecting inlet elevation (DI #2344 - 4949.89) plus 8" curb height.</i>					
L14053	4943.06	7.31	0	0	0

;MH rim elevation adj. to existing rim elevation (4951.39) plus 8" curb height.  
 ;There were no connecting inlets shown in the survey file.  
 L14054 4942.28 9.58 0 0 0  
 ;MH rim elevation adj. to lowest connecting inlet grate elevation (DI #2218 - 4948.59) plus 8" curb height.  
 L14055 4939.88 9.56 0 0 0  
 L14141 4936.08 12.24 0 0 0  
 ;MH rim elevation adj. to lowest connecting inlet grate elevation (DI #2134 - 4947.71) plus 8" curb height.  
 L14142 4935.24 13.23 0 0 0  
 ;MH rim elevation adj. to lowest connecting inlet grate elevation (DI #2156 - 4947.10) plus 8" curb height.  
 L14143 4935.66 12.35 0 0 0  
 L14146 4943.06 5.41 0 0 0  
 ;MH rim elevation adj. to lowest connecting inlet grate elevation (DI #2210 - 4947.80) plus 8" curb height.  
 L14151 4939.08 9.82 0 0 0  
 ;MH rim elevation adj. to lowest connecting inlet grate elevation (DI #2180 - 4947.83) plus 8" curb height.  
 L14152 4937.58 11.05 0 0 0  
 L14999 4937.3 11.28 0 0 0  
 L14JB145 4934.73 13.55 0 0 0

[OUTFALLS]

;	Invert	Outfall	Stage/Table	Tide
;:Name	Elev.	Type	Time Series	Gate
L14JB2	4952.21	FREE		NO
Unknown	4952	FREE		NO

[STORAGE]

;	Invert	Max.	Init.	Storage	Curve	Ponded	Evap.
;:Name	Elev.	Depth	Depth	Curve	Params	Area	Frac.
L14PS1	4926.99	22.67	0	TABULAR	L14PS1	0	0
Park	4941.5	7	0	TABULAR	Park	0	0
SBBroadwayPond	4938.42	16.54	0	TABULAR	SB.DepthArea	0	0

[CONDUITS]

;	Inlet	Outlet	Manning	Inlet	Outlet	Init.	Max.
;:Name	Node	Node	N	Offset	Offset	Flow	Flow
Commercial_Sag	K14941	DI2046	92.157	0.017	10.86	2.56	0
E.Cromwell	K14953	K14952	170	0.017	6.31	5.28	0
Link-13	DI2024	K14951	18.41	0.013	0	0.35	0
Link-14	DI2046	K14944	46.09	0.013	0	0.15	0
Link-16	L14054	Park	58.69	0.013	0	0	0
Link-17	Park	L14151	407.62	0.013	0	0	0
N.Commercial	K14942	K14941	234	0.017	12.68	10.86	0
N.William	DI2024	K14952	355	0.017	3.02	5.28	0
Pacific	K14945	DI2046	268	0.017	11.53	2.56	0
Pacific_William	K14954	K14951	34	0.017	11.14	3.45	0
S.Commercial_A	L14142	L14141	330	0.017	12.47	11.57	0
S.Commercial_B	L14141	L14143	36	0.017	11.57	11.44	0
S.William	L14152	L14151	370	0.017	10.25	8.72	0
SDP1	SBBroadwayPond	K14954	410.95811861	0.013	0	0.1	0
SDP10	L14143	L14JB145	85.40790915	0.013	0	0.42	0
SDP11	K14951	K14952	354.71353104	0.013	0	0.08000000000000001	0
SDP12	K14952	L14052	158.63026916	0.013	0	0.4	0
SDP13	L14052	L14055	223.09104012	0.013	0	0.5	0
SDP14	L14055	L14151	339.09603202	0.013	0	0.37	0
SDP15	L14151	L14152	369.53315621	0.013	0	0.85	0
SDP16	L14152	L14999	34.22873553	0.013	0	0.77	0
SDP17	L14999	L14JB145	260.75662555	0.013	0	0.7	0
SDP19	L14055	L14051	183.13	0.013	4.69	0	0
SDP2	K14954	K14945	43.12332141	0.013	0	0.0500000000000003	0
SDP20	L14053	L14054	199.67810886	0.013	0	0	0
SDP21	K14953	L14053	191.14310008	0.013	0	0	0
SDP22	L14054	L14055	244.301283	0.013	2.38	4.487	0
SDP24	L14042	L14041	19.24659539	0.013	0	6.907	0
SDP25	L14051	L14042	180.48	0.01	0	0.6	0
SDP26	L14146	L14142	35.18528675	0.013	0	7.778	0
SDP27	K14943	K14942	47.70588865	0.013	0	9	0
SDP28	L14JB145	L14PS1	53.05839922	0.013	0	6.45	0
SDP3	K14945	K14944	267.90742539	0.013	0	0.15	0
SDP4	K14944	K14941	92.15714564	0.013	0	0.15	0

SDP5	K14941	K14942	233.62246284	0.013	0	0.2	0	0
SDP6	K14942	L14041	392.68031286	0.013	0	0.12	0	0
SDP7	L14041	L14142	349.68993313	0.013	0	0.0999999999999996	0	0
SDP8	L14142	L14141	330.32958193	0.013	0	0.0800000000000001	0	0
SDP9	L14141	L14143	35.81241274	0.013	0	1.22	0	0
W.Garfield_A	L14051	L14042	181	0.017	2.32	2.77	0	0
W.Garfield_B	L14055	L14051	183	0.017	8.71	2.32	0	0
;Initial_invert_4943.53_to_4936.6_switted_direction								
W.Garfield_Junct	L14041	L14042	19	0.017	10.29	2.77	0	0
William_1A	K14952	L14052	158	0.017	5.28	6.38	0	0
William_1B	L14055	L14052	223	0.017	8.71	6.38	0	0
William_Pacific	K14945	K14954	43	0.017	11.53	11.14	0	0
[PUMPS]								
;;Name	Inlet Node	Outlet Node	Pump Curve	Init. Status	Startup Depth	Shutoff Depth		
PUMP_CT-3500-135_02	L14PS1	L14JB2	PumpStation	OFF	3.5	0		
[WEIRS]								
;;Name	Inlet Node	Outlet Node	Weir Type	Crest Height	Disch. Coeff.	Flap Gate	End Con.	End Coeff.
EmergencySpillway	SBroadwayPond	Unknown	TRANSVERSE	14.25	0.7	NO	0	2.60
[XSECTIONS]								
;;Link	Shape	Geom1	Geom2	Geom3	Geom4	Barrels		
Commercial_Sag	IRREGULAR	Standard	0	0	0	1		
E.Cromwell	IRREGULAR	Standard	0	0	0	1		
Link-13	CIRCULAR	1.5	0	1	1	1		
Link-14	CIRCULAR	1.5	0	1	1	1		
Link-16	CIRCULAR	2.5	0	1	1	1		
Link-17	CIRCULAR	1.5	0	1	1	1		
N.Commercial	IRREGULAR	Standard	0	0	0	1		
N.William	IRREGULAR	Standard	0	0	0	1		
Pacific	IRREGULAR	Standard	0	0	0	1		
Pacific_William	IRREGULAR	Standard	0	0	0	1		
S.Commercial_A	IRREGULAR	Standard	0	0	0	1		
S.Commercial_B	IRREGULAR	Standard	0	0	0	1		
S.William	IRREGULAR	Standard	0	0	0	1		
SDP1	CIRCULAR	2.5	0	1	1	1		
SDP10	CIRCULAR	4	0	1	1	1		
SDP11	CIRCULAR	2	0	1	1	1		
SDP12	CIRCULAR	2	0	1	1	1		
SDP13	CIRCULAR	2.5	0	1	1	1		
SDP14	CIRCULAR	3	0	1	1	1		
SDP15	CIRCULAR	3	0	1	1	1		
SDP16	CIRCULAR	4	0	1	1	1		
SDP17	CIRCULAR	4	0	1	1	1		
SDP19	CIRCULAR	1	0	1	1	1		
SDP2	CIRCULAR	2.5	0	1	1	1		
SDP20	CIRCULAR	2	0	1	1	1		
SDP21	CIRCULAR	2.0000000000	0	1	1	1		
SDP22	CIRCULAR	1	0	1	1	1		
SDP24	CIRCULAR	2	0	1	1	1		
SDP25	CIRCULAR	1	0	1	1	1		
SDP26	CIRCULAR	1.5	0	1	1	1		
SDP27	CIRCULAR	1.5	0	1	1	1		
SDP28	CIRCULAR	4	0	1	1	1		
SDP3	CIRCULAR	2.5	0	1	1	1		
SDP4	CIRCULAR	2.5	0	1	1	1		
SDP5	CIRCULAR	2.5	0	1	1	1		
SDP6	CIRCULAR	2.5	0	1	1	1		
SDP7	CIRCULAR	2.5000000000	0	1	1	1		
SDP8	CIRCULAR	2.5	0	1	1	1		
SDP9	CIRCULAR	4	0	1	1	1		
W.Garfield_A	IRREGULAR	Standard	0	0	0	1		
W.Garfield_B	IRREGULAR	Standard	0	0	0	1		
W.Garfield_Junct	IRREGULAR	Standard	0	0	0	1		
William_1A	IRREGULAR	Standard	0	0	0	1		

William_1B	IRREGULAR	Standard	0	0	0	1
William_Pacific	IRREGULAR	Standard	0	0	0	1
EmergencySpillway	RECT_OPEN	1.29	73	1	1	

[TRANSECTS]

;Standard roadway section for the project with 2% cross slope, 32' full street section and 8" curb and gutter.

NC 0	0	0.017							
X1 Standard		5	0.0	0.0	0.0	0.0	0.0	0.0	0.0
GR 0.67	0	0	0.1	0.32	16	0	31.9	0.67	32

[LOSSES]

;;Link	Inlet	Outlet	Average	Flap Gate
Link-13	0.5	0.6	0	NO
Link-14	0.5	0.6	0	NO
Link-16	0.025	0.025	0	NO
Link-17	0.025	0.025	0	NO
SDP1	0.025	0.025	0	NO
SDP10	.025	.025	0	NO
SDP11	.025	.025	0	NO
SDP12	.025	.025	0	NO
SDP13	.025	.025	0	NO
SDP14	0.025	0.025	0	NO
SDP15	.025	.025	0	NO
SDP16	.025	.025	0	NO
SDP17	0.025	0.025	0	NO
SDP19	0.025	0.025	0	NO
SDP2	0.025	0.025	0	NO
SDP20	.025	.025	0	NO
SDP21	0.025	0.025	0	NO
SDP22	.025	.025	0	NO
SDP24	0.025	0.025	0	NO
SDP25	0.025	0.025	0	NO
SDP26	0.025	0.025	0	NO
SDP27	0.025	0.025	0	NO
SDP28	.025	.025	0	NO
SDP3	.025	0.025	0	NO
SDP4	0.025	0.025	0	NO
SDP5	0.025	0.025	0	NO
SDP6	0.025	0.025	0	NO
SDP7	.025	.025	0	NO
SDP8	0.025	0.025	0	NO
SDP9	.025	.025	0	NO

[INFLOWS]

;;Node	Parameter	Time Series	Param Type	Units Factor	Scale Factor	Baseline Value	Baseline Pattern
DI2024	FLOW	UH-B2	FLOW	1.0	1.0		
DI2046	FLOW	UH-B1	FLOW	1.0	1.0		
K14941	FLOW	UH-B3	FLOW	1.0	1.0		
K14952	FLOW	UH-B4	FLOW	1.0	1.0		
K14953	FLOW	UH-B5	FLOW	1.0	1.0		
L14042	FLOW	UH-B6	FLOW	1.0	1.0		
L14052	FLOW	UH-B7	FLOW	1.0	1.0		
L14053	FLOW	UH-B8	FLOW	1.0	1.0		
L14055	FLOW	UH-B9	FLOW	1.0	1.0		
L14142	FLOW	UH-B10	FLOW	1.0	1.0		
L14143	FLOW	UH-B12	FLOW	1.0	1.0		
L14151	FLOW	UH-B11	FLOW	1.0	1.0		
L14152	FLOW	UH-B13	FLOW	1.0	1.0		
SBroadwayPond	FLOW	S Broadway Pond	FLOW	1.0	1.0		

[CURVES]

;;Name	Type	X-Value	Y-Value
PumpStation	Pump3	26.5	80
PumpStation		28	75
PumpStation		29.5	70
PumpStation		31.5	65

PumpStation		32	60	
PumpStation		33	55	
PumpStation		33.5	50	
PumpStation		35	45	
PumpStation		37	40	
PumpStation		47	0	
L14PS1	Storage	0	0	
L14PS1		25	496	
;Conic Reservoir Volume assuming 140 ft x 140 ft footprint w/ 6:1 side slopes.				
Park	Storage	0.0	19600.00	
Park		0.5	21316.00	
Park		1.0	23104.00	
Park		1.5	24964.00	
Park		2.0	26896.00	
Park		2.5	28900.00	
Park		3.0	30976.00	
Park		3.5	33124.00	
Park		4.0	35344.00	
Park		4.5	37636.00	
Park		5.0	40000.00	
Park		5.5	42436.00	
Park		6.0	44944.00	
Park		6.5	47524.00	
Park		7.0	50176.00	
SB.DepthArea	Storage	0	0	
SB.DepthArea		3.94	21780	
SB.DepthArea		4.42	43560	
SB.DepthArea		5.19	65340	
SB.DepthArea		8.74	87120	
SB.DepthArea		14.36	108900	
SB.DepthArea		16.54	121968	
[TIMESERIES]				
; <td>:Name</td> <td>Date</td> <td>Time</td> <td>Value</td>	:Name	Date	Time	Value
;	-----	-----	-----	-----
;5/6/2016	0:02	38.99		
S Broadway Pond	5/6/2016	0:04	57.38	
S Broadway Pond	5/6/2016	0:05	72.32	
S Broadway Pond	5/6/2016	0:06	91.17	
S Broadway Pond	5/6/2016	0:07	113.22	
S Broadway Pond	5/6/2016	0:08	141.27	
S Broadway Pond	5/6/2016	0:09	183.18	
S Broadway Pond	5/6/2016	0:10	241.75	
S Broadway Pond	5/6/2016	0:11	309.74	
S Broadway Pond	5/6/2016	0:12	354.16	
S Broadway Pond	5/6/2016	0:13	409.71	
S Broadway Pond	5/6/2016	0:14	444.98	
S Broadway Pond	5/6/2016	0:15	477.07	
S Broadway Pond	5/6/2016	0:16	502.79	
S Broadway Pond	5/6/2016	0:17	517.66	
S Broadway Pond	5/6/2016	0:18	527.53	
S Broadway Pond	5/6/2016	0:19	523.01	
S Broadway Pond	5/6/2016	0:20	513.8	
S Broadway Pond	5/6/2016	0:21	503.6	
S Broadway Pond	5/6/2016	0:22	491.45	
S Broadway Pond	5/6/2016	0:23	483.62	
S Broadway Pond	5/6/2016	0:24	475.38	
S Broadway Pond	5/6/2016	0:25	470.44	
S Broadway Pond	5/6/2016	0:26	466.98	
S Broadway Pond	5/6/2016	0:27	464.4	
S Broadway Pond	5/6/2016	0:28	462.93	
S Broadway Pond	5/6/2016	0:29	461.78	
S Broadway Pond	5/6/2016	0:30	460.75	
S Broadway Pond	5/6/2016	0:31	460.69	
S Broadway Pond	5/6/2016	0:32	458.8	
S Broadway Pond	5/6/2016	0:33	454.5	
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S Broadway Pond	5/6/2016	0:36	429.75
S Broadway Pond	5/6/2016	0:37	422.16
S Broadway Pond	5/6/2016	0:38	415.24
S Broadway Pond	5/6/2016	0:39	408.53
S Broadway Pond	5/6/2016	0:40	401.43
S Broadway Pond	5/6/2016	0:41	395.71
S Broadway Pond	5/6/2016	0:42	390.7
S Broadway Pond	5/6/2016	0:43	386
S Broadway Pond	5/6/2016	0:44	382
S Broadway Pond	5/6/2016	0:45	377.89
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S Broadway Pond	5/6/2016	0:50	344.71
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S Broadway Pond	5/6/2016	0:52	327.2
S Broadway Pond	5/6/2016	0:53	318.7
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S Broadway Pond	5/6/2016	13:26	6.76
S Broadway Pond	5/6/2016	13:27	6.76
S Broadway Pond	5/6/2016	13:28	6.76
S Broadway Pond	5/6/2016	13:29	6.76
S Broadway Pond	5/6/2016	13:30	6.76
S Broadway Pond	5/6/2016	13:31	6.76
S Broadway Pond	5/6/2016	13:32	6.76
S Broadway Pond	5/6/2016	13:33	6.76
S Broadway Pond	5/6/2016	13:34	6.76
S Broadway Pond	5/6/2016	13:35	6.76
S Broadway Pond	5/6/2016	13:36	6.77







S Broadway Pond	5/6/2016	17:10	6.81
S Broadway Pond	5/6/2016	17:11	6.8
S Broadway Pond	5/6/2016	17:12	6.8
S Broadway Pond	5/6/2016	17:13	6.8
S Broadway Pond	5/6/2016	17:14	6.8
S Broadway Pond	5/6/2016	17:15	6.8
S Broadway Pond	5/6/2016	17:16	6.8
S Broadway Pond	5/6/2016	17:17	6.8
S Broadway Pond	5/6/2016	17:18	6.8
S Broadway Pond	5/6/2016	17:19	6.8
S Broadway Pond	5/6/2016	17:20	6.8
S Broadway Pond	5/6/2016	17:21	6.8
S Broadway Pond	5/6/2016	17:22	6.8
S Broadway Pond	5/6/2016	17:23	6.8
S Broadway Pond	5/6/2016	17:24	6.8
S Broadway Pond	5/6/2016	17:25	6.8
S Broadway Pond	5/6/2016	17:26	6.8
S Broadway Pond	5/6/2016	17:27	6.8
S Broadway Pond	5/6/2016	17:28	6.8
S Broadway Pond	5/6/2016	17:29	6.8
S Broadway Pond	5/6/2016	17:30	6.8
S Broadway Pond	5/6/2016	17:31	6.8
S Broadway Pond	5/6/2016	17:32	6.8
S Broadway Pond	5/6/2016	17:33	6.8
S Broadway Pond	5/6/2016	17:34	6.8
S Broadway Pond	5/6/2016	17:35	6.8
S Broadway Pond	5/6/2016	17:36	6.8
S Broadway Pond	5/6/2016	17:37	6.8
S Broadway Pond	5/6/2016	17:38	6.8
S Broadway Pond	5/6/2016	17:39	6.8
S Broadway Pond	5/6/2016	17:40	6.8
S Broadway Pond	5/6/2016	17:41	6.8
S Broadway Pond	5/6/2016	17:42	6.8
S Broadway Pond	5/6/2016	17:43	6.8
S Broadway Pond	5/6/2016	17:44	6.8
S Broadway Pond	5/6/2016	17:45	6.8
S Broadway Pond	5/6/2016	17:46	6.8
S Broadway Pond	5/6/2016	17:47	6.8
S Broadway Pond	5/6/2016	17:48	6.8
S Broadway Pond	5/6/2016	17:49	6.8
S Broadway Pond	5/6/2016	17:50	6.8
S Broadway Pond	5/6/2016	17:51	6.8
S Broadway Pond	5/6/2016	17:52	6.8
S Broadway Pond	5/6/2016	17:53	6.8
S Broadway Pond	5/6/2016	17:54	6.8
S Broadway Pond	5/6/2016	17:55	6.8
S Broadway Pond	5/6/2016	17:56	6.8
S Broadway Pond	5/6/2016	17:57	6.8
S Broadway Pond	5/6/2016	17:58	6.8
S Broadway Pond	5/6/2016	17:59	6.8

;Unit hydrograph for sub-basin #1 developed using COA DPM Modified Rational Method.

UH-B1	5/6/2016	0:00	0
UH-B1	5/6/2016	00:12	10.21
UH-B1	5/6/2016	00:24	10.21
UH-B1	5/6/2016	00:43	0

;Unit hydrograph for sub-basin #10 using COA DPM Modified Rational Method.

UH-B10	5/6/2016	0:00	0
UH-B10	5/6/2016	0:14	5.03
UH-B10	5/6/2016	0:21	5.03
UH-B10	5/6/2016	0:45	0

;Unit hydrograph for sub-basin #11 using COA DPM Modified Rational Method.

UH-B11	5/6/2016	0:00	0
UH-B11	5/6/2016	0:15	17.84
UH-B11	5/6/2016	0:19	17.84
UH-B11	5/6/2016	0:45	0

;Unit hydrograph for sub-basin 12 using COA DPM Modified Rational Method.

UH-B12	5/6/2016	0:00	0
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UH-B12      5/6/2016  0:14      7.08
UH-B12      5/6/2016  0:20      7.08
UH-B12      5/6/2016  0:45      0

;Unit hydrograph for sub-basin 13 using COA DPM Modified Rational Method.
UH-B13      5/6/2016  0:00      0
UH-B13      5/6/2016  0:14     26.51
UH-B13      5/6/2016  0:21     26.51
UH-B13      5/6/2016  0:45      0

;Unit hydrograph for sub-basin #2 using COA DPM Modified Rational Method.
UH-B2       5/6/2016  0:00      0
UH-B2       5/6/2016  0:14    21.48
UH-B2       5/6/2016  0:21    21.48
UH-B2       5/6/2016  0:45      0

;Unit hydrograph for sub-basin #3 using COA DPM Modified Rational Method.
UH-B3       5/6/2016  0:00      0
UH-B3       5/6/2016  0:14     6.43
UH-B3       5/6/2016  0:20     6.43
UH-B3       5/6/2016  0:45      0

;Unit hydrograph for sub-basin #4 using COA DPM Modified Rational Method.
UH-B4       5/6/2016  0:00      0
UH-B4       5/6/2016  0:14    6.96
UH-B4       5/6/2016  0:21    6.96
UH-B4       5/6/2016  0:45      0

;Unit hydrograph for sub-basin #5 using COA DPM Modified Rational Method
UH-B5       5/6/2016  0:00      0
UH-B5       5/6/2016  0:14   14.64
UH-B5       5/6/2016  0:21   14.64
UH-B5       5/6/2016  0:45      0

;Unit hydrograph for sub-basin #6 using COA DPM Modified Rational Method.
UH-B6       5/6/2016  0:00      0
UH-B6       5/6/2016  0:14   11.73
UH-B6       5/6/2016  0:21   11.73
UH-B6       5/6/2016  0:45      0

;Unit hydrograph for sub-basin #7 using COA DPM Modified Rational Method.
UH-B7       5/6/2016  0:00      0
UH-B7       5/6/2016  0:14    9.15
UH-B7       5/6/2016  0:21    9.15
UH-B7       5/6/2016  0:45      0

;Unit hydrograph for sub-basin #8 using COA DPM Modified Rational Method.
UH-B8       5/6/2016  0:00      0
UH-B8       5/6/2016  0:14    8.71
UH-B8       5/6/2016  0:21    8.71
UH-B8       5/6/2016  0:45      0

;Unit hydrograph for sub-basin #9 using COA DPM Modified Rational Method.
UH-B9       5/6/2016  0:00      0
UH-B9       5/6/2016  0:14   11.05
UH-B9       5/6/2016  0:21   11.05
UH-B9       5/6/2016  0:45      0

[REPORT]
INPUT      YES
CONTROLS   YES
SUBCATCHMENTS ALL
NODES ALL
LINKS ALL

[TAGS]

[MAP]
DIMENSIONS 1520851.510 1481094.290 1521703.260 1483184.090
Units      Feet

```

[COORDINATES]

Node	X-Coord	Y-Coord
DI2024	1521464.400	1482627.250
DI2046	1521132.640	1482625.370
K14941	1521127.100	1482521.880
K14942	1521091.600	1482287.960
K14943	1521139.220	1482285.170
K14944	1521142.930	1482609.670
K14945	1521410.070	1482629.980
K14951	1521435.400	1482633.430
K14952	1521444.180	1482278.820
K14953	1521650.770	1482287.060
K14954	1521432.240	1482666.960
L14041	1521030.200	1481900.110
L14042	1521049.290	1481897.720
L14051	1521229.760	1481896.040
L14052	1521448.090	1482120.240
L14053	1521660.680	1482098.580
L14054	1521657.120	1481896.300
L14055	1521412.850	1481899.950
L14141	1520921.980	1481228.760
L14142	1520974.200	1481554.940
L14143	1520918.320	1481193.130
L14146	1521008.940	1481549.410
L14151	1521357.360	1481565.420
L14152	1521298.170	1481200.660
L14999	1521264.330	1481195.520
L14JB145	1521003.650	1481189.280
L14JB2	1520997.600	1481244.150
Unknown	1521215.440	1483020.040
L14PS1	1520997.730	1481242.010
Park	1521664.540	1481838.510
SBroadwayPond	1521440.510	1483089.100

[VERTICES]

Link	X-Coord	Y-Coord
Commercial_Sag	1521091.070	1482563.310
E_Cromwell	1521561.320	1482327.700
Link-17	1521643.100	1481566.870
N.Commercial	1521030.160	1482398.290
N.William	1521487.300	1482435.300
Pacific	1521246.240	1482667.330
Pacific_William	1521465.120	1482651.400
S.Commercial_A	1520895.830	1481387.380
S.Commercial_B	1520890.230	1481220.240
S.William	1521398.890	1481381.210
W.Garfield_A	1521152.260	1481980.280
W.Garfield_B	1521238.510	1481985.020
W.Garfield_Junct	1521043.840	1481915.030
William_1A	1521513.220	1482191.700
William_1B	1521505.120	1482095.360
William_Pacific	1521394.650	1482667.620

[PROFILES]

Name	Links
"SouthBroadwaytoPump"	SDP1 SDP2 SDP3 SDP4 SDP5
"SouthBroadwaytoPump"	SDP6 SDP7 SDP8 SDP9 SDP10
"SouthBroadwaytoPump"	SDP28
"SouthBroadway Pond to Pump Station through Road"	SDP1 William_Pacific Pacific_Commercial SDP4 SDP5
"SouthBroadway Pond to Pump Station through Road"	SDP6 SDP7 SDP8 SDP9 SDP10
"SouthBroadway Pond to Pump Station through Road"	SDP28
"	SDP1 Williams_Pacific SDP4 SDP5 SDP6
"	SDP7 SDP8 SDP9 SDP10 SDP28
"SouthBroadway Street Profile"	SDP1 Williams_Pacific SDP4 SDP5 SDP6
"SouthBroadway Street Profile"	SDP7 SDP8 SDP9 SDP10 SDP28
"FullStreet"	SDP1 Williams_Pacific Commercial SDP5 SDP6
"FullStreet"	SDP7 SDP8 SDP9 SDP10 SDP28

## SWMM Model Output - Recommended Improvements

EPA STORM WATER MANAGEMENT MODEL - VERSION 5.0 (Build 5.0.022)

\*\*\*\*\*  
NOTE: The summary statistics displayed in this report are  
based on results found at every computational time step,  
not just on results from each reporting time step.  
\*\*\*\*\*

\*\*\*\*\*  
Analysis Options  
\*\*\*\*\*  
Flow Units ..... CFS  
Process Models:  
Rainfall/Runoff ..... NO  
Snowmelt ..... NO  
Groundwater ..... NO  
Flow Routing ..... YES  
Ponding Allowed ..... NO  
Water Quality ..... NO  
Flow Routing Method ..... DYNWAVE  
Starting Date ..... MAY-06-2016 00:00:00  
Ending Date ..... MAY-07-2016 00:00:00  
Antecedent Dry Days ..... 0.0  
Report Time Step ..... 00:00:15  
Routing Time Step ..... 15.00 sec

WARNING 02: maximum depth increased for Node L14051

\*\*\*\*\*  
Element Count  
\*\*\*\*\*  
Number of rain gages ..... 0  
Number of subcatchments ... 0  
Number of nodes ..... 31  
Number of links ..... 47  
Number of pollutants ..... 0  
Number of land uses ..... 0

\*\*\*\*\*  
Node Summary  
\*\*\*\*\*

Name	Type	Invert Elev.	Max. Depth	Ponded Area	External Inflow
DI2024	JUNCTION	4945.64	3.87	0.0	Yes
DI2046	JUNCTION	4945.39	3.50	0.0	Yes
K14941	JUNCTION	4937.35	11.75	0.0	Yes
K14942	JUNCTION	4936.82	13.35	0.0	
K14943	JUNCTION	4946.02	3.75	0.0	
K14944	JUNCTION	4937.60	11.85	0.0	
K14945	JUNCTION	4937.94	12.20	0.0	
K14951	JUNCTION	4945.21	4.60	0.0	
K14952	JUNCTION	4943.04	6.05	0.0	Yes
K14953	JUNCTION	4943.86	7.17	0.0	Yes
K14954	JUNCTION	4937.94	11.81	0.0	
L14041	JUNCTION	4936.60	11.17	0.0	
L14042	JUNCTION	4943.53	3.70	0.0	Yes
L14051	JUNCTION	4944.35	2.99	0.0	
L14052	JUNCTION	4941.40	7.23	0.0	Yes
L14053	JUNCTION	4943.06	7.31	0.0	Yes
L14054	JUNCTION	4942.28	9.58	0.0	
L14055	JUNCTION	4939.88	9.56	0.0	Yes
L14141	JUNCTION	4936.08	12.24	0.0	
L14142	JUNCTION	4935.24	13.23	0.0	Yes
L14143	JUNCTION	4935.66	12.35	0.0	Yes
L14146	JUNCTION	4943.06	5.41	0.0	

L14151	JUNCTION	4939.08	9.82	0.0	Yes
L14152	JUNCTION	4937.58	11.05	0.0	Yes
L14999	JUNCTION	4937.30	11.28	0.0	
L14JB145	JUNCTION	4934.73	13.55	0.0	
L14JB2	OUTFALL	4952.21	0.00	0.0	
Unknown	OUTFALL	4952.00	15.54	0.0	
L14PS1	STORAGE	4926.99	22.67	0.0	
Park	STORAGE	4941.50	7.00	0.0	
SBroadwayPond	STORAGE	4938.42	16.54	0.0	Yes

\*\*\*\*\*  
Link Summary  
\*\*\*\*\*

Name	From Node	To Node	Type	Length	%Slope	Roughness
Commercial_Sag	K14941	DI2046	CONDUIT	92.2	0.2821	0.0170
E.Cromwell	K14953	K14952	CONDUIT	170.0	1.0883	0.0170
Link-13	DI2024	K14951	CONDUIT	18.4	0.4346	0.0130
Link-14	DI2046	K14944	CONDUIT	46.1	16.8088	0.0130
Link-16	L14054	Park	CONDUIT	58.7	1.3291	0.0130
Link-17	Park	L14151	CONDUIT	407.6	0.5937	0.0130
N.Commercial	K14942	K14941	CONDUIT	234.0	0.5513	0.0170
N.William	DI2024	K14952	CONDUIT	355.0	0.0958	0.0170
Pacific	K14945	DI2046	CONDUIT	268.0	0.5672	0.0170
Pacific_William	K14954	K14951	CONDUIT	34.0	1.2354	0.0170
S.Commercial_A	L14142	L14141	CONDUIT	330.0	0.0182	0.0170
S.Commercial_B	L14141	L14143	CONDUIT	36.0	1.5280	0.0170
S.William	L14152	L14151	CONDUIT	370.0	0.0081	0.0170
SDP1	SBroadwayPond	K14954	CONDUIT	411.0	0.0925	0.0130
SDP10	L14143	L14JB145	CONDUIT	85.4	0.5971	0.0130
SDP11	K14951	K14952	CONDUIT	354.7	0.5892	0.0130
SDP12	K14952	L14052	CONDUIT	158.6	0.7817	0.0130
SDP13	L14052	L14055	CONDUIT	223.1	0.4572	0.0130
SDP14	L14055	L14151	CONDUIT	339.1	0.1268	0.0130
SDP15	L14151	L14152	CONDUIT	369.5	0.1759	0.0130
SDP16	L14999	L14152	CONDUIT	34.2	1.4317	0.0130
SDP17	L14999	L14JB145	CONDUIT	260.8	0.7172	0.0130
SDP19	L14055	L14051	CONDUIT	183.1	0.1201	0.0130
SDP2	K14945	K14954	CONDUIT	43.1	0.1159	0.0130
SDP20	L14053	L14054	CONDUIT	199.7	0.3906	0.0130
SDP21	K14953	L14053	CONDUIT	191.1	0.4185	0.0130
SDP22	L14054	L14055	CONDUIT	244.3	0.1199	0.0130
SDP24	L14042	L14041	CONDUIT	19.2	0.1195	0.0130
SDP25	L14051	L14042	CONDUIT	180.5	0.1219	0.0100
SDP26	L14146	L14142	CONDUIT	35.2	0.1194	0.0130
SDP27	K14943	K14942	CONDUIT	47.7	0.4192	0.0130
SDP28	L14JB145	L14PS1	CONDUIT	53.1	2.4320	0.0130
SDP3	K14945	K14944	CONDUIT	267.9	0.0709	0.0130
SDP4	K14944	K14941	CONDUIT	92.2	0.1085	0.0130
SDP5	K14941	K14942	CONDUIT	233.6	0.1413	0.0130
SDP6	K14942	L14041	CONDUIT	392.7	0.0255	0.0130
SDP7	L14041	L14142	CONDUIT	349.7	0.3603	0.0130
SDP8	L14141	L14142	CONDUIT	330.3	0.2785	0.0130
SDP9	L14143	L14141	CONDUIT	35.8	2.2344	0.0130
W.Garfield_A	L14051	L14042	CONDUIT	181.0	0.2044	0.0170
W.Garfield_B	L14055	L14051	CONDUIT	183.0	1.0492	0.0170
W.Garfield_Junct	L14041	L14042	CONDUIT	19.0	3.1068	0.0170
William_1A	K14952	L14052	CONDUIT	158.0	0.3418	0.0170
William_1B	L14055	L14052	CONDUIT	223.0	0.3632	0.0170
William_Pacific	K14945	K14954	CONDUIT	43.0	0.9070	0.0170
PUMP_CT-3500-135_02L14PS1		L14JB2	TYPE3 PUMP			
EmergencySpillway	SBroadwayPond	Unknown	WEIR			

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Cross Section Summary  
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Conduit	Shape	Full Depth	Full Area	Hyd. Rad.	Max. Width	No. of Barrels	Full Flow
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Commercial_Sag	Standard	0.67	16.28	0.49	32.00	1	46.94
E.Cromwell	Standard	0.67	16.28	0.49	32.00	1	92.19
Link-13	CIRCULAR	1.50	1.77	0.38	1.50	1	6.92
Link-14	CIRCULAR	1.50	1.77	0.38	1.50	1	43.07
Link-16	CIRCULAR	2.50	4.91	0.63	2.50	1	47.29
Link-17	CIRCULAR	1.50	1.77	0.38	1.50	1	8.09
N.Commercial	Standard	0.67	16.28	0.49	32.00	1	65.61
N.William	Standard	0.67	16.28	0.49	32.00	1	27.35
Pacific	Standard	0.67	16.28	0.49	32.00	1	66.55
Pacific_William	Standard	0.67	16.28	0.49	32.00	1	98.22
S.Commercial_A	Standard	0.67	16.28	0.49	32.00	1	11.92
S.Commercial_B	Standard	0.67	16.28	0.49	32.00	1	109.23
S.William	Standard	0.67	16.28	0.49	32.00	1	7.96
SDP1	CIRCULAR	2.50	4.91	0.63	2.50	1	12.47
SDP10	CIRCULAR	4.00	12.57	1.00	4.00	1	111.00
SDP11	CIRCULAR	2.00	3.14	0.50	2.00	1	17.37
SDP12	CIRCULAR	2.00	3.14	0.50	2.00	1	20.00
SDP13	CIRCULAR	2.50	4.91	0.63	2.50	1	27.73
SDP14	CIRCULAR	3.00	7.07	0.75	3.00	1	23.75
SDP15	CIRCULAR	3.00	7.07	0.75	3.00	1	27.97
SDP16	CIRCULAR	4.00	12.57	1.00	4.00	1	171.87
SDP17	CIRCULAR	4.00	12.57	1.00	4.00	1	121.64
SDP19	CIRCULAR	1.00	0.79	0.25	1.00	1	1.23
SDP2	CIRCULAR	2.50	4.91	0.63	2.50	1	13.97
SDP20	CIRCULAR	2.00	3.14	0.50	2.00	1	14.14
SDP21	CIRCULAR	2.00	3.14	0.50	2.00	1	14.64
SDP22	CIRCULAR	1.00	0.79	0.25	1.00	1	1.23
SDP24	CIRCULAR	2.00	3.14	0.50	2.00	1	7.82
SDP25	CIRCULAR	1.00	0.79	0.25	1.00	1	1.62
SDP26	CIRCULAR	1.50	1.77	0.38	1.50	1	3.63
SDP27	CIRCULAR	1.50	1.77	0.38	1.50	1	6.80
SDP28	CIRCULAR	4.00	12.57	1.00	4.00	1	224.01
SDP3	CIRCULAR	2.50	4.91	0.63	2.50	1	10.92
SDP4	CIRCULAR	2.50	4.91	0.63	2.50	1	13.51
SDP5	CIRCULAR	2.50	4.91	0.63	2.50	1	15.42
SDP6	CIRCULAR	2.50	4.91	0.63	2.50	1	6.55
SDP7	CIRCULAR	2.50	4.91	0.63	2.50	1	24.62
SDP8	CIRCULAR	2.50	4.91	0.63	2.50	1	21.65
SDP9	CIRCULAR	4.00	12.57	1.00	4.00	1	214.72
W.Garfield_A	Standard	0.67	16.28	0.49	32.00	1	39.95
W.Garfield_B	Standard	0.67	16.28	0.49	32.00	1	90.52
W.Garfield_Junct	Standard	0.67	16.28	0.49	32.00	1	155.76
William_1A	Standard	0.67	16.28	0.49	32.00	1	51.66
William_1B	Standard	0.67	16.28	0.49	32.00	1	53.26
William_Pacific	Standard	0.67	16.28	0.49	32.00	1	84.16

\*\*\*\*\*  
Transect Summary  
\*\*\*\*\*

Transect Standard  
Area:

0.0005	0.0022	0.0049	0.0088	0.0137
0.0198	0.0269	0.0352	0.0445	0.0550
0.0665	0.0791	0.0929	0.1077	0.1236
0.1407	0.1588	0.1780	0.1984	0.2198
0.2423	0.2660	0.2907	0.3165	0.3428
0.3690	0.3953	0.4215	0.4478	0.4740
0.5003	0.5266	0.5528	0.5791	0.6054
0.6317	0.6580	0.6843	0.7106	0.7369
0.7632	0.7895	0.8158	0.8421	0.8684
0.8947	0.9210	0.9473	0.9737	1.0000

Hrad:

0.0134	0.0268	0.0402	0.0536	0.0670
0.0805	0.0939	0.1073	0.1207	0.1341
0.1475	0.1609	0.1743	0.1877	0.2011
0.2145	0.2279	0.2414	0.2548	0.2682
0.2816	0.2950	0.3084	0.3234	0.3499
0.3764	0.4028	0.4292	0.4556	0.4819

0.5082	0.5344	0.5606	0.5868	0.6129
0.6390	0.6650	0.6910	0.7170	0.7429
0.7688	0.7947	0.8205	0.8462	0.8719
0.8976	0.9233	0.9489	0.9745	1.0000
Width:				
0.0417	0.0835	0.1252	0.1670	0.2087
0.2504	0.2922	0.3339	0.3756	0.4174
0.4591	0.5009	0.5426	0.5843	0.6261
0.6678	0.7096	0.7513	0.7930	0.8348
0.8765	0.9182	0.9600	0.9968	0.9969
0.9970	0.9971	0.9972	0.9974	0.9975
0.9976	0.9977	0.9979	0.9980	0.9981
0.9983	0.9984	0.9985	0.9986	0.9988
0.9989	0.9990	0.9991	0.9992	0.9994
0.9995	0.9996	0.9998	0.9999	1.0000

\*\*\*\*\*  
Control Actions Taken  
\*\*\*\*\*

*****	Volume	Volume
Flow Routing Continuity	acre-feet	10^6 gal
*****	-----	-----
Dry Weather Inflow .....	0.000	0.000
Wet Weather Inflow .....	0.000	0.000
Groundwater Inflow .....	0.000	0.000
RDII Inflow .....	0.000	0.000
External Inflow .....	59.383	19.351
External Outflow .....	59.297	19.323
Internal Outflow .....	0.000	0.000
Storage Losses .....	0.000	0.000
Initial Stored Volume ....	0.000	0.000
Final Stored Volume .....	0.050	0.016
Continuity Error (%) .....	0.060	

\*\*\*\*\*  
Highest Continuity Errors  
\*\*\*\*\*  
Node L14051 (-6.25%)

\*\*\*\*\*  
Time-Step Critical Elements  
\*\*\*\*\*  
Link SDP9 (81.67%)  
Link SDP28 (7.73%)  
Link SDP2 (2.24%)  
Link SDP24 (1.49%)  
Link Link-16 (1.23%)

\*\*\*\*\*  
Highest Flow Instability Indexes  
\*\*\*\*\*  
Link SDP28 (2)  
Link SDP10 (2)  
Link SDP9 (2)  
Link SDP11 (1)  
Link SDP16 (1)

\*\*\*\*\*  
Routing Time Step Summary  
\*\*\*\*\*  
Minimum Time Step : 0.50 sec  
Average Time Step : 2.68 sec  
Maximum Time Step : 15.00 sec  
Percent in Steady State : 0.00

Average Iterations per Step : 2.14

\*\*\*\*\*  
Node Depth Summary  
\*\*\*\*\*

Node	Type	Average Depth Feet	Maximum Depth Feet	Maximum HGL Feet	Time of Max Occurrence days hr:min
DI2024	JUNCTION	0.32	3.57	4949.21	0 00:24
DI2046	JUNCTION	0.70	3.33	4948.72	0 00:39
K14941	JUNCTION	6.74	11.37	4948.72	0 00:38
K14942	JUNCTION	6.18	11.30	4948.12	0 00:36
K14943	JUNCTION	0.03	2.10	4948.12	0 00:36
K14944	JUNCTION	6.92	11.23	4948.83	0 00:39
K14945	JUNCTION	7.79	11.38	4949.32	0 00:42
K14951	JUNCTION	0.40	3.92	4949.13	0 00:38
K14952	JUNCTION	0.32	5.64	4948.68	0 00:27
K14953	JUNCTION	0.07	3.01	4946.87	0 00:21
K14954	JUNCTION	8.00	11.49	4949.43	0 00:46
L14041	JUNCTION	4.64	10.63	4947.23	0 00:32
L14042	JUNCTION	0.09	3.70	4947.23	0 00:32
L14051	JUNCTION	0.06	2.89	4947.24	0 00:33
L14052	JUNCTION	0.34	6.92	4948.32	0 00:29
L14053	JUNCTION	0.09	3.05	4946.11	0 00:12
L14054	JUNCTION	0.11	1.64	4943.92	0 00:44
L14055	JUNCTION	0.43	7.47	4947.35	0 00:25
L14141	JUNCTION	2.06	9.39	4945.47	0 00:24
L14142	JUNCTION	4.41	11.08	4946.32	0 00:26
L14143	JUNCTION	1.49	9.80	4945.46	0 00:24
L14146	JUNCTION	0.04	3.26	4946.32	0 00:25
L14151	JUNCTION	0.54	7.62	4946.70	0 00:24
L14152	JUNCTION	0.88	8.29	4945.87	0 00:24
L14999	JUNCTION	0.40	8.51	4945.81	0 00:24
L14JB145	JUNCTION	1.19	10.70	4945.43	0 00:24
L14JB2	OUTFALL	0.00	0.00	4952.21	0 00:00
Unknown	OUTFALL	0.00	0.00	4952.00	0 00:00
L14PS1	STORAGE	0.57	18.25	4945.24	0 00:24
Park	STORAGE	0.28	2.22	4943.72	0 00:43
SBroadwayPond	STORAGE	9.63	16.51	4954.93	0 01:15

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Node Inflow Summary  
\*\*\*\*\*

Node	Type	Maximum Lateral Inflow CFS	Maximum Total Inflow CFS	Time of Max Occurrence days hr:min	Lateral Inflow Volume 10^6 gal	Total Inflow Volume 10^6 gal
DI2024	JUNCTION	21.48	21.48	0 00:14	0.251	0.253
DI2046	JUNCTION	10.21	11.20	0 00:27	0.126	0.153
K14941	JUNCTION	6.43	33.06	0 01:12	0.074	13.509
K14942	JUNCTION	0.00	33.18	0 00:55	0.000	13.504
K14943	JUNCTION	0.00	0.61	0 00:20	0.000	0.001
K14944	JUNCTION	0.00	33.06	0 01:12	0.000	13.419
K14945	JUNCTION	0.00	33.06	0 01:11	0.000	13.316
K14951	JUNCTION	0.00	22.30	0 00:48	0.000	0.911
K14952	JUNCTION	6.96	24.70	0 00:24	0.081	1.098
K14953	JUNCTION	14.64	14.64	0 00:14	0.171	0.171
K14954	JUNCTION	0.00	47.27	0 01:15	0.000	14.080
L14041	JUNCTION	0.00	33.82	0 00:55	0.000	13.636
L14042	JUNCTION	11.73	12.86	0 00:21	0.137	0.148
L14051	JUNCTION	0.00	5.94	0 00:27	0.000	0.024
L14052	JUNCTION	9.15	30.42	0 00:24	0.107	1.206
L14053	JUNCTION	8.71	23.37	0 00:15	0.102	0.273

L14054	JUNCTION	0.00	26.15	0 00:21	0.000	0.312
L14055	JUNCTION	11.05	40.04	0 00:15	0.129	1.351
L14141	JUNCTION	0.00	38.76	0 00:52	0.000	13.684
L14142	JUNCTION	5.03	35.50	0 00:53	0.059	13.693
L14143	JUNCTION	7.08	43.66	0 00:52	0.081	13.761
L14146	JUNCTION	0.00	1.88	0 00:18	0.000	0.001
L14151	JUNCTION	17.84	56.44	0 00:15	0.196	1.903
L14152	JUNCTION	26.51	78.77	0 00:15	0.309	2.106
L14999	JUNCTION	0.00	78.10	0 00:15	0.000	2.105
L14JB145	JUNCTION	0.00	104.54	0 00:13	0.000	15.869
L14JB2	OUTFALL	0.00	80.00	0 00:10	0.000	15.873
Unknown	OUTFALL	0.00	118.36	0 01:15	0.000	3.449
L14PS1	STORAGE	0.00	97.86	0 00:12	0.000	15.872
Park	STORAGE	0.00	36.49	0 00:21	0.000	0.412
SBroadwayPond	STORAGE	527.47	527.47	0 00:18	17.525	17.530

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Node Surcharge Summary  
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Surcharging occurs when water rises above the top of the highest conduit.

Node	Type	Hours Surcharged	Max. Height Above Crown	Min. Depth Below Rim
DI2046	JUNCTION	0.15	0.104	0.166
K14943	JUNCTION	0.31	0.595	1.655
K14944	JUNCTION	17.89	8.584	0.616
L14042	JUNCTION	0.22	0.260	0.000
L14053	JUNCTION	0.25	1.050	4.260
L14146	JUNCTION	0.39	1.757	2.153
L14999	JUNCTION	0.45	3.738	2.772
L14JB145	JUNCTION	0.59	6.001	2.849
L14PS1	STORAGE	0.69	7.795	4.425
SBroadwayPond	STORAGE	0.92	0.967	0.033

\*\*\*\*\*  
Node Flooding Summary  
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Flooding refers to all water that overflows a node, whether it ponds or not.

Node	Hours Flooded	Maximum Rate CFS	Time of Max Occurrence days hr:min	Total Flood Volume 10^6 gal	Maximum Ponded Depth Feet
L14042	0.02	0.11	0 00:33	0.000	3.70

\*\*\*\*\*  
Storage Volume Summary  
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Storage Unit	Average Volume 1000 ft3	Avg Pcnt Full	E&I Pcnt Loss	Maximum Volume 1000 ft3	Max Pcnt Full	Time of Max Occurrence days hr:min	Maximum Outflow CFS
L14PS1	0.083	2	0	3.302	65	0 00:24	80.00
Park	5.892	2	0	52.296	22	0 00:43	10.75
SBroadwayPond	505.729	43	0	1169.582	100	0 01:15	165.63

\*\*\*\*\*  
Outfall Loading Summary  
\*\*\*\*\*

Outfall Node	Flow	Avg.	Max.	Total
	Freq. Pcnt.	Flow CFS	Flow CFS	Volume $10^6$ gal
L14JB2	99.79	30.94	80.00	15.873
Unknown	13.31	54.29	118.36	3.449
System	56.55	85.23	173.33	19.322

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Link Flow Summary  
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Link	Type	Maximum  Flow  CFS	Time of Max Occurrence days hr:min	Maximum  Veloc  ft/sec	Max/ Full Flow	Max/ Full Depth
Commercial_Sag	CHANNEL	7.22	0 00:35	0.81	0.15	0.88
E.Cromwell	CHANNEL	0.00	0 00:00	0.00	0.00	0.27
Link-13	CONDUIT	15.24	0 00:13	8.63	2.20	1.00
Link-14	CONDUIT	10.20	0 00:12	10.53	0.24	1.00
Link-16	CONDUIT	26.14	0 00:21	16.46	0.55	0.77
Link-17	CONDUIT	10.97	0 00:24	6.53	1.36	1.00
N.Commercial	CHANNEL	0.00	0 00:00	0.00	0.00	0.38
N.William	CHANNEL	12.62	0 00:24	1.39	0.46	0.67
Pacific	CHANNEL	0.00	0 00:00	0.00	0.00	0.50
Pacific_William	CHANNEL	18.62	0 00:46	3.13	0.19	0.58
S.Commercial_A	CHANNEL	0.00	0 00:00	0.00	0.00	0.00
S.Commercial_B	CHANNEL	0.00	0 00:00	0.00	0.00	0.00
S.William	CHANNEL	0.00	0 00:00	0.00	0.00	0.00
SDP1	CONDUIT	47.27	0 01:15	9.63	3.79	1.00
SDP10	CONDUIT	45.48	0 00:52	7.89	0.41	1.00
SDP11	CONDUIT	22.38	0 00:51	7.13	1.29	1.00
SDP12	CONDUIT	25.46	0 00:46	8.23	1.27	1.00
SDP13	CONDUIT	30.08	0 00:34	6.27	1.08	1.00
SDP14	CONDUIT	38.60	0 00:15	5.46	1.63	1.00
SDP15	CONDUIT	52.26	0 00:15	7.45	1.87	1.00
SDP16	CONDUIT	78.10	0 00:15	10.18	0.45	1.00
SDP17	CONDUIT	73.36	0 00:13	9.73	0.60	1.00
SDP19	CONDUIT	3.50	0 00:39	4.70	2.83	1.00
SDP2	CONDUIT	33.06	0 01:11	6.73	2.37	1.00
SDP20	CONDUIT	23.38	0 00:15	8.57	1.65	0.83
SDP21	CONDUIT	14.67	0 00:21	4.67	1.00	1.00
SDP22	CONDUIT	3.27	0 00:25	4.44	2.65	0.89
SDP24	CONDUIT	12.23	0 00:19	5.50	1.56	1.00
SDP25	CONDUIT	1.72	0 00:44	2.19	1.06	1.00
SDP26	CONDUIT	1.88	0 00:18	2.12	0.52	1.00
SDP27	CONDUIT	0.61	0 00:20	1.33	0.09	1.00
SDP28	CONDUIT	97.86	0 00:12	16.64	0.44	1.00
SDP3	CONDUIT	33.06	0 01:12	6.73	3.03	1.00
SDP4	CONDUIT	33.06	0 01:12	6.73	2.45	1.00
SDP5	CONDUIT	33.17	0 00:55	6.76	2.15	1.00
SDP6	CONDUIT	33.76	0 00:55	6.88	5.16	1.00
SDP7	CONDUIT	35.49	0 00:53	7.23	1.44	1.00
SDP8	CONDUIT	38.76	0 00:52	8.18	1.79	1.00
SDP9	CONDUIT	43.66	0 00:52	7.82	0.20	1.00
W.Garfield_A	CHANNEL	4.12	0 00:27	0.39	0.10	0.92
W.Garfield_B	CHANNEL	0.00	0 00:00	0.00	0.00	0.42
W.Garfield_Junct	CHANNEL	2.90	0 00:35	0.35	0.02	0.75
William_1A	CHANNEL	11.29	0 00:27	1.57	0.22	0.67
William_1B	CHANNEL	0.00	0 00:00	0.00	0.00	0.40
William_Pacific	CHANNEL	0.00	0 00:00	0.00	0.00	0.26
PUMP_CT-3500-135_02	PUMP	80.00	0 00:10		1.00	
EmergencySpillway	WEIR	118.36	0 01:15		1.00	

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Flow Classification Summary

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Conduit	Adjusted /Actual Length	Fraction of Time in Flow Class								Avg. Froude Number	Avg. Flow Change
		Up Dry	Down Dry	Sub Dry	Sup Crit	Up Crit	Down Crit	Up Crit	Down Crit		
Commercial_Sag	1.00	0.97	0.00	0.00	0.01	0.00	0.01	0.00	0.00	0.00	0.0000
E.Cromwell	1.00	0.99	0.01	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.0000
Link-13	1.00	0.00	0.00	0.00	0.25	0.00	0.00	0.75	0.01	0.0003	
Link-14	1.00	0.03	0.46	0.00	0.48	0.02	0.00	0.00	0.05	0.0000	
Link-16	1.00	0.00	0.85	0.00	0.11	0.03	0.00	0.00	0.11	0.0003	
Link-17	1.00	0.00	0.00	0.00	1.00	0.00	0.00	0.00	0.53	0.0002	
N.Commercial	1.00	0.99	0.01	0.00	0.00	0.00	0.00	0.00	0.00	0.0000	
N.William	1.00	0.96	0.00	0.00	0.01	0.00	0.00	0.03	0.01	0.0000	
Pacific	1.00	0.97	0.03	0.00	0.00	0.00	0.00	0.00	0.00	0.0000	
Pacific_William	1.00	0.75	0.01	0.00	0.01	0.00	0.00	0.23	0.27	0.0000	
S.Commercial_A	1.00	1.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.0000	
S.Commercial_B	1.00	1.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.0000	
S.William	1.00	1.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.0000	
SDP1	1.00	0.00	0.00	0.00	0.97	0.00	0.00	0.03	0.02	0.0003	
SDP10	1.00	0.00	0.00	0.00	0.04	0.01	0.00	0.95	1.22	0.0001	
SDP11	1.00	0.00	0.00	0.00	0.05	0.00	0.00	0.95	0.22	0.0003	
SDP12	1.00	0.00	0.00	0.00	0.03	0.00	0.00	0.97	0.38	0.0001	
SDP13	1.00	0.00	0.00	0.00	0.05	0.00	0.00	0.95	0.31	0.0001	
SDP14	1.00	0.00	0.00	0.00	0.14	0.00	0.00	0.86	0.21	0.0001	
SDP15	1.00	0.00	0.00	0.00	0.03	0.00	0.00	0.97	0.68	0.0001	
SDP16	1.00	0.00	0.00	0.00	0.03	0.00	0.97	0.00	0.24	0.0001	
SDP17	1.00	0.00	0.00	0.00	0.72	0.02	0.00	0.26	0.53	0.0000	
SDP19	1.00	0.01	0.96	0.00	0.02	0.00	0.02	0.00	0.01	0.0003	
SDP2	1.00	0.00	0.00	0.00	0.98	0.00	0.02	0.00	0.02	0.0003	
SDP20	1.00	0.00	0.00	0.00	0.97	0.03	0.00	0.00	0.05	0.0002	
SDP21	1.00	0.00	0.90	0.00	0.10	0.00	0.00	0.00	0.02	0.0001	
SDP22	1.00	0.98	0.00	0.00	0.00	0.00	0.02	0.00	0.01	0.0002	
SDP24	1.00	0.00	0.00	0.00	0.03	0.00	0.00	0.97	0.09	0.0007	
SDP25	1.00	0.00	0.00	0.00	0.04	0.00	0.00	0.95	0.08	0.0002	
SDP26	1.00	0.02	0.00	0.00	0.02	0.00	0.00	0.96	0.02	0.0002	
SDP27	1.00	0.03	0.00	0.00	0.03	0.00	0.00	0.95	0.02	0.0001	
SDP28	1.00	0.00	0.00	0.00	0.04	0.00	0.00	0.96	2.43	0.0001	
SDP3	1.00	0.00	0.00	0.00	0.96	0.00	0.00	0.04	0.03	0.0003	
SDP4	1.00	0.00	0.00	0.00	0.96	0.00	0.00	0.04	0.04	0.0003	
SDP5	1.00	0.00	0.00	0.00	0.96	0.00	0.00	0.04	0.03	0.0002	
SDP6	1.00	0.00	0.00	0.00	1.00	0.00	0.00	0.00	0.02	0.0005	
SDP7	1.00	0.00	0.00	0.00	1.00	0.00	0.00	0.00	0.01	0.0002	
SDP8	1.00	0.00	0.00	0.00	0.99	0.00	0.00	0.00	0.58	0.0002	
SDP9	1.00	0.00	0.00	0.00	0.03	0.00	0.96	0.00	1.01	0.0001	
W.Garfield_A	1.00	0.99	0.00	0.00	0.01	0.00	0.00	0.00	0.00	0.0000	
W.Garfield_B	1.00	0.99	0.01	0.00	0.00	0.00	0.00	0.00	0.00	0.0000	
W.Garfield_Junct	1.00	0.99	0.01	0.00	0.01	0.00	0.00	0.00	0.00	0.0000	
William_1A	1.00	0.99	0.00	0.00	0.01	0.00	0.00	0.00	0.01	0.0000	
William_1B	1.00	0.99	0.01	0.00	0.00	0.00	0.00	0.00	0.00	0.0000	
William_Pacific	1.00	0.76	0.24	0.00	0.00	0.00	0.00	0.00	0.00	0.0000	

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#### Conduit Surcharge Summary

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Conduit	Hours Full			Hours Above Full Normal Flow	Hours Capacity Limited
	Both Ends	Upstream	Dnstream		
Link-13	1.45	1.45	1.45	0.28	0.51
Link-14	5.11	5.11	5.11	0.01	0.01
Link-17	1.00	1.00	1.00	0.38	0.14
SDP1	17.96	17.96	17.96	17.89	17.68
SDP10	0.58	0.58	0.58	0.01	0.01
SDP11	0.64	0.64	0.64	0.16	0.14
SDP12	0.56	0.56	0.56	0.31	0.24
SDP13	0.56	0.56	0.56	0.15	0.18

SDP14	0.55	0.55	0.55	0.70	0.55
SDP15	0.50	0.50	0.50	0.76	0.50
SDP16	0.45	0.45	0.45	0.01	0.01
SDP17	0.50	0.50	0.50	0.01	0.01
SDP19	0.34	0.34	0.34	0.39	0.17
SDP2	17.95	17.95	17.95	17.76	0.01
SDP20	0.01	0.01	0.01	0.37	0.01
SDP21	0.20	0.20	0.20	0.05	0.13
SDP22	0.01	0.01	0.01	0.34	0.01
SDP24	0.44	0.44	0.44	0.26	0.12
SDP25	0.46	0.46	0.46	0.01	0.01
SDP26	0.39	0.39	0.39	0.01	0.01
SDP27	0.31	0.31	0.32	0.01	0.01
SDP28	0.62	0.62	0.62	0.01	0.01
SDP3	17.89	17.89	17.90	17.92	17.60
SDP4	17.95	17.95	17.95	17.95	17.77
SDP5	18.02	18.02	18.02	17.89	17.89
SDP6	17.79	17.79	17.79	18.41	17.79
SDP7	17.95	17.95	17.95	12.93	13.09
SDP8	0.64	0.64	0.64	15.38	0.01
SDP9	0.51	0.51	0.51	0.01	0.01

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Pumping Summary  
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Pump	Percent Utilized	Number of Start-Ups	Min Flow CFS	Avg Flow CFS	Max Flow CFS	Total Volume 10^6 gal	Power Usage Kw-hr	% Time Pump Low	% Time Pump High
PUMP_CT-3500-135_02	99.82	1	0.00	30.94	80.00	15.872	1182.59	0.0	100.0

Analysis begun on: Tue Dec 13 10:33:44 2016  
 Analysis ended on: Tue Dec 13 10:33:46 2016  
 Total elapsed time: 00:00:02