CITY OF ALBUQUERQUE

Planning Department Brennon Williams, Director



February 28, 2020

David Soule, P.E. Rio Grande Engineering P.O. Box 93924 Albuquerque, NM 87199

RE: Lots 43A – 47B, Brookline Oak Subdivision

Grading & Drainage Plan and Drainage Report

Engineer's Stamp Date: 02/17/20 Hydrology File: A10D002A

Dear Mr. Soule:

Based upon the information provided in your submittal received 02/18/2020, the Grading &

Drainage Plan and Drainage Report are approved for Building Permit, Work Order, and for

action by the DRB on the Preliminary Plat.

Albuquerque Once the grading is complete, a pad certification will be required prior to release of Building

Permit. Please attach a copy of this approved plan in the construction sets for Building Permit

processing along with a copy of this letter and the pad certification approval letter.

NM 87103

As a reminder, if the project total area of disturbance (including the staging area and any work

within the adjacent Right-of-Way) is 1 acre or more, then an Erosion and Sediment Control (ESC) Plan and Owner's certified Notice of Intent (NOI) is required to be submitted to the

Stormwater Quality Engineer (Doug Hughes, PE, jhughes@cabq.gov, 924-3420) 14 days prior to

any earth disturbance.

If you have any questions, please contact me at 924-3995 or rbrissette@cabq.gov.

Renée C. Brissette

www.cabq.gov

Renée C. Brissette, P.E. CFM Senior Engineer, Hydrology

Planning Department

DRAINAGE REPORT

For

BROOK LINE OAK SUBDIVISION LOTS 43A -47B

Albuquerque, New Mexico

Prepared by

Rio Grande Engineering PO Box 93924 Albuquerque, New Mexico 87199

February 2020



David Soule P.E. No. 14522

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<u>Map</u> Site Grading and Drainage Plan

PURPOSE

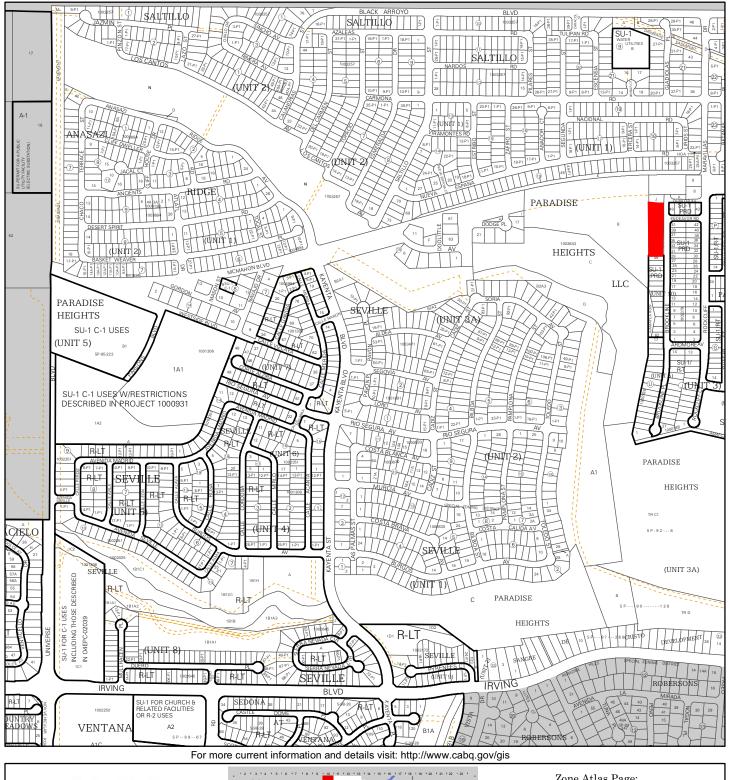
The purpose of this report is to provide the Drainage Management Plan for the subdivision and development of 10 single family lots located on Brookline Dr NW. This plan was prepared in accordance with the City of Albuquerque design regulations, utilizing the City of Albuquerque's Development Process Manual drainage guidelines. This report will demonstrate that the grading does not adversely affect the surrounding properties, nor the upstream or downstream facilities.

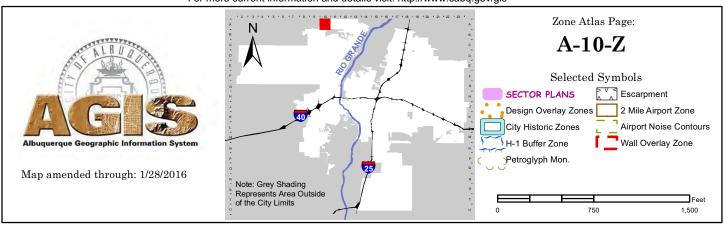
INTRODUCTION

The subject of this report, as shown on the Exhibit A, is a 1.1 acre parcel of land located on the west side of Brookline south of McMahon. The current legal is lots 43-47 block 4 Paradise Heights unit 4. As shown on FIRM map35013C0104H, the entire site is located within Flood Zone X. The site is adjacent to a roadway and located within the boundaries of a previously approved and constructed subdivision drainage plan (A10D004)

EXISTING CONDITIONS

The site currently is undeveloped and drains to Brookline and the adjacent AMAFCA maintained Calabacillas Arroyo. The adjacent roadway is constructed with the exception of curb and gutter and sidewalks. The down stream conveyance is completed. Minor flows enter the site from an adjacent HOA tract on the north.





PROPOSED CONDITIONS

The proposed improvements consist of grading the lots to construct 10 homes. The lots will be designed to drain to brook line. Due to maximum allowed wall heights, a portion between the two required retaining walls will continue to drain 0.23 cfs to the Calabacillas. The flow from each lot draining to the west will be .023 cfs, which is negligible. The cumulative discharge from the 10 lots to Brookline drive is calculated to be 3.88 cfs. The entire tract of land is identified as offsite basin within the Paradise Skies drainage management plan (A15-D004). The anticipated discharge of 3.9 cfs was designed to be accommodated within the roadway and captured by inlets downstream of the project. The flow is conveyed via a city maintained storm drain that discharges to the Calabacillas arroyo after passing through an energy dissipater. The minor upland flow from the north will be diverted to Brookline by minor grading with permission on the Saltillo common area tract that generates the flow.

Each lot will have a small water quality pond in the rear and front yard. Each lot shall retain the first 86 cfs during a rain event. The required volume for each lot is 68 cfs. The adjacent roadway and sidewalks will be constructed by city work order.

SUMMARY AND RECOMMENDATIONS

This project is a new residential development on an existing road within a fully developed subdivision. The proposed development will discharge less than allowed to Brookline. The development will manage the required first flush and drain to the required outfall. This drainage plan and report conforms to the governing drainage regulations of the City as well as the overall Silver oaks grading plan. Since the effected area site encompasses more than 1 acre, a NPDES permit will be required prior to any construction activity.

APPENDIX A

Excerpts from A10D004 drainage plan

PREPARED BY OTHERS, OBTAINED FROM CABQ.GOV

DRAINAGE STUDY FOR PARADISE SKIES UNIT 10 SUBDIVISION

APRIL 2003

Prepared for:

LONGFORD HOMES 7301 JEFFERSON NE ALBUQUERQUE, NM 87109

Prepared by:

BOHANNAN HUSTON, INC.
COURTYARD I
7500 JEFFERSON STREET NE
ALBUQUERQUE, NM 87109

Prepared by:

Prepared by:

Rick L. Beltramo, P.E.

Date

Bohannan Huston₂

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APPENDIX A - AHYMO INPUT, SUMMARY FILES

APPENDIX B - STREET CAPACITY ANALYSIS

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PLATES

PLATE 1 - PRELIMINARY PLAT

PLATE 2 - GRADING PLAN

PLATE 3 - DEVELOPED CONDITIONS BASIN MAP

Bohannan Huston₂

I. INTRODUCTION

This drainage study establishes a drainage management plan for the proposed development of the property legally described as Lots 1-26 and 35-42 of Paradise Heights Unit 4. This property is approximately 8 acres of residential (R-1) land to be re-subdivided as the "Paradise Skies Unit 10 Subdivision" with proposed R-T zoning. The subject property is located on Albuquerque's northwest mesa, west of Unser Boulevard and south of McMahon Boulevard. (See vicinity map on the *Preliminary Plat* for location, *Plate 1*). Paradise Skies Unit 10 is planned to consist of 53 single-family, residential dwellings.

This study provides hydrologic and hydraulic analysis and provides a drainage management plan as necessary to support the planned 53-unit development. More specifically, this report is submitted in conjunction with the preliminary plat application. Preliminary plat approval and grading plan approval is requested. Prior to final plat and building permit approvals of this project, the City of Albuquerque (COA) must approve final grading plans and work order construction plans.

II. METHODOLOGY

Existing and proposed site hydrological conditions were analyzed for the 100-year, 6-hour storm in accordance with the revised Section 22.2, Hydrology, of the Development Process Manual (DPM) for the City of Albuquerque, dated January 1993. Street capacities were analyzed using Manning's equation, consistent with the revised DPM Section 22.2. All data and calculations supporting this study are located in *Appendix B*. The AHYMO computer program was utilized to determine peak flow rates for design of the storm drainage improvements within the project. The 100-year, 6-hour storm is used as the design event. The results are included in *Appendix A*.

Bohannan Huston &

III. EXISTING CONDITIONS

A. Topography

Paradise Skies Unit 10 is currently undeveloped land, sloping from north to south and east to west with grades ranging from approximately 5% to 10%. Soils consist of deep, well-drained loamy fine sands typical of the West Mesa. Vegetation is light and consists mostly of native grasses and sand sagebrush.

B. Existing Drainage Patterns

Paradise Skies Unit 10 is located within the Calabacillas Arroyo Drainage Basin and will indirectly discharge into the Calabacillas Arroyo to the south. Adjacent properties and public roadways have been or will be constructed which intercept much of the potential off-site runoff. A portion of undeveloped flows from the north currently discharge through the property. Paradise Skies Units 6, 7, 8, and 9, to the east of Paradise Skies Unit 10, slope in an easterly direction away from the proposed site. Undeveloped lands to the northwest discharge into the Calabacillas Arroyo. The proposed area of Paradise Skies Unit 10 currently flows west into the Calabacillas Arroyo and south into Paradise Skies Unit 5.

IV. PROPOSED CONDITIONS

Paradise Skies Unit 10 Subdivision is a proposed single-family, detached-unit residential development with 53 lots on 8 acres, producing a density of approximately 6.6 dwelling units per acre. Proposed street configurations are shown on *Plate 1*, the *Preliminary Plat*. The drainage concepts for this property were presented in "Drainage Calculations for Paradise Skies Unit 5" by Mark Goodwin & Associates. This drainage report follows the approved concepts with a few modifications.

A. Offsite Flows

The proposed development will be graded so that it will not be affected by offsite flows from the north side of McMahon Blvd. Lands to the west will drain undeveloped flows into the Calabacillas Arroyo. When Lots 43-47 are developed, they will drain developed flows into Brookline Drive. Properties to the north will convey flow into McMahon Blvd. The developed residential properties to the east currently flow away from Paradise Skies Unit 10.

B. Onsite Flows

The internal street system conveys flow south into Paradise Skies Unit 5 and into the existing storm drain. The Paradise Skies Unit 5 Subdivision has a storm drain extended to its northern boundary in Brookline Place to accommodate flow from Paradise Skies Unit 10. The Unit 5 drainage study states that 18 cfs will be accepted from the offsite Unit 10 basin. This drainage plan proposes discharging 40 cfs from Unit 10 into Unit 5. This flow has been routed through the streets to demonstrate that a lower peak flow will occur at the southern boundary. In addition, the existing storm drain system in Unit 5 has been analyzed and it is determined to have excess capacity. Therefore, Unit 5 can accommodate fully developed discharge from the proposed Unit 10 and a small offsite basin. A more detailed analysis of the storm drain is provided in *Appendix C*.

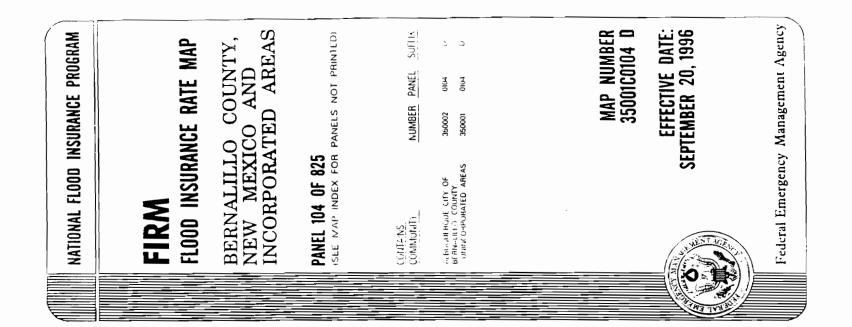
Due to existing grades of Rockcliff Drive, a small portion of Paradise Skies Unit 10 will discharge into McMahon Blvd. The McMahon Blvd. basin has also been analyzed with routed flows to determine that it can accommodate the minor addition of flows from Paradise Skies Unit 10. Please refer to the basin map and AHYMO analysis for additional information.

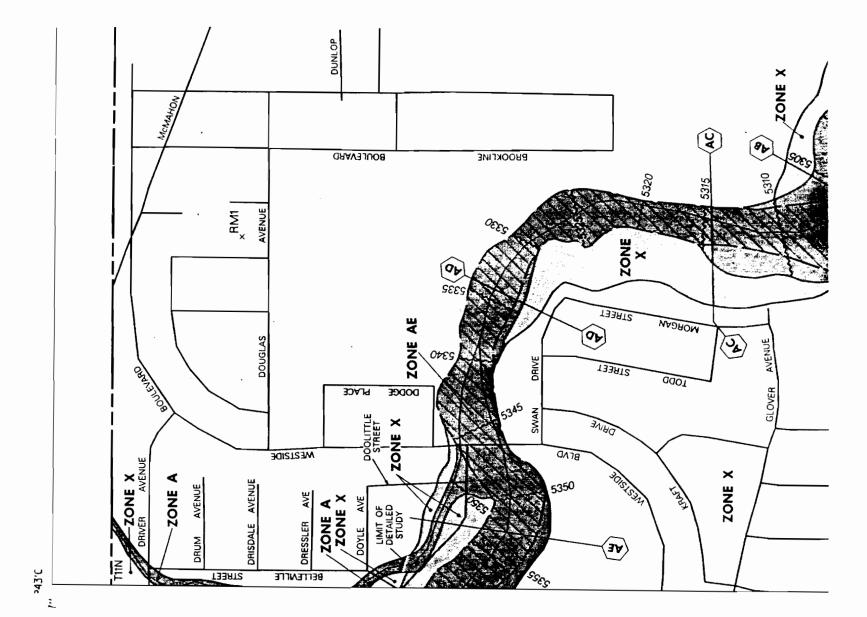
C. FEMA Floodplain

The property does not lie within a flood hazard zone, as designated on Panel 104 of 825 of the National Flood Insurance Program, Flood Insurance Rate Map published by FEMA for Bernalillo County, New Mexico (September 20, 1996). See the FEMA Floodplain exhibit provided at the end of the report text.

V. CONCLUSION

This report provides a detailed study of the developed runoff and street capacities for the proposed Paradise Skies Unit 10 Subdivision. Included is the preliminary plat, proposed conditions basin map, grading plan, infrastructure list, and all necessary hydrologic and hydraulic analyses. This drainage plan maintains the overall drainage pattern of the area as proposed in "Paradise Skies Drainage Master Plan", and allows for safe management of storm runoff in permanent as well as interim conditions.





Paradise Skies Unit 10 Subdivision

DETERMINATION OF LAND TREATMENT PERCENTAGES FOR RESIDENTIAL AREAS Ref: Basin Map

<u>Lots</u>

Minimum Lot Dimensions: 50'x100'

Total Lots = 53

Total Lot Area = 6.1 acres

Total Impervious Area = 3.5 acres

Percent A=0

Percent B=42.6

Percent C=0

Percent D=57.4

Misc. Lot Areas (Represents Extra Large Lots)

Total Area = 1.4 acres

Percent A=0

Percent B=100

Percent C=0

Percent D=0

Right-of-Way

Total Area = 2.2 acres

Percent A=0

Percent B=10

Percent C=0

Percent D=90

Cumulative Percentages for Residential Areas

Total Area of Subdivision = 9.7 acres

$$\frac{6.1}{9.7}(57.4) + \frac{1.4}{9.7}(0) + \frac{2.2}{9.7}(90) \Rightarrow \Rightarrow PercentD = 56.5$$

Percent A = 0
Percent B = 43.5
Percent C = 0
Percent D = 56.5

PARADISE SKIES UNIT 10 SUBDIVISION Internal Street Capacity Calculations April 7, 2003

1. Brookline Drive (Unit 10)

Basin C (See Basin Map)

S = 3%

Q = 25 cfs

Total flow from Basin C and the Offsite Basin is 25 cfs. At 3%, a road with mountable curb can handle 10 cfs. Therefore, no inlets are needed. Roll curb will be placed along Brookline Drive from the north boundary to Lot 43. See PC stream output. Water flows south into Brookline Place.

2. Brookline Drive (Unit 10)

Basin C (See Basin Map)

S = 7%

Q = 25 cfs

Total flow at the south end of Brookline Drive is 25 cfs. At 7%, a road with standard curb can handle 16.5 cfs. Three inlets will be placed at the southern boundary of Brookline Drive in Unit 10. These inlets will capture 19.2 cfs. Therefore, 5.7 cfs will surface flow into Basin E. See PC stream output and inlet nomograph.

3. Rockcliff Drive (Unit 10)

Basin B (See Basin Map)

S = 2%

Q = 15.2 cfs

At 2%, a road with mountable curb can handle 9 cfs. Therefore, roll curb will be placed along Rockcliff Drive from the north boundary until Lot 19. See PC stream output. Water flows south into Rockcliff Court.

4. Rockcliff Drive (Unit 10)

Basin B (See Basin Map)

S = 8%

Q = 15.2 cfs

At 8%, a road with standard curb can handle 21 cfs. Therefore, no inlets are needed. See PC stream output. Water flows into south into Rockcliff Court.

5. Brookline Place (Unit 5)

Basin E (See Basin Map)

S = 8%

Q = 18.0 cfs

Total street flow in this basin is 22.5 cfs. At 8%, a road with standard curb can handle 28 cfs. There are existing inlets in the cul-de-sac terminus to accept this flow. See PC stream output and storm drain analysis.

6. Rockcliff Court (Unit 5)

Basin D (See Basin Map)

S = 8%

Q = 25.8 cfs

Total flow in this basin is 25.8 cfs. At 8%, a road with standard curb can handle 28.9 cfs. Existing inlets in the culde-sac terminus will accommodate this flow. See PC stream output and storm drain analysis.

PARADISE SKIES UNIT 10 SUBDIVISION Storm Drain and Inlet Calculations April 7, 2003

1. Brookline Drive

S = 7%Q = 24.9 cfs

Total flow in Brookline Drive at the southern boundary is 24.9 cfs. There are three proposed inlets to capture a portion of this flow. At a flow depth of 0.33', each inlet will capture 6.4 cfs. An amount of 5.7 cfs will bypass and continue into Brookline Place. The existing storm drain can easily accommodate this flow. See inlet nomograph and analyzer report.

2. Brookline Place Cul-de-Sac

S = 2.45%Q = 22.5 cfs

At the cul-de-sac terminus, the total flow in the street is 22.5 cfs. There are two double 'C' inlets to capture flow from the 100-year storm event without exceeding top-of-curb height. This flow discharges into an existing 30" storm drain, through an energy dissipator and into the Calabacillas Arroyo. Should a storm event greater than the 100-year design storm occur or the inlets become clogged, wall drains are proposed for the existing retaining wall. In addition, there is excess capacity in the existing storm drain system. See inlet nomograph and analyzer report.

3. Rockcliff Court Cul-de-Sac

S = 9%Q = 25.8 cfs

At this cul-de-sac terminus, there are two double 'A' inlets to capture the entire flow from Rockcliff without exceeding top-of-curb height. This flow discharges into an existing 18" storm drain and connects to the energy dissipater. Wall drains are proposed as an additional safety factor. See inlet nomograph and analyzer report.

4. Energy Dissipator

Q = 67.5 cfs

The total flow from Paradise Skies Units 5 and 10 will discharge into an existing energy dissipator. The 36" storm drain pipe connecting the two cul-de-sacs into the dissipator can accommodate 103 cfs. Flow is discharged into the Calabacillas Arroyo.

Drainage Structure Analyzer

Pipe Hydraulic Analysis

BROOKLINE STORM DRAIN

(3 INLETS)

Date: Monday, April 07, 2003 5:27:18 PM

Input Data

Shape Circular Material RC C76-A Roughness 0.013000 Manning 19.2 cfs 7.000% Method Flow Rate Slope Size (W x T): 18.00 x 2.0000

Output Results

Velocity

19.2 cfs FROM INLET NOMOGRAPH Flow Rate Slope 7.000% 0.61 27.8 cfs d/D Capacity

16.98 ft/s 0.92 ft Depth Critical Depth 1.46 ft Size (W x T): 18.00 x 2.0000

Drainage Structure Analyzer

Pipe Hydraulic Analysis

BROOKLINE STORM ORAIN

CUL-DE-SAC TERMINUS

Date: Monday, April 07, 2003 5:30:26 PM

Input Data

Shape Circular
Material RC C76-A
Roughness 0.013000
Method Manning
Flow Rate 41.7 cfs
Slope 1.840%
Size (W x T): 30.00 x 2.7500

Output Results

Flow Rate
Slope
d/D
Capacity
Velocity

Depth Critical Depth Size (W x T): 41.7 cfs From AHYMO

1.840% 0.65 55.6 cfs 12.44 ft/s 1.61 ft 2.16 ft 30.00 x 2.7500

C-5

Drainage Structure Analyzer

Pipe Hydraulic Analysis

ROCKCLIFF STORM DRAIN CUL-DE-SAC TERMINUS

Date: Monday, April 07, 2003 5:29:40 PM

Essession and the second secon

Input Data

 Shape
 Circular

 Material
 RC C76-A

 Roughness
 0.013000

 Method
 Manning

 Flow Rate
 25.8 cfs

 Slope
 9.000%

 Size (W x T):
 18.00 x 2.0000

Output Results

Flow Rate 25.8 cfs From AHYMO 9.000%

 Stope
 3.000%

 d/D
 0.69

 Capacity
 31.5 cfs

 Velocity
 19.90 ft/s

 Depth
 1.03 ft

 Critical Depth
 1.48 ft

 Size (W x T):
 18.00 x 2.0000

Drainage Structure Analyzer

Pipe Hydraulic Analysis

TOTAL FLOW INTO ENERGY DISSIPATOR

Date: Monday, April 07, 2003 5:30:56 PM

Input Data

 Shape
 Circular

 Material
 RC C76-A

 Roughness
 0.013000

 Method
 Manning

 Flow Rate
 67.5 cfs

 Slope
 2.420%

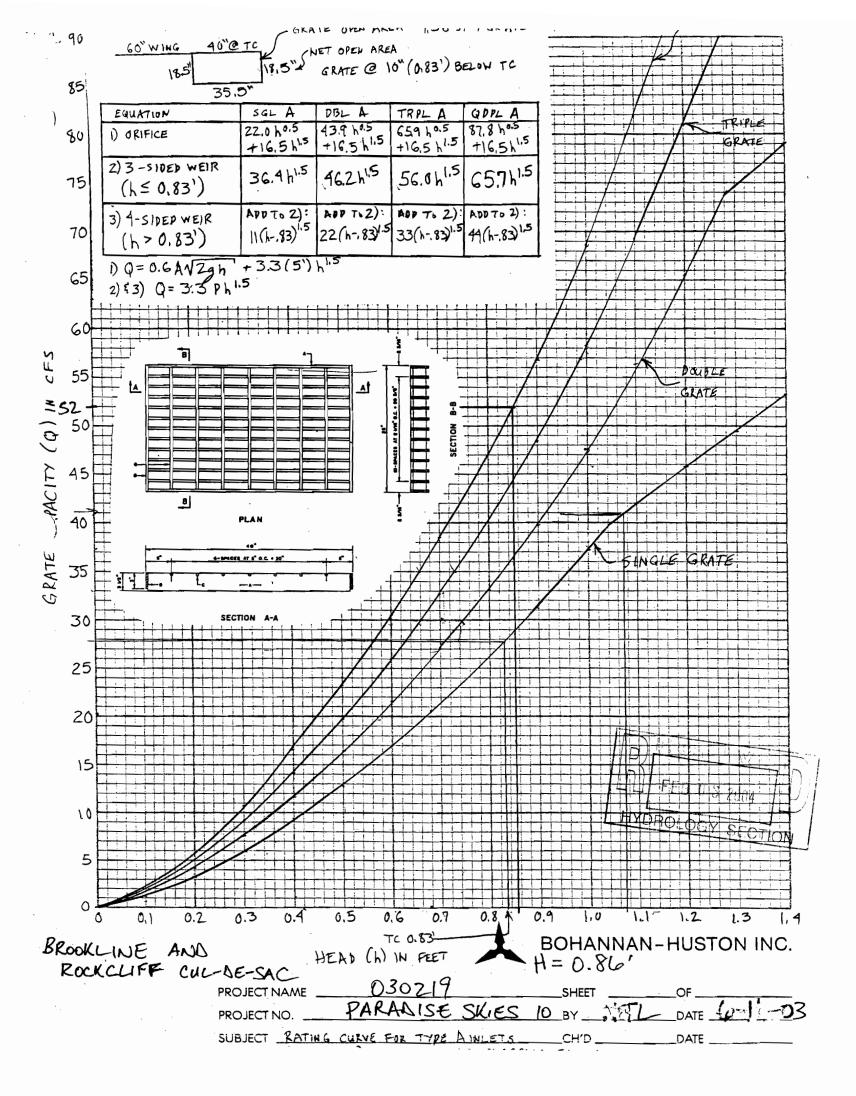
 Size (W x T):
 36.00 x 3.0000

Output Results

Flow Rate
Slope
d/D
Capacity
Velocity
Depth

Critical Depth Size (W x T): 67.5 cfs FRom AHYMO
2.420%
0.59
103.8 cfs
15.64 ft/s
1.76 ft
2.62 ft
36.00 x 3.0000

C-8/8



Design of Spun concrete connector pipes flowing full $\hat{Q} = \frac{A\sqrt{ZgH}}{\sqrt{1.2 + \frac{0.021L}{D^{1.2}}}}$ (Plate 22.3 D-8)

Rockcliff Court

Qooin Storm drain = 25.8 cfs

Right-of-way elev. (above grate) = 19.85

Invert Out = 2.15 H = 17.7' L = 77' of 18'' PCP \Longrightarrow D = 1.5' A = 1.77 H^2

$$Q = \frac{1.77\sqrt{64.4(17.7)}}{\sqrt{1.2 + (1.62/1.63)}} = 40.4 \text{ cfs}$$
(This is 1.5 times 100-year flow)

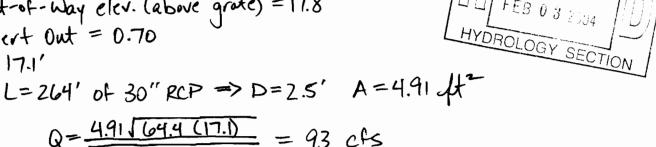
Brookline Place

Quo in Storm drain = 41.7 cfs

Right-of-Way elev. (above grate) = 17.8

Invert Out = 0.70

H= 17.1'



 $Q = \frac{4.91\sqrt{64.4(17.1)}}{\sqrt{1.2+(5.54/3)}} = 93 \text{ cfs}$ (More than twice 100-year flow)

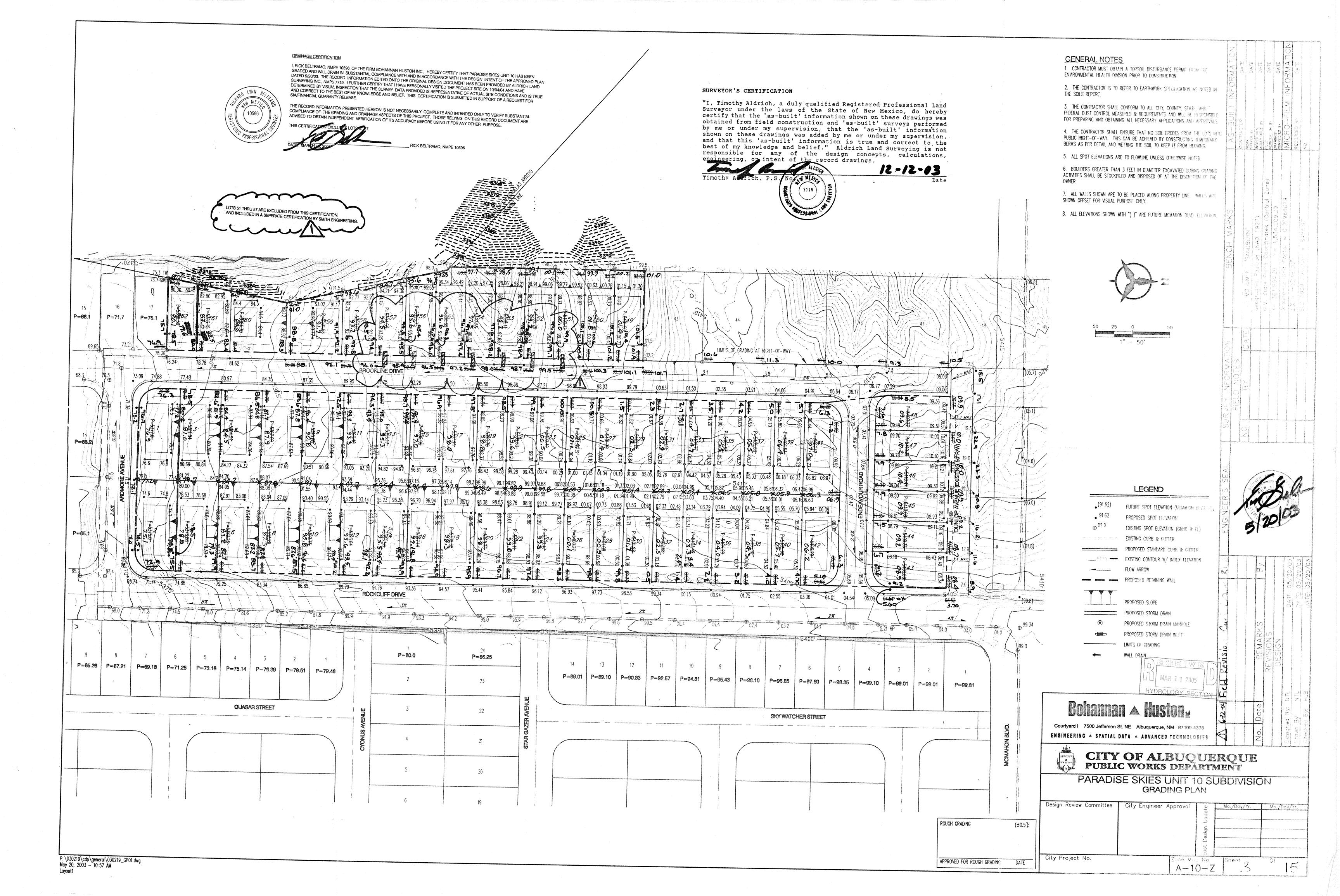
Bohannan ▲ Huston ≥

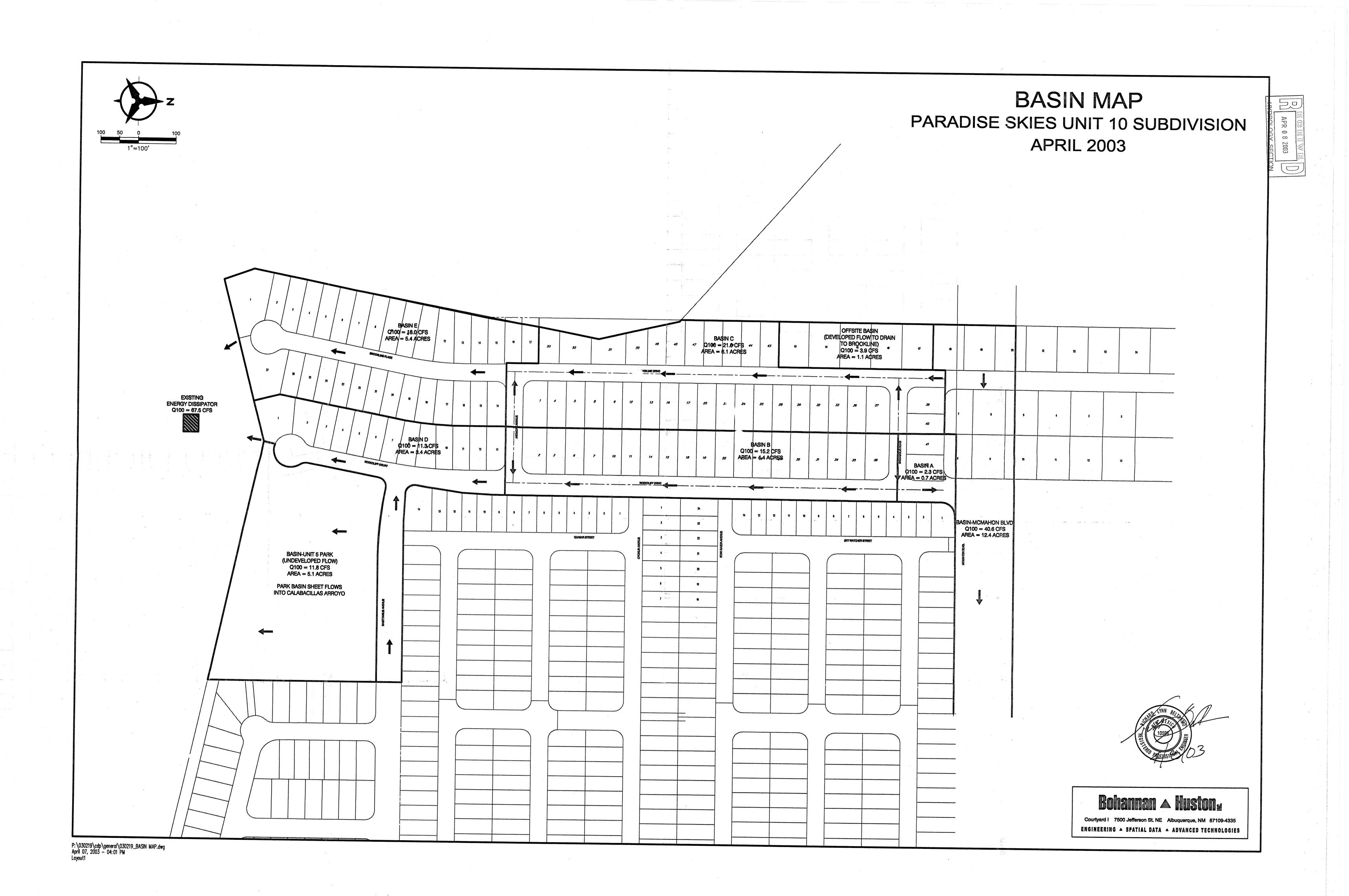
PROJECT NAME	SHEET	OF	ENGINEERING A
PROJECT NO.	BY	DATE	SPATIAL DATA -
SUBJECT	CH'D	DATE	ADVANCED TECHNOLOGIES &

36.00 x 3.0000

Critical Depth

Size (W x T):





APPENDIX B SITE HYDROLOGY

								100-Year, 6-hr.						
Basin	Area	Area	Treati	ment A	Treat	ment B	Treati	ment C	Treati	ment D	Weighted E	Volume	Flow	
	(sf)	(acres)	%	(acres)	%	(acres)	%	(acres)	%	(acres)	(ac-ft)	(ac-ft)	cfs	
ALLOWED**	47926.00	1.100												3.90
PROPOSED TO STREE	47926.00	1.100	0%	0	20%	0.220	25%	0.2751	55%	0.605	1.465	0.134		3.88
REAR YARD TO BACK	4063.00	0.093	0%	0	0%	0.000	95%	0.0886	5%	0.005	1.039	0.008	-	0.27
COMPARISON														

^{**} IDENTIFIED AS OFFSITE BASIN IN DRAINAGE ANALYSIS FOR PARADISE SKIES UNIT 10 FILE A10-D004 **Equations**:

Weighted E = Ea*Aa + Eb*Ab + Ec*Ac + Ed*Ad / (Total Area)

Volume = Weighted D * Total Area

Flow = Qa * Aa + Qb * Ab + Qc * Ac + Qd * Ad

Where for 100-year, 6-hour storm-zone 1

Ea= 0.44 Qa= 1.29 Eb= 0.67 Qb= 2.03 Ec= 0.99 Qc= 2.87 Ed= 1.97 Qd= 4.37

ONSITE Conditions

WATER QUALITY

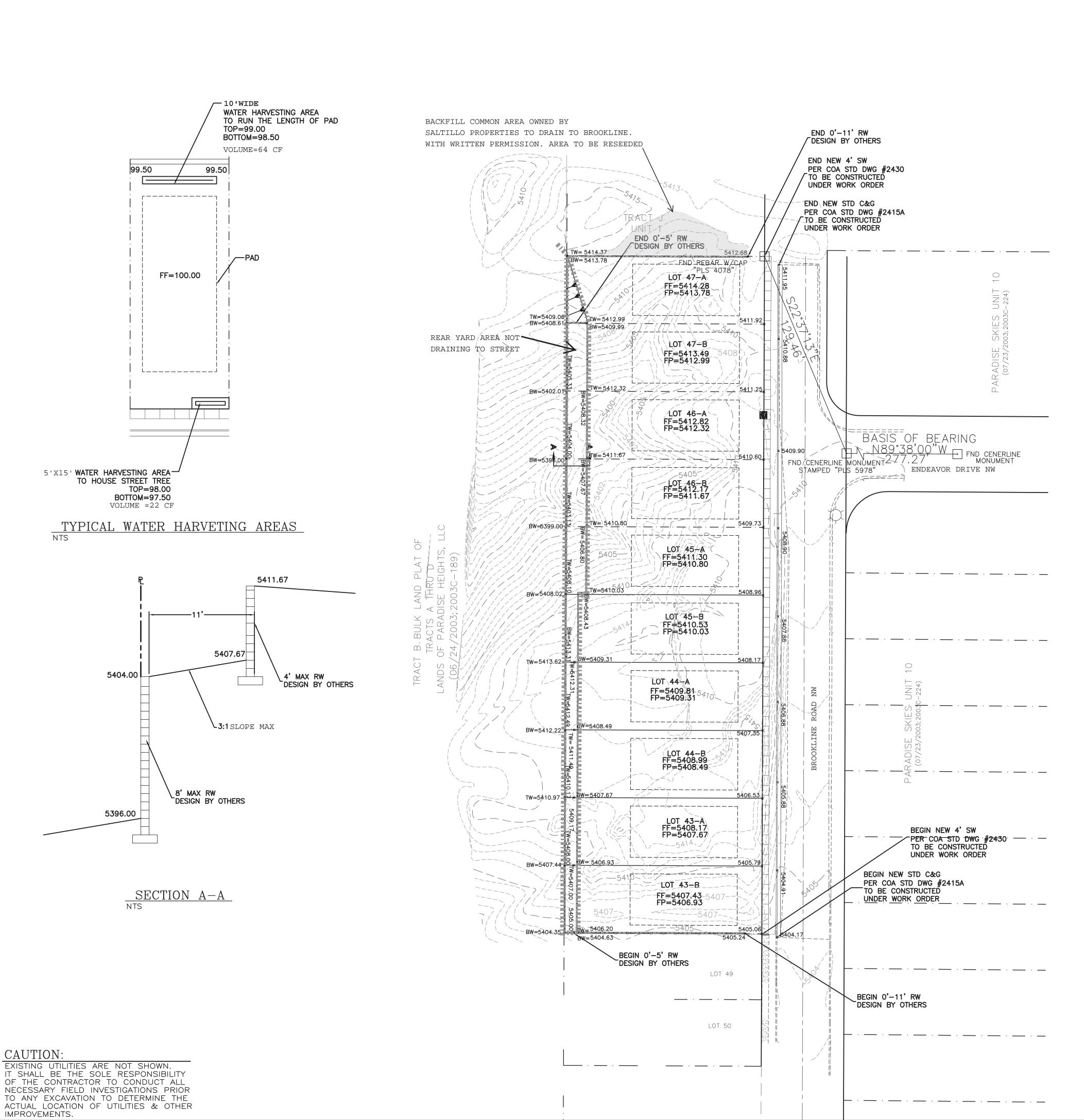
FLOOD CONTROL

FIRST FLUSH WATER QUALITY VOLUME per lot(2400 sf impervious)

REQUIRED PROVIDED
(CF) (CF)
68 86
0 86

Narrative

This site is within the identified offsite basin of Paradise Skies Unit 10 (A10-D004). The flow is identified as 3.90 cfs. The flow is programed to drain down Brookline. The flow is captured by a bank of inlets and conveyed to the calabacillas arroyou via an underground conduit and entering the arroyo through a constructed energy disipator. Minor upland flows impact the northern lot. This minor flow will be directed to Brookli by grading adjacent lot with permission from owner. Due to maximum allowed wall heights a portion of the rear yard will drain to the adjacent AMAFCA tract. The average flow leaving each lot is less than .03 cfs. This development conforms to the governing drainage management plan



EROSION CONTROL NOTES:

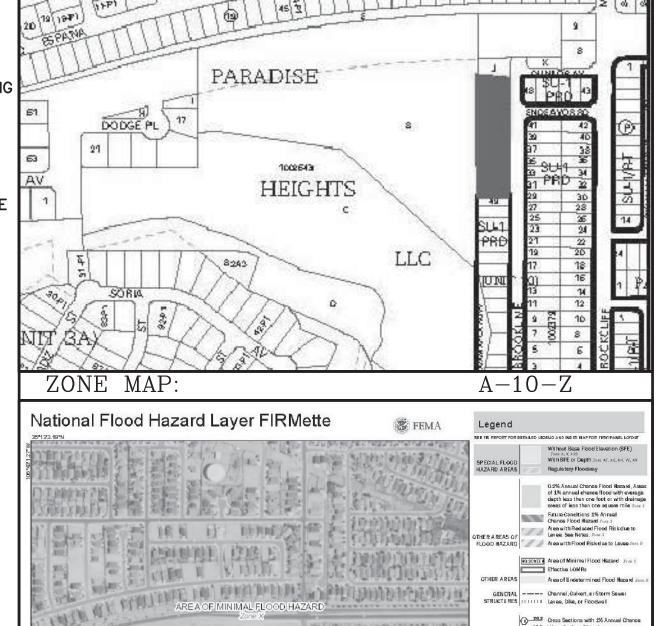
1. CONTRACTOR IS RESPONSIBLE FOR OBTAINING A TOPSOIL DISTURBANCE PERMIT PRIOR TO BEGINNING WORK.

2. CONTRACTOR IS RESPONSIBLE FOR MAINTAINING RUN-OFF ON SITE DURING CONSTRUCTION.

3. CONTRACTOR IS RESPONSIBLE FOR CLEANING ALL SEDIMENT THAT GETS INTO EXISTING RIGHT-OF-WAY.

4. REPAIR OF DAMAGED FACILITIES AND CLEANUP OF SEDIMENT ACCUMULATIONS ON ADJACENT PROPERTIES AND IN PUBLIC FACILITIES IS THE RESPONSIBILITY OF THE CONTRACTOR.

5. ALL EXPOSED EARTH SURFACES MUST BE PROTECTED FROM WIND AND WATER EROSION PRIOR TO FINAL ACCEPTANCE OF ANY PROJECT.



Digital Barrier

Digital Data Available

No Digital Data Available

LEGAL DESCRIPTION:

Feet 1,500 2,000

LOTS 43A THRU 47-B, BLOCK 4, PARADISE HEIGHTS UNIT 4

NOTES

1. ALL SPOT ELEVATIONS REPRESENT FLOWLINE ELEVATION UNLESS OTHERWISE NOTED.

2. ALL CURB AND GUTTER TO 6" HEADER UNLESS OTHERWISE NOTED.

3. ALL RETAINING WALL DESIGN SHALL BE BY OTHERS.

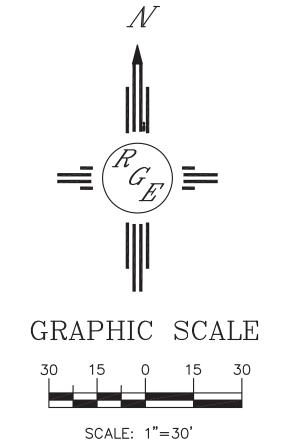
4. NOT USED

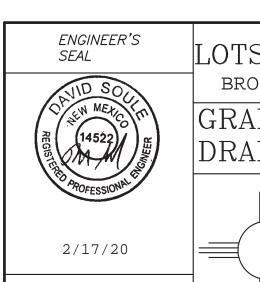
5. ANY CURBS OR PAVEMENT NEGATIVELY IMPACTED BY CONSTRUCTION ACTIVITY SHALL BE REPLACED TO MATCH EXISTING CONDITIONS.

6. ALL SITE WORK SHALL CONFORM TO CITY OF ALBUQUERQUE STANDARDS FOR PUBLIC WORKS CONSTRUCTION EDITION 9

LEGEND

EXISTING CONTOUR EXISTING INDEX CONTOUR — PROPOSED CONTOUR PROPOSED INDEX CONTOUR SLOPE TIE × 4048.25 EXISTING SPOT ELEVATION × 4048.25 PROPOSED SPOT ELEVATION BOUNDARY CENTERLINE — RIGHT—OF—WAY — PROPOSED CURB EXISTING CURB AND GUTTER PROPOSED SIDEWALK EXISTING SIDEWALK 3' ADA PATH-2% MAX SLOPE





DAVID SOULE

P.E. #14522

TS 43A	THRU 47-B	BY WCWJ
ROOKLINE	SUBDIVISION	DATE
ADING	AND	2-12-20
AINAGE	PLAN	2102012-LAYOUT-2-12-2
	Rio Grande	SHEET #
	Engineering	_

1606 CENTRAL AVENUE SE

JOB #

2102012

SUITE 201

ALBUQUERQUE, NM 87106

(505) 872-0999