# CITY OF ALBUQUERQUE



Richard J. Berry, Mayor

June 19, 2017

Fred C. Arfman, P.E. Isaacson & Arfman, P.A. 128 Monroe St. N.E Albuquerque, NM, 87108

RE: McMahon Market Place Lots 9C&D

Grading and Drainage Plan Stamp Date: 6/13/17

Hydrology File: A11D011F

Dear Mr. Arfman:

PO Box 1293

Based upon the information provided in your submittal received 6/16/2017, the Grading and Drainage Plan **is not** approved for Grading and Paving Permits. The following comments need to be addressed for approval of the above referenced project:

Albuquerque

New Mexico 87103

www.cabq.gov

- 1. More than 1 acre of disturbance is proposed, therefore an Erosion and Sediment Control Plan is required and is to be submitted to the storm water quality engineer (Curtis Cherne, PE, ccherne@cabq.gov). An approval for this must be given prior to Hydrology's approval.
- 2. On Sheet C-1. On the Vicinity Map, please show the site area from the Taco Bell site to Fineland Drive.
- 3. On Sheet C-1. Please show the existing Taco Bell development, about 25 feet min. into the site. Please show the existing retaining wall with top of wall and bottom spot elev., existing sidewalk, curb, and hatch the existing pavement.
- 4. On Sheet C-1. Please provide a cross section of the retaining wall. This section should include the existing block wall along the property line, the proposed drive, and the proposed grades on the other side of the drive.
- 5. On Sheet C-1. It appears that the proposed retaining wall is taller than 8 feet high in a spot. The maximum height for a retaining wall is 8 feet without an approved variance. This can be handled with changing the spot elevation at the bottom of the wall so it does not go past the 8 feet height maximum. Please clarify.

## CITY OF ALBUQUERQUE



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- 6. On Sheet C-1. At both of the proposed curb cuts, there appears to be patch of rip rap or gravel. If so, then please label it and give an area for them.
- 7. On Sheet C-2. Please label both of the basins in the Drainage Basin Map.
- 8. On Sheet C-2. Please enlarge both basins' ponding charts so that it is legible. These also should be place adjacent to the Basin Map.
- 9. On Sheet C-2. Please show the basins' calculations for the 100 year 6 hour storm along with the overall development calculations which you provided.

If you have any questions, please contact me at 924-3995 or rbrissette@cabq.gov.

Sincerely,

PO Box 1293

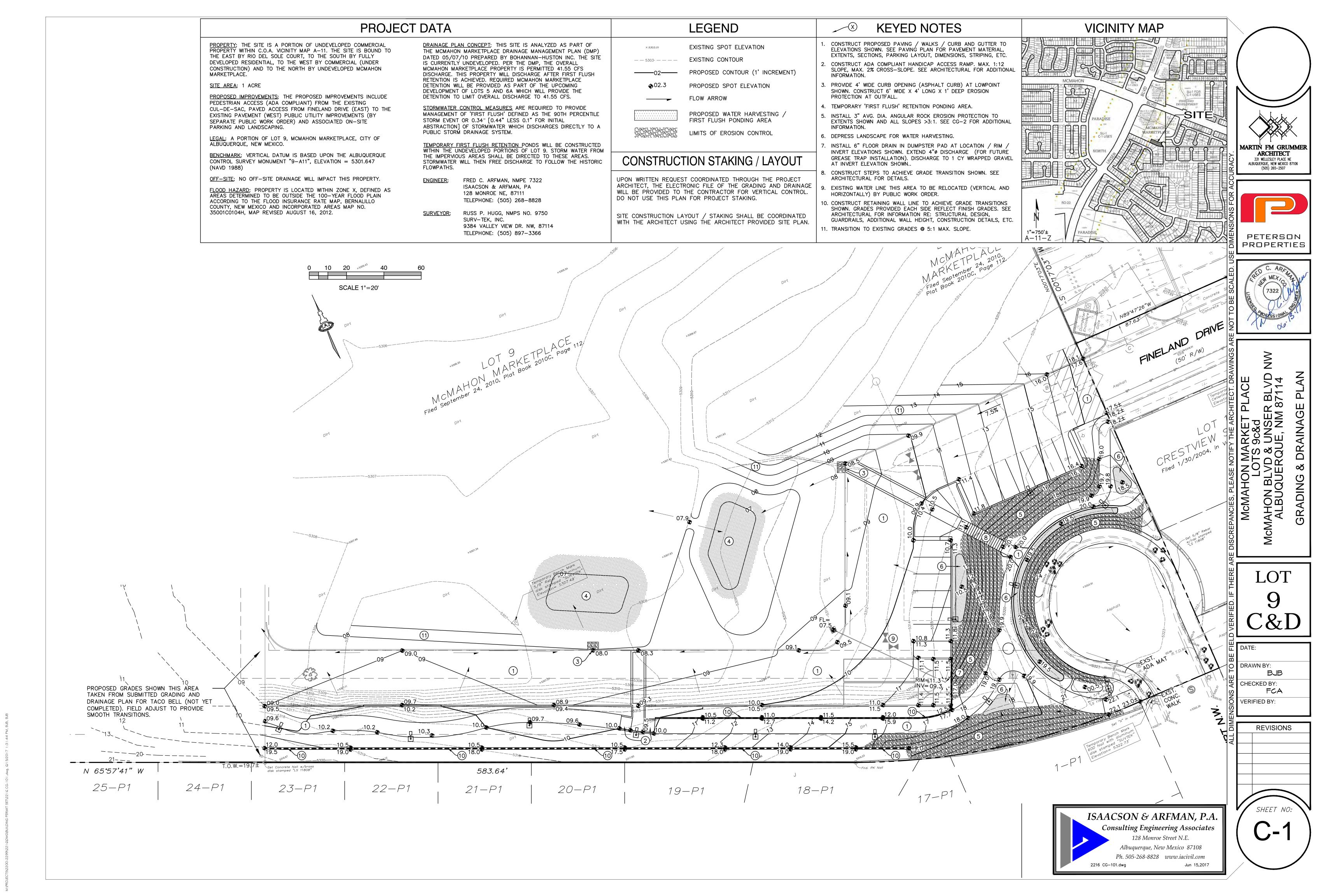
Renee C. Brissell

Albuquerque

Reneé C. Brissette, P.E. Senior Engineer, Hydrology Planning Department

New Mexico 87103

www.cabq.gov



DRAINAGE BASINS

#### GENERAL GRADING AND STORM DRAIN NOTES

- A. PROPOSED SPOT AND CONTOUR ELEVATIONS SHOWN REPRESENT TOP OF FINISH MATERIAL (I.E. TOP OF CONCRETE, TOP OF CONCRETE BUILDING PAD, TOP OF PAVEMENT MATERIAL, TOP OF LANDSCAPING MATERIAL, ETC.). CONTRACTOR SHALL GRADE, COMPACT SUBGRADE AND DETERMINE EARTHWORK ESTIMATES BASED ON ELEVATIONS SHOWN MINUS FINISH MATERIAL THICKNESSES.
- B. IF FIELD GRADE ADJUSTMENTS ARE REQUIRED, THE CONTRACTOR SHALL NOTIFY THE ARCHITECT FOR INSTRUCTIONS.
- C. THE ENVIRONMENTAL PROTECTION AGENCY (EPA) AND THE CITY OF ALBUQUERQUE REQUIRE A STORM WATER POLLUTION PREVENTION PLAN (SWPPP), AN NDPES PERMIT, AND AN EROSION AND SEDIMENT CONTROL (ESC) PERMIT FOR PROJECTS WHERE CONSTRUCTION ACTIVITIES MEET THE EPA THRESHOLD. (SWPPP, NPDES PERMIT, AND ESC PLAN BY OTHERS.) A CURRENT CITY-APPROVED ESC PERMIT MUST BE INCLUDED WITH THE CONTRACTOR'S SUBMITTAL FOR A ROUGH GRADING, GRADING, PAVING, BUILDING, OR WORK ORDER PERMIT. CONTRACTOR SHALL COORDINATE WITH OWNER TO DETERMINE WHO WILL PREPARE SWPPP AND INSPECT REQUIRED ELEMENTS.
- D. IF THE SITE IS SMALL ENOUGH NOT TO REQUIRE A SWPPP/NPDES PERMIT (LESS THAN ONE ACRE), THE CONTRACTOR SHALL STILL BE RESPONSIBLE FOR USING EROSION AND SEDIMENT CONTROL BEST MANAGEMENT PRACTICES (BMP'S) TO ENSURE THAT NO SOIL ERODES FROM THE SITE ONTO ADJACENT PUBLIC RIGHT—OF—WAY.
- E. MEASURES REQUIRED FOR EROSION AND SEDIMENT CONTROL SHALL BE INCIDENTAL TO THE PROJECT COST.
- F. WHERE GRADES BETWEEN NEW AND EXISTING ARE SHOWN AS 'MATCH' OR '±', TRANSITIONS SHALL BE SMOOTH
- G. PAVEMENT GRADES IN MARKED HANDICAPPED PARKING AREAS SHALL NOT EXCEED 2.0% IN ANY DIRECTION. FOR ALL ACCESSIBLE ROUTES, MAXIMUM ALLOWABLE CROSS SLOPE IS 2.0% AND MAXIMUM LONGITUDINAL SLOPE WITHOUT RAMP IS 5.0%. FOLLOW ALL ADA ACCESSIBILITY GUIDELINES OR CITY CODES, WHICHEVER IS MORE STRINGENT.
- H. ALL EROSION PROTECTION TO BE INSTALLED AS 3" AVG. DIA. ANGULAR FACED ROCK (F.F. ROCK) PLACED OVER GEOTEX 501 NON-WOVEN GEOTEXTILE (O.E.)
- SIDESLOPES STEEPER THAN 3:1 BUT LESS THAN 2:1 MUST HAVE PERMANENT EROSION PROTECTION INSTALLED, TYPICAL, NO SLOPE SHALL BE STEEPER THAN 2:1.
- J. FIRST FLUSH BASIN DESIGN PARAMETERS AND STORMWATER CONTROL MEASURES SHOWN ON THIS PLAN TO BE STRICTLY ADHERED TO FOR CERTIFICATION PURPOSES.
- K. POST-CONSTRUCTION MAINTENANCE FOR PRIVATE STORMWATER FACILITIES WILL BE THE RESPONSIBLITY OF THE FACILITIES OWNER. PERIODIC INSPECTION AND CERTIFICATIONS OF THE FACILITIES MAY BE REQUIRED BY THE CITY ENGINEER. ENGINEER RECOMMENDS THAT OWNER INSPECT SITE YEARLY AND AFTER EACH RAINFALL TO IDENTIFY NEW AREAS OF EROSION AND INSTALL ADDITIONAL EROSION PROTECTION AS NEEDED BASED ON ACTUAL OCCURRENCES.
- FOR ENGINEER'S CERTIFICATION OF SUBSTANTIAL COMPLIANCE (FOR CERTIFICATE OF OCCUPANCY) CONTRACTOR SHALL PROVIDE AN AUTOCAD FORMAT AS-BUILT SURVEY PREPARED BY A LICENSED SURVEYOR WHICH INCLUDES:
- M. AS-BUILT SPOT ELEVATIONS AT EACH DESIGN SPOT ELEVATION SHOWN ON THE APPROVED PLAN;
- N. TOP AND BOTTOM ELEVATIONS AS REQUIRED TO DEFINE THE PERIMETER OF PONDS (TO BE USED BY ENGINEER
- TO CALCULATE AS-BUILT VOLUME PROVIDED); O. ALL CONSTRUCTION, INCLUDING DRAIN INLETS, PIPES AND PONDS SHOWN ON THIS PLAN MUST BE CONSTRUCTED IN SUBSTANTIAL COMPLIANCE WITH THE APPROVED PLAN IN ORDER TO RECEIVE ENGINEER'S CERTIFICATION.
- P. GRADING OF FIRST FLUSH BASINS WILL BE INSPECTED AS PART OF ENGINEER'S CERTIFICATION FOR CERTIFICATE OF OCCUPANCY. DURING LANDSCAPING, FIRST FLUSH BASINS WILL BE SMOOTHLY INTEGRATED INTO LANDSCAPING WHILE MAINTAINING REQUIRED TOP AND BOTTOM ELEVATION, VOLUME AND INLET / OVERFLOW ELEVATIONS.
- Q. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE COMPLETE INSTALLATION OF ALL WORK RELATED TO PROPOSED STORM DRAINS SHOWN ON THIS PLAN INCLUDING: TRENCHING, BACKFILL, SUPPORTS, INLETS, WATER QUALITY FEATURES, EROSION CONTROL FEATURES, TESTING AND CLEANING. ANY WORK NOT ACCEPTED BY THE ARCHITECT OR ENGINEER DUE TO IMPROPER WORKMANSHIP OR LACK OF PROPER COORDINATION SHALL BE REMOVED AND CORRECTLY INSTALLED AT THE CONTRACTOR'S EXPENSE, AS DIRECTED.
- R. MINIMUM COVER FOR STORM DRAIN PIPES SHALL BE 12", UNLESS OTHERWISE NOTED.
- S. STORM DRAINS SHALL BE INSTALLED PRIOR TO SURFACE IMPROVEMENTS SUCH AS PAVEMENT, SIDEWALKS, AND LANDSCAPING.
- T. CONTRACTOR SHALL BE RESPONSIBLE FOR CONNECTIONS TO ROOF DOWNSPOUTS AND ALL NECESSARY FITTINGS. FITTING COSTS SHALL BE INCIDENTAL.
- U. TRENCHING, BORING, AND JACKING SHALL BE CONSTRUCTED IN ACCORDANCE WITH COA SPEC. SECT. 700. ALL BACKFILL SHALL BE COMPACTED TO A MINIMUM 95% DENSITY PER ASTM D-1557.
- V. ALL INLET AND AREA DRAIN RINGS & GRATES, MANHOLE RINGS & COVERS, AND OTHER SURFACE FEATURES SHALL BE ADJUSTED TO FINISHED GRADE, UNLESS OTHERWISE NOTED ON THE PLANS.
- W. ALL STORM DRAIN CROSSINGS OF WATER AND SEWER LINES SHALL HAVE 18" MIN CLEARANCE. IF 18" CLEARANCE IS NOT POSSIBLE, CONTACT THE ENGINEER AND / OR ARCHITECT IMMEDIATELY.
- X. HDPE PIPE SHALL BE ADS N-12 (WATERTIGHT) OR ENGINEER APPROVED EQUIVALENT. HDPE PIPE SHALL BE INSTALLED PER MANUFACTURER'S RECOMMENDATIONS.
- Y. PVC PIPES SHALL BE PVC SDR-35, INSTALLED PER MANUFACTURER'S RECOMMENDATIONS.
- Z. STORM DRAINS SHALL BE INSTALLED AT INVERTS AND SLOPES SPECIFIED ON THE PLANS. THE PIPE SHALL DRAIN AT A CONSTANT SLOPE BETWEEN FITTINGS AND MANHOLES. THE PIPE SHALL DRAIN TOWARD THE OUTLET AT ALL LOCATIONS.

#### **BASIN CALCULATIONS**

THE DEVELOPED AREA WILL DISCHARGE 3.8± CFS DURING A 100-YEAR 6-HOUR STORM

BASIN 1: APPROXIMATELY 0.4 CFS WILL DISCHARGE FROM THE PROPOSED PARKING TO THE EXISTING WEST PAVEMENT. AREA OF SITE:

BASIN 2:: APPROXIMATELY 1.5 CFS WILL DISCHARGE FROM THE PROPOSED PAVEMENT VIA A 4' WIDE OPENING AT THE CENTER PAVEMENT LOWPOINT.

BASIN 3: APPROXIMATELY 1.9 CFS WILL DISCHARGE FROM THE PROPOSED PAVEMENT VIA A 4' WIDE OPENING AT THE EAST PAVEMENT LOWPOINT.

DASINIO. I		DESCRIPTION					
Area of basin flows =	4449	SF	=	0.1 Ac.			
The following calculation	ons are based on	Treatment areas as shown in t	able to the right	LANDTR	REATMENT		
	Sub-basin Weighted Excess Precipitation (see formula above)				0%		
	Weighted E	= 1,60 ii	1.	B =	0%		
	Sub-basin Volun	ie of Runoff (see formula abov	/e)	C –	38%		On-Site Weighted Excess Precipitation
	V360	= 592	CF	D =	62%		Weighted $E =$
	Sub-basin Peak Discharge Rate: (see formula above) FIRST FLUSH VOL.						
	Qр	- 0.4	efs		78 CF		
BASIN NO. 2		DESCRIPTION					
Area of basin flows =	16503	SF	=	0.4 Ac.			
The following calculations are based on Treatment areas as shown in table to the right  LAND TREATMENT							On-Site Volume of Runoff: V360 =
	hted Excess Precipitation (see	A =	0%				
	Weighted E	d E = 1.60 in.		B =	0%		
	Sub-basin Volun	ne of Runoff (see formula abov	/e)	C =	38%		On-Site Peak Discharge Rate: On = O

Sub-basin Peak Discharge Rate: (see formula above) 1.4 cfs BASIN NO. DESCRIPTION Area of basin flows = 22160 SF

Ор =

LAND TREATMENT The following calculations are based on Treatment areas as shown in table to the right Sub-basin Weighted Excess Precipitation (see formula above)  $\Lambda = 0\%$ Weighted E = 1.59 in. B = 0% C = 39%Sub-basin Volume of Runoff (see formula above) D = 61%Sub-basin Peak Discharge Rate: (see formula above) FIRST FLUSH VOL

D = 62%

FIRST FLUSH VOL.

290 CF

383 CF

### 100-YEAR 6-HOUR STORM CALCULATIONS

CALCULATIONS: McMahon Marketplace - South Access: June 7, 2017

ON-SITE

43112 SF

Area D Total Area

Based on Drainage Design Criteria for City of Albuquerque Section 22.2, DPM, Vol 2, dated Jan., 1993

100-year, 6-hour **DEVELOPED FLOWS: EXCESS PRECIP:** Treatment SF % Precip. Zone  $E_{A} = 0.44$ Area A 0 | 0% | 6898 | 16% |  $E_{\rm B} = 0.67$ Area B 4311 10%  $E_{\rm C} = 0.99$ Area C 31903 | 74% |  $E_{D} = 1.97$ 

= 43112

=

0.99

100%

5978 CF

On-Site Weighted Excess Precipitation (100-Year, 6-Hour Storm)

Weighted E = $E_AA_A + E_BA_B + E_CA_C + E_DA_D$ 

 $A_A + A_B + A_C + A_D$ Developed E 1.66 in. E\*A / 12

Developed V<sub>360</sub>

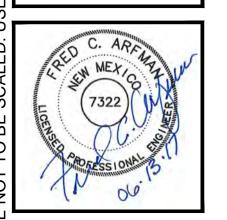
On-Site Peak Discharge Rate:  $Qp = Q_{pA}A_A + Q_{pB}A_B + Q_{pC}A_C + Q_{pD}A_D / 43,560$ For Precipitation Zone 1

 $Q_{pA} = 1.29$  $Q_{pC} = 2.87$  $Q_{pD} = 4.37$  $Q_{pB} = 2.03$ Developed  $Q_n =$ 3.8 CFS

**ARCHITECT** 331 WELLESLEY PLACE NE ALBUQUERQUE, NEW MEXICO 87106 (505) 265-2507

MARTIN FM GRUMMER

PETERSON **PROPERTIES** 



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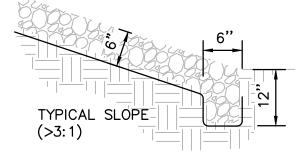
DATE: DRAWN BY: CHECKED BY: FCA VERIFIED BY:

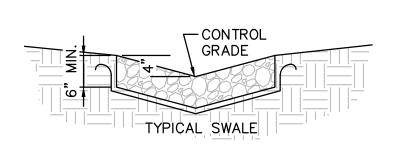
REVISIONS SHEET NO

• 2" AVG. DIA. ANGULAR ROCK (1"-3")

 PLACE GEOTEX 501 NON-WOVEN GEOTEXTILE (O.E.) BENEATH ALL EROSION PROTECTION

 CONSTRUCT ALL EROSION PROTECTION INSET INTO (NOT ON TOP OF) GRADE TO ENSURE RUNOFF CAN BE CAPTURED AND CONVEYED PROPERLY





**ROCK EROSION PROTECTION** 

SCALE: N.T.S.

