

CITY OF ALBUQUERQUE



January 31, 2018

David Soule, P.E.
Rio Grande Engineering
PO Box 93924
Albuquerque, NM 87199

RE: **Kidz Academy**
McMahon and Fineland NW
Grading Plan and Drainage Report
Engineer's Stamp Date 1/18/18 (File: A11D016)

Dear Mr. Soule:

Based on the information provided in your submittal received 1/18/18, the Grading Plan and Drainage Report cannot be approved until the following are corrected:

Prior to Site Plan for Building Permit:

1. The current approved grading plan (Goodwin-2018) and the new plat allow blanket cross lot drainage to a new pond (not constructed) with zero discharge from this larger site. This plan will need to restrict upstream discharge from Lot A1B with a new pond on lot A1B restricting discharge to at least 1.3cfs/ac. A Drainage Covenant (no public easement) will be required for this pond and be granted by the owner of Lot A1B.
2. On the Infrastructure List include Pond with Drainage Covenant (no public easement) on Lot A1B and include the required volume and allowable discharge rate.
3. Ponding on Lot A1A of similar nature is required (restrict to 1.3 cfs/ac); on the Infrastructure List include Pond(s) with Drainage Covenant (no public easement) on Lot A1A and include the required volume and allowable discharge rate.
4. The approved Drainage Master Plan that allows 1.3 cfs/acre also includes the half streets of Crown Road and Fineland Road. Please add the free discharge associated with these areas to your runoff analysis.
5. Provide RCP CL III pipe in the Crown Road ROW. There should also be an existing 18" RCP pipe with drop inlet coming from this manhole and onto the property; why not make use of that? This drainage connection will also need to be listed on the Infrastructure List.
6. Show the new lot lines on the grading plan, including the ROW takes for Crown and Fineland.

CITY OF ALBUQUERQUE



7. Please specify a square opening on the orifice plate.
8. Please label this Grading Plan as "conceptual, not for construction" or similar.
9. How is the roof drainage routing through a first flush/detention pond? The drains appear to be plumbed directly to the private storm drain. More detail is need on the ponds and storm drains to demonstrate that they will intercept flows and adequately restrict discharge from the site.
10. Quantify the volume of first flush bypass and state on plans. It appears there may be some bypass from the drive entrance, but it is unclear with the new ROW line.
11. This site has been graded several times and no land treatment A remains, Please update the proposed Hydrology to reflect.
12. There are two Basin C's on the subbasin map and no Basin A; Please correct.

Prior to Building Permit:

13. This project requires an ESC Plan, submitted to the Stormwater Quality Engineer (Curtis Cherne PE, ccherne@cabq.gov or 924-3420).
14. Additional comments may be provided at Building Permit, based on the outcome of the above remarks and level of detail shown on plans.

Prior to Certificate of Occupancy:

15. The new ponds will need to be certified (along with the rest of the site). Once certified, the City can release the previous covenant on the temporary retention pond (not constructed).
16. City acceptance and close-out of the public Work Order will be required, unless financial guarantee has been posted.

If you have any questions, please contact me at 924-3695 or dpeterson@cabq.gov.

Sincerely,

Dana Peterson, P.E.
Senior Engineer, Planning Dept.
Development Review Services



City of Albuquerque

Planning Department

Development & Building Services Division

DRAINAGE AND TRANSPORTATION INFORMATION SHEET (REV 09/2015)

Project Title: _____ **Building Permit #:** _____ **City Drainage #:** _____

DRB#: _____ **EPC#:** _____ **Work Order#:** _____

Legal Description: _____

City Address: _____

Engineering Firm: _____ **Contact:** _____

Address: _____

Phone#: _____ **Fax#:** _____ **E-mail:** _____

Owner: _____ **Contact:** _____

Address: _____

Phone#: _____ **Fax#:** _____ **E-mail:** _____

Architect: _____ **Contact:** _____

Address: _____

Phone#: _____ **Fax#:** _____ **E-mail:** _____

Other Contact: _____ **Contact:** _____

Address: _____

Phone#: _____ **Fax#:** _____ **E-mail:** _____

Check all that Apply:

DEPARTMENT:

- ☐ HYDROLOGY/ DRAINAGE
☐ TRAFFIC/ TRANSPORTATION
☐ MS4/ EROSION & SEDIMENT CONTROL

TYPE OF SUBMITTAL:

- ☐ ENGINEER/ ARCHITECT CERTIFICATION
- ☐ CONCEPTUAL G & D PLAN
☐ GRADING PLAN
☐ DRAINAGE MASTER PLAN
☐ DRAINAGE REPORT
☐ CLOMR/LOMR
- ☐ TRAFFIC CIRCULATION LAYOUT (TCL)
☐ TRAFFIC IMPACT STUDY (TIS)
☐ EROSION & SEDIMENT CONTROL PLAN (ESC)
- ☐ OTHER (SPECIFY) _____

CHECK TYPE OF APPROVAL/ACCEPTANCE SOUGHT:

- ☐ BUILDING PERMIT APPROVAL
☐ CERTIFICATE OF OCCUPANCY
- ☐ PRELIMINARY PLAT APPROVAL
☐ SITE PLAN FOR SUB'D APPROVAL
☐ SITE PLAN FOR BLDG. PERMIT APPROVAL
☐ FINAL PLAT APPROVAL
☐ SIA/ RELEASE OF FINANCIAL GUARANTEE
☐ FOUNDATION PERMIT APPROVAL
☐ GRADING PERMIT APPROVAL
☐ SO-19 APPROVAL
☐ PAVING PERMIT APPROVAL
☐ GRADING/ PAD CERTIFICATION
☐ WORK ORDER APPROVAL
☐ CLOMR/LOMR
- ☐ PRE-DESIGN MEETING
☐ OTHER (SPECIFY) _____

IS THIS A RESUBMITTAL?: ☐ Yes ☐ No

DATE SUBMITTED: _____ **By:** _____

COA STAFF: _____ ELECTRONIC SUBMITTAL RECEIVED: _____

DRAINAGE REPORT

For

KIDZ ACADEMY
Parcel A1A and A1B Fineland Subdivision
Albuquerque, New Mexico

Prepared by

Rio Grande Engineering
PO Box 93924
Albuquerque, New Mexico 87199

JANUARY 2018



David Soule P.E. No. 14522

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Site Hydrology	A
Hydraulic Model and calculations.....	B

Map

Site Grading and Drainage Plan

PURPOSE

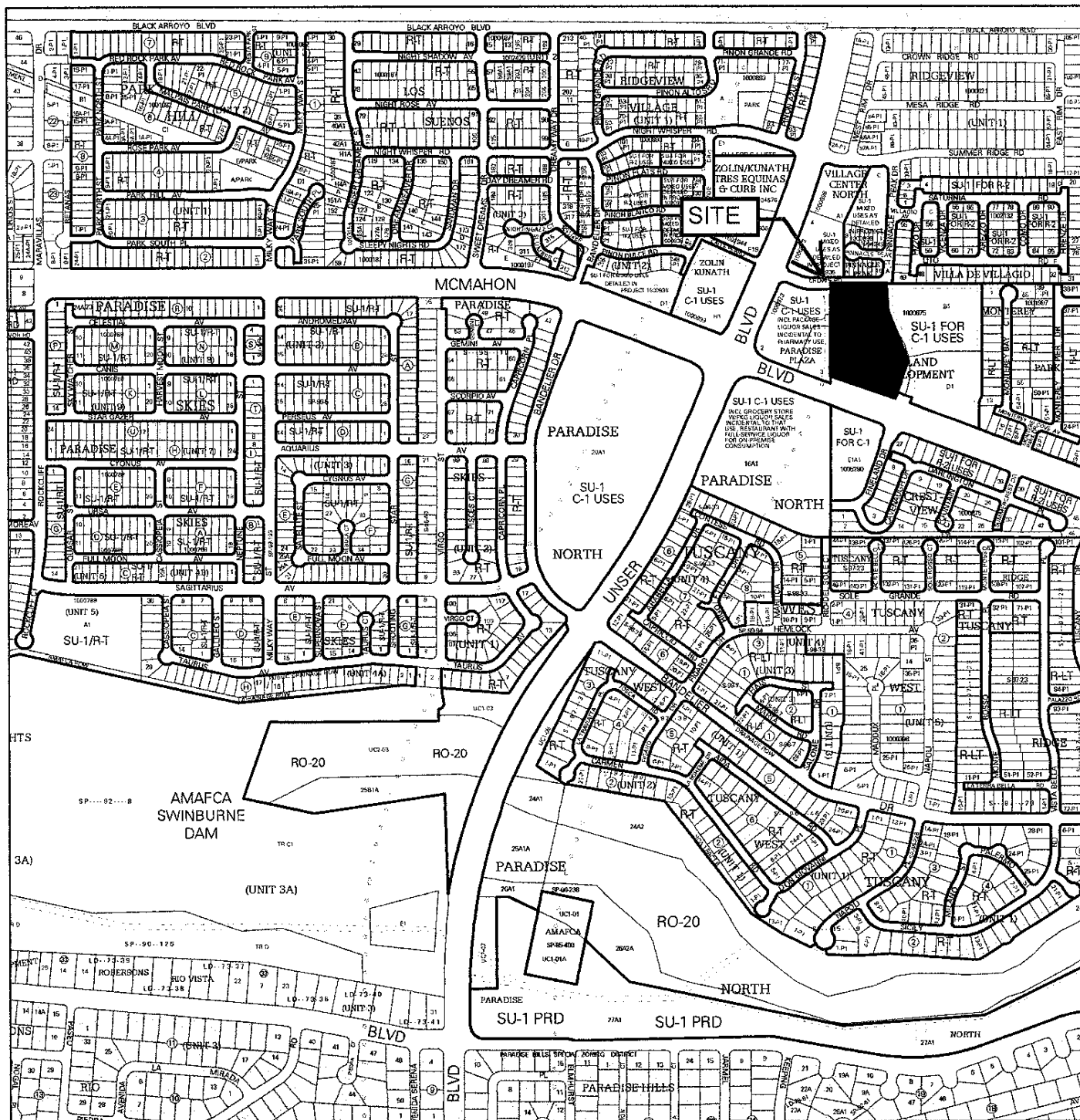
The purpose of this report is to provide the Drainage Management Plan for the subdivision of a 4.33 acre tract and the construction of a Kidz Daycare building with associated parking lot and playground on the northern 1.03 acres. This plan was prepared in accordance with the City of Albuquerque design regulations, utilizing the City of Albuquerque's Development Process Manual drainage guidelines. This report will demonstrate that the grading does not adversely affect the surrounding properties, nor the upstream or downstream facilities.

INTRODUCTION

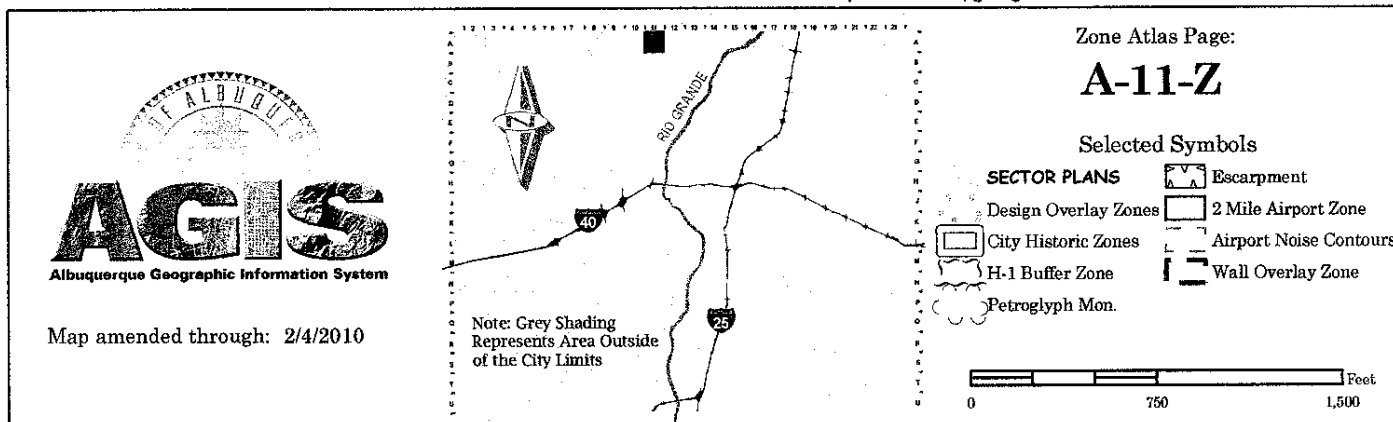
The subject of this report, as shown on the Exhibit A, is a 4.33 -acre parcel of land located on the west side of Fineland drive between McMahon and Crown road. The legal description of this site is tract A1A and A1B Fineland Subdivision. As shown on FIRM map35001C0104H, the entire site is located within Flood Zone X. The site is bound on all sides by roadways and not impacted by upland flows. The site is an undeveloped site. The site free discharges 5.63 cfs to the intersection of Crown road and Fineland. The site is located within basin O as shown in the area drainage plan (A11D009). The proposed improvements include the construction of a day care with parking and play ground areas on tract A1A. The remaining A1B will not be developed at this time. This site must conform to the 1.3 cfs per acre assigned within the master drainage plan and discharge to the existing storm drain system within Crown. The site must manage the first flush volume onsite.

EXISTING CONDITIONS

The site is currently undeveloped. The site currently discharges native flow of 5.63 or 1.3 cfs per acre to the intersection of crown and Fineland. The flows are captured by inlets and conveyed north to the Black Arroyo. Due to being higher than the surrounding roadways, the site is not impacted by upland flows.



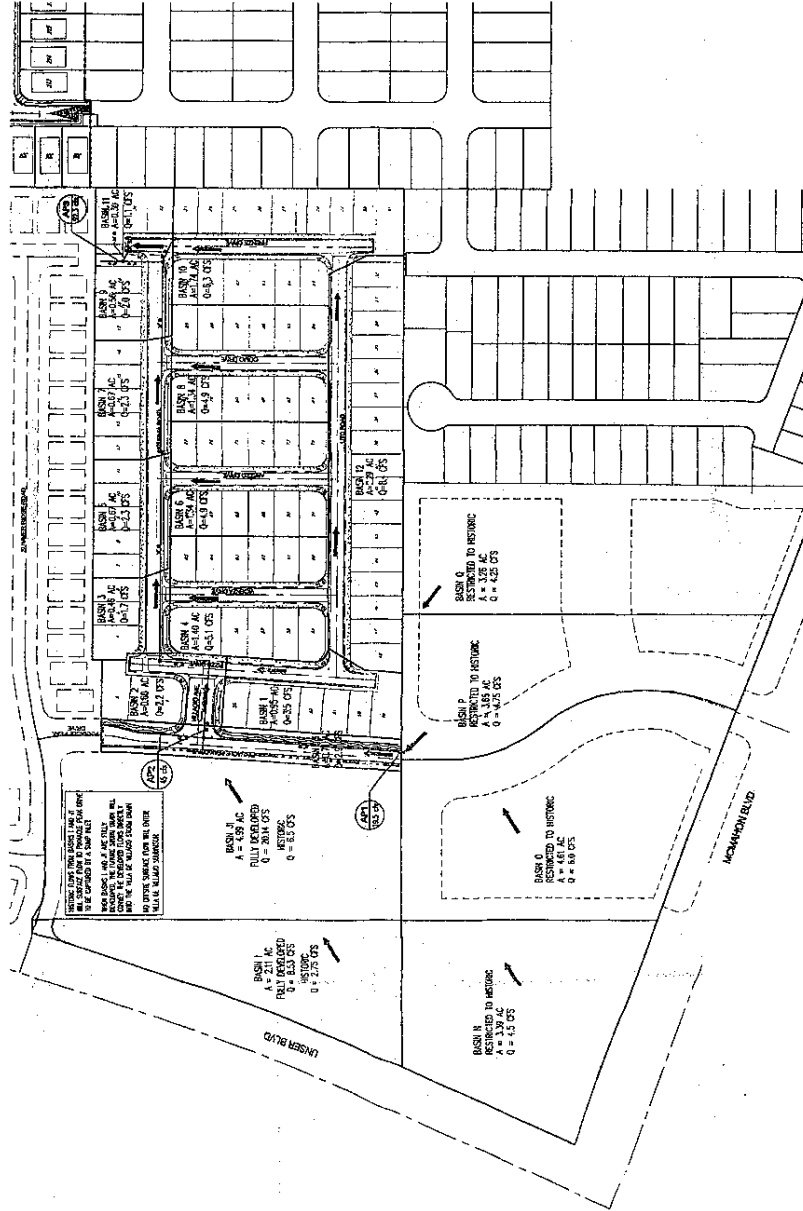
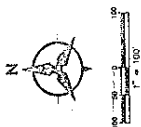
For more current information and more details visit: <http://www.cabq.gov/gis>



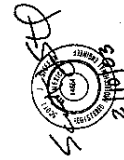
APPENDIX A
SITE HYDROLOGY

A11D009

BASIN MAP **VILLA DE VILLAGIO SUBDIVISION** **FEBRUARY 2003**



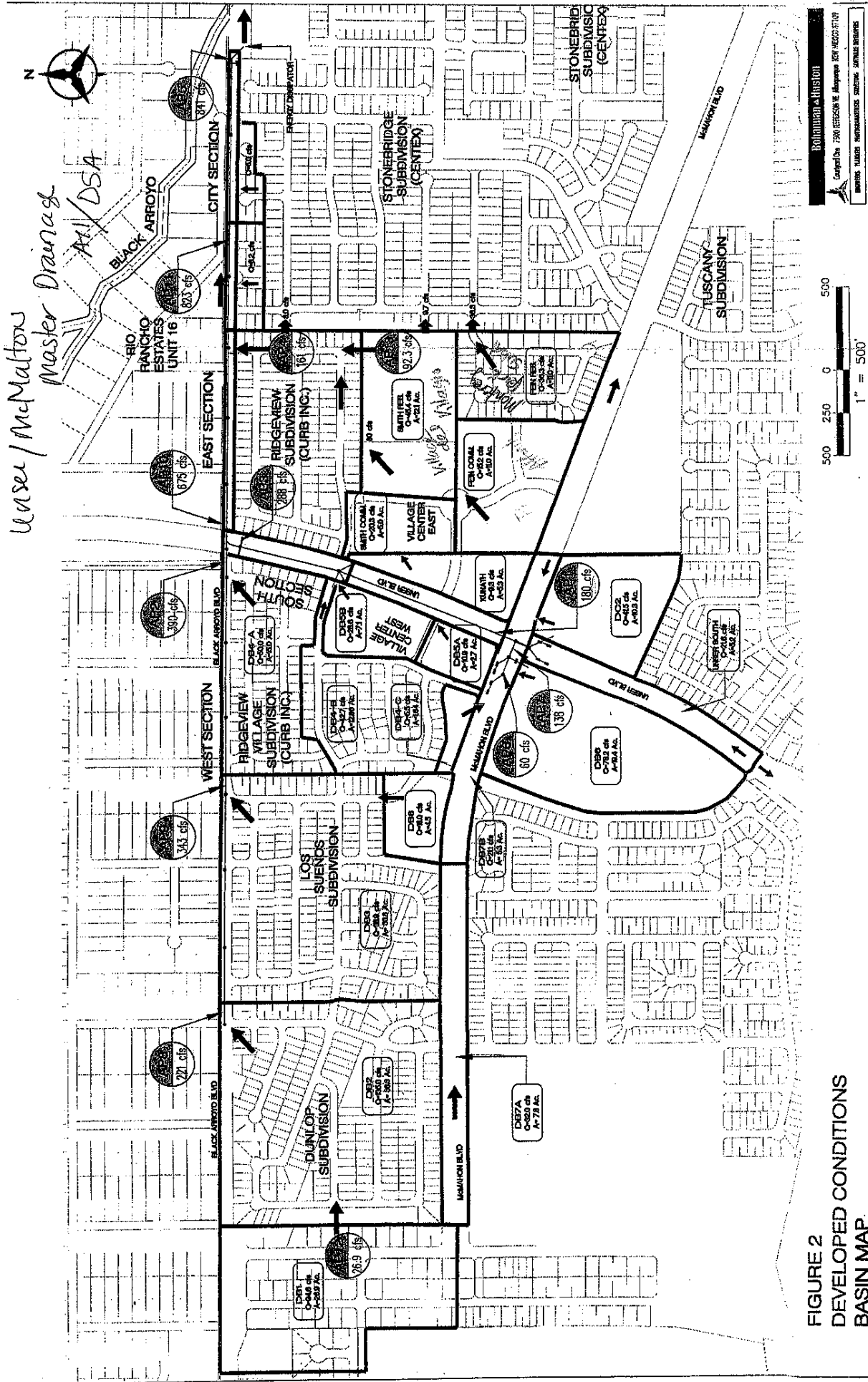
THIS SYMBOL REPRESENTS THE AMOUNT OF FLOW
 CONTAINED WITHIN THE STORM DRAIN AT EACH
 ANALYSIS POINT DURING THE 100-YEAR STORM



Behnam & Hoston

Civil Engineers
 1100 West 19th Street, Suite 100
 Fort Worth, Texas 76102
 (817) 342-1111

Unser / McMatton
Master Drainage
AUL/DSA



Weighted E Method **KIDZ ACADEMY**

Existing Developed Basins

Basin	Area (sf)	Area (acres)	Treatment A		Treatment B		Treatment C		Treatment D		100-Year, 6-hr.		
			%	(acres)	%	(acres)	%	(acres)	%	(acres)	Weighted E (ac-ft)	Volume (ac-ft)	Flow cfs
BASIN A	13656	0.313	0%	0	79.0%	0.248	16.0%	0.05016	5%	0.016	0.786	0.021	0.72
BASIN B	31296	0.718	0%	0	6.0%	0.043	15.0%	0.10777	79%	0.568	1.745	0.104	2.88
BASIN C	117128	2.689	100%	2.6889	0.0%	0.000	0.0%	0	0%	0.000	0.440	0.099	3.47

Equations:

Weighted E = Ea*Aa + Eb*Ab + Ec*Ac + Ed*Ad / (Total Area)

Volume = Weighted D * Total Area

Flow = Qa * Aa + Qb * Ab + Qc * Ac + Qd * Ad

Where for 100-year, 6-hour storm (zone 1)

- Ea= 0.44
- Eb= 0.67
- Ec= 0.99
- Ed= 1.97

- Qa= 1.29
- Qb= 2.03
- Qc= 2.87
- Qd= 4.37

- FIRST FLUSH
- BASIN A 19
- BASIN B 701
- BASIN C 0
- TOTAL 720 CF

pondrout011718.txt

*S AHYMO - DETENTION-KIDZ ACADEMY
*S POND ROUTING

START TIME=0.0 PUNCH CODE=0

RAINFALL TYPE=2
QUARTER=0.0 ONE= 1.87 IN
SIX=2.20 IN DAY= 2.66 IN DT = 0.05 HR

COMPUTE NM HYD ID=1 HYD NO=101 DA= .0011226 SQ MI
PER A=0 PER B=6 PER C=15 PER D=79
TP=-.142 MASSRAIN=-1

PRINT HYD ID=1 CODE=3

* ROUTE THE TOTAL FLOW THROUGH THE PROPOSED RESERVOIR
ROUTE RESERVOIR ID=2 HYD NO=102 INFLOW=1 CODE=3
OUTFLOW(CFS) STORAGE(AC-FT) ELEV(FT)
0.00 0.000 85.50
0.37 0.001 86.50

0.52 0.004 87.50
0.58 0.020 88.00
0.64 0.082 88.50
0.67 0.134 88.75

FINISH

			AHYMO.OUT				
TIME	FLOW	CFS	TIME	FLOW	CFS	HRS	CFS
HRS	HRS		HRS	HRS			
	0.000	0.0		4.950	0.0	9.900	0.0
14.850	0.0	0.0	19.800	0.0	0.0	10.050	0.0
15.000	0.150	0.0	19.950	5.100	0.0	10.200	0.0
15.150	0.300	0.0	20.100	5.250	0.0	10.350	0.0
15.300	0.450	0.0	20.250	5.400	0.0	10.500	0.0
15.450	0.600	0.0	20.400	5.550	0.0	10.650	0.0
15.600	0.750	0.0	20.550	5.700	0.0	10.800	0.0
15.750	0.900	0.0	20.700	5.850	0.0	10.950	0.0
15.900	1.050	0.2	20.850	6.000	0.0	11.100	0.0
16.050	1.200	0.4	21.000	6.150	0.0	11.250	0.0
16.200	1.350	0.9	21.150	6.300	0.0	11.400	0.0
16.350	1.500	2.9	21.300	6.450	0.0	11.550	0.0
16.500	1.650	1.9	21.450	6.600	0.0	11.700	0.0
16.650	1.800	0.9	21.600	6.750	0.0	11.850	0.0
16.800	1.950	0.5	21.750	6.900	0.0	12.000	0.0
16.950	2.100	0.3	21.900	7.050	0.0	12.150	0.0
17.100	2.250	0.2	22.050	7.200	0.0	12.300	0.0
17.250	2.400	0.1	22.200	7.350	0.0	12.450	0.0
17.400	2.550	0.0	22.350	7.500	0.0	12.600	0.0
17.550	2.700	0.0	22.500	7.650	0.0	12.750	0.0
17.700	2.850	0.0	22.650	7.800	0.0	12.900	0.0
17.850	3.000	0.0	22.800	7.950	0.0	13.050	0.0
18.000	3.150	0.0	22.950	8.100	0.0	13.200	0.0
18.150	3.300	0.0	23.100	8.250	0.0	13.350	0.0
18.300	3.450	0.0	23.250	8.400	0.0	13.500	0.0
18.450	3.600	0.0	23.400	8.550	0.0	13.650	0.0
18.600	3.750	0.0	23.550	8.700	0.0	13.800	0.0
18.750	3.900	0.0	23.700	8.850	0.0	13.950	0.0
18.900	4.050	0.0	23.850	9.000	0.0	14.100	0.0
19.050	4.200	0.0	24.000	9.150	0.0	14.250	0.0
19.200	4.350	0.0	24.150	9.300	0.0		

				AHYMO.OUT			
	4.500	0.0	9.450	0.0	14.400	0.0	
19.350	0.0	24.300	0.0				
	4.650	0.0	9.600	0.0	14.550	0.0	
19.500	0.0	24.450	0.0				
	4.800	0.0	9.750	0.0	14.700	0.0	
19.650	0.0						

RUNOFF VOLUME = 2.12017 INCHES = 0.1269 ACRE-FEET
 PEAK DISCHARGE RATE = 2.89 CFS AT 1.500 HOURS BASIN AREA =
 0.0011 SQ. MI.

* ROUTE THE TOTAL FLOW THROUGH THE PROPOSED RESERVOIR
 ROUTE RESERVOIR ID=2 HYD NO=102 INFLOW=1 CODE=3
 OUTFLOW(CFS) STORAGE(AC-FT) ELEV(FT)
 0.00 0.000 85.50
 0.37 0.001 86.50
 0.52 0.004 87.50
 0.58 0.020 88.00
 0.64 0.082 88.50
 0.67 0.134 88.75

* * * * *	* * * * *	* * * * *	* * * * *	* * * * *
TIME (HRS)	INFLOW (CFS)	ELEV (FEET)	VOLUME (AC-FT)	OUTFLOW (CFS)
0.00	0.00	85.50	0.000	0.00
0.15	0.00	85.50	0.000	0.00
0.30	0.00	85.50	0.000	0.00
0.45	0.00	85.50	0.000	0.00
0.60	0.00	85.50	0.000	0.00
0.75	0.00	85.50	0.000	0.00
0.90	0.01	85.52	0.000	0.01
1.05	0.16	85.82	0.000	0.12
1.20	0.38	86.40	0.001	0.33
1.35	0.94	87.27	0.003	0.49
1.50	2.89	88.00	0.020	0.58
1.65	1.88	88.20	0.045	0.60
1.80	0.91	88.27	0.054	0.61
1.95	0.52	88.28	0.055	0.61
2.10	0.28	88.26	0.052	0.61
2.25	0.16	88.22	0.047	0.61
2.40	0.11	88.17	0.041	0.60
2.55	0.04	88.12	0.035	0.59
2.70	0.02	88.06	0.028	0.59
2.85	0.01	88.01	0.021	0.58
3.00	0.01	87.81	0.014	0.56
3.15	0.01	87.60	0.007	0.53
3.30	0.01	86.59	0.001	0.38
3.45	0.01	85.52	0.000	0.01

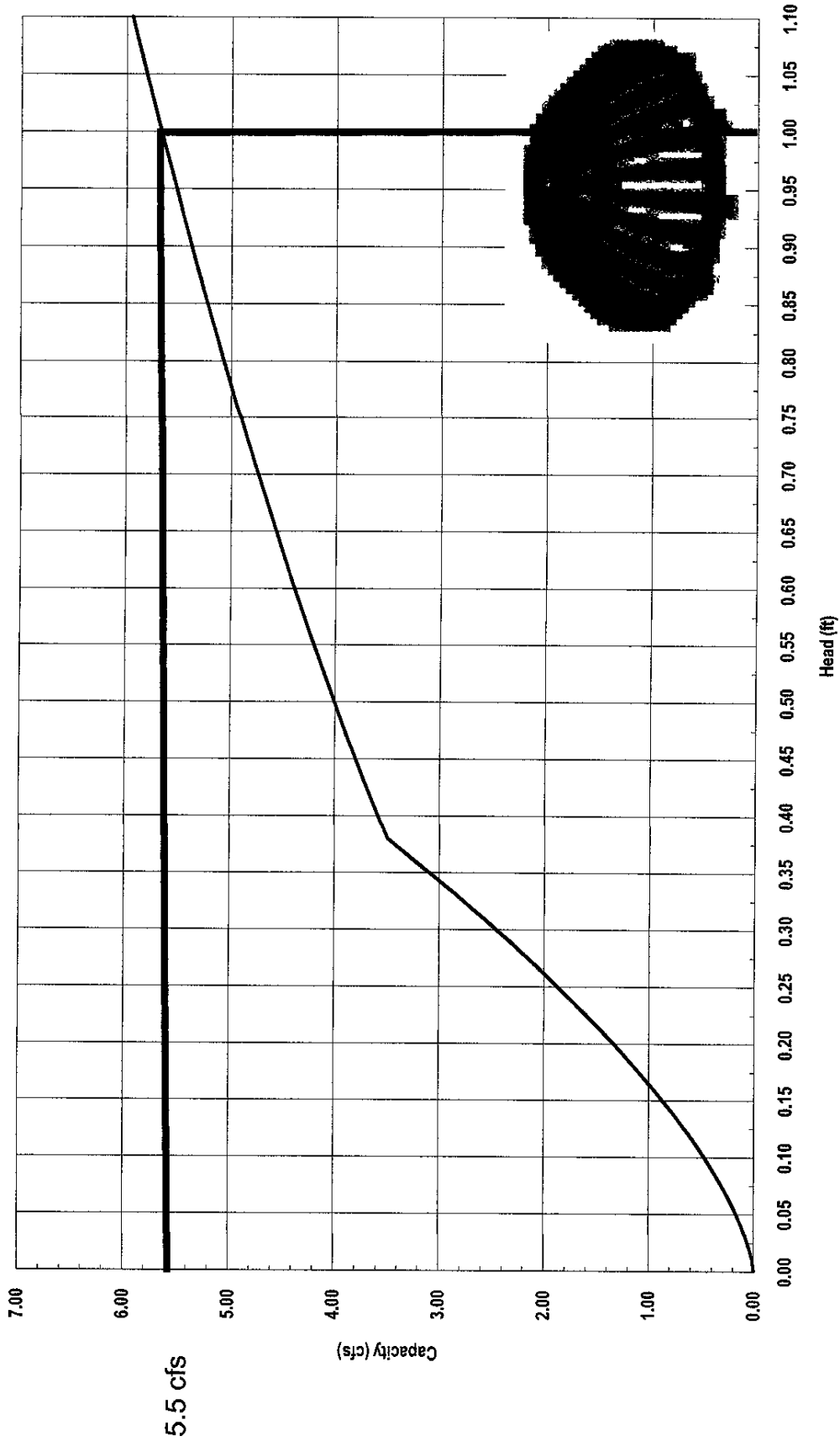
AHYMO.OUT
3.60 0.00 85.51 0.000 0.00
PEAK DISCHARGE = 0.614 CFS - PEAK OCCURS AT HOUR 1.90
MAXIMUM WATER SURFACE ELEVATION = 88.281
MAXIMUM STORAGE = 0.0548 AC-FT INCREMENTAL TIME= 0.050000HRS

FINISH

NORMAL PROGRAM FINISH

END TIME (HR:MIN:SEC) = 17:17:40

Nyloplast 18" Dome Grate Inlet Capacity Chart



3130 Verona Avenue • Buford, GA 30518
 (866) 888-8479 / (770) 932-2443 • Fax: (770) 932-2490
 © Nyloplast Inlet Capacity Charts June 2012

DROP INLET CALCULATIONS

INLET	TYPE OF INLET	AREA (SF)	Q (CFS)	H (FT)	H ALLOW (FT)
INLET A	SINGLE	3.84	2.88	0.0243	1.5

ORIFICE EQUATION

$$Q = CA \sqrt{2gH}$$

$$C = 0.6$$

$$g = 32.2$$

Pipe Capacity

Pipe	D	Slope	Area	R	Q Provided	Q Required	Velocity
	(in)	(%)	(ft^2)		(cfs)	(cfs)	(ft/s)
18HDPE	18	4	1.77	0.375	18.26	4.18	2.37

Manning's Equation:

$$Q = 1.49/n * A * R^{(2/3)} * S^{(1/2)}$$

A = Area

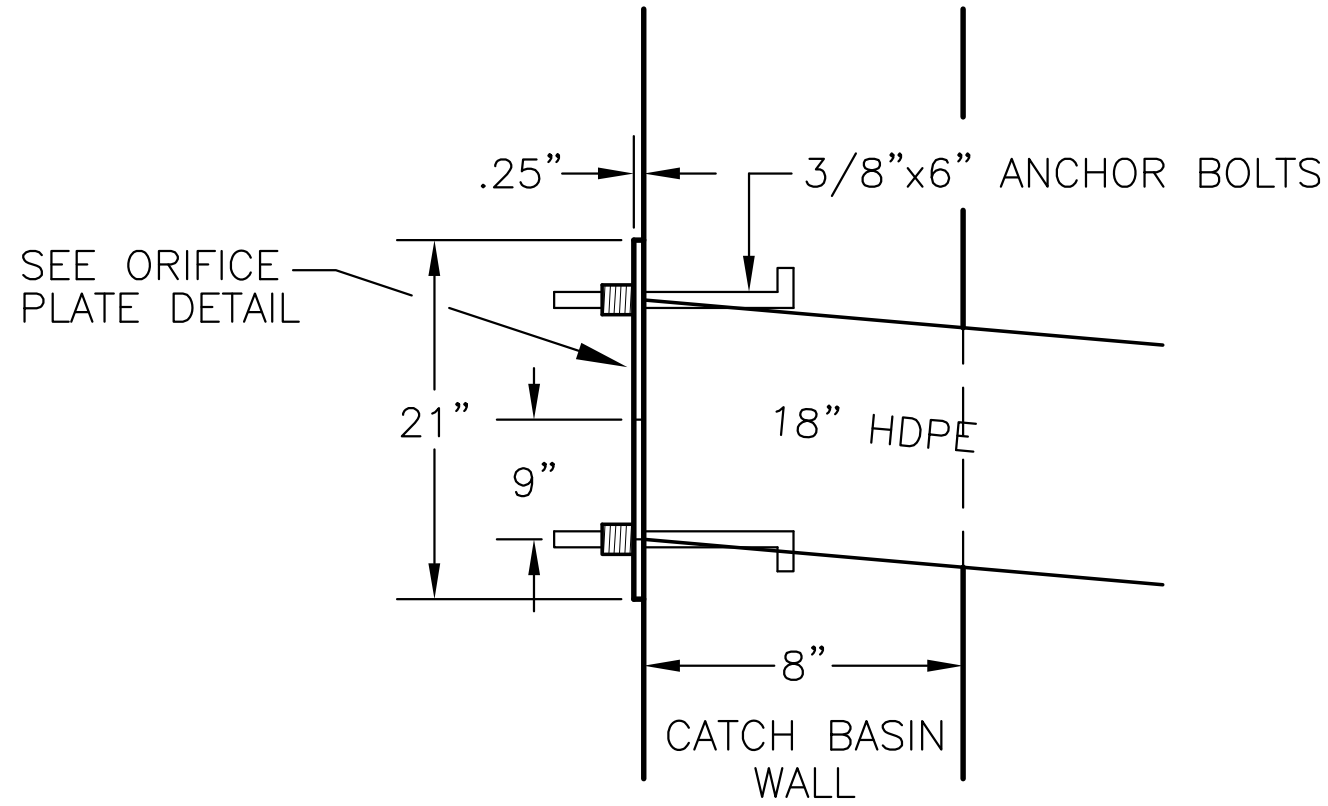
R = D/4

S = Slope

n = 0.015

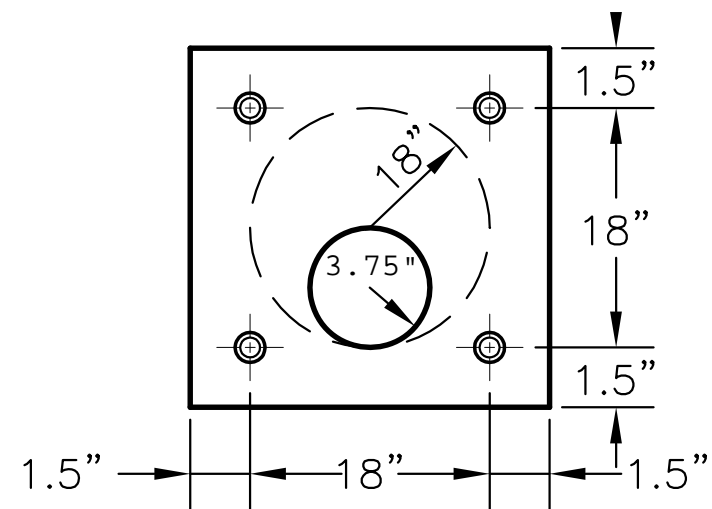
EROSION CONTROL NOTES:

1. CONTRACTOR IS RESPONSIBLE FOR OBTAINING A TOPSOIL DISTURBANCE PERMIT PRIOR TO BEGINNING WORK.
2. CONTRACTOR IS RESPONSIBLE FOR MAINTAINING RUN-OFF ON SITE DURING CONSTRUCTION.
3. CONTRACTOR IS RESPONSIBLE FOR CLEANING ALL SEDIMENT THAT GETS INTO EXISTING RIGHT-OF-WAY.
4. REPAIR OF DAMAGED FACILITIES AND CLEANUP OF SEDIMENT ACCUMULATIONS ON ADJACENT PROPERTIES AND IN PUBLIC FACILITIES IS THE RESPONSIBILITY OF THE CONTRACTOR.
5. ALL EXPOSED EARTH SURFACES MUST BE PROTECTED FROM WIND AND WATER EROSION PRIOR TO FINAL ACCEPTANCE OF ANY PROJECT.



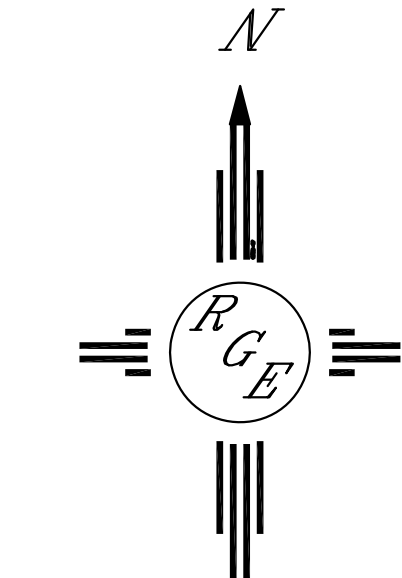
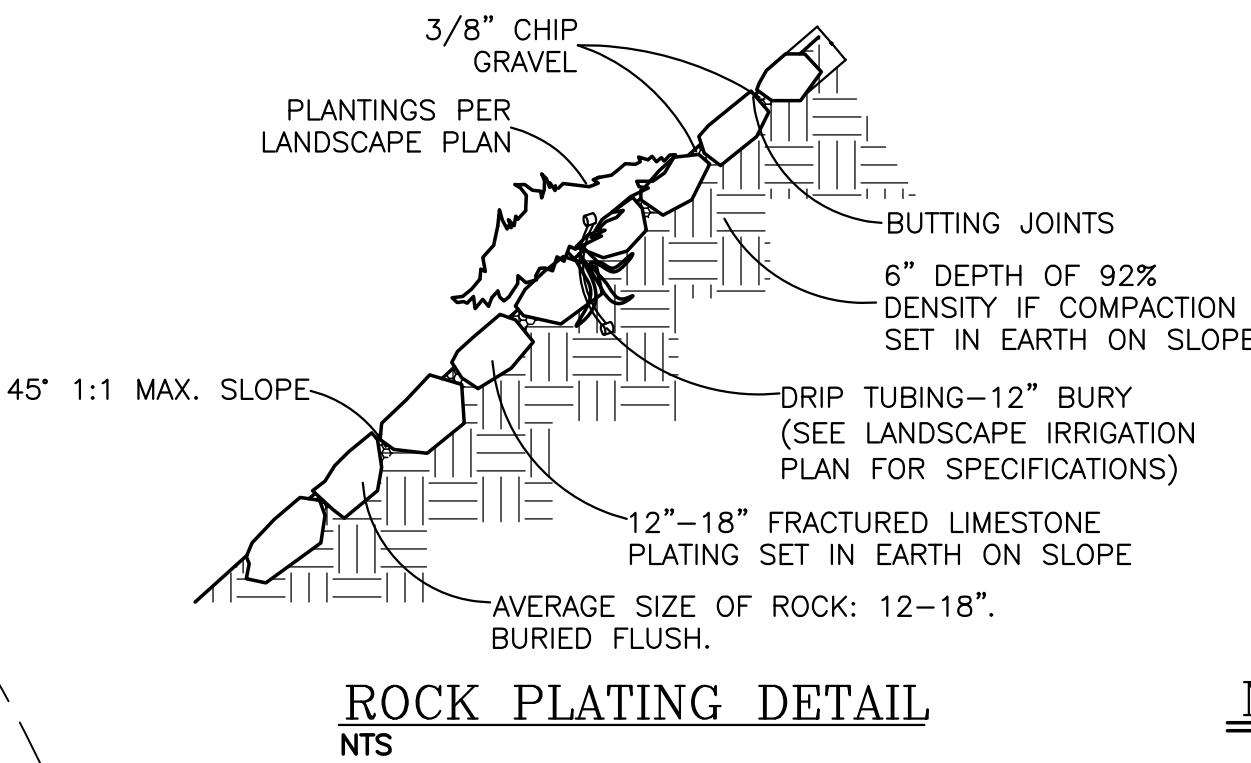
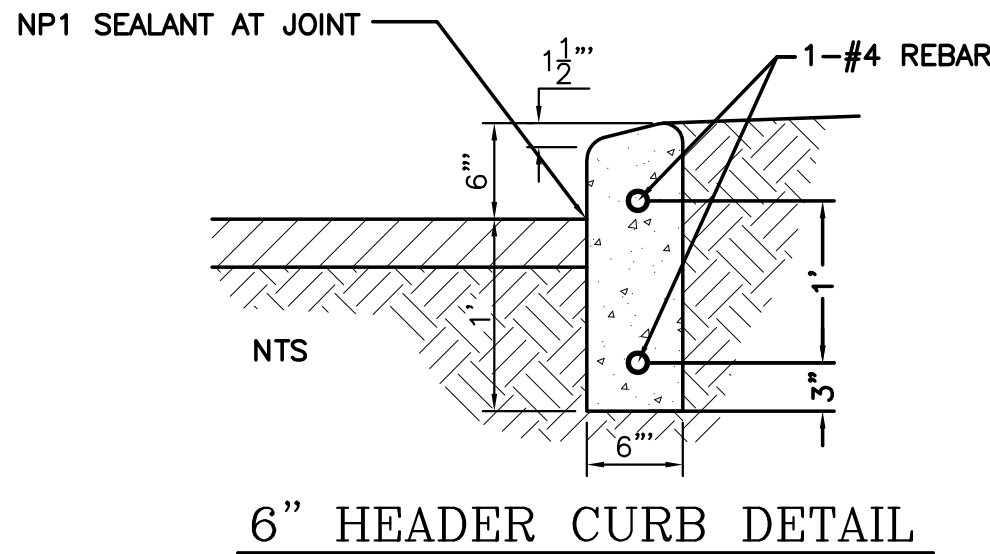
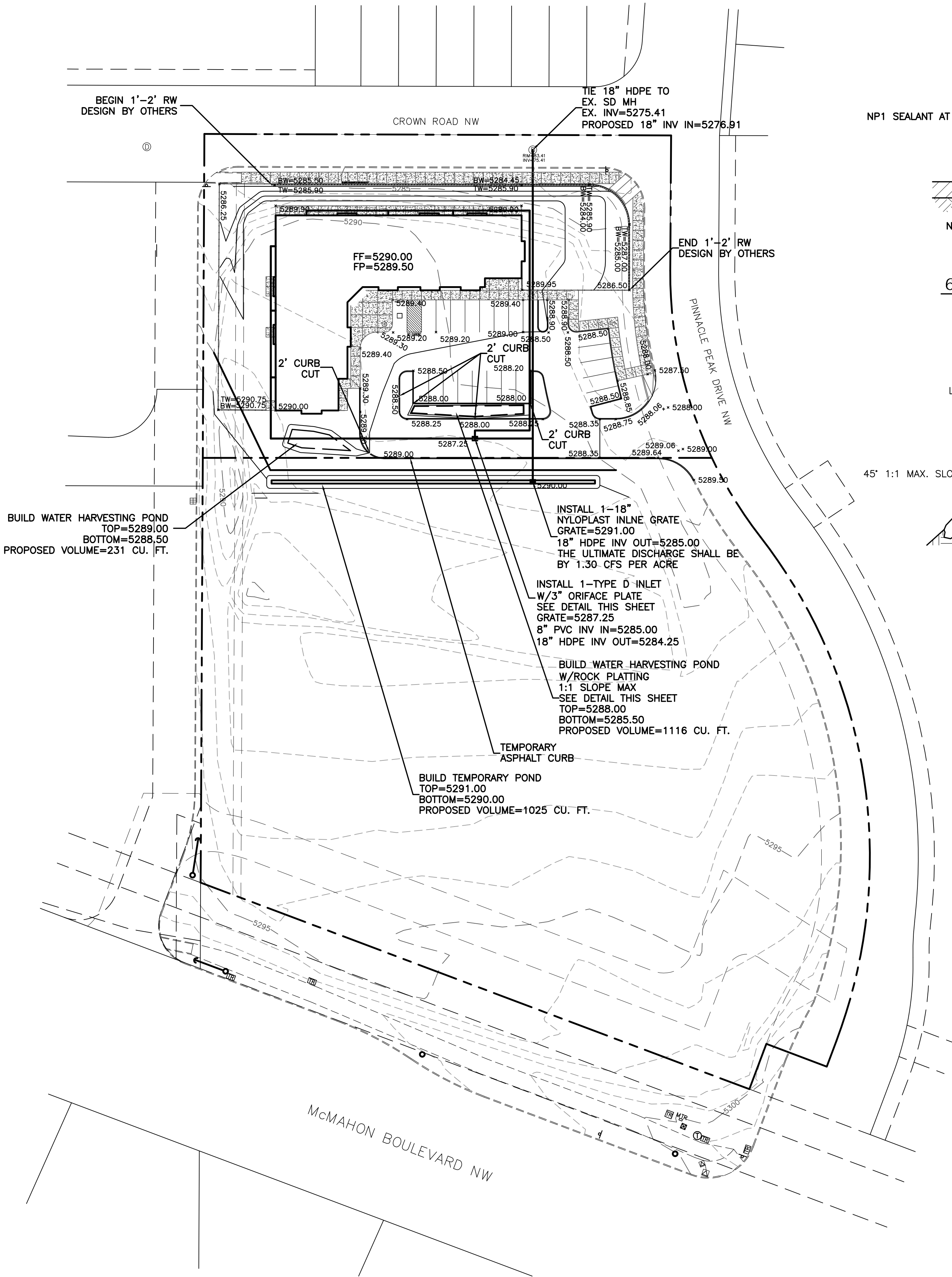
DETAIL A

TO BE INSTALLED @ THE OUTFLOW OF THE CATCH BASINS (SEE THIS PLAN FOR ORIFICE PLATE SIZES)

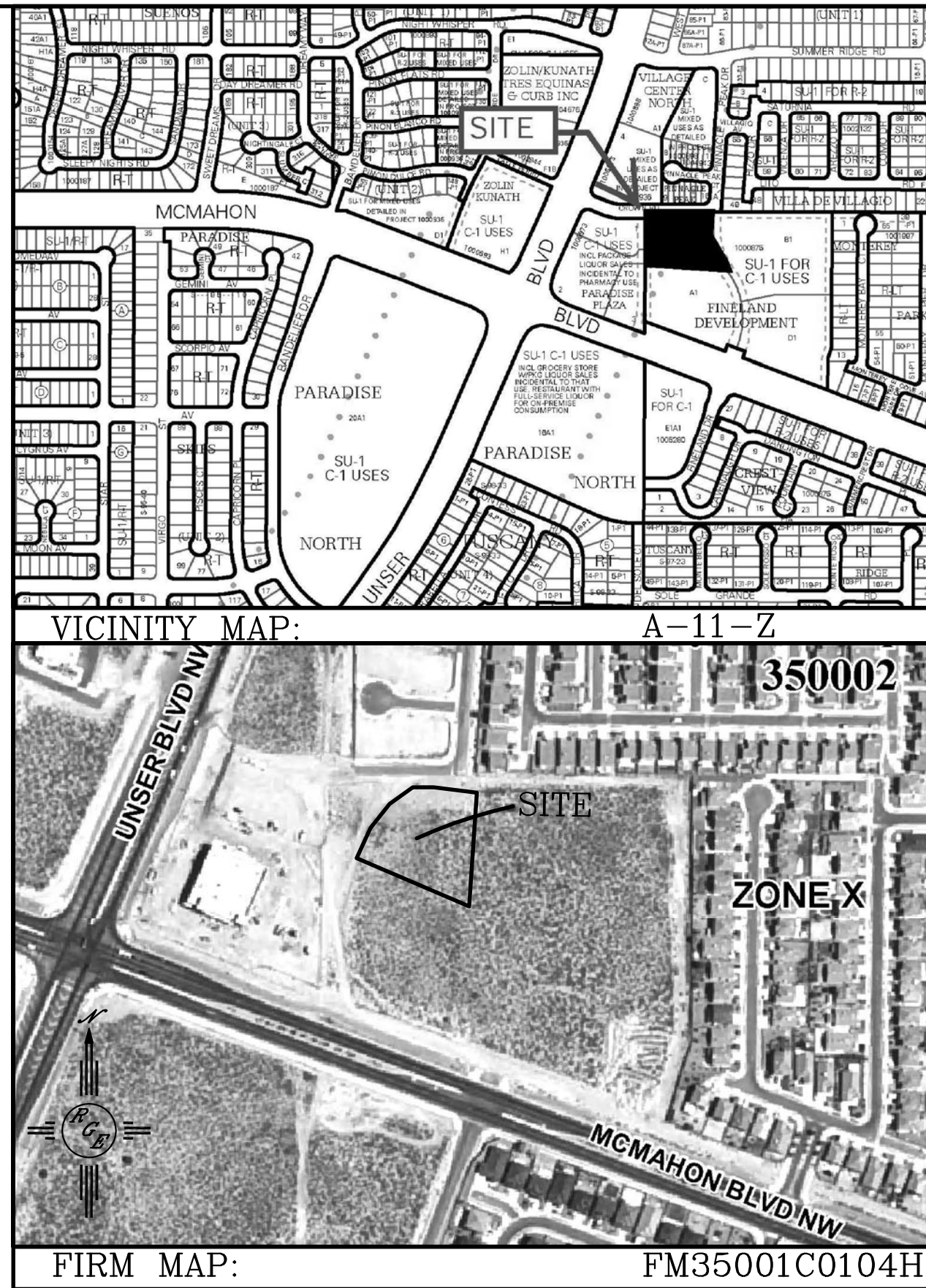


TYP. ORIFICE PLATE DETAIL
N.T.S.

CAUTION:
EXISTING UTILITIES ARE NOT SHOWN. IT SHALL BE THE SOLE RESPONSIBILITY OF THE CONTRACTOR TO CONDUCT ALL NECESSARY FIELD INVESTIGATIONS PRIOR TO ANY EXCAVATION TO DETERMINE THE ACTUAL LOCATION OF UTILITIES & OTHER IMPROVEMENTS.



GRAPHIC SCALE
40 20 0 20 40
SCALE: 1"=40'



LEGAL DESCRIPTION:
PARCEL A-1, FINELAND DEVELOPMENT

- NOTES:**
- ALL SPOT ELEVATIONS REPRESENT FLOWLINE ELEVATION UNLESS OTHERWISE NOTED.
 - ALL CURB AND GUTTER TO 6" HEADER UNLESS OTHERWISE NOTED.
 - ALL RETAINING WALL DESIGN SHALL BE BY OTHERS.
 - ANY CURBS OR PAVEMENT NEGATIVELY IMPACTED BY CONSTRUCTION ACTIVITY SHALL BE REPLACED TO MATCH EXISTING CONDITIONS.
 - ALL SITE WORK SHALL CONFORM TO CITY OF ALBUQUERQUE STANDARDS FOR PUBLIC WORKS CONSTRUCTION EDITION 9

LEGEND	
---	EXISTING CONTOUR
- - -	EXISTING INDEX CONTOUR
---	PROPOSED CONTOUR
---	PROPOSED INDEX CONTOUR
---	SLOPE TIE
x	EXISTING SPOT ELEVATION
x	PROPOSED SPOT ELEVATION
---	BOUNDARY
---	CENTERLINE
---	RIGHT-OF-WAY
---	PROPOSED CURB
---	EXISTING CURB AND GUTTER
---	PROPOSED SIDEWALK
---	EXISTING SIDEWALK
---	PROPOSED RETAINING WALL (SEE STRUCTURAL DRAWINGS)
---	6" CONCRETE OVER 4" AGGREGATE BASE COURSE, WITH 12" SUBGRADE PREP
---	REMOVE AND REPLACE EX.SW PER COA STD DWG #2430

ENGINEER'S SEAL 1/18/18	KIDZ ACADEMY	DRAWN BY WCWJ
	GRADING AND DRAINAGE PLAN	DATE 9-25-17
	 1606 CENTRAL AVENUE SE SUITE 201 ALBUQUERQUE, NM 87106 (505) 872-0989	21801-LAYOUT-9-25-17
DAVID SOULE P.E. #14522		SHEET # 21801