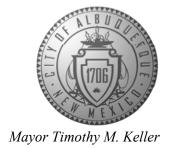
CITY OF ALBUQUERQUE

Planning Department Alan Varela, Interim Director



July 6, 2022

Thomas C. David, JR, P.E. Pan American Engineers, LLC 1717 Jackson Street Alexandria, LA 71301

RE: **Murphy Express** 10601 Unser Blvd NW **Grading and Drainage Plan** Engineer's Stamp Date: 6/13/2022 **Hydrology File: A11D017A**

Dear Mr. David:

Based upon the information provided in your submittal received 7/5/22, the Grading and Drainage Plan is approved for Building Permit and Grading Permit. Please attach a copy of this approved plan in the construction sets for Building Permit processing along with a copy of this letter. Please attach a copy of this approved plan in the construction sets for Building Permit processing along with a copy of this letter. Prior to approval in support of Permanent Release of Occupancy by Hydrology, Engineer Certification per the DPM checklist will be required.

As a reminder, if the project total area of disturbance (including the staging area and any work within the adjacent Right-of-Way) is 1 acre or more, then an Erosion and Sediment Control (ESC) Plan and Owner's certified Notice of Intent (NOI) is required to be submitted to the Stormwater Quality Engineer (Doug Hughes, PE, jhughes@cabq.gov, 924-3420) 14 days prior to any earth disturbance.

Please provide Drainage Covenant for the stormwater quality ponds per Article 6-15(C) of the DPM prior to Permanent Release of Occupancy. Please submit an electronic file of the Covenant and Exhibit for completeness to Marion G. Velasquez at mgvelasquez@cabq.gov. Once the electronic file is approved for completeness, please submit the original copies along with the \$\\$ 25.00 recording fee check made payable to Bernalillo County to Marion on the 4th floor of Plaza de Sol. Please note that Hydrology will need a pdf copy of the recorded Drainage Covenant prior to Hydrology's approval of Permanent Release of Occupancy.

If you have any questions, please contact me at 924-3695 or dggutierrez@cabq.gov.

Sincerely,

David G. Gutierrez, P.E. Senior Engineer, Hydrology

Wir Gul

Planning Department

PO Box 1293

Albuquerque

NM 87103

www.cabq.gov

Drainage Calculations For Murphy Express

Located at Lot 5 of Unser and McMahon Center Subdivision 10601 Unser Boulevard Northwest Albuquerque, New Mexico



Prepared by
Pan American Engineers, LLC
May 28, 2022
PAE Job No. 11605

Drainage Calculations



Prepared By: Wesley D. Miller, P.E.

Pan American Engineers, LLC

1717 Jackson Street Alexandria, LA 71301 (318) 473-2100

Wesley@paealex.com

Date: June 17, 2022

Re: Murphy Express

10601 Unser Boulevard NW Albuquerque, New Mexico

PAE Job No. 11605

Introduction

The referenced development consist of the construction of a Murphy Express Fueling Station (2,824 s.f. building with eight fuel islands under canopy). These calculations will also account for and include the adjacent eastern access road to be constructed as part of the Murphy Express project.

The Murphy Express lot will be located on Lot 5 of Unser and McMahan Center Subdivision. The adjacent access road will be located partially on Lot 5 and partially on Lot 6 of the Unser and McMahon Center Subdivision.

The drainage of this site shall be in compliance with the approved "Master Drainage Plan for Paradise North SW Corner of Unser/McMahon" prepared by Tierra West, LLC dated August 3, 2020 (Master Drainage Plan).

Murphy Express 10601 Unser Boulevard NW Albuquerque, New Mexico PAE Job No. 11605

Stormwater Runoff Calculations

The Master Drainage Plan divides the subdivision into drainage basins P1-P17. The Master Drainage Plan outlines the allowable runoff for each basin. The Murphy Development is located within the following basins: P6, P7, P13, P14, and P15 (see enclosed Drainage Basin Exhibit and excerpts from the Master Drainage Plan).

The discharge from the Murphy Development was calculated using Chapter 6-1(A) of the City of Albuquerque Drainage Manual. The site is in Zone 1. Table 6.8 and equation 6.6 were used to calculate the discharge.

The calculated allowable and actual discharges for the Murphy Development can be seen on the enclosed Drainage Calculations Table. As shown in the table, the Murphy Development is within the allowable rate of discharge for each individual basin as well as overall discharge.

Stormwater Quality Volume

Per City of Albuquerque plan review comments, stormwater quality volume (SWQV) shall be calculated as follows: 0.42" x impervious area.

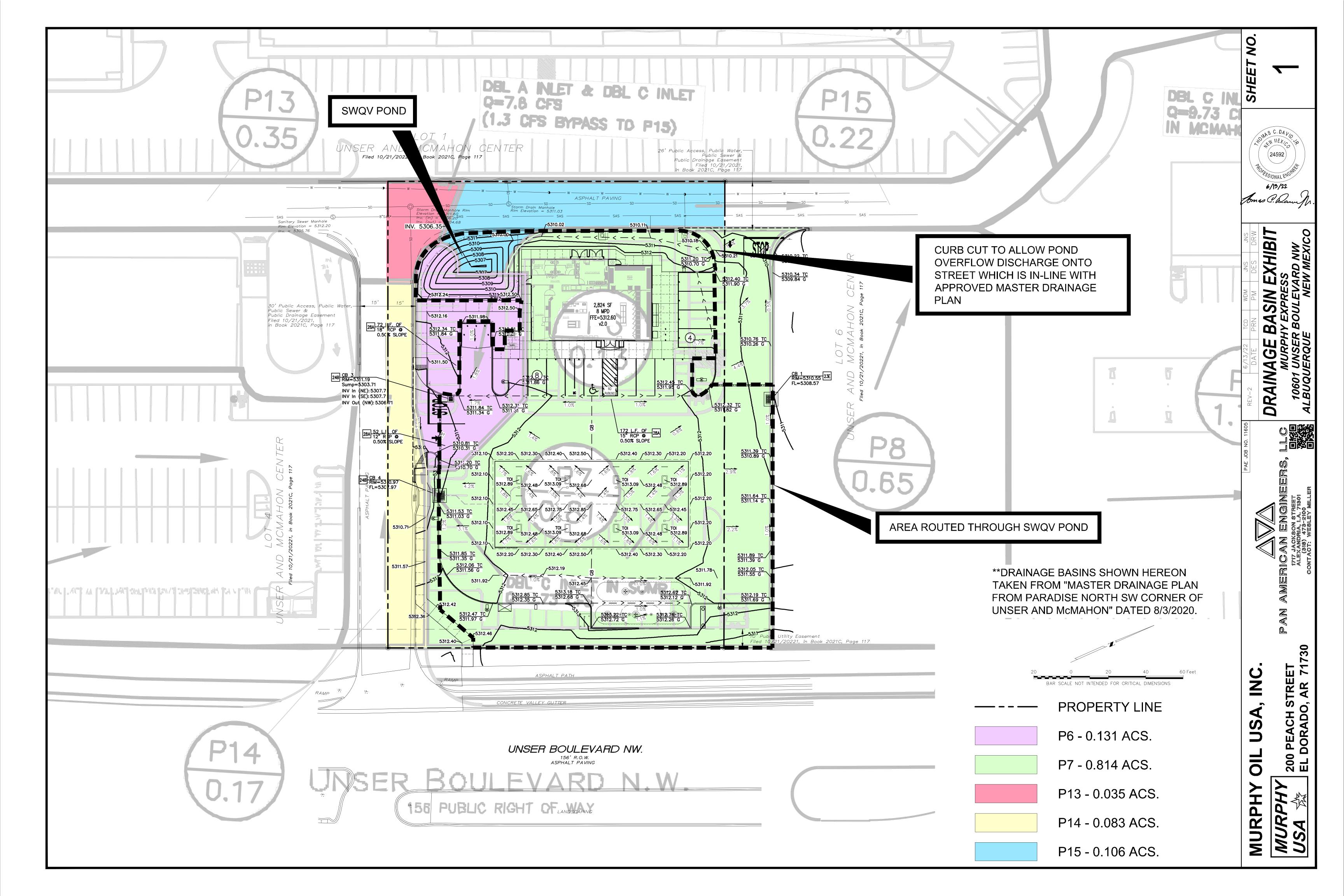
We are able to collect and provide SWQV for approximately 84% of the development. Due to site grades and layout, we are unable to capture the entire development.

The portion of the site that will be captured will require 1,064 c.f. of SWQV (see enclosed SWQV Map). The SWQV shown in the development plans provides 1,428.53 c.f. of storage.

The portion of the site that cannot be captured requires 175 c.f. of SWQV. It is requested that a payment-in-lieu be approved for this portion. Payment-in-lieu required for this portion is \$1,400 (175 c.f. x \$8/c.f.).

Drainage Calculations Table
May 28, 2022

	May 28, 2022											
Ma	Master Drainage Plan Murphy Development											
	Basin Area	Allowable Discharge	Area of Murphy Development Within M	Master Drainage Plan Basins	Allowable Discharge (C x E)	Land Treatment "C"	Land Treatment "D"	Discharge ((2.87 x G) + (4.12 x H))				
Basin Designation	Acres	CFS	Acres	Percent Area of Master Drainage Plan Basin (D / B)	CFS	Acres	Acres	CFS				
(A)	(B)	(C)	(D)	(E)	(F)	(G)	(H)	(I)				
P6	0.131	0.52	0.131	100%	0.52	0.055	0.076	0.47				
P7	0.814	3.25	0.814	100%	3.25	0.122	0.692	3.20				
P13	0.352	1.45	0.035	10%	0.14	0.000	0.035	0.14				
P14	0.168	0.69	0.083	49%	0.34	0.000	0.083	0.34				
P15	0.221	0.91	0.106	48%	0.44	0.018	0.088	0.41				
Total	1.69	6.82	1.17		4.69			4.57				



Z

SET PK NAIL WITH DISK ELEVATION= 5311.15 SET PK NAIL DISK ELEVATION= 5312.78

PROPOSED CURB CUT

THE AREA SHOWN WITHIN SWQV 1 WILL BE ROUTED THROUGH THE SWQV POND. WHEN THE POND IS OVERTOPPED THE SITE DISCHARGE WILL FLOW THROUGH THE CURB CUT TO ALLOW POND OVERFLOW DISCHARGE ONTO STREET WHICH IS INLINE WITH THE APPROVED MASTER DRAINAGE PLAN

DRAINAGE AREA										
DRAINAGE AREA	AREA (ACS.)	IMPERVIOUS AREA (ACS.)	SWQV REQUIRED (0.42" × IMP. AREA) (CF)	SWQV PROVIDED						
SWQV 1	0.827	0.67	1,064	1428.53						
SWQV 2	0.157	0.11	175	PAYMENT-IN-LIEU: \$1,400						





Hydrant

Water Meter

Concrete Symbol

Irrigation Box

Set Monument

EXISTING		
	Storm Drain Manhole	ی
Ś	Sanitary Sewer Manhole	
SAS	Sanitary Sewer Line	
SD	Storm Drain Line	
# 	Storm Drain Inlet	
——— W ———	Underground Water Line	

PROPOSED	
	PROPERTY BOUNDARY LINE
—GB— —	GRADE BREAK
- - 15	EXISTING CONTOUR ELEVATIONS

XX.XX

FLOW DIRECTION SPOT ELEVATIONS: XX.XX TC = TOP OF CURB XX.XX G = GUTTERXX.XX TOI = TOP OF ISLAND XX.XX = FINISHED GRADEDRAINAGE SLOPE AND DIRECTION

PROPOSED STORM PIPE

PROPOSED CONTOUR ELEVATIONS

GENERAL GRADING NOTES

- A. PRIOR TO INSTALLATION OF STORM OR SANITARY SEWER, WATER MAIN, OR ANY OTHER UTILITIES, THE CONTRACTOR SHALL EXCAVATE, VERIFY, AND CALCULATE ALL POINTS OF CONNECTION AND ALL UTILITY CROSSING AND INFORM THE OWNER AND THE ENGINEER OF ANY CONFLICTS OR REQUIRED DEVIATIONS FROM THE PLAN PRIOR TO CONSTRUCTION, NOTIFICATION SHALL BE MADE A MINIMUM OF 72 HOURS PRIOR TO CONSTRUCTION, THE ENGINEER AND ITS CLIENTS SHALL BE HELD HARMLESS IN THE EVENT THAT THE CONTRACTOR FAILS TO MAKE
- B. ALL SLOPES AND AREAS DISTURBED BY CONSTRUCTION SHALL BE GRADED SMOOTH AND GROUND COVER ESTABLISHED. ANY RELOCATED TREES SHALL BE MAINTAINED UNTIL SUCH POINT AS TREE IS RE-ESTABLISHED. ANY AREAS DISTURBED FOR ANY REASON PRIOR TO FINAL ACCEPTANCE OF THE PROJECT SHALL BE STABILIZE BY THE CONTRACTOR AT NO ADDITIONAL COST TO THE
- C. THE CONTRACTOR IS SPECIFICALLY CAUTIONED THAT THE LOCATION AND/OR ELEVATION OF EXISTING UTILITIES AS SHOWN ON THESE PLANS IS BASED ON RECORDS OF THE VARIOUS UTILITY COMPANIES, AND WHERE POSSIBLE, MEASUREMENTS TAKEN IN THE FIELD. THE INFORMATION IS NOT TO BE RELIED ON AS BEING EXACT OR COMPLETE. THE CONTRACTOR MUST CALL THE APPROPRIATE UTILITY COMPANY AT LEAST 72 HOURS BEFORE ANY EXCAVATION TO REQUEST EXACT FIELD LOCATION OF UTILITIES.
- D. REFER TO GEOTECHINCAL REPORT FOR SPECIFIC SITE SOIL CONDITIONS AND CONSIDERATIONS.
- E. CONTRACTOR SHALL COMPLY COMPLETELY WITH THE LATEST STANDARDS OF OSHA DIRECTIVES OR ANY OTHER AGENCY HAVING JURISDICTION FOR EXCAVATION AND TRENCHING PROCEDURES. THE CONTRACTOR SHALL USE SUPPORT SYSTEMS, SLOPING, BENCHING AND OTHER MEANS OF PROTECTION. THIS IS TO INCLUDE, BUT NOT LIMITED FOR ACCESS AND EGRESS FROM ALL EXCAVATION AND TRENCHING. CONTRACTOR IS RESPONSIBLE FOR COMPLYING WITH PERFORMANCE CRITERIA AS REQUIRED BY OSHA.
- F. ALL HDPE PIPE IN SANDY OR HIGHLY EROSIVE, OR EXPANSIVE SOILS SHALL BE N-12 WT IB (OR EQUIVALENT WITH SMOOTH INTERIOR AND ANNULAR EXTERIOR CORRUGATIONS. 4"-48" PIPE SHALL MEET ASTM F2648 (OR AASHTO M252 TYPE S) REQUIREMENTS AND SHALL HAVE A MINIMUM MANNINGS "n" DESIGN VALUE OF 0.012. JOINTS SHALL BE WATERTIGHT ACCORDING TO ASTM D3212 (OR AASHTO M252, M294) REQUIREMENTS. GASKETS SHALL MEET THE REQUIREMENTS OF ASTM F477. JOINT PERFORMANCE, FITTINGS, MATERIAL PROPERTIES AND INSTALLATION SHALL BE DONE PER THE COMPLETE ADS SPECIFICATION FOR ADS N-12 WE IB PIPE FOUND IN THE ADS, INC. DRAINAGE HANDBOOK, LATEST EDITION.
- G. ALL OTHER HDPE PIPE SHALL BE N-12 ST IB (OR EQUIVALENT WITH SMOOTH INTERIOR AND ANNULAR EXTERIOR CORRUGATIONS. 4"-48" SHALL MEET ASTM F2648 (OR AASHTO M252 TYPE S OR SP) REQUIREMENTS AND SHALL HAVE A MINIMUM MANNINGS "n" DESIGN VALUE OF 0.012. JOINT PERFORMANCE, FITTINGS, MATERIAL PROPERTIES AND INSTALLATION SHALL BE DONE PER THE COMPLETE ADS SPECIFICATIONS FOR ADS N-12 ST IB PIPE FOUND IN THE ADS, INC. HAND BOOK, LATEST EDITION.
- H. IF USING HDPE PERFORATED PIPE FOR SUBSURFACE DRAINAGE AND DETENTION/RETENTION SYSTEMS, THE PERFORATION SHALL MEET THE AASHTO CLASS II STANDARD PERFORATION PATTERN REQUIREMENTS.
- I. STORM SEWER LINES 18"-54" DIAMETER ARE TO BE REINFORCED CONCRETE PIPE ACCORDING TO ASTM C-76 (WATERTIGHT) UNLESS OTHERWISE INDICATED.
- J. CORRUGATED METAL PIPE, WHERE SPECIFICALLY SPECIFIED ON PLAN, SHALL BE TYPE II OF AASHTO M 36 GALVANIZED WITH TYPE 3 JOINTS.
- K. CONTRACTOR SHALL ESTABLISH GRADES OF FINISH PAVEMENT TO ENSURE PROPER (POSITIVE) DRAINAGE AND PREVENT PUDDLING OF WATER, SPECIALLY IN PEDESTRIAN WALKWAYS. UNPAVED AREAS OF SITE SHALL ALSO BE GRADED FOR POSITIVE DRAINAGE. CONSULT ENGINEER SHOULD THEIR BE CONFLICTS WITH CRITICAL GRADES SHOWN HEREON.
- L. CONTRACTOR SHALL VERIFY ACTUAL FINISH FLOOR ELEVATION AFTER BUILDING SET AND NOTIFY ENGINEER AND PM WITHIN 48 HOURS OF ANY DISCREPANCY.
- M. PIPE JOINTS SHALL BE WRAPPED WITH GEOTEXTILE FABRIC (LAPPED 18").

GRADING NOTES

18D MATCH EXISTING PAVEMENT ELEVATIONS

GRADING DETAILS

23E COMBINATION INLET

- 24B AREA INLET
- 27B ACO DRAIN SK300 PRE SLOPED CHANNEL WITH D.I. SLOTTED GRATE
- 28A STORM SEWER TRENCH & BEDDING
- 42F ENVIROHOOD BY ADS

CB 1 RIM=5310.55 23E

FL=5308.57

5311.89 TC 5311.39 G

,____

MATCH ELEVATION AT ASPHALT TRAIL

ASPHALT PAVING

2,824 SF

8 MPD

FFE=5312.60 v2.0

/5312.40 /5312.30 /5312.20 /5312.20

TOI 5313.09 5312.48

5312.40 5312.30 5312.20

Storm Drain Manhole Rim Elevation = 5311.03

TOI 5312.48 5313.09 5312.68

5312.20 5312.30 5312.40 5312.50

ASPHALT PATH

ASPHALT TRAIL

CONCRETE VALLEY GUTTER

5312.50

LOT 1

UNSER AND MCMAHON CENTER

5306.35-

-5312.16

28A 18" RCP ©

0.50% SLOPE

Filed, 10/21/20221, in Book 2021C, Page 117

SWQV POND

24B CB 3 25 RIM=5311.19

Sump=5303.71

INV In (NE): 5307.71 INV In (SE): 5307.71

INV Out (NW): 5306.7

nitary Sewer Manho/e m Elevation = 5312.20

v. = 5305.76

MASTER DRAINAGE PLAN

For

Paradise North SW Corner of Unser/McMahon

Prepared by:

Tierra West, LLC 5571 Midway Park Place NE Albuquerque, New Mexico 87109

August 3, 2020

I certify that this report was prepared under my supervision, and I am a registered professional engineer in the State of New Mexico in good standing.

7868

OB-04-20

Ronald R. Bohannan
PE # 7868

Job No. 2020015

DPM Weighted E Method

Precipitation Zone 1

SW Corner Unser Blvd/McMahon Blvd

Tracts 20A-1A thru 20A-1I Paradise North (Formerly Tract 20A-1 Paradise North)

TWLLC VP Date 7/22/2020

Existing Conditions

	Basin Descriptions										100	-Year, 6-Hr		10	-Year, 6-Hr		
Basin	Area	Area	Area	Treatr	nent A	Treatr	nent B	Treati	ment C	Treatr	ment D	Weighted E	Volume	Flow	Weighted E	Volume	Flow
ID	(sf)	(acres)	(sq miles)	%	(acres)	%	(acres)	%	(acres)	%	(acres)	(in)	(ac-ft)	cfs	(in)	(ac-ft)	cfs
E1	852,497.83	19.571	0.03058	100%	19.571	0%	0.000	0%	0.000	0%	0.000	0.550	0.897	30.14	0.110	0.179	5.87
E2	44,474.92	1.021	0.00160	100%	1.021	0%	0.000	0%	0.000	0%	0.000	0.550	0.047	1.57	0.110	0.009	0.31
E3	6,826.56	0.157	0.00024	100%	0.157	0%	0.000	0%	0.000	0%	0.000	0.550	0.007	0.24	0.110	0.001	0.05
Total	903,799.31	20.748	0.03242		0.000		0.000		0.000		0.000		0.951	31.95		0.190	6.22

Proposed Conditions

Порозоц	Basin Descriptions										100	-Year, 6-Hr		10-	Year, 6-Hr		
Basin	Area	Area	Area	Treatr	nent A	Treatr	nent B	Treatr	nent C	Treatr	ment D	Weighted E	Volume	Flow	Weighted E	Volume	Flow
ID	(sf)	(acres)	(sq miles)	%	(acres)	%	(acres)	%	(acres)	%	(acres)	(in)	(ac-ft)	cfs	(in)	(ac-ft)	cfs
P1	389,121.54	8.933	0.01396	0%	0.000	0%	0.000	10%	0.893	90%	8.040	2.111	1.571	35.69	1.330	0.990	21.97
P2	46,633.06	1.071	0.00167	0%	0.000	0%	0.000	10%	0.107	90%	0.963	2.111	0.188	4.28	1.330	0.119	2.63
Р3	26,588.77	0.610	0.00095	0%	0.000	0%	0.000	10%	0.061	90%	0.549	2.111	0.107	2.44	1.330	0.068	1.50
P4	34,092.59	0.783	0.00122	0%	0.000	0%	0.000	10%	0.078	90%	0.704	2.111	0.138	3.13	1.330	0.087	1.92
P5	60,232.39	1.383	0.00216	0%	0.000	0%	0.000	10%	0.138	90%	1.244	2.111	0.243	5.52	1.330	0.153	3.40
P6	5,706.02	0.131	0.00020	0%	0.000	0%	0.000	10%	0.013	90%	0.118	2.111	0.023	0.52	1.330	0.015	0.32
P7	35,443.02	0.814	0.00127	0%	0.000	0%	0.000	10%	0.081	90%	0.732	2.111	0.143	3.25	1.330	0.090	2.00
P8	28,224.99	0.648	0.00101	0%	0.000	0%	0.000	10%	0.065	90%	0.583	2.111	0.114	2.59	1.330	0.072	1.59
P9	75,044.37	1.723	0.00269	0%	0.000	0%	0.000	10%	0.172	90%	1.551	2.111	0.303	6.88	1.330	0.191	4.24
P10	29,615.39	0.680	0.00106	0%	0.000	0%	0.000	10%	0.068	90%	0.612	2.111	0.120	2.72	1.330	0.075	1.67
P11	36,890.30	0.847	0.00132	0%	0.000	0%	0.000	10%	0.085	90%	0.762	2.111	0.149	3.38	1.330	0.094	2.08
P12	9,404.71	0.216	0.00034	0%	0.000	0%	0.000	0%	0.000	100%	0.216	2.240	0.040	0.89	1.430	0.026	0.55
P13	15,328.54	0.352	0.00055	0%	0.000	0%	0.000	0%	0.000	100%	0.352	2.240	0.066	1.45	1.430	0.042	0.90
P14	7,315.89	0.168	0.00026	0%	0.000	0%	0.000	0%	0.000	100%	0.168	2.240	0.031	0.69	1.430	0.020	0.43
P15	9,646.85	0.221	0.00035	0%	0.000	0%	0.000	0%	0.000	100%	0.221	2.240	0.041	0.91	1.430	0.026	0.57
P16	9,520.30	0.219	0.00034	0%	0.000	0%	0.000	0%	0.000	100%	0.219	2.240	0.041	0.90	1.430	0.026	0.56
P17	11,897.56	0.273	0.00043	0%	0.000	0%	0.000	0%	0.000	100%	0.273	2.240	0.051	1.13	1.430	0.033	0.70
Total	830,706.29	19.070	0.02980		0.000		0.000	·	1.762	·	17.308		3.370	76.37		2.126	47.05

Equations:

Weighted E = Ea*Aa + Eb*Ab + Ec*Ac + Ed*Ad / (Total Area)

Volume = Weighted E * Total Area

Flow = Qa*Aa + Qb*Ab + Qc*Ac + Qd*Ad

Excess Precipitation, E (in.)									
Zone 1	100-Year	10-Year							
Ea	0.55	0.11							
Eb	0.73	0.26							
Ec	0.95	0.43							
Ed	2.24	1.43							

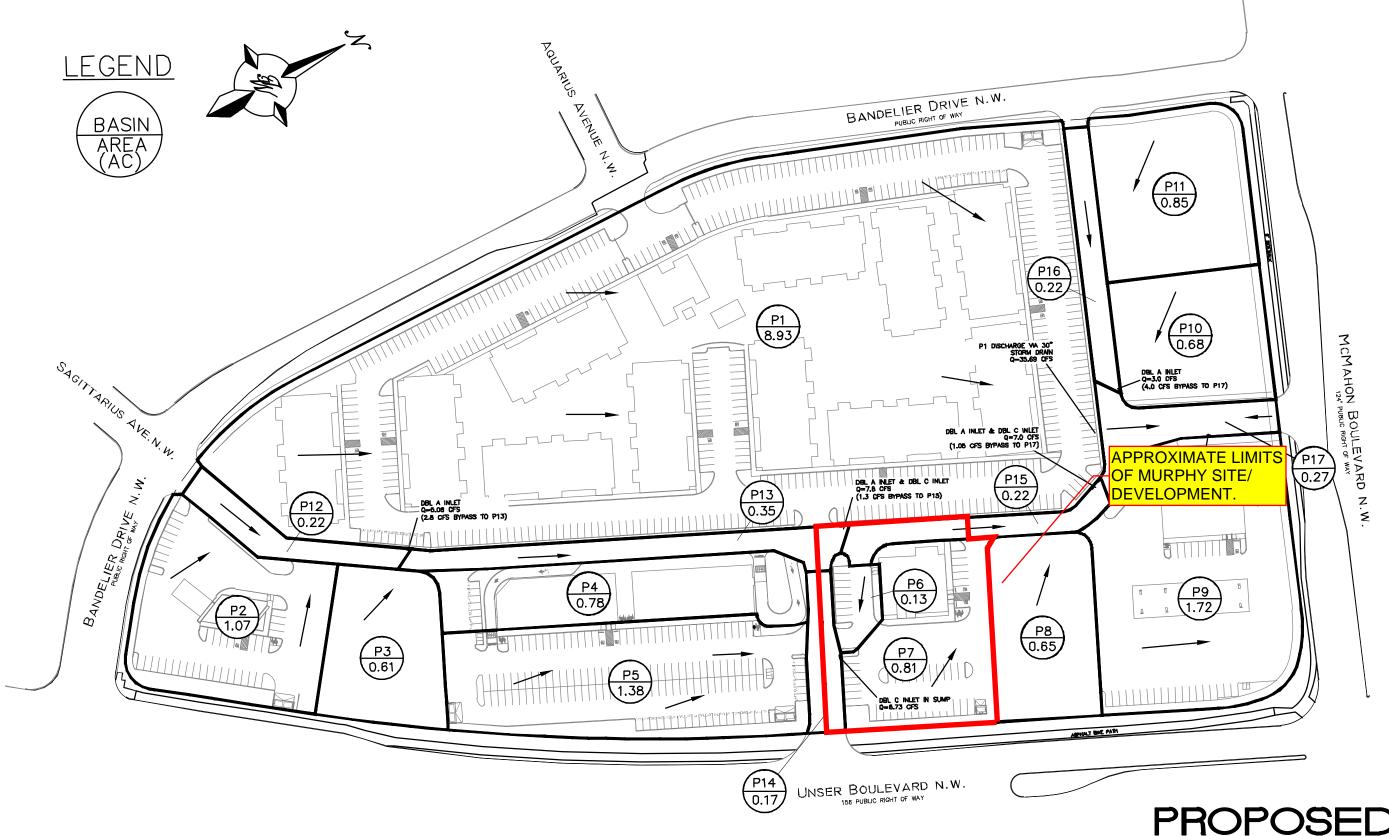
Peak Discharge (cfs/acre)										
Zone 1	100-Year	10-Year								
Qa	1.54	0.3								
Qb	2.16	0.81								
Qc	2.87	1.46								
Qd	4.12	2.57								

Water Quality Volume (Onsite)

Total Impervious Area = 17.307 Acres = 753,892.92 SF

Retainage depth = 0.42" = 0.035' (COA DPM Article 6-12)

Retention Volume = 0.035*753,892.92 = 26,386.25 **CF = 0.606 Ac-Ft**



PROPOSED BASIN MAP

HOLLY PARTNERS, LLC



June 17, 2022

City of Albuquerque Planning Department Plaza Del Sol Building 600 2nd Street, NW Suite 201 Albuquerque, New Mexico 87102

Attention: Mr. David G. Gutierrez, P.E.

Re: Murphy Express

10601 Unser Boulevard NW

Albuquerque, NM

Dear Mr. Gutierrez:

We are the owners/developers of the Unser and McMahon Center Subdivision. Murphy Oil USA, Inc. is developing Lot 5 into a Murphy Express Fueling Station. They are also constructing an access drive to the north of the site, which will be partially located on Lot 6. As the owners of Lots 5 and 6, we approve of this construction. Additionally, Section 5.1(D) of the Recorded Declaration of Easements, Covenants, and Restrictions for the Subdivision (ECR) (Doc # 2022036822, recorded on 04/14/2022) provides for a non-exclusive access easement across all lots of the Subdivision. Enclosed is the recorded ECR, and an exhibit partially depicting the above referenced access easement.

If you have any questions or require additional information, please feel free to Tushar Patel at (505) 362-1702 for assistance.

Yours very truly,

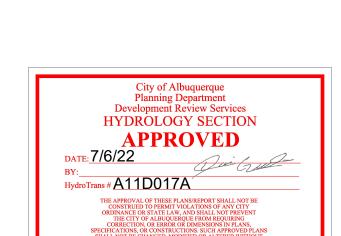
Holly Partners, LLC

Yogesh Patel, Member

Enclosures

CB 1 RIM=5310.55 23E

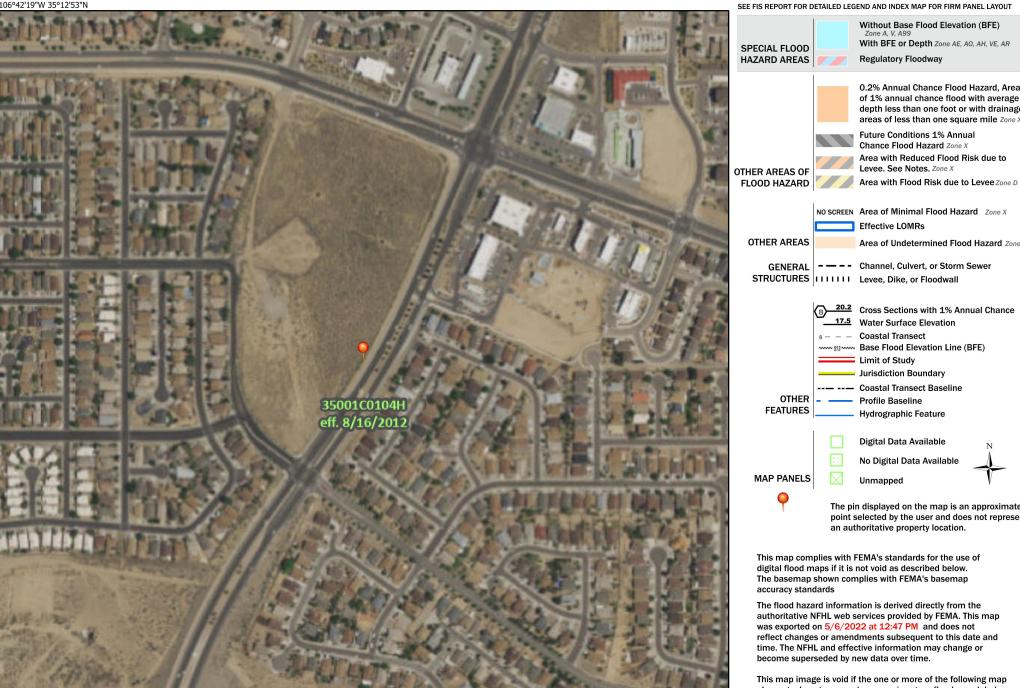
FL=5308.57



Legend

S FEMA

National Flood Hazard Layer FIRMette



Irving Blvd NW

VICINITY MAP

N.T.S.

STRUCTURES | LILLI Levee, Dike, or Floodwall B 20.2 Cross Sections with 1% Annual Chance <u>17.5</u> Water Surface Elevation – – Coastal Transect ----- Base Flood Elevation Line (BFE) Limit of Study Jurisdiction Boundary --- -- Coastal Transect Baseline OTHER - Profile Baseline Hydrographic Feature Digital Data Available No Digital Data Available The pin displayed on the map is an approximate

point selected by the user and does not represent an authoritative property location. This map complies with FEMA's standards for the use of

Without Base Flood Elevation (BFE)

Regulatory Floodway

Future Conditions 1% Annual Chance Flood Hazard Zone X

Effective LOMRs

GENERAL - - - Channel, Culvert, or Storm Sewer

Area with Reduced Flood Risk due to

NO SCREEN Area of Minimal Flood Hazard Zone X

Levee. See Notes. Zone X

With BFE or Depth Zone AE, AO, AH, VE, AR

0.2% Annual Chance Flood Hazard, Areas

of 1% annual chance flood with average

depth less than one foot or with drainage

areas of less than one square mile Zone >

Area of Undetermined Flood Hazard Zone D

digital flood maps if it is not void as described below. The basemap shown complies with FEMA's basemap The flood hazard information is derived directly from the authoritative NFHL web services provided by FEMA. This map was exported on 5/6/2022 at 12:47 PM and does not

time. The NFHL and effective information may change or This map image is void if the one or more of the following map elements do not appear: basemap imagery, flood zone labels,

legend, scale bar, map creation date, community identifiers. FIRM panel number, and FIRM effective date. Map images fo regulatory purposes.



, Hydrant Storm Drain Manhole Sanitary Sewer Manhole Water Meter Sanitary Sewer Line Water Valve Storm Drain Line Concrete Symbol Irrigation Box ——— W —— Underground Water Line Set Monument

PROPOSED

_____20____ XX.XX

PROPOSED CONTOUR ELEVATION FLOW DIRECTION SPOT ELEVATIONS: XX.XX TC = TOP OF CURB XX.XX G = GUTTERXX.XX TOI = TOP OF ISLAND

GRADE BREAK

PROPERTY BOUNDARY LINE

EXISTING CONTOUR ELEVATION

XX.XX = FINISHED GRADEDRAINAGE SLOPE AND DIRECTION PROPOSED STORM PIPE

GENERAL GRADING NOTES

- A. PRIOR TO INSTALLATION OF STORM OR SANITARY SEWER, WATER MAIN, OR ANY OTHER UTILITIES, THE CONTRACTOR SHALL EXCAVATE, VERIFY, AND CALCULATE ALL POINTS OF CONNECTION AND ALL UTILITY CROSSING AND INFORM THE OWNER AND THE ENGINEER OF ANY CONFLICTS OR REQUIRED DEVIATIONS FROM THE PLAN PRIOR TO CONSTRUCTION. NOTIFICATION SHALL BE MADE A MINIMUM OF 72 HOURS PRIOR TO CONSTRUCTION, THE ENGINEER AND ITS CLIENTS SHALL BE HELD HARMLESS IN THE EVENT THAT THE CONTRACTOR FAILS TO MAKE
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- C. THE CONTRACTOR IS SPECIFICALLY CAUTIONED THAT THE LOCATION AND/OR ELEVATION OF EXISTING UTILITIES AS SHOWN ON THESE PLANS IS BASED ON RECORDS OF THE VARIOUS UTILITY COMPANIES, AND WHERE POSSIBLE, MEASUREMENTS TAKEN IN THE FIELD. THE INFORMATION IS NOT TO BE RELIED ON AS BEING EXACT OR COMPLETE. THE CONTRACTOR MUST CALL THE APPROPRIATE UTILITY COMPANY AT LEAST 72 HOURS BEFORE ANY EXCAVATION TO REQUEST EXACT FIELD LOCATION OF UTILITIES.
- D. REFER TO GEOTECHINCAL REPORT FOR SPECIFIC SITE SOIL CONDITIONS AND
- . CONTRACTOR SHALL COMPLY COMPLETELY WITH THE LATEST STANDARDS OF OSHA DIRECTIVES OR ANY OTHER AGENCY HAVING JURISDICTION FOR EXCAVATION AND TRENCHING PROCEDURES. THE CONTRACTOR SHALL USE SUPPORT SYSTEMS, SLOPING, BENCHING AND OTHER MEANS OF PROTECTION. THIS IS TO INCLUDE, BUT NOT LIMITED FOR ACCESS AND EGRESS FROM ALL EXCAVATION AND TRENCHING. CONTRACTOR IS RESPONSIBLE FOR COMPLYING WITH PERFORMANCE CRITERIA AS REQUIRED BY OSHA.
- ALL HDPE PIPE IN SANDY OR HIGHLY EROSIVE, OR EXPANSIVE SOILS SHALL BE N-12 WT IB (OR EQUIVALENT WITH SMOOTH INTERIOR AND ANNULAR EXTERIOR CORRUGATIONS. 4"-48" PIPE SHALL MEET ASTM F2648 (OR AASHTO M252 TYPE S) REQUIREMENTS AND SHALL HAVE A MINIMUM MANNINGS "n" DESIGN VALUE OF 0.012. JOINTS SHALL BE WATERTIGHT ACCORDING TO ASTM D3212 (OR AASHTO M252, M294) REQUIREMENTS. GASKETS SHALL MEET THE REQUIREMENTS OF ASTM F477. JOÍNT PERFORMANCE, FITTINGS, MATERIAL PROPERTIES AND INSTALLATION SHALL BE DONE PER THE COMPLETE ADS SPECIFICATION FOR ADS N-12 WE IB PIPE FOUND IN THE ADS, INC. DRAINAGE HANDBOOK, LATEST
- . ALL OTHER HDPE PIPE SHALL BE N-12 ST IB (OR EQUIVALENT WITH SMOOTH INTERIOR AND ANNULAR EXTERIOR CORRUGATIONS. 4"-48" SHALL MEET ASTM F2648 (OR AASHTO M252 TYPE S OR SP) REQUIREMENTS AND SHALL HAVE A MINIMUM MANNINGS "n" DESIGN VALUE OF 0.012. JOINT PERFORMANCE, FITTINGS, MATERIAL PROPERTIES AND INSTALLATION SHALL BE DONE PER THE COMPLETE ADS SPECIFICATIONS FOR ADS N-12 ST IB PIPE FOUND IN THE ADS, INC. HAND
- H. IF USING HDPE PERFORATED PIPE FOR SUBSURFACE DRAINAGE AND DETENTION/RETENTION SYSTEMS, THE PERFORATION SHALL MEET THE AASHTO CLASS II STANDARD PERFORATION PATTERN REQUIREMENTS.
- I. STORM SEWER LINES 18"-54" DIAMETER ARE TO BE REINFORCED CONCRETE PIPE ACCORDING TO ASTM C-76 (WATERTIGHT) UNLESS OTHERWISE INDICATED.
- J. CORRUGATED METAL PIPE, WHERE SPECIFICALLY SPECIFIED ON PLAN, SHALL BE
- TYPE II OF AASHTO M 36 GALVANIZED WITH TYPE 3 JOINTS. K. CONTRACTOR SHALL ESTABLISH GRADES OF FINISH PAVEMENT TO ENSURE PROPER (POSITIVE) DRAINAGE AND PREVENT PUDDLING OF WATER, SPECIALLY IN PEDESTRIÀN WALKWAYS. UNPAVED AREAS OF SITE SHALL ALSO BE GRADED FOR POSITIVE DRAINAGE. CONSULT ENGINEER SHOULD THEIR BE CONFLICTS WITH
- CONTRACTOR SHALL VERIFY ACTUAL FINISH FLOOR ELEVATION AFTER BUILDING SET AND NOTIFY ENGINEER AND PM WITHIN 48 HOURS OF ANY DISCREPANCY.
- M. PIPE JOINTS SHALL BE WRAPPED WITH GEOTEXTILE FABRIC (LAPPED 18").

⟨ ⟩ GRADING NOTES

18D MATCH EXISTING PAVEMENT ELEVATIONS

CRITICAL GRADES SHOWN HEREON.

GRADING DETAILS

23E COMBINATION INLET

- 24B AREA INLET 27B ACO DRAIN - SK300 PRE SLOPED CHANNEL WITH D.I. SLOTTED GRATE
- 28A STORM SEWER TRENCH & BEDDING
- 42F ENVIROHOOD BY ADS

<u>Б</u>

Z

=



FLOOD ZONE:

TRACT BEING LOT 5 OF UNSER AND MCMAHON CENTER SUBDIVISION WITHIN THE TOWN OF ALAMEDA GRANT, PROJECTED SECTION 2, TOWNSHIP 11 NORTH, RANGE 2 EAST,

N.M.P.M., CITY OF ALBUQUERQUE, BERNALILLO COUNTY, NEW MEXICO

THE SUBJECT PROPERTY (AS SHOW HEREON) APPEARS TO LIE WITHIN "ZONE X"

(AREA OF MINIMAL FLOOD HAZARD) AS SHOWN ON FLOOD INSURANCE RATE MAP

0.50% SLOPE

0.50% SLOPE

24B CB 3 ST. 60

Sump=5303.71

<u>Project Bench Mark</u>

Elevation = 5312.78 (NAVD88)

The Murphy Express site is part of the Unser

Drainage Plan (A11D017) by Tierra West, LLC

and McMahon Center Subdivision Master

dated September 03, 2020.

NUMBER 35001C0104H, MAP REVISED AUGUST 16, 2012.

Set PK Nail with Disk

5312.36

INV In (NE): 5307.71

INV In (SE): 5307.71

INV Out (NW): 5306.71

SEE SHEET C-4.2 FOR **CANOPY DRAINAGE**

7/05-

5312.19 TC 5311.69 G

172 L.F. OF 28A

5313.09 _5312.48

-5312.75 -5312.65 -5312.45

-5312.40 **-**5312.30 **-**5312.20 **-**

 $\sqrt{5312.40}$ $\sqrt{5312.30}$ $\sqrt{5312.20}$ $\sqrt{5312.20}$

0.50% SLOPE

5312.20 \ 5312.30 \ 5312.40 \ 5312.50 \

=5312.48 / 5313.09 5312.68 /

ASPHALT PATH

CONCRETE VALLEY GUTTER

5312.45-

5312.20 5312.30 5312.40 5312.50

5311.92-

ASPHALT TRAIL

ALL CASTINGS SHALL BE COATED WITH BLACK ASPHALTIC VARNISH.

SET PK NAIL WITH DISK ELEVATION= 5311.15 SET PK NAIL DISK

ELEVATION= 5312.78

Basemap: USGS National Map: Orthoimagery: Data refreshed October, 2020 FIRM MAP

