DRAINAGE REPORT

for

Seven Bar Phase 2&3 Entry Driveway Cibola Loop N.E. Albuquerque, New Mexico

Prepared by:

Tierra West, LLC 5571 Midway Park Place NE Albuquerque, New Mexico 87109

November 20, 2009

I certify that this report was prepared under my supervision, and I am a registered professional engineer in the State of New Mexico in good standing.

O. HERNANDER 17893 PROFESSIONAL ENGINE

Joel D. Hernandez PF NO. 17893

Job No 29066

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PURPOSE

The purpose of this report is to provide a drainage management plan for the construction of a dedicated driveway to serve Phase 2 and Phase 3 (Tract B-9J-1A-1) of the Seven Bar Apartments. The proposed driveway will traverse Tract B-9H-1B-1 which was dedicated to the City of Albuquerque for pond/drainage purposes by the Vista Del Parque Subdivision plat. The grading configuration proposes to maintain the historical drainage patterns and detention pond volume in a manner which does not increase the existing maximum discharge. This design and analysis is in accordance with the DPM, Chapter 22, Hydrology Section. The purpose of this report is to provide the drainage analysis and management plan for approval of the new driveway alignment.

pm2 24 h 100 m

INTRODUCTION

The subject of this report is for the construction of an entry driveway to be located on a 0.78- acre tract (pond/drainage lot) located on Cibola Loop Road, between Luna Park Street and the Seven Bar Apartments entry driveway. The site appears on zone atlas page A-13-Z. As shown of FIRM map 35001C0108E, the subject property lies outside mapped flood zones. A previous drainage report by which the pond was designed was completed in April 1997 by Bohannan Huston and titled *Drainage Report for Tract B-9H-1*. This report limits the discharge from the pond to 11.6 cfs to be in conformance with the requirements of SAD No. 223. The proposed driveway was designed to maintain historic drainage patterns by not redirecting flows across property lines and by maintaining the pond volume such that discharge characteristics remain unchanged

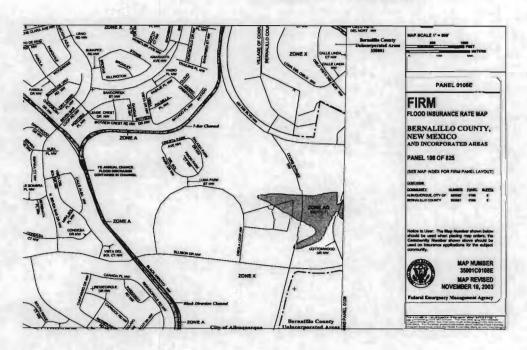
11.65 cds

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Exhibit A- Vicinity Map



Exhibit B- FIRM Map



EXISTING CONDITIONS

The property (Tract B-9J-1A-1) over which Phase 2 and Phase 3 of the Seven Bar Apartments will be constructed is currently undeveloped, however, this site was mass graded along with the construction of the existing apartments in Phase 1. The majority of this site currently drains to a detention pond located at the east side of the property, and a smaller area on the west side of the property drains south onto Phase 1. These flows are conveyed via an existing driveway into an onsite storm drain system.

The pond tract is configured to accept flows from the Vista Del Parque Subdivision conveyed via a 30-inch RCP and to discharge flows through a 24-inch RCP at a maximum flow rate of 11.6 CFS. An excerpt from the report dated April 1997 by Bohannan Huston and titled *Drainage Report for Tract B-9H-1*., is included for reference in Appendix B.

PROPOSED CONDITIONS

A new driveway to provide direct access to Tract B-9J-1A-1 is proposed over the drain-age/pond tract. The roadway is designed with sidewalk culverts near the property boundary to maintain flows onsite, and thusly, not divert existing flows. Because a portion of the graded fill necessary to construct the road would have reduced the pond volume, the design proposes a series of culverts under the road section to serve as underground storage. The road profile is maintained at an elevation above the pond top to maintain volume and all-weather access.

The reconfigured pond was analyzed using AHYMO to ensure the original design discharge would be maintained. Our analysis assumed the same inflow characteristics as the above referenced report with a maximum flow of 67.4 cfs. The model results indicate the reconfigured pond would have a peak discharge of 11.66 cfs, a maximum storage volume of 1.47 acre-feet, and a maximum water surface elevation of 100.00 allowing for a one foot of freeboard. Calculations are found in Appendix A for the detention pond sizing and preliminary design.

SUMMARY AND RECOMMENDATIONS

The drainage management plan presented in this report demonstrates the proposed road alignment is hydraulically feasible as the proposed improvements will be configured to not adversely effect the operation of existing infrastructure. The development of this site is consistent with the DPM, Chapter 22, Hydrology section. It is recommended this road alignment be approved for rough grading.

agreement and coverant for punt

GRANT OF ACCESS EASEMENT AND ENCROACHMENT AGREEMENT

This Grant of Access Easement and Encroachment Agreement is made and executed this ____ day of December, 2009, by the City of Albuquerque (the "City") and Titan 528, LLC, a New Mexico limited liability company ("Titan").

WHEREAS, the City is the owner of that certain real property within Bernalillo County, New Mexico more particularly described as:

Tract B-9H-1B-1, Seven Bar Ranch, as shown and	designated	on	the
plat entitled	_ recorded	in	the
office of the Bernalillo County Clerk on			
in Book, at Page (hereinafter the "City Pro	perty");		

WHEREAS, Titan is the owner of that certain real property within Bernalillo County, New Mexico more particularly described as:

Tract B-9J-1A-1, Seven Bar Ranch, as shown and designated on the plat entitled "Plat of Tracts B-9J-1A-1 and B-9J-1A-2" recorded in the office of the Bernalillo County Clerk on May 16, 2006 in Book 2006C, at Page 156 (hereinafter the "City Property");

WHEREAS, Titan desires, and the City is willing to grant, an easement over and across the City Property in accordance with the terms and conditions of Agreement;

NOW THEREFORE, in consideration of the covenants and conditions set forth herein and other good and valuable consideration, the receipt of which is hereby acknowledged, the City and Titan agree as follows:

1. The City hereby grants to Titan a perpetual, exclusive access easement over
and across that portion of the City Property identified on Exhibit A (the "Easement") for the
purpose of ingress and from the public right-of-way on Cibola Loop N.W. to the Titan
Property. The City understands and agrees that the Titan property will be developed with
multiple multi-family dwelling units and that the Easement will provide ingress and egress
for the residents, guests, common carriers, and others accessing the Titan Property. Titan, at
its sole cost and expense is permitted to construct road, landscaping, wall, signage, lighting,
utilities and other improvements on the Easement. Titan, at its sole cost and expense, may
modify and/or move the storm water drainage pond currently located on the Easement (the
"Pond"); provided, however, any such modification of this tract and the Pond is subject to the
reasonable design approval of the City per the approved Grading and Drainage Plan and
Report engineer stamp dated, file #, approved by the City or

^{2.} From and after the date that Titan makes any improvements or commences construction on the Easement, Titan shall be responsible for all costs and expenses related to

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the use or maintenance of the Easement and all improvements thereon including the maintenance of the City drainage pond located on the Easement. Prior to such date, the Easement and any improvements thereon shall be maintained by the City and any costs and expenses relating to or arising from the use or maintenance of the Easement shall be paid by the City.

- 3. Titan and its successors and assigns shall hold harmless and defend the City from any and all injuries, claims, damages, obligations and liability to the extent that such relate to or arise out of Titan's use of the Easement.
- 4. The rights and obligations of the parties set forth herein shall be binding upon the parties, their successors and assigns. The Easement and the parties' respective rights and obligations hereunder will run with and be appurtenant to the City Property and the Titan Property.

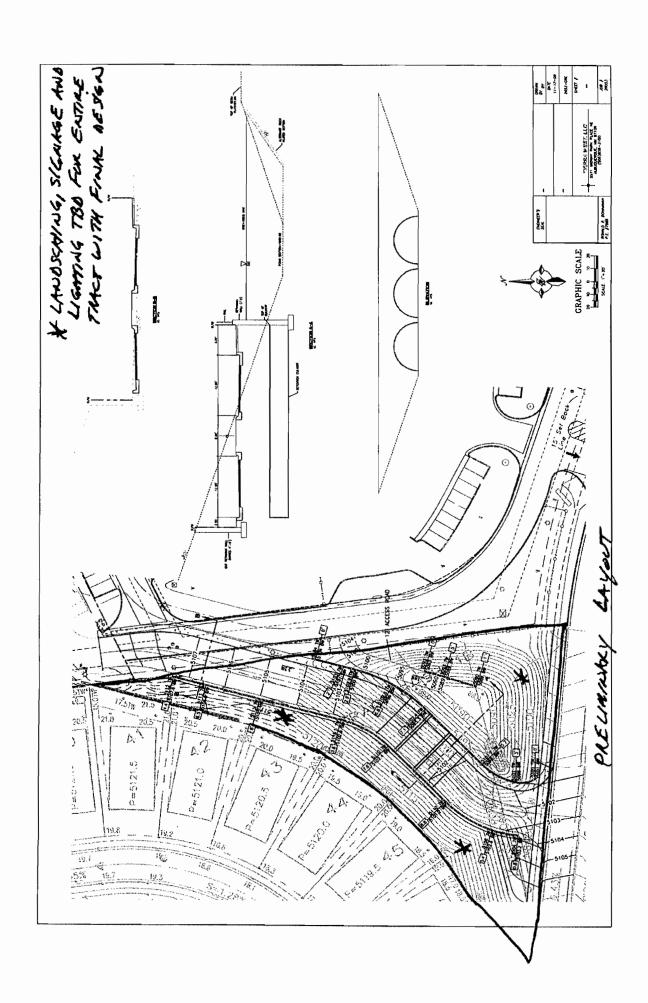
IN WITNESS WHEREOF, the parties forth their hands on the date first above written.

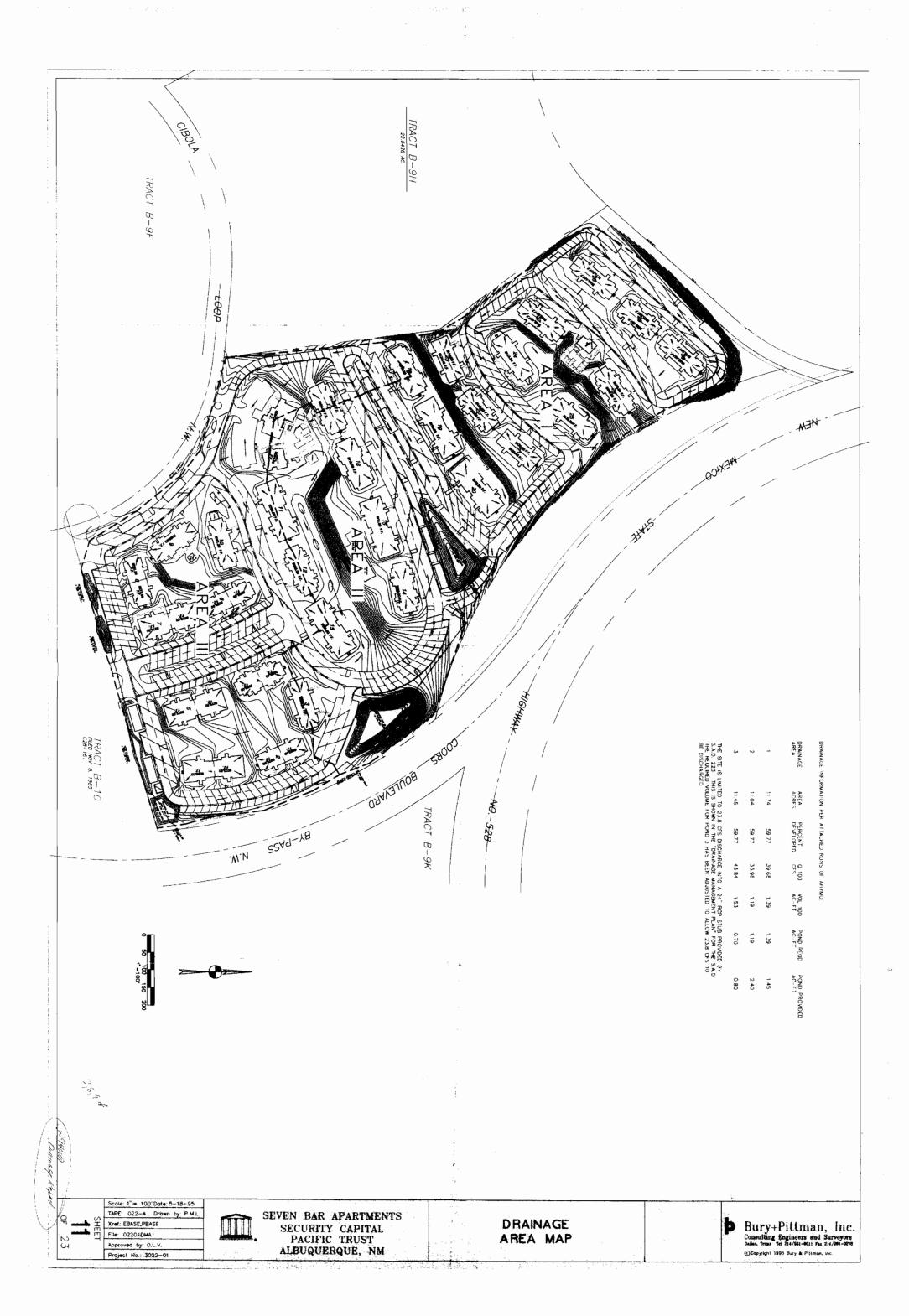
THE CITY OF ALBUQUERQUE

By:				
Its:				
Approved:				
Ву:				
Its:				
STATE OF NEW MEXICO)) ss. COUNTY OF BERNALILLO)				
The foregoing instrument was ack				
2009 by		01	the	City of
	Notary Publ	lic		
My Commission Expires:				

TITAN 528, LLC	
By:Ben F. Spencer, Manager	_
STATE OF NEW MEXICO COUNTY OF BERNALILLO) ss.
The foregoing instrument wa	as acknowledged before me thisth day of December, r of Titan 528, LLC, a New Mexico limited liability
	Notary Public
My Commission Expires:	

EXW. 617 "4" - DUTE 7 TACT 8-94-18-1 (City)





		POND VOLUME	ME		-	CULVERT VOLUME	LUME	TOTAL VOLUME	-UME
					19-FT S	SPAN, L=120'	19-FT SPAN, L=120' (TRIPLE 40-FT)	POND+CULVERT	VERT
ELEVATION	AREA	ELEVATION AREA INCREMENT CUM VOL	CUM VOL	COM VOL	AREA	AREA INCREMENT CUM VOL	CUM VOL	CUM VOL CUM VOL	COM VOL
	SF	VOL, CF	SF	AC-FT	SF	VOL, CF	CF	CF	AC-FT
						120			
88	2120								
6	2434	2277	2277	0.052273	18.96	2275.79	2275.79	4552.788	4552.788 0.104518
91	2771		4880	0.112018	18.75	2250.30	4526.09	9405.588	9405.588 0.215923
92	3136	2954		0.179821	18.32	2198.46	6724.55	14557.548	0.334195
93			_	0.25621	17.65	2118.31	8842.86	20003.360	0.459214
96	3925			0.341655	16.72	2006.44	10849.30	25731.796	0.590721
95		4138	19021	0.436651	15.48	1857.08	12706.38	31726.880	0.728349
96	4838			0.542126	13.83	1660.04	14366.42	37981.424	0.871934
97	5345	5092	İ	0.659011	11.62	1394.83	15761.26	44467.756	1.020839
86	5870		34314	0.787741	8.382	1005.84	16767.10	51081.096	1.172661
66	6400	6135	40449	0.928581	2.039	244.62	17011.72	57460.716	1.319117
100			47260	1.084929			17011.72	64271.216	1.475464
101	7751		54746	1.256784			17011.72	71757.216	1.647319

VOLUME CALCULATIONS

POND 4

Volume = from irregular pond & pipe culvert

ACTUAL	DEPTH	VOLUME	Q
ELEV.	(FT)	(AC-FT)	(CFS)
89	0	0	0.0000
90.00	1.00	0.1045	2.5736
91.00	2.00	0.2159	4.4212
92.00	3.00	0.3342	5.6983
93.00	4.00	0.4592	6.7375
94.00	5.00	0.5907	7.6366
95.00	6.00	0.7283	8.4405
96.00	7.00	0.8719	9.1742
97.00	8.00	1.0208	9.8534
98.00	9.00	1.1727	10.4887
99.00	10.00	1.3191	11.0877
100.00	11.00	1.4755	11.6559
101.00	12.00	1.6473	12.1977

Orifice Equation

Q = CA SQRT(2gH)

C = 0.6

Diameter (in) 11.7 (RESTRICTOR PLATE

Area (ft 2)= 0.747 g = 32.2

H (Ft) = Depth of water above center of orifice

Q(CFS)=Flow

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SHAPE CONSTANT, N = 7.106420
                                                                                                                                                                                                                                                                                                                                                                                                                                                                         COMPUTED 24-HOUR RAINFALL DISTRIBUTION BASED ON NOAA ATLAS 2 - PEAK AT 1.40 HR.
                                             USER NO. = AHYMO-S-9702d3TierraW-AH
- Version: 1997.02d
                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                  24.000000 HOURS
                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                          .545000
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                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                  PER A=4.4 PER B=19.8 PER C=19.8 PER D=56.00
TP=-0.1333 HR MASS RAINFALL=-1
                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                          K/TP RATIO =
                                                                                                                                                                                                                 100-YEAR, 24-HR STORM (PONDING CALCULATIONS)
                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                              ID=2 HYD NO=BAS9H AREA=0.0293 SQ MI
                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                  END TIME =
                                                                                                                                                                                                                                                                                                                                                                                                       Z
                                                                                                                                                                                                                                                                                                                                                                                                 RAIN ONE=1.87 IN RAIN SIX=2.20
RAIN DAY=2.66 IN DT=0.05 HR
                                                                                                                                                                                                                                                                                                                                                                                TYPE=-2 RAIN QUARTER=0.0 IN
                     RUN DATE (MON/DAY/YR) = 11/12/2009
                                             START TIME (HR:MIN:SEC) = 16:20:50
                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                        .133300HR
                                                                                                                                                                    DETENTION POND
                                                                  INPUT FILE = C:\2653pndl.txt
                                                                                                                                          TRACT B-9H-1B
                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                  .050000 HOURS
                                                                                                                                                                                                                                                                                                                                 ID=1 BULK=1.05
                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                            TP =
 AHYMO PROGRAM (AHYMO 97)
                                                                                                                                                                                                                                                              TIME=0.0
                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                            .072649HR
                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                  DT =
                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                               HYD
                                                                                                                                                                                                                                                                                                                                 SEDIMENT BULK
                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                               COMPUTE NM
                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                 * BASIN 9H
                                                                                                                                                                                                                                                                                                                                                                                  RAINFALL
                                                                                                                                                                                                                                                              START
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Н

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RU

20 21 22 24 25 26

MAXIM

PRINT

* PEAK

HYDROGRAPH FROM AREA BAS9H

SHAPE CONSTANT, N = 3.836889

.922392

K/TP RATIO =

.133300HR

.122955HR TP =

1.001

UNIT VOLUME =

CFS

33,284

UNIT PEAK =

AREA

.44750 INCHES

IA =

.012892 SQ MI

P60 = 1.8700

.04000 INCHES PER HOUR

526.28

II M

.9989

UNIT VOLUME =

CFS

UNIT PEAK = 64.780

.016408 SQ MI

AREA =

S N

FINISH

INF =

.10000 INCHES

.050000

RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT =

B = 344.14 P60 = 1.8700 INF = 1.10300 INCHES PER HOUR

RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT =

BULKING FACTOR APPLIED TO HYDROGRAPH. FACTOR = 1.05000 AT PEAK FLOW.

ID=2 CODE=1

PRINT HYD