"INTEL" HYMO RUN

HYMO SUMMARY TABLE

INTEL PHASE 2, WATERSHED FULLY DEV. 1000 CFS POND

0	DESCRIPTION	HYDROGRAI	FROM PH ID	TO ID			DISCHARGE	CURVE NO	TIME TO	CFS PER
		LABEL	NO	NO	SQ MI		CFS		PEAK HR	ACRE
	YDROGRAPH AT	31.0	0 1 1	- 4	0.40)90	294.5	NA	1.80	1.1
10	OUTED	31.00	0 1	_	0.40				1.80	
7-		31.Rt			0.40				1.80	
	YDROGRAPH AT		0 2 2 4	1	0.28			NA	1.70 1.75	1.8
	OMBINED FLOW		1	4						
	•	32.SM	1		0.69				1.75	
ч	YDROGRAPH AT	32.Rt	0 2		0.69			NA	1.75 1.55	3.6
	OMBINED FLOW		2& 4	1				1412	1.75	3.0
	OUTED		1	4						
		33.SM 33.Rt	1 4		0.74				1.75 1.80	
н	YDROGRAPH AT				0.74				1.70	1.9
С	OMBINED FLOW		2& 4	1					1.80	
R	OUTED	2.4 GW	1	4	0.00	200	905 0		1 00	
		34.SM 34.Rt	1 4				805.0 805.1		1.80 1.80	
	YDROGRAPH AT	35.0	0 2	_	0.26	550	330.5		1.65	1.9
	OMBINED FLOW	AT 35.SM	2& 4	1	1.14	150	1082.6		1.80	
R	OUTED	35.SM	1 1	4	1.14	450	1082.6		1.80	
		35.Rt	4		1.14		1030.2		1.85	
	YDROGRAPH AT				0.15	540	185.7	NA	1.70	1.8
	OMBINED FLOW	AT 36.SM	2 & 4	1 4	1.29	990	1176.2		1.85	
K	ROUTED	36.SM	1 1	-4	1.29	990	1176.2		1.85	
		36.Rt	4		1.29	990	1155.1		1.90	
	YDROGRAPH AT		2	_	0.10			NA	1.60	2.2
	COMBINED FLOW		2 & 4 50	50 49	1.40				1.85 2.20	
	YDROGRAPH AT						6.1	NA	1.50	4.3
Н	YDROGRAPH AT	S	2	_	0.00	070	13.0	NA	1.55	2.9
	OMBINED FLOW	AT S&T	1& 2	3	0.00)92	18.7		1.50	
Л	OUTED	S&T	3 3	23	0.00	192	18.7		1.50	
		T&S&Rt			0.00	092	5.9		1.80	
	YDROGRAPH AT	R	4	-	0.00	096	13.4	NA	1.65	2.1
	YDROGRAPH AT	В	5 5	_ 25	0.00)59	13.4	NA	1.50	3.5
1.	OUTED	В	5	23	0.00	059	13.4		1.50	
		B&Rt	25		0.00	059	4.5		1.70	
	YDROGRAPH AT	D2	6	26	0.00	38	8.6	NA	1.50	3.5
L	OUTED	D2	6 6	26	0.00	038	8.6		1.50	
		D2&Rt	26		0.00				1.70	

INTEL PHASE 2, WATERSHED FULLY DEV. 1000 CFS POND

0 DESCRIPTION	1	HYDROGRAPI LABEL	FROM H ID NO	TO ID NO	AREA SQ MI	DISCHARGE CFS	CURVE NO	TIME TO PEAK HR	CFS PER ACRE
HYDROGRAPH AT		5	7	_	0.0050		NA	1.50	2.2
COMBINED FLOW		ST&R	23& 4	29	0.0188			1.65	
COMBINED FLOW		&B	29&25	29	0.0247			1.65	
COMBINED FLOW		&D2	29&26	29	0.028			1.65	
COMBINED FLOW	AT	&5 P	29& 7 8	29	0.0339			1.65	1 7
HYDROGRAPH AT ROUTED		P	29	39	0.009	5 10.3	ΝA	1.70	1.7
ROUTED		& 5	29	39	0.033	30.3		1.65	
		&Rt	39		0.033			1.90	
COMBINED FLOW	ΑT	P&SM	39&8	39	0.0430			1.80	
HYDROGRAPH AT		A	9	_	0.0142		NA	1.50	3.6
RESERVOIR HYD		A	9	19	0.0142			1.65	
ROUTED 24"			19	20					
		A	19		0.0142			1.65	
		A&R24	20		0.0142	20.1		1.65	
ROUTED 36"			20	21					
		A&R24	20		0.0142			1.65	
HVDDOCDADH AM		A&R36 C1.1	21 20		0.0142		27.2	1.65	2 6
HYDROGRAPH AT COMBINED FLOW	λТ		20&21	20	0.019		NA	1.50 1.50	3.6
ROUTED 36"	ΛI	CIMMANSO	20 21	21	0.0232	49.0		1.50	
NOOTED 50		C1&A&R36	20		0.0292	49.6		1.50	
		C1&A&R60	21		0.0292			1.55	
HYDROGRAPH AT		C1.2	10	_	0.0050		NA	1.50	3.6
RECALL HYD		3A	20	_	0.0016			1.57	
HYDROGRAPH AT		D3	11	-	0.0030		NA	1.50	4.1
COMBINED FLOW	ΑT	3A&D3	11&20	11	0.0046	8.2		1.00	
ROUTED 24"		0	11	31					
		3A&D3	11		0.0046			1.00	
HVDDOCDADH AM		3AD3R24 1	31		0.0046		***	1.00	4 3
HYDROGRAPH AT COMBINED FLOW	λТ	1&R24	12 12&31	41	0.0068		NA	1.50	4.3
ROUTED 30"	ΑI	10124	41	42	0.0114	19.5		1.50	
NOOTED 30		1&R24	41	72	0.0114	19.5		1.50	
		1&D3&R30	42		0.0114			1.50	
COMBINED FLOW	ΑT	C12&R	10&42	43	0.0164			1.50	
ROUTED 30"			43	22					
		C12&R	43		0.0164			1.50	
		C12R30	22		0.0164			1.50	
HYDROGRAPH AT		2.1N	19	-	0.0030		NA	1.50	4.3
COMBINED FLOW ROUTED 36"	ΑT	21NR30	22&19	20	0.0194	38.9		1.50	
ROUTED 36"		21NR30	20 20	10	0.0194	38.9		1.50	
		2.1N&R30	10		0.0194			1.50	
HYDROGRAPH AT		FAB9	12	-	0.0085		NA	1.50	4.3
ROUTED 30"			12	11		20.0		3.00	
		FAB9	12		0.0085			1.50	
		FAB&R30	11		0.0085			1.50	
HYDROGRAPH AT		2.2N	19	-	0.0020	5.5	NA	1.50	4.3

INTEL PHASE 2, WATERSHED FULLY DEV. 1000 CFS POND

0	DESCRIPTION	ON	HYDROGRAP	FROM H ID	TO ID	AREA	DISCHARGE	CURVE NO	TIME TO	CFS PER
			LABEL	NO	NO	SQ MI	CFS		PEAK HR	ACRE
C	COMBINED FL	CA WO	22NFAB	11&19	11	0.010	5 29.1		1.50	
_		OW AT	22NFB36	11&10 22	22 20	0.029	9 67.5		1.50	
-		_	22NFB36	22		0.029			1.50	
			2NFAB&R4	20		0.029			1.50	
	YDROGRAPH .		2.3-NRT	12	-	0.004		NA	1.50	2.8
	COMBINED FLOOMBINED FLOOMBINED			20&12 20&21	20 23	0.0339			1.50 1.50	
		0" A.	R4ZRACI	23	24	0.003.	1 121.9		1.50	
•		•	R42RAC1	23		0.063	1 121.9		1.50	
			R&60	24		0.063			1.55	
	YDROGRAPH		2SOUTH	12	_	0.011	0 24.9	NA	1.50	3.5
F	ROUTED 3	6"	2 C O I I M I I	12 12	23	0 011	0 24 0		1 50	
			2SOUTH SOUTH&R3	23		0.011			1.50 1.50	
(OMBINED FL	CA WO		24&23	25	0.074			1.55	
	RECALL HYD	· · · · · · · · · · · · · · · · · · ·	PMP	12	_	0.010			1.57	
	COMBINED FLOUTED 7	OW AT	CLVRT&PS	25&12 24	24 26	0.084	149.0		1.53	
Ī		_	CLVRT&PS	24		0.084	4 149.0		1.53	
			CULVRT	26		0.084			1.55	
	IYDROGRAPH .		GNORTH	15	_	0.023		NA	1.50	4.0
	COMBINED FLOOR TOUTED 7	OW AT	CLVRT&GN	15&26 27	27 30	0.107	4 197.6		1.55	
			CLVRT&GN	27		0.107			1.55	
_			R72	30		0.107			1.55	
	IYDROGRAPH . ROUTED 5	AT 4"	GSOUTH	17 17	27	0.026	0 58.4	NA	1.50	3.5
r	COULED 3	4	GSOUTH	17	21	0.026	0 58.4		1.50	
			Rt54	27		0.026			1.50	
		CA WO		27&30	24	0.133			1.55	
F	ROUTED 9	0 "		24	27					
			GSO&Rt54	24		0.133			1.55	
	VDDOCDADU	5 m	Rt90 J	27		0.133			1.55	2 0
	IYDROGRAPH A ROUTED 4	2"	J	16 16	30	0.045	0 84.2	NA	1.65	2.9
•	.00125 4	-	J	16	30	0.045	0 84.2		1.65	
			Rt42	30		0.045			1.65	
			R42&R90	30&27	16	0.178	4 313.6		1.60	
F	ROUTED 9	0"	742-700	16	30					
			R42&R90 90.00	16 30		0.178			1.60	
F	YDROGRAPH A	ΔТ	GMID	36	_	0.178		NA	1.60 1.65	2.8
		4"	0.112	36	16	0.000	0 133.0	IAV	1.03	2.0
			GMID	36		0.086			1.65	
			Rt54	16		0.086	0 156.4		1.65	
	COMBINED FLO			16&30	36	0.264			1.60	
	IYDROGRAPH A ROUTED 9	AT 0"	TERMNUS	16 36	30	0.006	0 15.7	NA	1.50	4.0
1		•		30	50					

INTEL PHASE 2, WATERSHED FULLY DEV. 1000 CFS POND

0 DESCRIPTION	н	YDROGRAPH		TO ID	ARE		DISCHARGE	CURVE NO	TIME TO	CFS PER
		LABEL	NO	МО	SÇ Mi		CFS		PEAK HR	ACRE
		197 RtCUL	36 30			644			1.60 1.60	
COMBINED FLOW	ΑТ	COMBO	16&30	32		704			1.60	
COMBINED FLOW		COMBO	32&39	40	0.3	134	484.3		1.60	
RESERVOIR HYD		COMBO	40	40	0.3	3134	231.1		2.00	
ROUTED		СОМВО	40 40	39	0 3	3134	231.1		2.00	
		RtCOMBO	39			134			2.00	
COMBINED FLOW		37.2	49&39	1		144			2.15	
HYDROGRAPH AT		EX	2	_	0.0	031	7.8	NA	1.50	3.8
COMBINED FLOW	ΑT	ROADRRP	2& 1	1	1.7	176	1014.5		2.15	
ROUTED		ROADRRP	1	4	1 7	176	1014.5		2.15	
		7BAR&Rt	4			176			2.13	
HYDROGRAPH AT		38.00	2	_		.220		NA	1.65	2.3
COMBINED FLOW	ΑT	38&BAR	2& 4	2		396			2.20	
ROUTED		20.555	2	50	1 0		1050 0		2 20	
		38&BAR BLKCNFLU	2 50			396 396			2.20 2.20	
HYDROGRAPH AT		101.10	2	_		310		NA	1.55	1.3
ROUTED		101.10	2	14	•••	, , ,	2010	.,		1.5
		101.10	2			310			1.55	
		101.2	14			310			1.70	1 0
HYDROGRAPH AT COMBINED FLOW	λm	101.30 101.4	4 14& 4	15		.060 .370		NA	1.65 1.65	1.0
HYDROGRAPH AT	AT	101.4	3	15		. 3 / C		NA	1.60	1.1
ROUTED		101.50	3	16	•••	, , ,		.,	1.00	
		101.50	3			650			1.60	
		101.6	16			650			1.70	
HYDROGRAPH AT COMBINED FLOW	N CD	101.70 101.8	5 16& 5	_ 17)770 .420		NA	1.60	1.1
COMBINED FLOW		101.8	15&17	18		.420 2790			1.65 1.65	
ROUTED	***	101.5	18	19	0.2	.,,,	, 170.0		1.05	
		101.9	18			790			1.65	
		101.10	19			790			1.70	
HYDROGRAPH AT ROUTED		101.11	6 6	20	0.0	940	62.7	NA	1.60	1.0
KOUTED		101.11	6	20	0.0	940	62.7		1.60	
		101.12	20			940			1.65	
HYDROGRAPH AT		101.13	7	-		990	65.1	NA	1.65	1.0
COMBINED FLOW	ΑT	101.14	20& 7	21	0.1	.930	128.2		1.65	
ROUTED		101.14	21 21	22	0 1	.930	128.2		1.65	
		101.14	22			.930			1.65	
HYDROGRAPH AT		101.16	8		0.1	680	119.5	NA	1.60	1.1
COMBINED FLOW		101.17	19& 8	23		470			1.70	
COMBINED FLOW ROUTED	AT	101.18	23&22 24	24 25	0.6	400	396.8		1.65	
		101.18	24	23	0.6	400	396.8		1.65	

HYMO SUMMARY TABLE

INTEL PHASE 2, WATERSHED FULLY DEV. 1000 CFS POND

0 DESCRIPTION	HYDROGRAI	FROM PH ID	TO ID	AREA	DISCHARGE	CURVE NO	TIME TO	CFS PER
	LABEL	NO	NO	SQ MI	CFS		PEAK HR	ACRE
	101.19			0.6400			1.70	
HYDROGRAPH AT			26	0.1120		NA	1.65	0.9
COMBINED FLOW HYDROGRAPH AT			26	0.7520		NA	1.70 1.65	1.0
ROUTED	101.22	9	27	0.1590		NA	1.65	1.0
	101.23	27		0.1590			1.75	
ROUTED	202.20	26	28	0.255	0,11		2.70	
	101.21	26		0.7520			1.70	
	101.24	28		0.7520			1.75	
HYDROGRAPH AT			20	0.1540		NA	1.70	0.8
COMBINED FLOW HYDROGRAPH AT			29 -	0.9060		NA	1.75 1.65	0.9
COMBINED FLOW			30	0.2220		IVA	1.75	0.9
COMBINED FLOW		30&29	31	1.1280			1.75	
ROUTED		31	32	_,,				
	101.29	31		1.1280			1.75	
	101.30	32		1.1280			1.85	
HYDROGRAPH AT			-	0.1600		NA	1.75	0.8
COMBINED FLOW HYDROGRAPH AT		32&13	33	1.2880		NA	1.85 1.60	1.4
ROUTED	102.10		7	0.1140	100.1	11/17	1.00	1.4
1.00122	102.10) 2	•	0.1140	108.1		1.60	
	102.2	7		0.1140			1.65	
HYDROGRAPH AT ROUTED		5	- 8	0.0200	27.2	NA	1.50	2.1
	102.30	5		0.0200			1.50	
	102.4	8		0.0200			1.65	
HYDROGRAPH AT) 3 7& 3	_	0.1130		NA	1.60	1.5
COMBINED FLOW ROUTED		9	9 10	0.2270			1.60	
	102.5	9		0.2270			1.60	
HYDROGRAPH AT	102.6 102.70	10) 4	_	0.2270		NA	1.65 1.55	1.8
COMBINED FLOW		10& 4	11	0.287		IVA	1.60	1.0
ROUTED	102.8	11 11	12	0.2870	278.6		1 60	
	102.9	12		0.2870			1.60 1.65	
HYDROGRAPH AT				0.1730		NA	1.65	1.3
COMBINED FLOW		8& 6	13	0.1930			1.65	
COMBINED FLOW		13&12	14	0.4800			1.65	
ROUTED	4 . 4	33	15					
	AP101	33		1.2880			1.85	
COMBINED FLOW	102.13 AT AP102	15 14&15	16	1.2880			1.85 1.80	
HYDROGRAPH AT ROUTED) 2	- 7	0.139		NA	1.55	2.3
VOOLED	103.10	2	/	0.1390	208.0		1.55	
	103.2	7		0.1390			1.60	
1					_ ,			

INTEL PHASE 2, WATERSHED FULLY DEV. 1000 CFS POND

0 і	DESCRIPTION	H	YDROGRAPI LABEL	FROM H ID NO	TO ID NO	AREA SQ MI	DISCHARGE	CURVE NO	TIME TO PEAK HR	CFS PER ACRE
HYI	DROGRAPH AT		103.30	4	_	0.099		NA	1.60	2.3
COI	MBINED FLOW			7& 4 16	8 9	0.238			1.60	
			AP102 103.5	16 9		1.768 1.768			1.80 1.80	
	MBINED FLOW	ΑT	103.6	8 & 9	10	2.006			1.75	
ROU	JTED		103.6	10 10	42	2.006	0 1074.3		1.75	
нуі	DROGRAPH AT		103.7 103.80	42 3	_	2.006 0.259		NA	1.80 1.60	2.1
	JTED			3 3 3	12			1421		2.1
			103.80 103.9	12		0.259 0.259			1.60 1.65	
	OROGRAPH AT	ΑТ	103.10 AP1	6 12& 6	- 41	0.064		NA	1.55 1.60	2.6
HYI	DROGRAPH AT	•••	104.10	2	-	0.137	0 91.1	NA	1.80	1.0
	DROGRAPH AT		104.20	3 2	- 7	0.052		NA	1.65	1.4
			104.10 104.3	2 7		0.137			1.80 1.85	
	OROGRAPH AT	λT	104.40 104.5	4 7& 4	_ 8	0.121	0 114.9	NA	1.65	1.4
	JTED FLOW	N.		8	9					
			104.5 104.6	8 9 3		0.258 0.258			1.75 1.75	
ROU	JTED		104.20	3 3	10	0.052	0 49.3		1.65	
DOI	ıman		104.7	10	11	0.052			1.70	
ROC	JTED		104.7	10 10	11	0.052			1.70	
COI	MBINED FLOW	ΑT	104.8 AP109	11 9&11	12	0.052			1.75 1.75	
ROU	JTED		AP109	12 12	13	0.310			1.75	
			104.9	13		0.310	0 229.2		1.80	
CON	OROGRAPH AT	ΑT	104.10 AP103	6 13& 6	33	0.133		NA	1.60 1.75	1.7
	OROGRAPH AT	ΑТ	104.11 AP110	5 42& 5	42	0.147		NA	1.70 1.75	1.3
	JTED		AP110	42	14					
			103.11	14		2.153 2.153	0 1184.7		1.75 1.75	
	OROGRAPH AT	ΑT	103.12 AP2	5 14& 5	42	0.099		NA	1.65 1.75	2.0
	OROGRAPH AT		106.10 106.20	2	- -	0.009	0 13.7	NA NA	1.55 1.65	2.3
	JTED		106.10	2 2	13	0.009		.12.1		±• /
1			106.3	13		0.009			1.55 1.75	

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INTEL PHASE 2, WATERSHED FULLY DEV. 1000 CFS POND

0 DESCRIPTION	HYDROGRAPH LABEL	FROM I ID NO	TO ID NO	AREA SQ MI	DISCHARGE CFS	CURVE NO	TIME TO PEAK HR	CFS PER ACRE
COMBINED FLOW HYDROGRAPH AT ROUTED	AT 106.4 106.50	13& 3 4 4	14 - 15	0.1200 0.0880		NA	1.65 1.65	1.9
HYDROGRAPH AT COMBINED FLOW ROUTED	106.50 106.6 106.70 AT 106.8	4 15 6 15& 6 16	- 16 17	0.0880 0.0880 0.0760 0.1640	102.9	NA	1.65 1.70 1.70 1.70	1.6
HYDROGRAPH AT COMBINED FLOW ROUTED	106.8 106.9 106.10 AT 106.11	16 17 9 17& 9	- 18 19	0.1640 0.1640 0.0890 0.2530	178.2 83.2	NA	1.70 1.75 1.75 1.75	1.4
HYDROGRAPH AT COMBINED FLOW	106.11 106.13 106.14 AT 106.15	18 19 11 19&11	_ 20	0.2530 0.2530 0.1070 0.3600	239.1	NA	1.75 1.85 1.70 1.80	1.5
ROUTED HYDROGRAPH AT COMBINED FLOW		20 20 21 12 21&12	21	0.3600 0.3600 0.1350 0.4950	318.1 129.0	NA	1.80 1.90 1.75 1.85	1.4
ROUTED HYDROGRAPH AT COMBINED FLOW	106.4 106.19 106.20 AT 106.21	14 14 22 5 22& 5	22	0.1200 0.1200 0.0370 0.1570	129.4	NA	1.65 1.70 1.65 1.70	1.7
ROUTED HYDROGRAPH AT COMBINED FLOW	106.21 106.22 106.23 AT 106.24	23 23 24 8 24&8	24	0.1570 0.1570 0.0360 0.1930	167.5 50.4	NA	1.70 1.75 1.60 1.70	2.1
ROUTED HYDROGRAPH AT ROUTED	106.24 106.25 106.26	25 25 26 7 7	26 _ 27	0.1930 0.1930 0.0620	201.7	NA	1.70 1.75 1.60	2.1
COMBINED FLOW		7 27 26&27 28	28 29	0.0620 0.0620 0.2550	85.0 272.2		1.60 1.60 1.70	
HYDROGRAPH AT COMBINED FLOW ROUTED		28 29 10 29&10 30	30 31	0.2550 0.2550 0.1120 0.3670	272.6 144.4	NA	1.70 1.70 1.60 1.70	2.0
COMBINED FLOW	106.31 106.32 AT 106.33	30 31 31&43	43	0.3670 0.3670 0.8620	359.8		1.70 1.75 1.85	

INTEL PHASE 2, WATERSHED FULLY DEV. 1000 CFS POND

0 DESCRIPTION	HYDROGRAPI	FROM I ID	TO ID	AREA	DISCHARGE	CURVE NO	TIME TO	CFS PER
	LABEL	NO	NO	SQ MI	CFS		PEAK HR	ACRE
HYDROGRAPH AT ROUTED	107.10	2 2 2	<u> </u>	0.1260	103.3	NA	1.65	1.2
	107.10 107.2	5		0.1260			1.65 1.85	
HYDROGRAPH AT COMBINED FLOW		3 5& 3	- 6	0.2170	118.6	NA	2.30 2.05	0.4
COMBINED FLOW HYDROGRAPH AT	105.00	43& 6 35	43	1.2050	40.6	NA	1.85	1.3
HYDROGRAPH AT ROUTED	105.10	2 2	<u> </u>	0.1350		NA	1.65	1.5
HYDROGRAPH AT	105.10 105.2 105.30	2 5 4	_	0.1350 0.1350 0.0650	133.5	NA	1.65 1.70 1.65	1.6
COMBINED FLOW HYDROGRAPH AT		5& 4 38	37	0.2000	199.9	NA NA	1.65	1.6
HYDROGRAPH AT ROUTED	201.10	2 2 2	8	0.1510		NA	1.60	0.9
	201.10 201.2	8		0.1510	84.6		1.60	
HYDROGRAPH AT COMBINED FLOW ROUTED	201.30 AT 201.4	3 8 & 3 9	9 10	0.2160 0.3670		NA	1.65 1.65	0.8
	201.4 201.5	9 10		0.3670	197.4		1.65 1.65	
HYDROGRAPH AT COMBINED FLOW ROUTED	201.60 AT 201.7	10& 4 11	11 12	0.1520 0.5190		NA	1.65 1.65	0.8
	201.7 201.8	11 12		0.5190 0.5190	268.2		1.65 1.70	
HYDROGRAPH AT COMBINED FLOW ROUTED	201.90 AT 201.10	5 12& 5 13	13 14	0.1950 0.7140		NA	1.65 1.70	0.8
KOOTED	201.10 201.11	13 14	14	0.7140			1.70 1.70	
HYDROGRAPH AT COMBINED FLOW ROUTED	201.12	6 14& 6 15	15 16	0.0120 0.7260	8.7	NA	1.55 1.70	1.1
ROUTED	201.13 201.14	15 16	10	0.7260			1.70 1.80	
HYDROGRAPH AT COMBINED FLOW		7 16& 7	45	0.0550	357.1	NA	1.65 1.75	0.6
HYDROGRAPH AT ROUTED	202.10	2 2 2	12	0.0560		NA	1.55	0.9
HYDROGRAPH AT	202.10 202.2 202.30	12 3	_	0.0560	32.3	NA	1.55 1.65 1.60	0.9
ROUTED	202.30 202.4	3 3 13	13	0.0410			1.60 1.65	

INTEL PHASE 2, WATERSHED FULLY DEV. 1000 CFS POND

0 DESCRIPTION	HYDROGRAPI LABEL	FROM H ID NO	TO ID NO	AREA SQ MI	DISCHARGE CFS	CURVE NO	TIME TO PEAK HR	CFS PER ACRE
COMBINED FLOW ROUTED	AT AP210	12&13 14	14 15	0.0970	56.4		1.65	
HYDROGRAPH AT	AP210 202.5 202.60	14 15 6	_	0.0970 0.0970 0.0750	51.6 0 42.0	NA	1.65 1.70 1.60	0.8
COMBINED FLOW ROUTED		15& 6 16 16	16 17	0.1720			1.65	
ROUTED	202.7 202.8	16 17 41	18	0.1720 0.1720			1.65 1.75	
	AP1 202.9 202.10	41 18 4		0.3230 0.3230 0.0970	432.6	NA	1.60 1.65 1.65	0.8
HYDROGRAPH AT COMBINED FLOW ROUTED	AT 202.11	18& 4 19	19 20	0.4200	482.5	NA	1.65	0.0
DOMED	202.11 202.12	19 20 42	21	0.4200 0.4200			1.65 1.65	
ROUTED	AP2 202.13	42 42 21	21	2.2520 2.2520			1.75 1.80	
COMBINED FLOW ROUTED		20&21 22	22 23	2.6720			1.75	
COMBINED FLOW	AP211 202.14 AT AP212	22 23 17&23	24	2.6720 2.6720 2.8440	1713.1		1.75 1.75 1.75	
ROUTED	AP212 202.15	24 24 25	25	2.8440			1.75 1.75	
HYDROGRAPH AT COMBINED FLOW HYDROGRAPH AT	202.16 AT 202.17 202.18	5 25& 5 7	26 -	0.0940 2.9380 0.1370	0 1832.8	NA NA	1.65 1.75 1.60	0.7
ROUTED	202.18	7 7	27	0.1370	0 79.2		1.60	0.9
HYDROGRAPH AT COMBINED FLOW ROUTED	202.19 202.20 AT 202.21	27 9 27& 9 28	- 28 29	0.1370 0.0940 0.2310	0 41.3	NΑ	1.70 1.65 1.70	0.6
HYDROGRAPH AT	202.21 202.22 202.23	28 29 8	_	0.2310 0.2310 0.0770	0 113.8	NΑ	1.70 1.70 1.55	1.0
ROUTED	202.23 202.24	8 8 30	30	0.0770			1.55 1.70	
ROUTED	202.17	26 26 31	31	2.9380	0 1775.2		1.75	
COMBINED FLOW COMBINED FLOW HYDROGRAPH AT		29&30 32&31 10	32 49 -	0.3080 3.2460 0.1330	0 1908.5	NA	1.70 1.80 1.70	0.6

HYMO SUMMARY TABLE

INTEL PHASE 2, WATERSHED FULLY DEV. 1000 CFS POND

0	DESCRIPTION	F	IYD R OGRAPH	FROM I ID	TO ID	AR	EA	DISCHARGE	CURVE NO	TIME TO	CFS PER
			LABEL	МО	NO	S M	Q I	CFS		PEAK HR	ACRE
	COMBINED FLOW	ΑT	AP213	49&10 48	48 47	3.	3790	1959.8		1.80	
	1.00122		AP213	48		3.	3790	1959.8		1.80	
			202.28	47			3790			1.80	
	COMBINED FLOW ROUTED	ΑT	202.29	45&47 33	46 4	4.	1600	2323.2		1.80	
			AP103	33			4430			1.75	
			203.1	4			4430			1.80	
	HYDROGRAPH AT		203.20		_		1540		NA	1.65	0.8
	COMBINED FLOW ROUTED	AT	202.4	4& 2 5	5 6		5970			1.80	
			202.4 203.2	5 6			5970 5970			1.80 1.85	
	HYDROGRAPH AT		203.2	3	_		0400		NA	1.70	0.7
	COMBINED FLOW			6& 3	7		6370			1.85	0.7
	COMBINED FLOW ROUTED		AP201	46& 7	31 6		7970			1.80	
			AP201	31	•	4.	7970	2694.0		1.80	
			204.1	6			7970			1.80	
	HYDROGRAPH AT		204.20	2	=		0840		NA	1.70	0.6
	COMBINED FLOW	ΑT	202.4	6&_2	7	4.	881	2720.4		1.80	
	ROUTED		202.4	7 7	8	1	881	2720.4		1.80	
			203.2	8			881			1.80	
	HYDROGRAPH AT			3	-		121			1.75	0.5
	COMBINED FLOW	ΑT	202.4	8& 3	9	5.	0020	0 2752.0		1.80	
	ROUTED		202 4	9	10	_				1 00	
			202.4	9			0020			1.80	
	HYDROGRAPH AT		203.2 204.20	10 4	_		094			1.85 1.65	0.6
	COMBINED FLOW	ΑТ	202.4	10& 4	11		096		IVA	1.85	0.0
	ROUTED	•••	202.4	11	12	٠.	0,50	2755.4		1.05	
			202.4	11		5.	0960	0 2795.4		1.85	
			203.2	12			0960			1.85	
	HYDROGRAPH AT		204.20	5	- 1 2		089		NA	1.75	0.5
	COMBINED FLOW HYDROGRAPH AT	AT	202.4 206.10	12& 5 2	13		1850 2810		NA	1.85 1.60	0.9
	ROUTED		200.10	2	4	υ.	201	1/2.1	IVA	1.00	0.9
			206.10	2	•	0.	281	0 172.1		1.60	
			206.2	4			281			1.65	
	HYDROGRAPH AT		206.40	3	-		1640		NA	1.70	0.6
	COMBINED FLOW	AΤ	206.5	4&_3	5	0.	445	0 236.6		1.65	
	ROUTED		206.5	5 5	6	0	115	0 226 6		1 (5	
			206.5	6			4450			1.65 1.70	
	HYDROGRAPH AT		206.40	4	-		079		NA	1.70	0.6
	COMBINED FLOW		206.5	6& 4	7	0.	524	0 251.6		1.70	2.0
	COMBINED FLOW	ΑT	206.5	13& 7	36		709			1.85	
1	ROUTED			36	22						

HYMO SUMMARY TABLE

INTEL PHASE 2, WATERSHED FULLY DEV. 1000 CFS POND

0 DESCRIPTION	HYDROGRAPI	FROM I ID	TO ID	AREA	DISCHARGE	CURVE NO	TIME TO	CFS PER
	LABEL	NO	NO	SQ MI	CFS	110	PEAK HR	ACRE
	206.5 206.2	36 22		5.7090 5.7090			1.85 1.85	
HYDROGRAPH AT	207.10	2	- 5	0.2390		NA	1.60	1.0
ROOTED	207.10	2 2 5	3	0.2390			1.60	
HYDROGRAPH AT	207.2 207.30	3	_	0.2390		NA	1.70 1.70	0.6
COMBINED FLOW ROUTED	AT 207.4	5& 3 6	6 7	0.4870	230.2		1.70	
1.00122	207.4	6 7	·	0.4870			1.70 1.75	
HYDROGRAPH AT	207.40	4	_	0.0490	26.0		1.60	0.8
COMBINED FLOW ROUTED	AT 207.5	7& 4 8	8 9	0.5360	246.6		1.70	
	207.5 206.2	8 9		0.5360			1.70 1.75	
HYDROGRAPH AT	205.10	2	-	0.0860	37.5	NA	1.70	0.6
COMBINED FLOW COMBINED FLOW ROUTED	AT 207.5 AT 207.5	9& 2 22&10 11	10 11 12	0.6220 6.3310			1.75 1.85	
3.00122	207.5 206.2	11 12		6.3310			1.85 1.85	
HYDROGRAPH AT	207.40	5	-	0.0170	10.2	NA	1.60	0.9
COMBINED FLOW ROUTED	AT 207.5	12& 5 13	13 14	6.3480	3340.3		1.85	
	207.5 206.2	13 14		6.3480			1.85 1.85	
HYDROGRAPH AT	205.10	3	. -	0.0230	11.4	NA	1.65	0.7
COMBINED FLOW HYDROGRAPH AT	AT 207.5 212.10	14& 3	15	6.3710 0.1380		NA	1.85 1.65	0.7
ROUTED	212.10	2 2	4	0.1380			1.65	
HYDROGRAPH AT	207.2 212.30	4 3	_	0.1380		NA	1.70 1.55	1.1
COMBINED FLOW	AT 207.4	4& 3	5	0.1940	92.5	1121	1.65	
COMBINED FLOW ROUTED	AT 207.4	15& 5 37	27 6	6.5650	3422.8		1.85	
	AP107 207.2	37 6		0.2000			1.65 1.80	
HYDROGRAPH AT	208.20	2	=	0.1870	75.5	NA	1.70	0.6
COMBINED FLOW ROUTED		6& 2 7	7 8	0.3870			1.75	
	208.4 208.3	7 8		0.3870			1.75 1.75	
HYDROGRAPH AT COMBINED FLOW	208.30	3 8& 3	– 9	0.1670	58.3	NA	1.75	0.5
ROUTED		9	10				1.75	
	208.4 208.2	9 10		0.5540 0.5540			1.75 1.85	

HYMO SUMMARY TABLE

INTEL PHASE 2, WATERSHED FULLY DEV. 1000 CFS POND

0 DESCRIPTION	HYDROGRAPH LABEL	FROM I ID NO	TO ID NO	AREA SQ MI	DISCHARGE CFS	CURVE NO	TIME TO PEAK HR	CFS PER ACRE
HYDROGRAPH AT COMBINED FLOW ROUTED		4 10& 4 11	- 11 12	0.1510 0.7050		NA	1.75 1.85	0.5
HYDROGRAPH AT COMBINED FLOW	208.5 206.2 208.40 AT 208.5	11 12 5 12& 5	_ 13	0.7050 0.7050 0.0490 0.7540	335.0	NA	1.85 1.85 1.60 1.85	0.9
ROUTED ROUTED	108.00 208.2	38 38 6 43	6 7	0.1100			1.65 1.70	
COMBINED FLOW	AP3 208.2 AT 209.4	43 7 6& 7	8	1.2050 1.2050 1.3150	889.2		1.85 1.85 1.85	
ROUTED ROUTED	209.4 208.2	8 8 9 35	9	1.3150 1.3150			1.85 1.90	
COMBINED FLOW	105.00 208.2 AT 209.4	35 10 9&10 11	11 12	0.0470 0.0470 1.3620	29.9		1.65 1.80 1.90	
HYDROGRAPH AT	209.4 208.2 209.20 AT 209.4	11 12 2 12& 2	_ 14	1.3620 1.3620 0.2260 1.5880	997.8 95.0	NA	1.90 1.90 1.75 1.90	0.6
ROUTED HYDROGRAPH AT	209.4 209.3 209.30	14 14 15 3	15 -	1.5880 1.5880 0.1700	1063.7	NA	1.90 1.90 1.70	0.6
COMBINED FLOW ROUTED	209.4	15& 3 16 16	16 17	1.7580	1120.7 1120.7		1.90	
HYDROGRAPH AT COMBINED FLOW ROUTED	209.2 209.40 AT 209.5	17 4 17& 4 18	- 18 19	1.7580 0.1280 1.8860	62.0	NA	1.95 1.65 1.95	0.7
HYDROGRAPH AT COMBINED FLOW COMBINED FLOW HYDROGRAPH AT	AT 209.5	18 19 5 19& 5 13&20 2	- 20 24 -	1.8860 1.8860 0.0100 1.8960 2.6500 0.0950	1127.3 8.4 1128.5 1461.2	NA AN	1.95 1.95 1.50 1.95 1.95	1.3
ROUTED HYDROGRAPH AT HYDROGRAPH AT COMBINED FLOW	210.20 210.3 210.30 210.30 AT 210.4	2 2 8 3 5 8 3	8 - - 9	0.0950 0.0950 0.0660 0.0380 0.1610	37.1 37.1 43.6 25.1	NA NA	1.80 1.85 1.60 1.60	1.0

HYMO SUMMARY TABLE

INTEL PHASE 2, WATERSHED FULLY DEV. 1000 CFS POND

0 DESCRIPTION	HYDROGRAPI LABEL	FROM H ID NO	TO ID NO	AREA SQ MI	DISCHARGE CFS	CURVE NO	TIME TO PEAK HR	CFS PER ACRE
COMBINED FLOW	AT 210.4	9& 5 10	10 11	0.1990	90.4		1.65	
ROUTED	210.4 210.3	10 10 11	11	0.1990	77.6		1.65 1.80	
HYDROGRAPH AT COMBINED FLOW		11& 4	12 13	0.1910 0.3900	117.4	NA	1.65 1.70	0.9
ROUTED -	210.4 210.3	12 12 13	13	0.3900			1.70 1.70	
ROUTED	209.5	24 24	14	2.6500			1.95	
COMBINED FLOW	210.3 AT 210.4	14 14&13	15	2.6500 3.0400	1573.4		2.00	0.0
HYDROGRAPH AT COMBINED FLOW ROUTED	210.30 AT 210.4	6 15& 6 27	26 20	0.0770 3.1170		NA	1.65 1.95	0.9
	207.4 210.3	27 20		6.5650 6.5650	3422.8 3404.8		1.85 1.85	
ROUTED	210.4 210.3	26 26 21	21	3.1170 3.1170			1.95	
COMBINED FLOW HYDROGRAPH AT		20&21	22	9.6820	4797.1	NA	1.90 1.55	1.3
COMBINED FLOW HYDROGRAPH AT	AT 210.4 213.10	22& 7 2	23	0.0460 9.7280 0.1110	4807.0 64.4	NA	1.90 1.65	0.9
ROUTED	213.10 210.3	2 2 4	4	0.1110			1.65 1.65	
HYDROGRAPH AT COMBINED FLOW		3 4& 3	_ 28	0.0200 0.1310		NA	1.55 1.60	1.2
ROUTED	210.4 210.3	28 28 24	24	0.1310			1.60 1.65	
COMBINED FLOW PUNCHED HYD RESERVOIR HYD			25 - 40		4851.8 4851.8		1.90 1.90 2.35	
HYDROGRAPH AT COMBINED FLOW	210.30	8 40&8	40	0.0270 9.8860	67.1	NA	1.50	3.8
ROUTED	AP301 211.5	40 40 3	3	9.8860			2.35	
HYDROGRAPH AT HYDROGRAPH AT COMBINED FLOW	413.00 414.00 AT 414.10	5 2 2& 5	- - 2	0.0280 0.0720 0.1000	54.2 139.3 193.5	NA NA	1.50 1.50 1.50	3.0 3.0
COMBINED FLOW ROUTED	AT 414.20 414.20	3& 2 1 1	1 3	9.9860			2.35	
HYDROGRAPH AT	211.7 213.40	3 4	_	9.9860 0.1530	2453.7	NA	2.35	2.2

INTEL PHASE 2, WATERSHED FULLY DEV. 1000 CFS POND

0 DESCRIPTION	HYDROGRAPH LABEL	FROM ID NO	TO ID NO	AREA SQ MI	DISCHARGE CFS	CURVE NO	TIME TO PEAK HR	CFS PER ACRE
COMBINED FLOW HYDROGRAPH AT HYDROGRAPH AT COMBINED FLOW	410.00 411.00	4& 3 2 5 5& 2	4 - - 5	10.1390 0.0740 0.0030 0.0770	98.3 7.6	NA NA	2.35 1.60 1.50 1.60	2.0 3.9
ROUTED 54" HYDROGRAPH AT COMBINED FLOW	480.00 411.50 412.00 AT 412.1	5 5 2 6 2& 6	2 - 6	0.0770 0.0770 0.0240 0.1010	0 100.2 0 45.0	ΝA	1.60 1.65 1.50 1.60	2.9
ROUTED 42"	412.1 412.2 412.2	6 6 5 5 5 6	5 6	0.1010 0.1010	131.9		1.60 1.60	
ROUTED 36"	412.3 412.3 412.4	6 6 5 5	5	0.1010 0.1010 0.1010	132.0 132.0		1.60 1.60 1.60	
ROUTED 42" HYDROGRAPH AT HYDROGRAPH AT	412.4 412.5 415.00 416.00	5 5 6 5 2	6 _ _	0.1010 0.1010 0.0330 0.0180	130.7 61.8	NA NA	1.60 1.60 1.50 1.50	2.9 3.7
COMBINED FLOW HYDROGRAPH AT COMBINED FLOW COMBINED FLOW COMBINED FLOW	AT 416.10 417.00 AT 417.1 AT L11	2 & 5 3 3 & 2 3 & 6 4 & 3	2 - 3 3 3	0.0510 0.0520 0.1030 0.2040 10.3430	105.5 97.4 203.0 300.8	NA	1.50 1.50 1.50 1.55 2.30	2.9
COMBINED FLOW ROUTED	100 BLACK	3&50 3 3 4	3 4	12.1826 12.1826 12.1826	3555.7 3555.7 3548.1		2.20 2.20 2.25	
HYDROGRAPH AT HYDROGRAPH AT COMBINED FLOW ROUTED 42"	39.00 40.00 AT 38.1 38.1	5 6 5& 6 7 7	- 7 8	0.0020 0.0213 0.0234	53.1 58.6	NA NA	1.50 1.50 1.50	4.2 3.8
COMBINED FLOW	RCP1 AT 110	8 8 & 4	9	0.0234 12.2059	4 54.9		1.50	



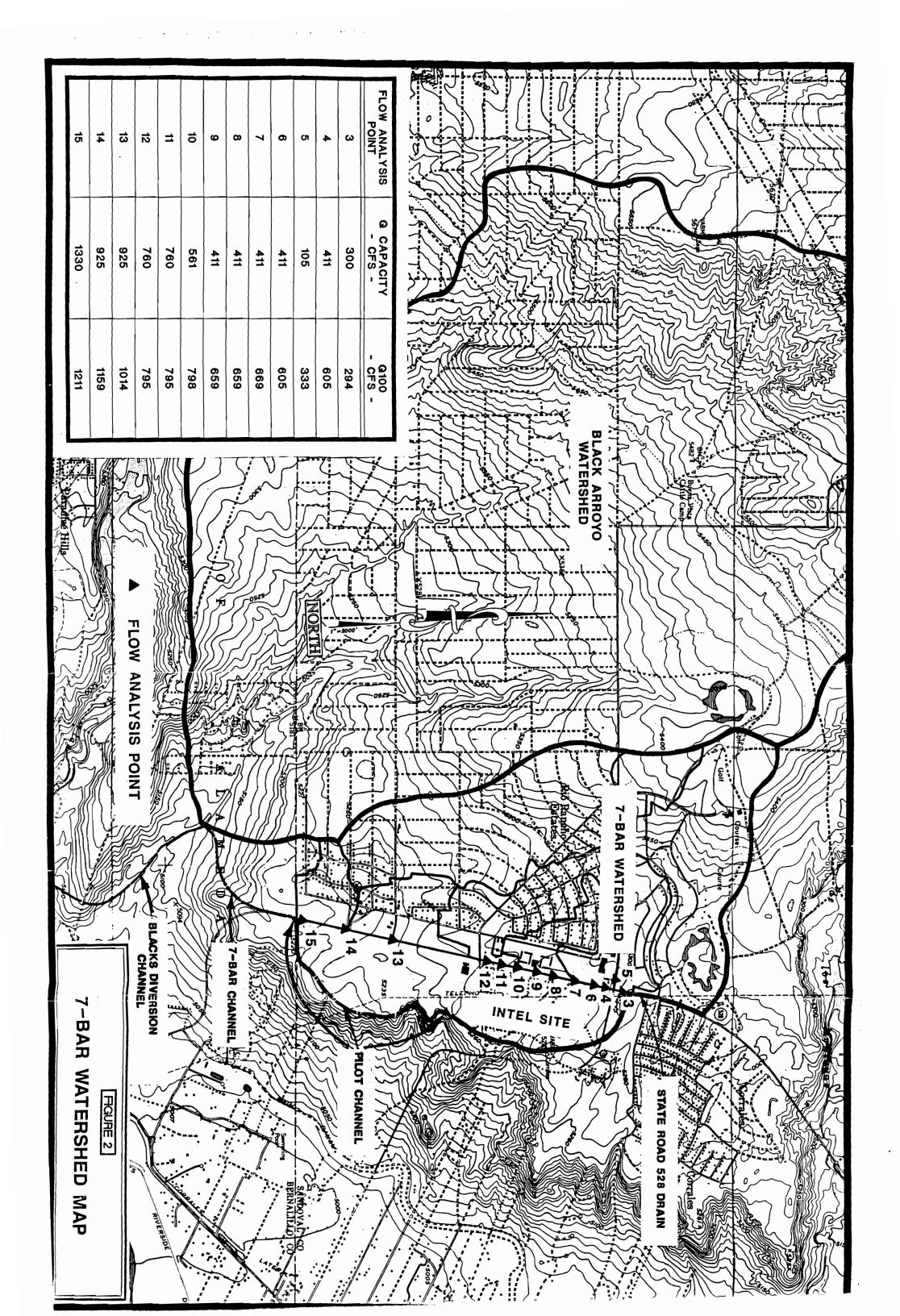
D. Black Arroyo and 7-Bar Watersheds

The model that simulates the hydrology for the Intel site was combined with the Black Arroyo and 7-Bar models that Bohannan-Huston Inc. had created for AMAFCA. This model was previously used to size the Black Arroyo Dam, which was constructed in 1992. The Black Arroyo Dam was constructed to capture sediment and reduce peak flows generated from the Black Arroyo Watershed. The majority of the Black Arroyo Watershed lies within the municipal boundaries of Rio Rancho. The Black Arroyo Dam is therefore designed to insure the Blacks Diversion is not overtopped during the 100-year storm event. The model utilized for the Black Arroyo and 7-Bar Watersheds is based on full development conditions.

The flows outletting from the Intel site flow into the NM 528 earthen channel. At that point, the flows combine with the rest of the basin in the 7-Bar Watershed which, in general, lies north and west of Intel (see Figure 2, 7-Bar Watershed Map). The flows are then carried in the 7-Bar Channel to its confluence with Blacks Diversion, at which point the flows combine with flows from the Black Arroyo (see Figure 3, Black Arroyo Watershed Map).

The 7-Bar Watershed, at the confluence with Intel flows, has a drainage area of approximately 1.4 square miles and drains most of the easterly portion of the Rio Rancho area (see Figure 2). The NM 528 earthen channel collects the flows from the portion of Rio Rancho in the 7-Bar Watershed. The earthen channel has a number of constrictions caused by culvert structures at road crossings which, in general, are not sufficiently large to carry the 100-year flows. Figure 2 shows a comparison of the flows that are anticipated from the contributing basins versus the capacity of each of the channel culvert structures. These crossing structures prevent the entire flow of 1234 cfs (see Table 4) from being delivered to the 7-Bar Channel from the Rio Rancho area. However, for this analysis, the assumption has been made that at

▲ BOHANNAN-HUSTON INC.



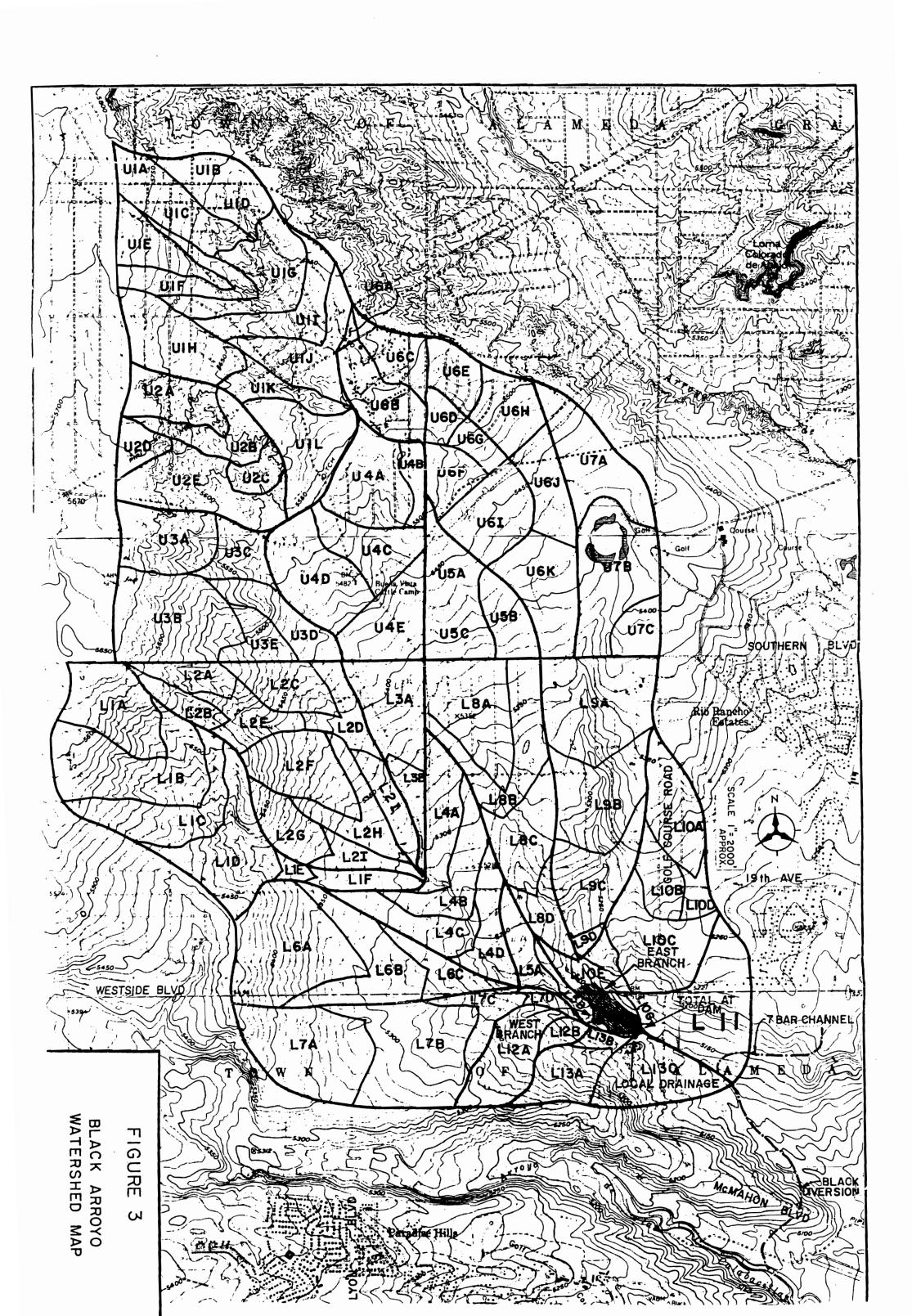


TABLE 4

BLACK ARROYO AND 7-BAR WATERSHED FLOWS 100-Year Flows

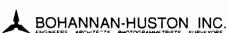
Flow Location	Phase 1 Watershed Developed cfs (1)	Phase 1 Existing Watershed cfs (2)	Phase 2 Watershed Developed No Ponding cfs (3)	Phase 2 Watershed Developed 7-Bar Max. is 1250 cfs cfs (4)	Phase 2 Watershed Developed 7-Bar Max. is 1000 cfs cfs (5)
Rio Rancho - Upstream of Intel	1234	970	1234	1234	1234
528 Pond - Upstream of Intel		•		964	802
Volume Required	•	•	••	20.6 acft.	30.3 acft.
Intel site Flow	186	186	490	484	484
Intel Detention Pond	:	•	•-	337.6	231
Volume Required	ŧ	-		8.7 acft.	13.15 acft.
Flow Entering 7-Bar Channel	1407	1150	1560	1230	1015
7-Bar Channel at Black Arroyo	1433	1192	1580	1295	1064
Black Arroyo Upstream of 7-Bar	2503	2440	2505	2505	2505
Flow Entering Blacks Diversion	3697	3640	3804	3711	3470
Blacks Diversion at Calabacillas	3693	3640	3818	3706	3468

Notes:

- 1. Phases 1 and 2 refer to Intel site expansions
- 2. Watershed Developed 7-Bar and Black Arroyo at ultimate development
- 3. Column 1 represents the flows in the downstream facilities assuming the undersized structure along the NM 528 Channel have been up-sized.
- 4. Column 2 shows the flows in the downstream facilities with the existing undersized structures in the NM 528 Channel. Flow calculation is based on the assumption that half the weir flow at each structure adds back into the channel.
- 5. Column 4 represents the capacity of 7-Bar Channel if the channel is upgraded from the present capacity of 1000 cfs to 1250 cfs.

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some point in the future these structures will be upgraded to accommodate the anticipated 100-year flows.

Table 4, Black Arroyo and 7-Bar Watershed Flows, shows the peak flows at key points for both Phase 1 and Phase 2 development conditions at the Intel site. From previous reports performed for AMAFCA by Bohannan-Huston Inc., the capacity of the Blacks Diversion is 3,700 cfs, and the capacity of the 7-Bar Channel is 1000 cfs. A comparison of the capacities of the drainage facilities and the 100-year flows are shown in Table 5.

TABLE 5

DOWNSTREAM DRAINAGE FACILITIES CAPACITY

	Structure	100-yea	ır Flows
Drainage Facility	Capacity cfs	Phase 1 cfs	Phase 2 cfs
3-42" RCPs - Intel Storm Outlet	190	186	490
7-Bar Channel at Upstream End	1,000	1,407	1,560
7-Bar Channel at Confluence with Blacks Diversion	1,000	1,433	1,580
Blacks Diversion	3,700	3,697	3,804

NOTE: Flows shown are with 7-Bar and Black Arroyo Watersheds fully developed.

Table 5 shows that the 7-Bar Channel is under capacity for Phase 1 and Phase 2, and the Blacks Diversion and the three 42" RCPs are under capacity for Phase 2. The flows shown in this table assume Rio Rancho has up-sized the drainage crossing structures along the NM 528 Channel. The impact on the Intel development is that either the drainage structures must be increased in capacity or a detention pond is

required to attenuate the peak flows. The following section on hydraulics examines the facilities drainage capacity issue.

V. HYDRAULICS

As part of this study, the downstream drainage facilities were examined to first review the capacity of the existing structures, and secondly, examine the potential for increasing their capacities.

A. Black Arroyo Dam

Constructed in 1992 by AMAFCA, this dam has a storage capacity of 200 acrefeet. At present, the entire storage capacity has not been completely excavated since the Black Arroyo Watershed is not fully developed. AMAFCA has indicated that additional storage capacity will be developed in the near future as part of a Corps of Engineers project which will remove earth from the storage pond.

One possibility to remedy the downstream system capacity problems would be to increase the amount of water storage behind the dam. Increasing the storage would make it possible to decrease the peak flow discharging from the dam.

The potential improvements that would be necessary to decrease the peak flow discharging from the dam (and thereby reduce the peak flow entering Blacks Diversion) entails increasing the storage capacity and/or modifying the principal spillway. An analysis to upgrade this facility was not performed, mainly due to the fact that the costs involved would be excessive. Additionally, the main system constraint is the 7-Bar Channel and Blacks Diversion, not the Black Arroyo Dam. The main reason for the high cost was due to the fact that additional land would be required and the dam outlet structure would need modification.





B. Blacks Diversion

The Blacks Diversion has a capacity of 3,700 cfs with sediment bulking (3,600 cfs without sediment bulking). Typically, flows calculated in a hydrologic model are for clear water. However, almost all runoff picks up and transports sediment as it moves across the earth's surface. The flow has therefore been increased by a factor (referred to as sediment bulking) to reflect the fact that a portion of the runoff is composed of sediment.

Bohannan-Huston Inc., as part of the design analysis for Black Arroyo Dam, investigated the possibility of increasing the capacity of the Blacks Diversion. For this project, increasing the Blacks Diversion to accommodate the additional 100 cfs required would entail placing an 8" block on top of the existing wells for the entire length of the channel. It is believed that the walls can handle the additional height without major reinforcement, but this has never been verified with the Soil Conservation Service who constructed the facility. However, from the Black Arroyo design analysis, it was not considered cost effective to attempt to increase the capacity of the Blacks Diversion.

C. Black Arroyo and 7-Bar Confluence

Easterling & Associates, Inc., analyzed and designed an upgrade to this confluence in 1990. In this effort, the confluence was designed for a maximum capacity of 4,200 cfs, which at the time would have been the maximum capacity of Blacks Diversion if an 8" block upgrade had been constructed. The flows calculated in this report indicate that the confluence does not need upgrading; however; this may have to be verified in a physical model study should the 7-Bar Channel be upgraded.

BOHANNAN-HUSTON INC.

D. 7-Bar Channel

The 7-Bar Channel was designed to carry a peak flow of 1,000 cfs. The channel was constructed in 1983, well before the Black Arroyo Dam was constructed. The channel was designed using a more conservative freeboard equation that has since been revised by both the City and AMAFCA. The present equation indicates that substantial reaches of the 7-Bar Channel have capacity greater than 1,000 cfs. The channel has markedly different slopes. The initial part of the channel has a fairly flat slope where it parallels NM 528. However, once the channel alignment veers away from NM 528, the channel leaves the mesa top and traverses through a steep slope area.

This study performed a detailed analysis of the channel capacity using the Corps of Engineers hydraulic program HEC-2. Once the water surface depths were determined from the HEC-2 analysis, the results were incorporated into a spreadsheet to determine the total freeboard and superelevation required based on the present freeboard equations currently used by AMAFCA. The results of the freeboard analysis are summarized in Table 6. Table 6 reflects the peak of 1150 cfs from Phase 1 with the existing undersized structures in the NM 528 channel.

A number of flow rates were analyzed in order to determine the optimal flow capacity. Table 7 reflects the flow capacity the majority of 7-Bar Channel can adequately pass is 1250 cfs. The exception to this is at the upstream end of the channel from Sta. 30+75 to Sta. 43+75, a distance of approximately 1,200 feet. In this portion, the channel freeboard is deficient by up to .5 feet.

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					Radius		Super		Channel	Existing
	Water		Flow		of Horiz.	Normal	Elevation	Total	Depth	Channel
Location	Depth ft.	Discharge cfs.	Area sq. ft.	Velocity fps.	Curve ft.	Freeboard ft.	Freeboard ft.	Freeboard ft.	Required ft.	Depth ft.
	(1)									
I rapezoidal Channei			1	!						
Sta. 43+75 7bar	5.25	1,150.0	107.63	10.7	0	1.7		1.7	7.0	7.0
Sta. 35+00 7bar	5.24	1,150.0	107.32	10.7	1395	1.7		1.8	7.1	7.0
Sta. 32+00 7bar	4.87	1,150.0	96.13	12.0	1395	1.8		1.9	6.7	7.0
Sta. 30+75 7bar	5.01	1,150.0	100.30	11.5	1395	1.7	0.1	1.8	6.9	7.0
Sta. 29+50 7bar	4.07	1,150.0	73.83	15.6	1395	1.8	0.2	2.0	6.1	7.0
Sta. 28+50 7bar	3.31	1,150.0	55.01	20.9	1395	1.9	0.3	2.2	5.5	7.0
Sta. 27+50 7bar	2.93	1,150.0	46.47	24.7	1395	2.0	0.3	2.4	5.3	6.5
Sta. 26+50 7bar	2.80	1,150.0	43.68	26.3	0	2.0	0.0	2.0	4.8	6.5
Sta. 25+50 7bar	2.89	1,150.0	45.60	25.2	0	2.0	0.0	2.0	4.9	6.5
Sta. 24+75 7bar	3.06	1,150.0	49.33	23.3	0	2.0	0.0	2.0	5.1	6.5
Sta. 24+25 7bar	3.09	1,150.0	20.00	23.0	0	2.0	0.0	2.0	5.1	9.9
Sta. 23+75 7bar	3.02	1,150.0	48.44	23.7	0	2.0	0.0	2.0	5.0	6.5
Sta. 23+00 7bar	2.81	1,150.0	43.89	26.2	0	2.0	0.0	2.0	4.9	6.5
Sta. 21+00 7bar	2.42	1,150.0	35.91	32.0	658	2.2	1.0	3.2	5.6	6.5
Sta. 20+00 7bar	2.36	1,150.0	34.74	33.1	658	2.2	1.1	3.3	5.6	6.5
Sta. 18+50 7bar	2.40	1,150.0	35.52	32.4	658	2.2	1.1	3.2	5.6	6.5
Sta. 16+50 7bar	2.45	1,150.0	36.51	31.5	658	2.1	1.0	3.2	5.6	6.5
Sta. 15+50 7bar	2.52	1,150.0	37.90	30.3	0	2.1	0.0	2.1	4.6	0.9
Sta. 14+50 7bar	2.71	1,150.0	41.79	27.5	0	2.1	0.0	2.1	4.8	0.9
Sta. 10+50 7bar	3.34	1,150.0	55.71	50.6	0	1.9	0.0	1.9	5.3	0.9
Begin transition										
Sta. 10+25 7bar	3.28	1,150.0	54.35	21.2	0	2.0	0.0	2.0	5.2	6.2
Sta. 9+75 7bar	3.45	1,150.0	58.31	19.7	0	1.9	0.0	1.9	5.4	6.9
	3.14	1,150.0	51.12	22.5	0	2.0	0.0	2.0	5.1	7.5
Rectangular channel										
Sta. 8+75 7bar	3.39	1,150.0	33.90	33.9	0.0	2.3	0.0	2.3	5.7	8.2
Sta. 8+25 7bar	4.18	1,150.0	41.80	27.5	0.0	2.2	0.0	2.2	6.4	8.9
Sta. 7+75 7bar	5.75	1,150.0	57.50	20.0	335.8	2.0	0.4		8.1	0.6
	5.47	1,150.0	54.70	21.0	335.8	2.0	0.4		7.9	9.7
Sta. 4+00 7bar	7.33	1,150.0	73.30	15.7	335.8	1.9	0.2		9.5	9.7
Sta. 3+00 7bar	7.41	1,150.0	74.10	15.5	335.8	1.9	0.2	2.2	9.6	10.0
Sta. 2+00 7bar	7.50	1,150.0	75.00	15.3	335.8	1.9	0.5	2.1	9.6	10.0
Confluence - Black's Diversion										

TABLE 6 7-BAR CHANNEL FREEBOARD - 1,150 CFS

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8.9 8.9 9.0 7.6 0.0 0.0 0.0

6.2 6.9 7.5

Channel Existing Depth ft. 6.3 6.3 8.7 8.7 9.5 9.5 5.4 5.5 5.3 7.2 7.3 7.1 6.3 5.7 Required Channel Depth 2.0 1.9 2.0 Freeboard Freeboard Total Super Elevation 0.0 0.0 0.3 0.3 0.3 0.0 0.0 7-BAR CHANNEL FREEBOARD - 1,250 CFS 20000123333333000 10000133333333000 1.7 1.8 1.8 1.9 2.0 2.0 Freeboard Normal 335.84 335.84 335.84 335.84 1395 1395 1395 1395 1395 00000 000 335.84 Radius of Horiz. Curve ₩ 37.9 30.8 22.6 19.9 17.5 17.2 11.6 16.0 25.2 25.2 26.9 25.7 25.7 25.7 25.7 25.7 25.7 32.8 32.8 32.8 33.2 32.1 21.8 20.2 23.2 Velocity ģ. 107.63 107.32 78.36 59.02 49.55 46.47 48.66 52.48 52.94 51.57 46.91 38.10 115.50 113.59 37.70 40.13 57.36 61.92 53.86 33.00 40.60 55.40 62.70 71.50 72.60 38.91 Area Flow ,250.0 ,250.0 ,250.0 250.0 ,250.0 ,250.0 ,250.0 ,250.0 ,250.0 ,250.0 ,250.0 ,250.0 ,250.0 ,250.0 ,250.0 ,250.0 ,250.0 ,250.0 ,250.0 ,250.0 ,250.0 ,250.0 ,250.0 Discharge Water 5.25 5.24 4.24 3.41 3.60 3.26 3.30 5.54 6.27 7.15 7.26 7.35 Water Depth Ξ Confluence - Black's Diversion Trapezoidal Channel Rectangular channel Begin transition _ocation Sta. 43+75 7bar Sta. 35+00 7bar Sta. 32+00 7bar Sta. 30+75 7bar Sta. 29+50 7bar Sta. 27+50 7bar Sta. 25+50 7bar Sta. 24+75 7bar Sta. 24+75 7bar Sta. 24+25 7bar Sta. 23+00 7bar Sta. 21+00 7bar Sta. 20+00 7bar Sta. 16+50 7bar Sta. 10+25 7bar Sta. 9+75 7bar Sta. 9+25 7bar Sta. 10+50 7bar Sta. 7+75 7bar Sta. 5+00 7bar Sta. 4+00 7bar Sta. 3+00 7bar Sta. 2+00 7bar Sta. 14+50 7bar 5+00 7bar 8+75 7bar 8+25 7bar