

Martin J. Chávez, Mayor

October 23,1997

Mark H. Burak PE Burak Engineering 1512 Sagebrush Trail SE Albuquerque, New Mexico 87103

RE: REVISED DRAINAGE PLAN FOR PARADISE VISTA SUBDIVISION (B11-D2A) REVISION DATED 10/16/97

Dear Mr. Burak:

Based on the information provided on your October 17,1997 resubmittal, the above referenced site is approved FOR Preliminary Plat, Site Development Plan FOR Subdivision, and Grading Permit.

Please be advised that prior to Final Plat approval, the Agreement & Covenant FOR the retention pond must have been executed and filed.

Also, prior to Financial Guarantee release, Engineer Certification per the DPM checklist will be required.

If I can be of further assistance, please feel free to contact me at 924-3986.

C: Andrew Garcia Felix Rabadi File Sincerely

Bernie J. Montoya CE Associate Engineer



## **DRAINAGE INFORMATION SHEET**



PROJECT TITLE:	Paradise Vista Subdivisi	on ZONE ATLAS/DRNG.FILE# B-11/D2A
DRB #:	EPC#:	WORK ORDER #:
LEGAL DESCRIPT	TON: Tracts A-2B, Par	adise Bluff
CITY ADDRESS:	Justin / Buglo	
ENGINEERING FIF ADDRESS:		eering Consulting CONTACT: Mark Burak Trail SE, ABQ, NM 87123 (505) 296-0461
OWNER: ADDRESS:	Felix Rabadi	
ARCHITECT: ADDRESS:		CONTACT:
SURVEYOR:		CONTACT:
CONTRACTOR: _		CONTACT:
TYPE OF SUBMIT	TAL CH	ECK TYPE OF APPROVAL SOUGHT:
DRAINAGE REDRAINAGE REDRAINAGE PECONCEPTUAL  XXX GRADING PLATE EROSION CO  ENGINEER'S  OTHER	LAN L GRADE & DRAIN PLAN AN NTROL PLAN	SKETCH PLAT APPROVAL  XXX PRELIMINARY PLAT APPROVAL  XXX SITE DEV. PLAN FOR SUBD. APPROVAL  SITE DEV. PLAN FOR BLDG. PERMIT APP.  SECTOR PLAN APPROVAL  FINAL PLAT APPROVAL  FOUNDATION PERMIT APPROVAL  BUILDING PERMIT APPROVAL  CERTIFICATION OF OCCUPANCY APPROVAL
PRE-DESIGN MEE  xxx YES  NO COPY PROVI		xxx GRADING PERMIT APPROVAL PAVING PERMIT APPROVAL S.A.D. DRAINAGE REPORT DRAINAGE REQUIREMENTS OTHER
DATE SUBMITTE		
BY: Ma	k Baral	-





1512 Sagebrush Trail SE Albuquerque, NM 87123

(505) 296-0461

235-2256 cell

296-0467 fax

October 17, 1997

Bernie J. Montoya Associate Engineer City of Albuquerque Public Works Albuquerque, NM 87110

RE: Paradise Vista Subdivision (B11-D2A) Grading and Drainage Plan

Dear Mr. Montoya:

This letter is in response to your comments dated September 16, 1997. I would like to thank you for faxing your comments to me to help speed up the revision process. Hopefully, the following will alleviate any concerns you may have so that we may gain approval and move on to DRB with this project.

- 1. The street capacity analysis with my spreadsheet template "Hydrapak" is more detailed and versatile than the DPM street capacity graphs. The template will calculate open channel flow capacity for streets with normal crown; inverted crown; streets with medians; with sidewalks or without; and variable curb and gutter dimensions. The DPM street section analyzes only a triangle flow area for a half street. Attached are some examples of the spreadsheet printout with the corresponding DPM graph. The DPM graph consistently shows a slightly higher street capacity than the spreadsheet. Calculations and graph attached.
- 2. After further analysis, it was found that the fully developed flow can be carried in the streets through the subdivision. The EGL in the streets is still well below the right-of-way limits. The wall openings to the easements were estimated assuming a five foot wide by one foot high box culvert calculation. These spreadsheet printouts are included with this letter. The permanent drainage easements are ten feet wide and one foot high and will flow at a depth of about 0.4-feet during the fully developed 100-year storm.
- $o^{1/2}$  3. All pad elevations have been converted to MSL.
  - 16 4. The typical detail for side lot swales is shown on the plan.
- 5. A note for concrete grouting of the rundowns has been added to the plan.

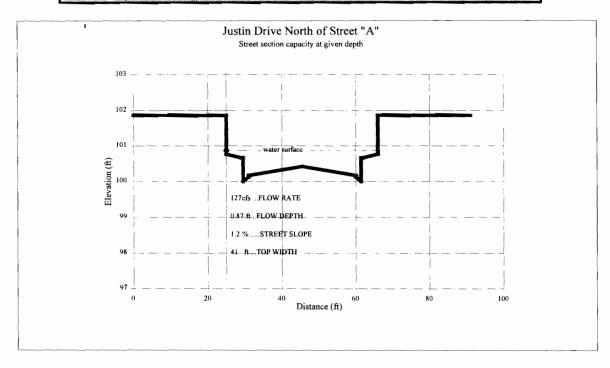
- 6. Top of curb and flow line elevations are shown at all turnouts and along all streets at periodic intervals. Plan and profile sheets showing the roadway construction have been generated also.
- , 7. The lot lines along the western boundary have been relocated to the north to allow proper minimal setbacks from the proposed drainage easements as shown on the plan. The detail of the easements has also been updated to show the required setbacks.
- 8. If offsite flows are allowed to pass through the site, the temporary storm drain inlets on Justin will not be needed. Even without the inlets, some of the existing offsite runoff will be intercepted by the sidewalk culverts. The inlets were intended solely to intercept all runoff exiting the site. Without them, approximately eight cfs will continue north on Justin primarily on the east side of the roadway.
- 9. The process for the Agreement and Covenant for the retention pond has been started. I do not want to proceed further with it until I am confident that all your concerns are met.
- 10. The drainage easements will be public and are designed as shown on the plan conforming to COA standard drawing 22-60. The ten foot easement will be concrete lined with an eight foot bottom, one foot deep and will flow the fully developed runoff rate at about 0.4 feet. The EGL was calculated as 0.7 feet, thus maintaining any potential hydraulic jump to be maintained within the curbs on each side of the street.
- 11. Traffic engineering does not have a problem with the driveway slopes.
  - 12. The height of the retaining wall on the west side of the property has been called out on the plan at each lot line. The wall will be 500 feet long and will range between two and four feet in height. It should be noted that runoff from the west will not discharge to the proposed easements until the commercial site to the west is developed. In the interim, a small amount of ponding may occur along the retaining wall.

Thanks for your attention to this project and please feel free to contact me at 296-0461 if you have any more questions or concerns that may further delay approval. Also, for your information, Mr. Rabadi is working on obtaining a slope easement from the property owner to the north to eliminate the need for the retaining wall.

Sincerely.

Mark Burok

Manning's Equation for flow capacity in a street section.  Justin Drive North of Street "A"							
Input variables:			Output Parameters:				
Depth of flow	0.87	ft	Capacity at d	127.1	cfs		
Width (back of curb)	33.0	ft	@ top of curb	68.5	cfs		
Crown height	0.42	ft	@ back of walk	79.9	cfs		
Street slope	1.20	%	Velocity at d	6.0	fps		
Sidewalk width	4.0	ft	V*d FACTOR	5.2			
Curb height	8	in	Gutter width	1.5	ft		
Median width	0.0	ft	Gutter depression	1.5	in		
Rt back of walk	100.0	%	Asphalt lip	0	in		
Lt back of walk	100.0	%	Manning's n	0.017			

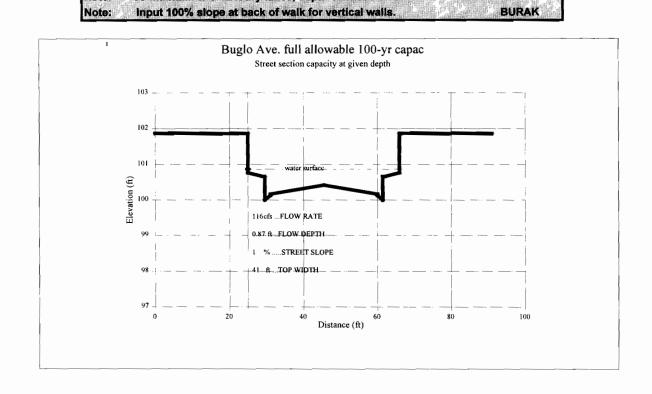


STREET FLOW'S  Manning's Equation for flow capacity in a street section.								
Buglo Ave. full allowable 100-yr capacity								
Input variables:		d	Output Parameters:					
Depth of flow	0.87 ft	c	Capacity at d	116.0	cfs			
Width (back of curb)	33.0 ft	1	@ top of curb	62.5	cfs			
Crown height	0.42 ft	1	@ back of walk	72.9	cfs			
Street slope	1.00 %	6 N	elocity at d	5.5	fps			
Sidewalk width	4.0 ft	· \	/*d FACTOR	4.8				
Curb height	8 in	1 (	Sutter width	1.5	ft			
Median width	0.0 ft		Sutter depression	1.5	in			
Rt back of walk	100.0 %	6 <b> </b> 4	Asphalt lip	0	in			
Lt back of walk	100.0 %	6 N	/lanning's n	0.017				

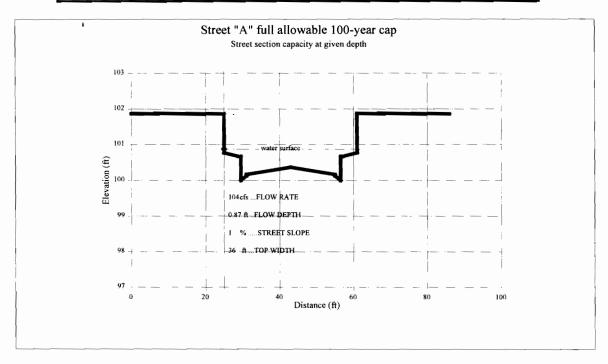
To maintain two 12-ft dry lanes, depth cannot exceed

Note:

0.185 feet



Street "A" full allo	wable	100-	year capacity		
Input variables:			Output Parameters:		
Depth of flow	0.87	ft	Capacity at d	103.9	cfs
Width (back of curb)	28.0	ft	@ top of curb	58.6	cfs
Crown height	0.37	ft	@ back of walk	65.6	cfs
Street slope	1.00	%	Velocity at d	5.5	fps
Sidewalk width	4.0	ft	V*d FACTOR	4.8	
Curb height	8	in	Gutter width	1.5	ft
Median width	0.0	ft	Gutter depression	1.5	in
Rt back of walk	100.0	%	Asphalt lip	0	in
Lt back of walk	100.0	%	Manning's n	0.017	



#### TRAPEZOIDAL CHANNEL

Normal depth and critical depth parameters

	Normal depth and	chtical depth paramet	
<b>Drainage Ease</b>	ment for Area "	B" - Fully Develo	ped
Input variables		Output Parame	ters:
Discharge	18 cfs	Normal depth	0.43 ft
Channel slope	0.01000 ft/ft	Normal velocity	5.23 fps
Manning's n	0.015	Froude number	1.41
Bottom width	8 ft	Critical depth	0.54 ft
Left side slope	0 H:1	Critical velocity	4.17 fps
Right side slope	0 H:1	Scour Depth	1.10 ft
		Superelevation	0.00 ft
Curve Radius	10000 ft	Freeboard	2.10 ft
		Channel Depth	2.53 ft
· · · · · · · · · · · · · · · · · · ·	ing a state of the same of	Sequent Depth	<b>0.67</b> ft
Note: Freeboar	d = (2+0.025(velocity))	depth)^(1/3))	BURAK

# TRAPEZOIDAL CHANNEL

	nd critical depth paramete					
Drainage Easement for Are	a "C" - Fully Develo	ped Lubblin				
Input variables:	Output Parame	Output Parameters:				
Discharge 19.56 cfs	Normal depth	0.46 ft				
Channel slope 0.01000 ft/ft	Normal velocity	5.32 fps				
Manning's n 0.015	Froude number	1.38				
Bottom width 8 ft	Critical depth	0.58 ft				
Left side slope 0 H:1	Critical velocity	4.22 fps				
Right side slope 0 H:1	Scour Depth	1.12 ft				
	Superelevation	0.00 ft				
Curve Radius 10000 ft	Freeboard	2.10 ft				
	Channel Depth	2.56 ft				
	Sequent Depth	0.70 ft				
Note: Freeboard = (2+0.025(velocit	tv)(depth)^(1/3))	BURAK				

### BOX CULVERT

Inlet control and outlet control parameters

#### Wall Opening to Easement for Area "B"

1.0 ft INLET CTRL HWo: Box height Box width tapered throat 1.11 ft 5.0 ft Number of boxes 45 degree bevels 1.19 ft 0.01000 ft/ft sq edge headwall 1.29 ft Slope OUTLET CTRL HW Manning's n 1.10 ft 0.015 Velocity Culvert length 1 ft 3.6 fps 0.74 ft Critical depth Discharge 18 cfs

Note: 1. Full flowing box assumed.

2. Critical depth cannot exceed the height of box culvert.

BURAK

### **BOX CULVERT**

Inlet control and outlet control parameters

100	555 S	1000	\$99038955385 <b>8</b> 2	***************************************	\$167°, 100° mark	10000		20 - 21
27.81	erre e	100 ato 7		B 2000 2000 2000	3	 ~~~		
8.7.1		255 937 4		3 2 2 1 0 222	5 0 132 mm	8 8 1 2.3 3 8 8	B I S I SSE " 1	
2.6.2	6431	200 A	ALC: NO.	ほんれんご 部札				rea "C"

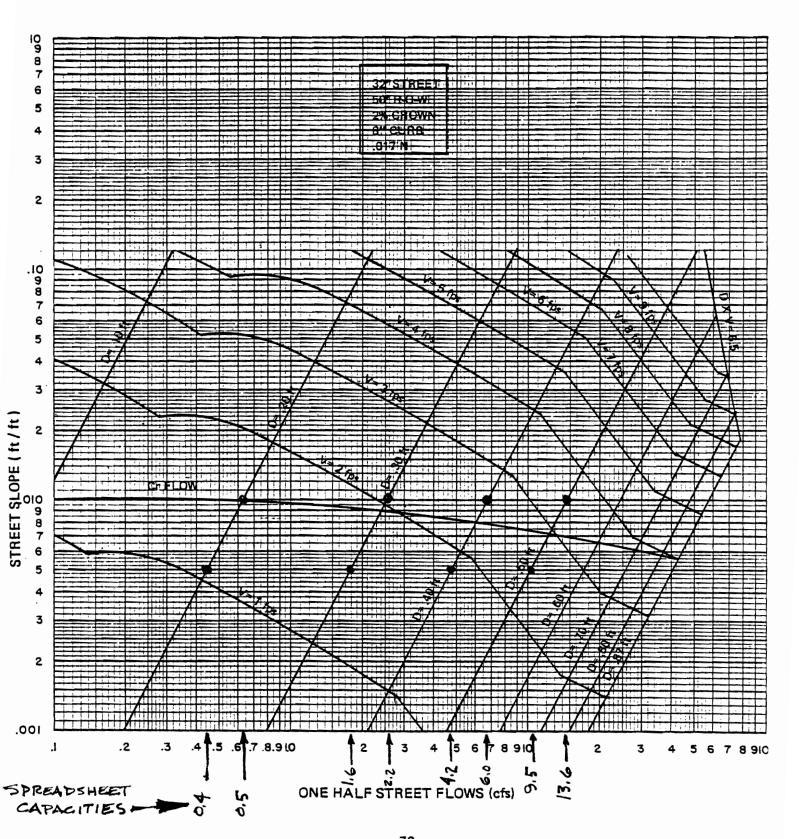
Box height 1.0 ft	INLET CTRL HWo:
Box width 5.0 ft	tapered throat 1.17 ft
Number of boxes 1	45 degree bevels 1.28 ft
Slope 0.01000 ft/ft	sq edge headwall 1.38 ft
Manning's n 0.015	OUTLET CTRL HW 1.17 ft
Culvert length 1 ft	Velocity 3.9 fps
Discharge 20 cfs	Critical depth 0.78 ft
图16.12.14.14.26.12.15.16.16.16.16.16.16.16.16.16.16.16.16.16.	"你是你在这个事情的。" 第一次,我们就是一个事情,我们就是一个事情,就是一个事情,就是一个事情,就是一个事情,就是一个事情,就是一个事情,就是一个事情,就是一个事情,就是一个事情,就是

Note: 1. Full flowing box assumed.

2. Critical depth cannot exceed the height of box culvert.

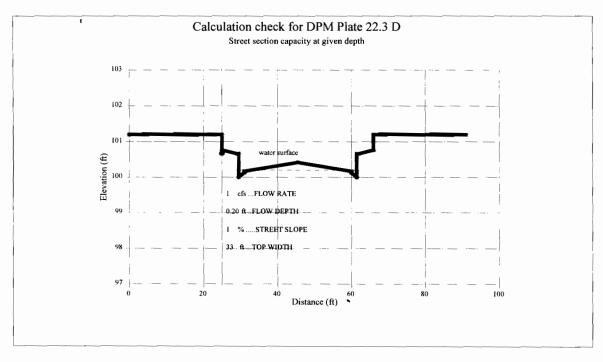
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#### STREET CAPACITY

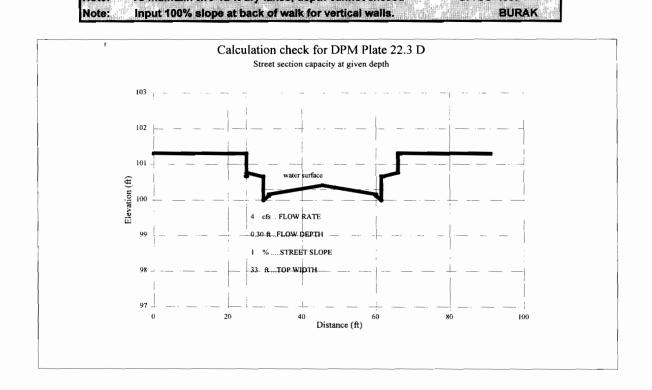


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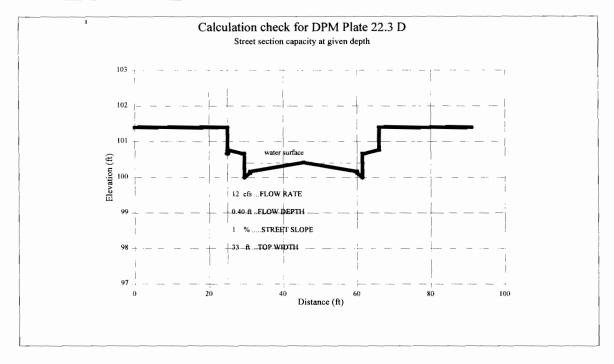
Calculation check	20 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	200000000000000000000000000000000000000	capacity in a street section late 22.3 D-1 (Street		city)
Input variables:			Output Parameters:		
Depth of flow	0.20	ft	Capacity at d	1.0	cfs
Width (back of curb)	33.0	ft	@ top of curb	63.1	cfs
Crown height	0.42	ft	@ back of walk	73.5	cfs
Street slope	1.00	%	Velocity at d	1.4	fps
Sidewalk width	4.0	ft	V*d FACTOR	0.3	
Curb height	8	in	Gutter width	1.5	ft
Median width	0.0	ft	Gutter depression	1.5	in
Rt back of walk	100.0	%	Asphalt lip	0	in
Lt back of walk	100.0	%	Manning's n	0.017	



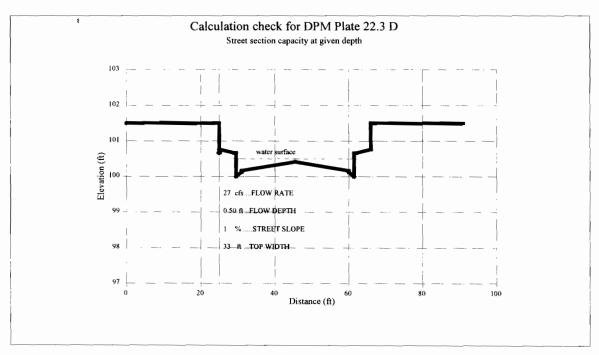
STREET FLOWS  Manning's Equation for flow capacity in a street section.						
			late 22.3 D-1 (Stree		city)	
Input variables:			Output Parameters:			
Depth of flow	0.30	ft	Capacity at d	4.4	cfs	
Width (back of curb)	33.0	ft	@ top of curb	63.1	cfs	
Crown height	0.42	ft	@ back of walk	73.5	cfs	
Street slope	1.00	%	Velocity at d	2.0	fps	
Sidewalk width	4.0	ft	V*d FACTOR	0.6		
Curb height	8	in	Gutter width	1.5	ft	
Median width	0.0	ft	Gutter depression	1.5	in	
Rt back of walk	100.0	%	Asphalt lip	0	in	
Lt back of walk	100.0	%	Manning's n	0.017		



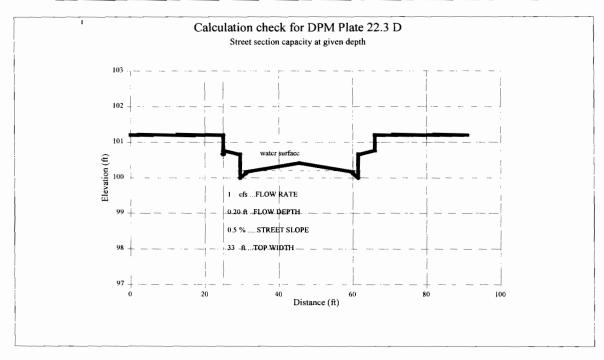
Manning's Equation for flow capacity in a street section.  Calculation check for DPM Plate 22.3 D-1 (Street Capacity)							
Input variables:			Output Parameters:				
Depth of flow	0.40	ft	Capacity at d	12.0	cfs		
Width (back of curb)	33.0	ft	@ top of curb	63.1	cfs		
Crown height	0.42	ft	@ back of walk	73.5	cfs		
Street slope	1.00	%	Velocity at d	2.5	fps		
Sidewalk width	4.0	ft	V*d FACTOR	1.0			
Curb height	8	in	Gutter width	1.5	ft		
Median width	0.0	ft	Gutter depression	1.5	in		
Rt back of walk	100.0	%	Asphalt lip	0	in		
Lt back of walk	100.0	%	Manning's n	0.017			



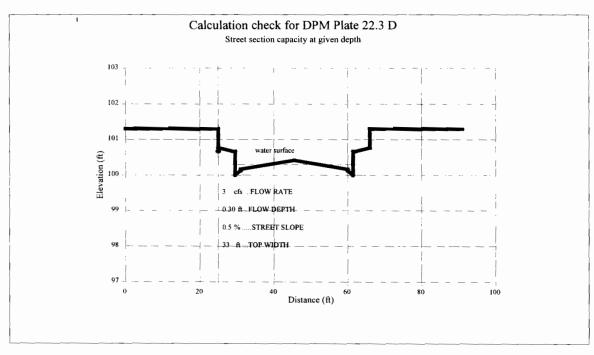
Calculation check for DPM Plate 22.3 D-1 (Street Capacity)								
Input variables:			Output Parameters:					
Depth of flow	0.50	ft	Capacity at d	27.1	cfs			
Width (back of curb)	33.0	ft	@ top of curb	63.1	cfs			
Crown height	0.42	ft	@ back of walk	73.5	cfs			
Street slope	1.00	%	Velocity at d	3.4	fps			
Sidewalk width	4.0	ft	V*d FACTOR	1.7				
Curb height	8	in	Gutter width	1.5	ft			
Median width	0.0	ft	Gutter depression	1.5	in			
Rt back of walk	100.0	%	Asphalt lip	0	in			
Lt back of walk	100.0	%	Manning's n	0.017				



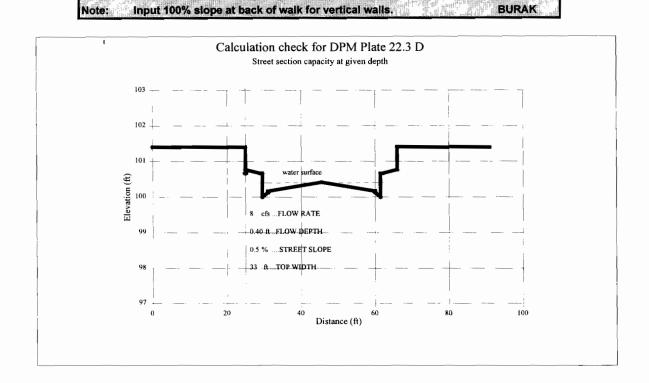
Manning's Equation for flow capacity in a street section.  Calculation check for DPM Plate 22.3 D-1 (Street Capacity)						
Input variables:			Output Parameters:			
Depth of flow	0.20	ft	Capacity at d	0.7	cfs	
Width (back of curb)	33.0	ft	@ top of curb	44.6	cfs	
Crown height	0.42	ft	@ back of walk	52.0	cfs	
Street slope	0.50	%	Velocity at d	1.0	fps	
Sidewalk width	4.0	ft	V*d FACTOR	0.2	-	
Curb height	8	in	Gutter width	1.5	ft	
Median width	0.0	ft	Gutter depression	1.5	in	
Rt back of walk	100.0	%	Asphalt lip	0	in	
Lt back of walk	100.0	%	Manning's n	0.017		



Calculation check for DPM Plate 22.3 D-1 (Street Capacity)						
Input variables:			Output Parameters:			
Depth of flow	0.30	ft	Capacity at d	3.1	cfs	
Width (back of curb)	33.0	ft	@ top of curb	44.6	cfs	
Crown height	0.42	ft	@ back of walk	52.0	cfs	
Street slope	0.50	%	Velocity at d	1.4	fps	
Sidewalk width	4.0	ft	V*d FACTOR	0.4		
Curb height	8	in	Gutter width	1.5	ft	
Median width	0.0	ft	Gutter depression	1.5	in	
Rt back of walk	100.0	%	Asphalt lip	0	in	
Lt back of walk	100.0	%	Manning's n	0.017		



STREET FLOWS  Manning's Equation for flow capacity in a street section.  Calculation check for DPM Plate 22.3 D-1 (Street Capacity)						
Depth of flow	0.40	ft	Capacity at d	8.5	cfs	
Width (back of curb)	33.0	ft	@ top of curb	44.6	cfs	
Crown height	0.42	ft	@ back of walk	52.0	cfs	
Street slope	0.50	%	Velocity at d	1.8	fps	
Sidewalk width	4.0	ft	V*d FACTOR	0.7		
Curb height	8	in	Gutter width	1.5	ft	
Median width	0.0	ft	Gutter depression	1.5	in	
Rt back of walk	100.0	%	Asphalt lip	0	in	
Lt back of walk	100.0	%	Manning's n	0.017		



#### STREET FLOWS

Manning's Equation for flow capacity in a street section.

Calculation check for DPM Plate 22.3 D-1 (Street Capacity

Input variables:		Output Parameters	Output Parameters:		
Depth of flow	0.50 ft	Capacity at d	19.1 cf	fs	
Width (back of curb)	33.0 ft	@ top of curb	44.6 ct	fs	
Crown height	0.42 ft	@ back of walk	52.0 ct	fs	
Street slope	0.50 %	Velocity at d	2.4 fp	s	
Sidewalk width	4.0 ft	V*d FACTOR	1.2		
Curb height	8 in	Gutter width	1.5 ft		
Median width	0.0 ft	Gutter depression	1.5 in	1	
Rt back of walk	100.0 %	Asphalt lip	0 in	1	
Lt back of walk	100.0 %	· ·	0.017		
Note: To maintain two	12-ft dry lane	s, depth cannot exceed	0.185 fe	et	

BURAK Note: Input 100% slope at back of walk for vertical walls.

