

DRAINAGE REPORT

for

NORTH GATEWAY

Prepared by

Mark Goodwin & Associates, PA
P.O. Box 90606
Albuquerque, NM 87199
(505) 828-2200



March 2007

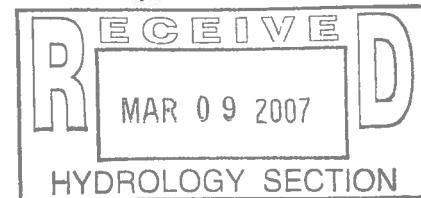


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I. PROJECT DESCRIPTION

The proposed project site covers approximately 22.79 Acres and is located along the North side of Balloon Fiesta Parkway NE. The current legal description of the site is Tract B, C-1 and C-2 North Gateway, Bernalillo County, New Mexico 6/2/06.

The purpose of this report is to present the master drainage management plan for the North Gateway Subdivision which is comprised of 4 tracts, of which only one is already developed. All applicable ordinances, the DPM and AHYMO were utilized to prepare this plan.

II. DRAINAGE DESIGN CRITERIA

The design criteria used in this report was in accordance with Section 22.2 Hydrology of the Development Process Manual. The 100-year, 6-hour storm event was utilized to determine site runoff rates using P (1 hr) = 2.10", P (6 hr) = 2.50" and P (24 hr) = 2.80". The onsite Land Treatment values used were Treatment D=85 and Treatment B=15.

III. EXISTING DRAINAGE CONDITION

The site presently consists of existing office building in Tract C-2, undeveloped land covered by native vegetation and small open areas of the native sandy surface. Slope is predominantly toward the north. At the present time there is no offsite runoff entering any portion of the site.

IV. DEVELOPED DRAINAGE CONDITIONS

The total 100 year developed conditions flow from the site is 102.86 cfs. The site is divided into 5 basins (4 Tracts and 1 cul-de-sac) as shown in the plan. According to AHYMO total developed flow generated within the site during the 100-year, 6-hour storm, is 11.06 cfs (Tract C-1-A), 2.87 cfs (Cul-de-sac), 36.92 cfs (Tract C-2-A), 29.34 cfs (Tract B-1) and 22.67 cfs(Tract C-2-B). The developed runoff from Tract B-1 and Tract C-2-B is designed to discharge into existing concrete lined channel on north side. According to approved drainage report dated 1-14-07 from Mark Goodwin and Associates, from existing development on Tract C-2-A, 9.2 cfs will be discharged freely onto Balloon Fiesta Parkway NE and remaining into the channel on north side. The developed runoff from Tract C-1-A and cul-de-sac is designed to discharge freely onto the Balloon Fiesta Parkway NE, this can be achieved by adding a Type "A" storm inlet at intersection of Balloon Fiesta Parkway NE and San Mateo Blvd NE along the north side of Balloon Fiesta Parkway NE.

All the capacity calculations and AHYMO printouts are attached in the Appendix A.

APPENDIX A

AHYMO PRINTOUTS
CAPACITY CALCULATIONS

```
*S AHYMO_97 MODEL FOR: NORTH GATEWAY DRAINAGE REPORT
*S PREPARED FOR: COA
*S PREPARED BY: MARK GOODWIN & ASSOCIATES
*S
*S MODEL DESCRIPTION -
*S 1. 100-YEAR 6-HOUR RAINFALL EVENT
*S 2. THIS MODEL ASSUMES DEVELOPED CONDITIONS
*S
START TIME=0.0
*S***** AHYMO -GATEWAY. DAT
*S***** January 4TH 2007
*S***** HYDROLOGY FOR NORTH GATEWAY
*
RAINFALL TYPE=1 RAIN QUARTER=0.0 IN
RAIN ONE=2.10 IN RAIN SIX=2.50 IN
RAIN DAY=2.80 IN DT=0.0333 HR
*
*S***** TRACT C-1-A - ONSITE DEVELOPED (2.4377 ACRES)
*
COMPUTE NM HYD ID=1 HYD NO=101 AREA=0.00379 SQ MI
PER A=0 PER B=15 PER C=0 PER D=85
TP=0.1333 HR MASS RAINFALL=-1
PRINT HYD ID=1 CODE=24
*
*S***** Cul-De-Sac - ONSITE DEVELOPED (0.6313 ACRES)
*
COMPUTE NM HYD ID=2 HYD NO=102 AREA=0.00098 SQ MI
PER A=0 PER B=15 PER C=0 PER D=85 /SO
TP=0.1333 HR MASS RAINFALL=-1
PRINT HYD ID=2 CODE=24
*
*S***** TRACT C-2-A - ONSITE DEVELOPED (8.2228 ACRES)
*
COMPUTE NM HYD ID=3 HYD NO=103 AREA=0.01266 SQ MI
PER A=0 PER B=15 PER C=0 PER D=85
TP=0.1333 HR MASS RAINFALL=-1
PRINT HYD ID=3 CODE=24
*
*S***** TRACT B-1 - ONSITE DEVELOPED (6.5111 ACRES)
*
COMPUTE NM HYD ID=4 HYD NO=104 AREA=0.01006 SQ MI
PER A=0 PER B=15 PER C=0 PER D=85
```

PRINT HYD
ID=4 CODE=24

*S***** TRACT C-2-B - ONSITE DEVELOPED (5.00 ACRES)

COMPUTE NM HYD
ID=5 HYD NO=105 AREA=0.00777 SQ MI
PER A=0 PER B=15 PER C=0 PER D=85
TP=0.1333 HR MASS RAINFALL=-1
ID=5 CODE=24

PRINT HYD

FINISH

Zone 3

AHYMO PROGRAM SUMMARY TABLE (AHYMO_97) -
=02/22/2007
INPUT FILE = F:\PAVAN\AHYMO1~1\CUKIDES~1.TXT
9702dGoodwinM-AH

- VERSION: 1997.02d RUN DATE (MON/DAY/YR) USER NO.= AHYMO-I-

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*S AHYMO_97 MODEL FOR: NORTH GATEWAY DRAINAGE REPORT
*S PREPARED FOR: COA
*S PREPARED BY: MARK GOODWIN & ASSOCIATES
*S
*S MODEL DESCRIPTION -
*S 1. 100-YEAR 6-HOUR RAINFALL EVENT
*S 2. THIS MODEL ASSUMES DEVELOPED CONDITIONS
*S
START TIME=0.0
*S **** AHYMO -GATEWAY. DAT
*S **** January 4TH 2007
*S **** HYDROLOGY FOR NORTH GATEWAY
2.4
2.6
OK
^Z/
RAINFALL
TYPE=1 RAIN QUARTER=0.0 IN
RAIN ONE=2.10 IN RAIN SIX=2.50 IN
RAIN DAY=2.80 IN DT=0.0333 HR

COMPUTED 6-HOUR RAINFALL DISTRIBUTION BASED ON NOAA ATLAS 2 - PEAK / HOURS
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.0158 .0183 .0208 .0234 .0261 .0288 .0317
.0345 .0375 .0406 .0438 .0470 .0504 .0539
.0575 .0613 .0652 .0693 .0736 .0780 .0827
.0876 .0928 .0983 .1041 .1103 .1169 .1306
.1619 .2103 .2799 .3750 .5001 .6598 .8587
1.1016 1.3348 1.4304 1.5109 1.5824 1.6474 1.7073
1.7628 1.8147 1.8634 1.9091 1.9523 1.9931 2.0317
2.0683 2.1029 2.1358 2.1671 2.1967 2.2051 2.2118
2.2181 2.2241 2.2299 2.2355 2.2408 2.2460 2.2509
2.2557 2.2604 2.2649 2.2692 2.2735 2.2776 2.2816
2.2856 2.2894 2.2931 2.2968 2.3004 2.3039 2.3073
2.3107 2.3139 2.3172 2.3203 2.3235 2.3265 2.3295
2.3325 2.3354 2.3383 2.3411 2.3438 2.3466 2.3493
2.3519 2.3545 2.3571 2.3597 2.3622 2.3647 2.3671
2.3695 2.3719 2.3743 2.3766 2.3789 2.3812 2.3834

```

2.3857	2.3879	2.3900	2.3922	2.3943	2.3964	2.3985
2.4006	2.4026	2.4046	2.4066	2.4086	2.4106	2.4126
2.4145	2.4164	2.4183	2.4202	2.4220	2.4239	2.4257
2.4275	2.4293	2.4311	2.4329	2.4347	2.4364	2.4381
2.4398	2.4416	2.4432	2.4449	2.4466	2.4482	2.4499
2.4515	2.4531	2.4547	2.4563	2.4579	2.4595	2.4611
2.4626	2.4641	2.4657	2.4672	2.4687	2.4702	2.4717
2.4732	2.4747	2.4761	2.4776	2.4790	2.4805	2.4819
2.4833	2.4847	2.4861	2.4875	2.4889	2.4903	2.4917
2.4930	2.4944	2.4958	2.4971	2.4984	2.4998	

*S***** TRACT C-1-A - ONSITE DEVELOPED (2.4377 ACRES)

COMPUTE NM HYD ID=1 HYD NO=101 AREA=0.00379 SQ MI
 PER A=0 PER B=15 PER C=0 PER D=85
 TP=0.1333 HR MASS RAINFALL=-1

K = .072649HR TP = .133300HR K/TP RATIO = .545000 SHAPE CONSTANT, N = 7.106420
 UNIT PEAK = 12.719 CFS UNIT VOLUME = .9985 B = 526.28 P60 = 2.1000
 AREA = .003222 SQ MI IA = .10000 INCHES INF = .04000 INCHES PER HOUR
 RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT = .033300

PRINT HYD ID=1 CODE=24

PARTIAL HYDROGRAPH 101.00

FLOW	TIME	FLOW	TIME	FLOW	TIME	FLOW	TIME
CFS	HRS	CFS	HRS	CFS	HRS	CFS	HRS
.000	.0	1.998	2.6	3.996	.1	5.994	.1
.666	.0	2.664	.3	4.662	.1	6.660	.0
1.332	3.2	3.330	.1	5.328	.1		

RUNOFF VOLUME = 2.05227 INCHES = .4148 ACRE-FEET
 PEAK DISCHARGE RATE = 11.06 CFS AT 1.499 HOURS BASIN AREA = .0038 SQ. MI.

*S***** Cul-De-Sac - ONSITE DEVELOPED (0.6313 ACRES)

COMPUTE NM HYD ID=2 HYD NO=102 AREA=0.00098 SQ MI
PER A=0 PER B=15 PER C=0 PER D=85
TP=0.1333 HR MASS RAINFALL=-1

K = .072649HR TP = .133300HR K/TP RATIO = .545000 SHAPE CONSTANT, N = 7.106420
UNIT PEAK = 3.2887 CFS UNIT VOLUME = .9960 B = 526.28 P60 = 2.1000
AREA = .000833 SQ MI IA = .10000 INCHES INF = .04000 INCHES PER HOUR
RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT = .033300

K = .133173HR TP = .133300HR K/TP RATIO = .999050 SHAPE CONSTANT, N = 3.533693
UNIT PEAK = .35595 CFS UNIT VOLUME = .9629 B = 322.78 P60 = 2.1000
AREA = .000147 SQ MI IA = .50000 INCHES INF = 1.25000 INCHES PER HOUR
RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT = .033300

PRINT HYD ID=2 CODE=24

PARTIAL HYDROGRAPH 102.00

FLOW	TIME	FLOW	TIME	FLOW	TIME	FLOW	TIME	FLOW	TIME
CFS	HRS	CFS	HRS	CFS	HRS	CFS	HRS	CFS	HRS
.000	.0	1.332	.8	2.664	.1	3.996	.0	5.328	
.666	.0	1.998	.7	3.330	.0	4.662	.0	5.994	
.0									

RUNOFF VOLUME = 2.05227 INCHES
PEAK DISCHARGE RATE = 2.87 CFS AT 1.499 HOURS BASIN AREA = .0010 SQ. MI.

*S***** TRACT C-2-A - ONSITE DEVELOPED (8.2228 ACRES)

COMPUTE NM HYD ID=3 HYD NO=103 AREA=0.01266 SQ MI
 PER A=0 PER B=15 PER C=0 PER D=85
 TP=0.1333 HR MASS RAINFALL=-1

K = .072649HR TP = .133300HR K/TP RATIO = .545000 SHAPE CONSTANT, N = 7.106420
 UNIT PEAK = 42.485 CFS UNIT VOLUME = .9991 B = 526.28 P60 = 2.1000
 AREA = .010761 SQ MI IA = .10000 INCHES INF = .04000 INCHES PER HOUR
 RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT = .033300

K = .133173HR TP = .133300HR K/TP RATIO = .999050 SHAPE CONSTANT, N = 3.533693
 UNIT PEAK = 4.5983 CFS UNIT VOLUME = .9973 B = 322.78 P60 = 2.1000
 AREA = .001899 SQ MI IA = .50000 INCHES INF = 1.25000 INCHES PER HOUR
 RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT = .033300

PRINT HYD ID=3 CODE=24

PARTIAL HYDROGRAPH 103.00					
TIME	FLOW	TIME	FLOW	TIME	FLOW
HRS	CFS	HRS	CFS	HRS	CFS
.000	.0	1.998	8.8	3.996	.3
.666	.0	2.664	.9	4.662	.2
1.332	10.6	3.330	.3	5.328	.2

RUNOFF VOLUME = 2.05227 INCHES
 PEAK DISCHARGE RATE = 36.92 CFS AT 1.499 HOURS = 1.3857 ACRE-FEET
 BASIN AREA = .0127 SQ. MI.

*\$***** TRACT B-1 - ONSITE DEVELOPED (6.5111 ACRES)

COMPUTE NM HYD ID=4 HYD NO=104 AREA=0.01006 SQ MI
 PER A=0 PER B=15 PER C=0 PER D=85
 TP=0.1333 HR MASS RAINFALL=-1

K = .072649HR TP = .133300HR K/TP RATIO = .545000 SHAPE CONSTANT, N = 7.106420
 UNIT PEAK = 33.760 CFS UNIT VOLUME = .9990 B = 526.28 P60 = 2.1000
 AREA = .008551 SQ MI IA = .10000 INCHES INF = .04000 INCHES PER HOUR

RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT = .033300

K = .133173HR TP = .133300HR K/TP RATIO = .999050 SHAPE CONSTANT, N = 3.5333693
UNIT PEAK = 3.6540 CFS UNIT VOLUME = .9967 B = 322.78 P60 = 2.1000
AREA = .001509 SQ MI IA = .50000 INCHES INF = 1.25000 INCHES PER HOUR
RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT = .033300

PRINT HYD

ID=4 CODE=24

PARTIAL HYDROGRAPH 104.00

FLOW	TIME	FLOW	TIME	FLOW	TIME	FLOW	TIME	FLOW	TIME
CFS	HRS	CFS	HRS	CFS	HRS	CFS	HRS	CFS	HRS
.000	.0	1.998	7.0	3.996	.2	5.994	.2		
.666	.0	2.664	.7	4.662	.2	6.660	.0		
1.332	8.4	3.330	.3	5.328	.2				

RUNOFF VOLUME = 2.05227 INCHES = 1.1011 ACRE-FEET
PEAK DISCHARGE RATE = 29.34 CFS AT 1.499 HOURS BASIN AREA = .0101 SQ. MI.

*S***** TRACT C-2-B - ONSITE DEVELOPED (5.00 ACRES)

COMPUTE NM HYD ID=5 HYD NO=105 AREA=0.00777 SQ MI
PER A=0 PER B=15 PER C=0 PER D=85
TP=0.1333 HR MASS RAINFALL=-1

K = .072649HR TP = .133300HR K/TP RATIO = .5450000 SHAPE CONSTANT, N = 7.106420
UNIT PEAK = 26.075 CFS UNIT VOLUME = .9989 B = 526.28 P60 = 2.1000
AREA = .006605 SQ MI IA = .10000 INCHES INF = .04000 INCHES PER HOUR
RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT = .033300

K = .133173HR TP = .133300HR K/TP RATIO = .999050 SHAPE CONSTANT, N = 3.5333693
UNIT PEAK = 2.8222 CFS UNIT VOLUME = .9953 B = 322.78 P60 = 2.1000
AREA = .001166 SQ MI IA = .50000 INCHES INF = 1.25000 INCHES PER HOUR
RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT = .033300

PRINT HYD

ID=5 CODE=24

PARTIAL HYDROGRAPH

105.00

FLOW CFS	TIME HRS	FLOW CFS	TIME HRS	FLOW CFS	TIME HRS	FLOW CFS	TIME HRS
.000	.0	1.998	.4	3.996	.2	5.994	.2
.666	.0	2.664	.5	4.662	.1	6.660	.0
1.332	6.5	3.330	.2	5.328	.2		

RUNOFF VOLUME = 2.05227 INCHES = .8505 ACRE-FEET
PEAK DISCHARGE RATE = 22.67 CFS AT 1.499 HOURS BASIN AREA = .0078 SQ. MI.

FINISH

NORMAL PROGRAM FINISH

END TIME (HR:MIN:SEC) = 15:13:38

dmg

D. Mark Goodwin & Associates, P.A.
Consulting Engineers
P.O. BOX 90606, ALBUQUERQUE, NM 87199
(505) 828-2200 FAX 797-9539
e-mail: goodwinengrs@comcast.net

PROJECT BALLOON FIESTA PARK
SUBJECT STREET & INLET CAPACITIES
BY Pavam DATE Mar 1 '06
CHECKED _____ DATE _____
SHEET 1 OF 1

FROM APPROVED REPORT FROM CITI CORP DRAINAGE (96177 A.2417)

We have 19.3 CFS on Balloon Fiesta Park.

This 19.3 CFS IS INTERCEPTED BY SERIES OF INLETS
ON BALLOON FIESTA AT INTERSECTION OF SAN MATEO AND
BALLOON FIESTA.

The developed flow from proposed cul-de-sac at Farm
(TRACT C-1-A)
Credit is 13.93 CFS.

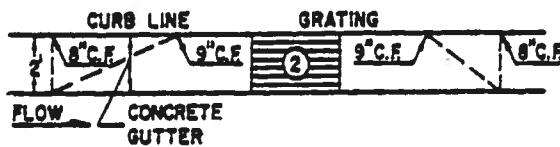
Existing
Capacity of Catch Basin on North side of the Crown
on Balloon Fiesta = 25.2 CFS. (2-TYPE A Inlets.
(slope = 1.978% ; D = 0.55') Existing on North side)

Required Capacity = Half of 19.3 CFS + 13.93 CFS + 9.2 CFS
(cul-de-sac + from TRACT C-2-A)
= 9.2 + 9.65 + 13.93 = 32.78 CFS.

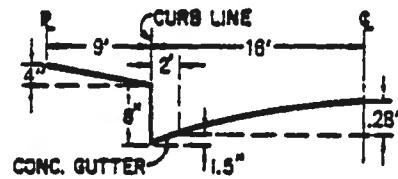
ADDITION CAPACITY OF 7.58 CFS is provided by
adding Storm Inlet "TYPE A", ON DOWNSTREAM SIDE
AT THE INTERSECTION.

By adding one 'TYPE A' Inlet the capacity
increase by 33.61 CFS > 32.78 CFS (Req) \approx 0 k
24" or 3300 cu ft

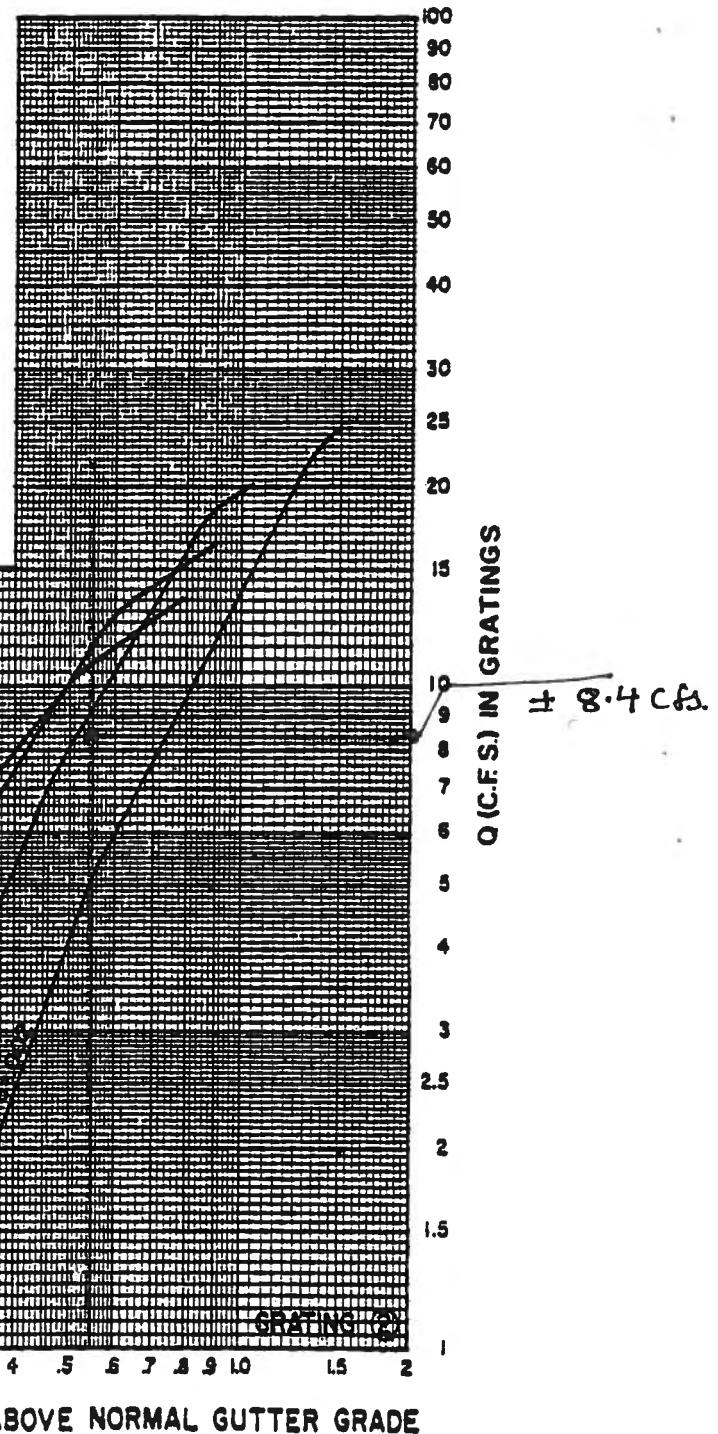
GRATING CAPACITIES FOR TYPE 'A' , 'C' and "D"



GRATING & GUTTER PLAN



TYPICAL HALF STREET SECTION
(ABOVE BASIN)



APPENDIX B

GRADING AND DRAINAGE PLAN