CITY OF ALBUQUERQUE

July 10, 2017

Diane Hoelzer, P.E.
Mark Goodwin & Associates
PO Box 90606
Albuquerque, NM, 87199

RE: Glendesto Subdivision
Grading Plan and Drainage Report
Engineers Stamp Date: 7/7/17
Hydrology File: B18D020

Dear Ms. Hoelzer:

Based on the information provided in your submittal received on 7/7/17, the Grading Plan and Drainage Report are approved for Grading Permit and Amended Preliminary Plat.

If you have any questions, please contact me at 924-3695 or dpeterson@cabq.gov.

Sincerely,

Dana Peterson, P.E.
Senior Engineer, Planning Dept.
Development Review Services

PO Box 1293

Albuquerque

New Mexico 87103

www.cabq.gov

Orig: Drainage File

City of Albuquerque

Planning Department

Development & Building Services Division

DRAINAGE AND TRANSPORTATION INFORMATION SHEET (REV 09/2015)

Project Title: Glendesto Subdivision Building Permit #: _____ City Drainage #: B18 / D020

DRB#: 1004472 EPC#: _____ Work Order#: _____

Legal Description: Lot 8-A, Block 25, Tract A, Unit B, N.A.A.

City Address: Glendale Avenue

Engineering Firm: Mark Goodwin and Associates, PA Contact: William Travis Barr

Address: PO Box 90606, Abq, 87199

Phone#: 505-828-2200 Fax#: _____ E-mail: Travis@goodwinengineers.com

Owner: Scott Clark (Managing member) Contact: Scott Clrak

Address: _____

Phone#: 883-1414 Fax#: _____ E-mail: scottbrockclark@aol.com

Architect: NA Contact: _____

Address: _____

Phone#: _____ Fax#: _____ E-mail: _____

Other Contact: Mark Goodwin and Associates, PA Contact: Diane Hoelzer, PE

Address: PO Box 90606, Abq, 87199

Phone#: 505-828-2200 Fax#: _____ E-mail: Diane@goodwinengineers.com

Check all that Apply:

DEPARTMENT:

x HYDROLOGY/ DRAINAGE

_____ TRAFFIC/ TRANSPORTATION

_____ MS4/ EROSION & SEDIMENT CONTROL

TYPE OF SUBMITTAL:

_____ ENGINEER/ ARCHITECT CERTIFICATION

_____ CONCEPTUAL G & D PLAN

X GRADING PLAN

_____ DRAINAGE MASTER PLAN

X DRAINAGE REPORT

_____ CLOMR/LOMR

_____ TRAFFIC CIRCULATION LAYOUT (TCL)

_____ TRAFFIC IMPACT STUDY (TIS)

_____ EROSION & SEDIMENT CONTROL PLAN (ESC)

_____ OTHER (SPECIFY) _____

CHECK TYPE OF APPROVAL/ACCEPTANCE SOUGHT:

_____ BUILDING PERMIT APPROVAL

_____ CERTIFICATE OF OCCUPANCY

_____ PRELIMINARY PLAT APPROVAL

_____ SITE PLAN FOR SUB'D APPROVAL

_____ SITE PLAN FOR BLDG. PERMIT APPROVAL

_____ FINAL PLAT APPROVAL

_____ SIA/ RELEASE OF FINANCIAL GUARANTEE

_____ FOUNDATION PERMIT APPROVAL

X GRADING PERMIT APPROVAL

_____ SO-19 APPROVAL

_____ PAVING PERMIT APPROVAL

_____ GRADING/ PAD CERTIFICATION

_____ WORK ORDER APPROVAL

_____ CLOMR/LOMR

_____ PRE-DESIGN MEETING

x OTHER (SPECIFY) Amended Preliminary Plat Approval

IS THIS A RESUBMITTAL?: x Yes _____ No

DATE SUBMITTED: July 5, 2017 By: William Travis Barr, EI

COA STAFF: ELECTRONIC SUBMITTAL RECEIVED: ___

Drainage Management Plan
for
Glendesto Subdivision
(aka Tierra Serena Subdivision)
(27 lots)

Prepared by
Mark Goodwin & Associates, P.A.

July 2017

City of Albuquerque

Planning Department

Development & Building Services Division

DRAINAGE AND TRANSPORTATION INFORMATION SHEET (REV 09/2015)

Project Title: Glendesto Subdivision Building Permit #: _____ City Drainage #: B18 / D020

DRB#: 1004472 EPC#: _____ Work Order#: _____

Legal Description: Lot 8-A, Block 25, Tract A, Unit B, N.A.A.

City Address: Glendale Avenue

Engineering Firm: Mark Goodwin and Associates, PA Contact: William Travis Barr

Address: PO Box 90606, Abq, 87199

Phone#: 505-828-2200 Fax#: _____ E-mail: Travis@goodwinengineers.com

Owner: Scott Clark (Managing member) Contact: Scott Clrak

Address: _____

Phone#: 883-1414 Fax#: _____ E-mail: scottbrockclark@aol.com

Architect: NA Contact: _____

Address: _____

Phone#: _____ Fax#: _____ E-mail: _____

Other Contact: Mark Goodwin and Associates, PA Contact: Diane Hoelzer, PE

Address: PO Box 90606, Abq, 87199

Phone#: 505-828-2200 Fax#: _____ E-mail: Diane@goodwinengineers.com

Check all that Apply:

DEPARTMENT:

x HYDROLOGY/ DRAINAGE
_____ TRAFFIC/ TRANSPORTATION
_____ MS4/ EROSION & SEDIMENT CONTROL

TYPE OF SUBMITTAL:

_____ ENGINEER/ ARCHITECT CERTIFICATION

_____ CONCEPTUAL G & D PLAN
X GRADING PLAN
_____ DRAINAGE MASTER PLAN
X DRAINAGE REPORT
_____ CLOMR/LOMR

_____ TRAFFIC CIRCULATION LAYOUT (TCL)
_____ TRAFFIC IMPACT STUDY (TIS)
_____ EROSION & SEDIMENT CONTROL PLAN (ESC)

_____ OTHER (SPECIFY) _____

IS THIS A RESUBMITTAL?: x Yes _____ No

CHECK TYPE OF APPROVAL/ACCEPTANCE SOUGHT:

_____ BUILDING PERMIT APPROVAL
_____ CERTIFICATE OF OCCUPANCY

_____ PRELIMINARY PLAT APPROVAL
_____ SITE PLAN FOR SUB'D APPROVAL
_____ SITE PLAN FOR BLDG. PERMIT APPROVAL
_____ FINAL PLAT APPROVAL
_____ SIA/ RELEASE OF FINANCIAL GUARANTEE
_____ FOUNDATION PERMIT APPROVAL
X GRADING PERMIT APPROVAL
_____ SO-19 APPROVAL
_____ PAVING PERMIT APPROVAL
_____ GRADING/ PAD CERTIFICATION
_____ WORK ORDER APPROVAL
_____ CLOMR/LOMR

_____ PRE-DESIGN MEETING
x OTHER (SPECIFY) Amended Preliminary Plat Approval

DATE SUBMITTED: July 5, 2017 By: William Travis Barr, EI

COA STAFF: ELECTRONIC SUBMITTAL RECEIVED: ___

D. Mark Goodwin & Associates, P.A.
Consulting Engineers
P.O. BOX 90606, ALBUQUERQUE, NM 87199
(505) 828-2200 FAX 797-9539

~ 2012 ACEC/NM Award Winner for Engineering Excellence ~
~ 2008 ACEC/NM Award Winner for Engineering Excellence ~

July 7, 2017

Mr. Jack Cloud
DRB Chairman
City of Albuquerque
PO Box 1293
Albuquerque, NM 87102

Re: Glendesto Subdivision – (DRB 1004472)
Amended Preliminary Plat, Grading and Drainage Plan, Sidewalk Variance, Sidewalk Waiver and Deferral.

Dear Mr. Cloud,

In response to DRB comments two weeks ago, the revised Amended preliminary plat, grading plan and infrastructure list, sidewalk deferral, waiver, variance exhibits is being submitted.

The following is a summary of the revisions:

1. The site was lowered to reduce retaining wall height in the southwest corner,
2. A 16' wide drainage channel was placed down the middle of the 30' wide easement,
3. The drainage report was revised to address the Hydrology Dept. comments,
4. A sidewalk design variance is being requested along the south side of the hammerhead street. The Transportation Engineer requested sidewalk on both sides of the hammerhead to create sidewalk continuity within the development. We are asking for a variance along the south side of the hammerhead adjacent to Lots 27 and 7 because if the landscape easement is added there will not be enough width within the lot for the home product planned for this development. The lot depths adjacent to lots 7 and 8 along Yawkey Way are already at a minimum depth of 105 feet.
5. A sidewalk waiver along lot 24 is being requested as well.

Should you have any questions or concerns, please don't hesitate to contact myself at or 505-828-2200.

Sincerely,
MARK GOODWIN & ASSOCIATES, PA

Diane Hoelzer, PE
Senior Engineer

DLH/dlh

CITY OF ALBUQUERQUE

June 21, 2017

Diane Hoelzer, P.E.
Mark Goodwin & Associates
PO Box 90606
Albuquerque, NM, 87199

RE: Glendesto Subdivision
Grading Plan and Drainage Report
Engineers Stamp Date: 6/15/17
Hydrology File: B18D020

Dear Ms. Hoelzer:

Based on the information provided in your submittal received on 6/16/17, the Grading Plan and Drainage Report are not approved for Grading Permit or Preliminary Plat. The following comments must be addressed prior to approval:

1. The proposed channel on Tract A needs to be designed to accommodate 1.0' freeboard according to Ch. 22 of the DPM. Also the channel top width in the drainage report is 9' wide, but the top width shown on the grading plan is only 5' wide.

2. What happens where the Tract A channel crosses the sidewalks on Glendale and at the hammerhead? It seems like arrays of sidewalk culverts need to be sized and designed here and included on the Infrastructure List.

3. The Glendale roadside channel designed in the drainage report does not match the channel shown on the plans.

4. What happened to the double retaining wall along the west side of lot 23, formerly lot 8? The single retaining wall topped with a garden wall will exceed the 8' maximum and the double wall may infringe on the 5' setback for the pad.

5. Recheck the pad elevations. Many pads have enough elevation difference to require retaining walls in between lots such as: lot 21, and lot 24/25.

6. A revised ESC plan is required prior to Grading Permit approval.

PO Box 1293

Albuquerque

New Mexico 87103

www.cabq.gov

Orig: Drainage File

CITY OF ALBUQUERQUE

If you have any questions, please contact me at 924-3695 or dpeterson@cabq.gov.

Sincerely,

Dana Peterson, P.E.
Senior Engineer, Planning Dept.
Development Review Services

PO Box 1293

Albuquerque

New Mexico 87103

www.cabq.gov

Orig. Drainage File

GLENDESTO SUBDIVISION

Table of Contents

I. PROJECT DESCRIPTION

The Glendesto subdivision project site covers an area of approximately 7.46 acres. This project encompasses grading and infrastructure construction that will support the development of 27 single family residential lots within a gated community.

The project site is bounded to the south by the Eagle Rock Business Park and a parking lot with a 30 foot wide vacant strip separating the two properties; to the west by the Sandia Memorial Gardens; to the north is Glendale Avenue; to the east is Modesto Avenue and a vacant acre parcel owned by the Sandia Indian Pueblo.

The proposed subdivision will connect to Modesto Avenue to the east through a gate.

II. DESIGN CRITERIA AND PREVIOUS DEVELOPMENT

The design criteria used in this report was in accordance with Section 22.2 Hydrology of the Development Process Manual, Volume 2, Design Criteria, January 1993 edition. The 100-year 24-hour storm event was analyzed to determine street capacities and sizing of the storm drain system using P(1 hr)=2.10", P(6 hr)=2.50" and the P(24 hr)=2.85". The onsite Land Treatment values used were based on Table A-5, in the DPM. (Refer to Exhibit 1Vicinity Map and Exhibit 2 Existing Drainage Conditions- google earth).

The site lies in Zone X as shown on Firm Panel 129, map number 3500120129H, revised August 16, 2012.

This project site lies within the ***North Albuquerque Acres Master Drainage Plan Report*** *(RTI, Inc., October 1998). Excerpts from this report can be found in Appendix C. Under existing drainage conditions, this property is split between the south third (sub basin 116.0) flowing to the west and the northern two-thirds (sub basin 115.0) flowing to the north and west as shown in Figure 3A. In Figure 4A Future Conditions, the south three acres are to flow to the west and the north two acres are to flow down Glendale. Since there isn't a conveyance system in place to intercept any of the runoff from our site, it was decided to take the entire runoff from our project site to Glendale.*

According to Figure 5A, there is a planned future storm drain in San Pedro extending from Glendale to the La Cueva arroyo channel. At this time, there is only a 6' manhole in San Pedro immediately south of the La Cueva channel that connects to an inlet on the east side of San Pedro which intercepts existing flows from the remnant natural La Cueva arroyo.

III. EXISTING DRAINAGE CONDITIONS

Under existing drainage conditions, runoff from the project site is in a general westerly direction. Refer to "Existing Drainage Conditions" Exhibit. Runoff from the north third flows to Glendale through two incised arroyos. The middle third appears to sheet flow directly to the west to the Sandia Memorial Gardens Cemetery. Runoff from the south third flows through the Cemetery through an incised arroyo where it continues across San Pedro and westward.

Runoff from a portion of Modesto Avenue continues in a westerly direction down the same southerly arroyo through the Cemetery.

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IV. DEVELOPED DRAINAGE CONDITIONS

Under developed drainage conditions, runoff from the project site will discharge into Glendale Avenue and be intercepted by a hardlined swale running along the north side of Glendale. Runoff in the swale will be intercepted by a proposed Double D inlet that will connect to a new 36" RCP storm drain at the San Pedro intersection. A new 36" storm drain in San Pedro will continue north and tie into the existing storm manhole in San Pedro. Refer to the "Proposed Drainage Conditions" Exhibit. The swale is designed to convey flows from Glendale and the project site.

1) *A portion of the runoff from "N-S" Modesto Avenue will flow through the existing apron rundown on the La Cueva channel right where Modesto turns west again and becomes Glendale Avenue.*
2) *The south portion of the flows from "W-E" Modesto Avenue will continue westward in the existing historical flow path through the 30 foot wide easement area.*
3) *Flows from the vacant Lot 10, immediately east of our project site will continue in the same general flow path to Glendale Avenue.*
4) *The 30' wide vacant area immediately south of this project site will be regraded to continue runoff in the same historic flow path as before.*

V. FIRST FLUSH

First flush for the project site will be handled on each individual residential lot. The area between the back of curb and sidewalk will be depressed 6 inches and the front yards will also be depressed by 6" as shown in the standard detail on the grading and drainage plan. First flush calculations can be found in Appendix A Hydrology.

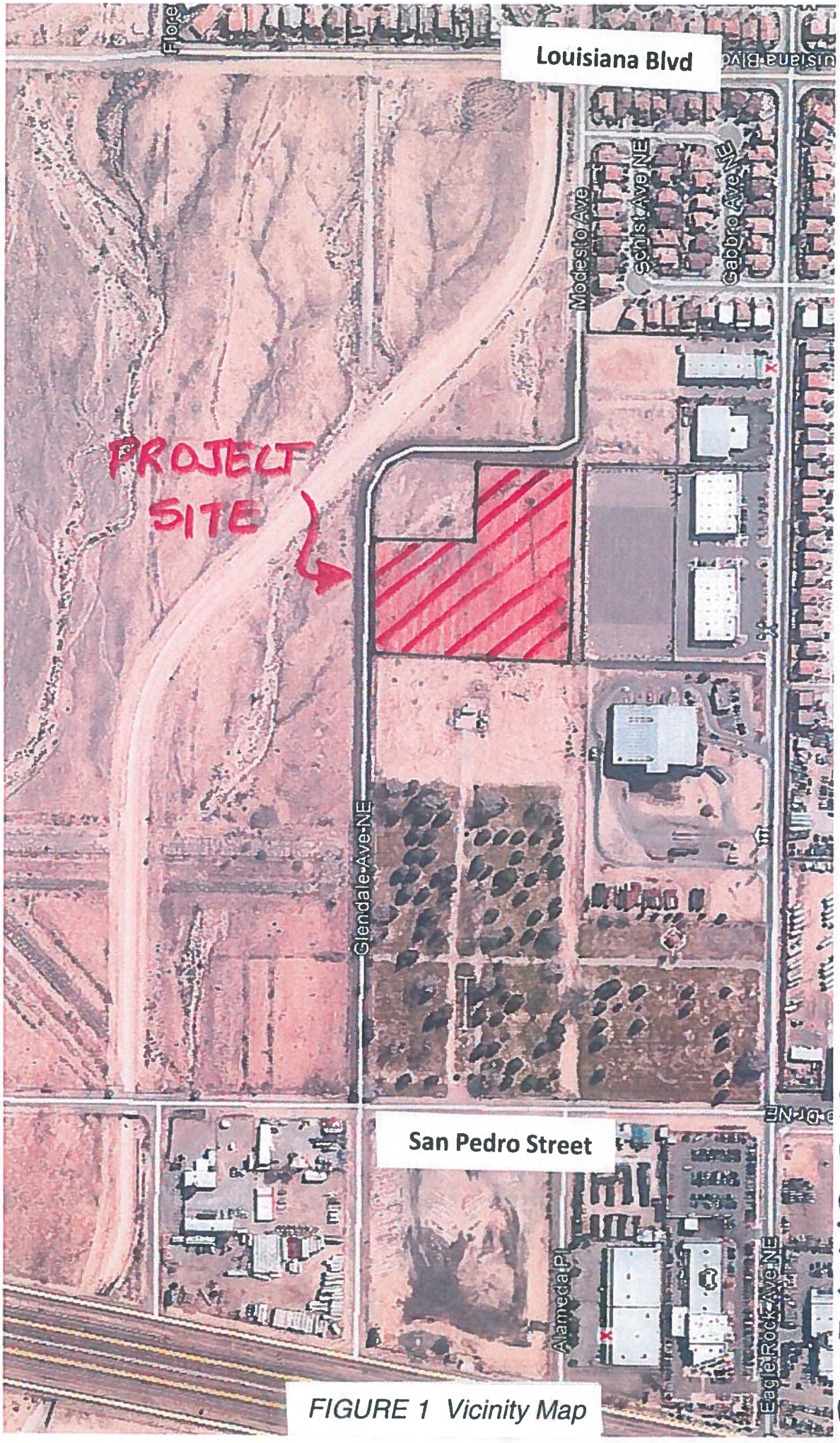


FIGURE 1 Vicinity Map

ZONE ATLAS B-18

FIGURE 2 Existing Drainage Conditions

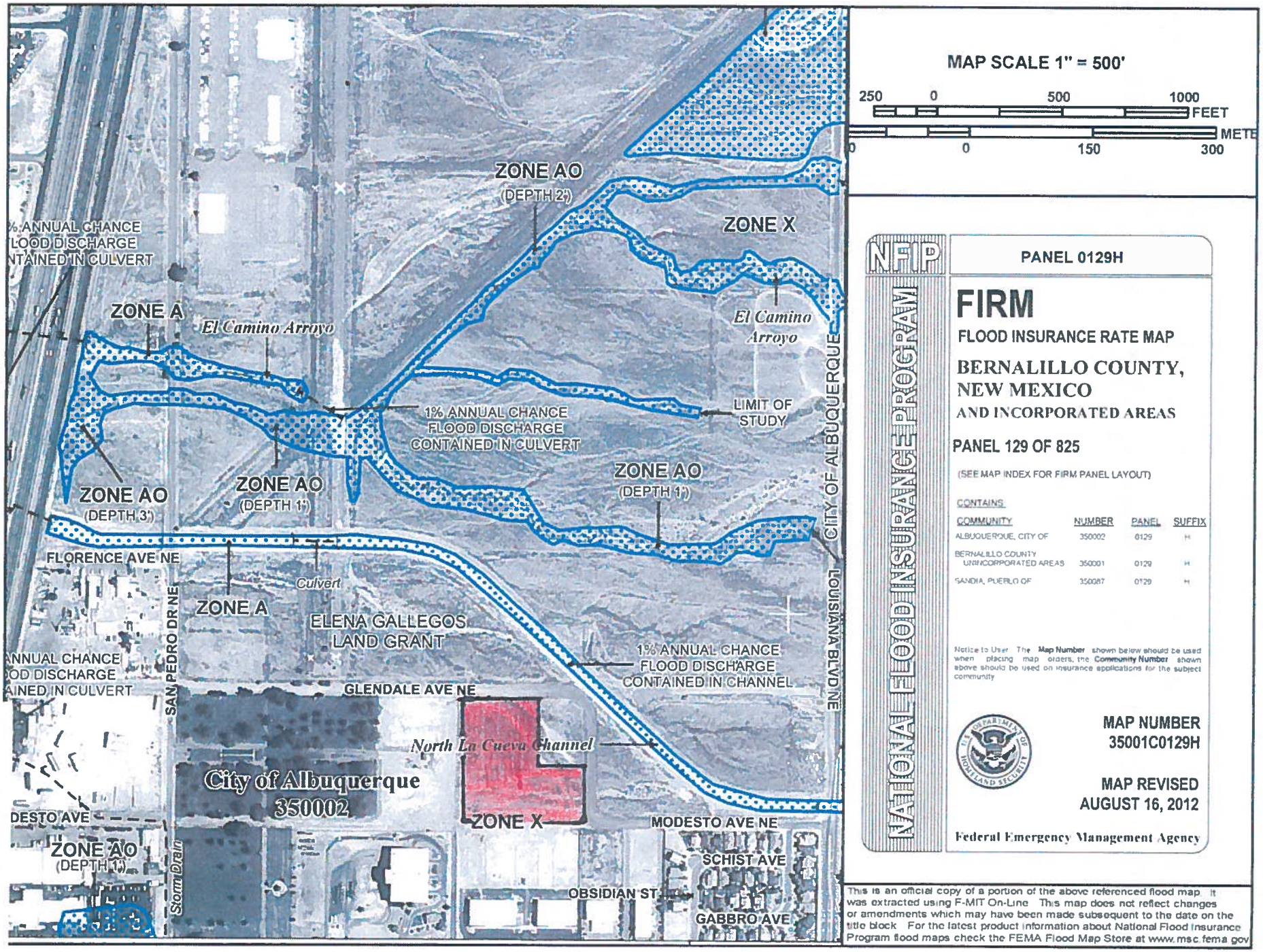


FIGURE 3 FEMA Map

Current DRC
Project Number:_____

FIGURE 12

INFRASTRUCTURE LIST

EXHIBIT "A"

TO SUBDIVISION IMPROVEMENTS AGREEMENT

DEVELOPMENT REVIEW BOARD (D.R.B.) REQUIRED INFRASTRUCTURE LIST

Date Submitted: July 7, 2017
Date Site Plan Approved:_____
Date Preliminary Plat Approved:_____
Date Preliminary Plat Expires:_____
DRB Project No.: 1004472
DRB Application No.: 17DRB -70169

Glendesto Subdivision

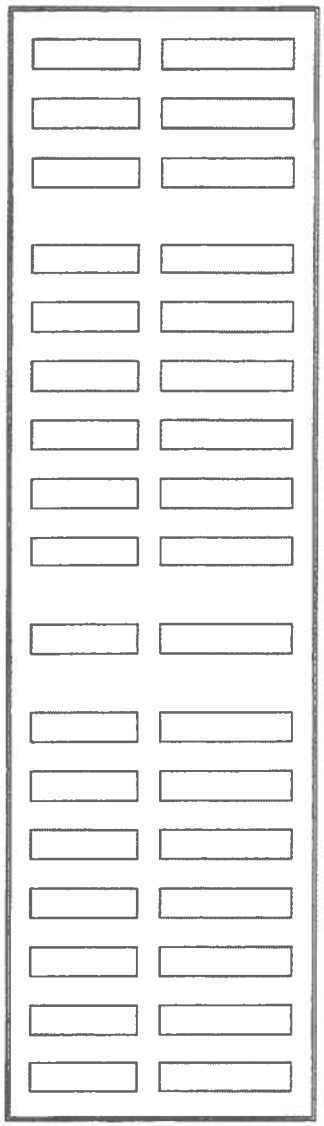
PROPOSED NAME OF PLAT AND/OR SITE DEVELOPMENT PLAN

Lot 8-A, Block 25, Tract A, Tract B, N.A.A.

EXISTING LEGAL DESCRIPTION PRIOR TO PLATTING ACTION

Following is a summary of PUBLIC/PRIVATE Infrastructure required to be constructed or financially guaranteed for the above development. This Listing is not necessarily a complete listing. During the SIA process and/or in the review of the construction drawings, if the DRC Chair determines that appurtenant items and/or unforeseen items have not been included in the infrastructure listing, the DRC Chair may include those items in the listing and related financial guarantee. Likewise, if the DRC Chair determines that appurtenant or non-essential items can be deleted from the listing, those items may be deleted as well as the related portions of the financial guarantees. All such revisions require approval by the DRC Chair, the User Department and agent/owner. If such approvals are obtained, these revisions to the listing will be incorporated administratively. In addition, any unforeseen items which arise during construction which are necessary to complete the project and which normally are the Subdivider's responsibility will be required as a condition of project acceptance and close out by the City.

SIA Sequence #	COA DRC Project #	Size	Type of Improvement	Location	From	To	Private Inspector	City Inspector	City Cnst Engineer
			PAVING						
		26' F-F	Res Pvmt	Lansdowne Place N.E.	Hammerhead	Yawkey Way N.E.	/	/	/
			G&G (both sides)						
		4'	Sidewalk (westside) Deferred	Lansdowne Place N.E.	Hammerhead	Yawkey Way N.E.	/	/	/
		4'	Sidewalk (eastside) Deferred	Lansdowne Place N.E.	Hammerhead	Lot 9	/	/	/
		4'	Sidewalk (eastside)	Lansdowne Place N.E.	Lot 9	Yawkey Way N.E.	/	/	/
		26' F-F	Res Pvmt	Yawkey Way N.E.	Lot 23	Modesto Ave. N.E.	/	/	/
			C&G (both sides)						
		4'	Sidewalk (southside) Deferred	Yawkey Way N.E.	Lot 23	Modesto Ave. N.E.	/	/	/
		4'	Sidewalk (northside) Deferred	Yawkey Way N.E.	Lansdowne Place N.E.	Modesto Ave. N.E.	/	/	/
		4'	Sidewalk (northside)	Yawkey Way N.E.	Lansdowne Place N.E.	Lot 23	/	/	/
		4'	Sidewalk (southside)	Hammerhead	Lot 1	Lot 3	/	/	/
		4'	Sidewalk (southside)	Hammerhead	Lot 4	Lot 6	/	/	/
		4'	Crusher Fine Trail (westside)	Tract A	Hammerhead	Glendale Ave.	/	/	/
		20' F-F	Res Pvmt	Hammerhead	Lot 1	Lot 6	/	/	/
			C&G (both sides)						
		4'	Sidewalk (northside) Deferred	Hammerhead	Lot 1	Lot 3	/	/	/
		4'	Sidewalk (northside)	Hammerhead	Lot 3	Lot 4	/	/	/
		4'	Sidewalk (northside) Deferred	Hammerhead	Lot 4	Lot 6	/	/	/
			OFF-SITE PAVING						
		24' F-E	Perm Pvmt	Glendale Ave.	W. Lot 1 Property Line	E. Lot 6 Property Line	/	/	/
			C&G (southside)						
		6'	Sidewalk (southside)	Glendale Ave.	W. Lot 1 Property Line	E. Lot 6 Property Line	/	/	/



24' F-E	Perm Pvmt C&G (westside)	Modesto Ave.	N. Lot 14	S. Lot 15	/ / /
6'	Sidewalk (westside)	Modesto Ave.	N. Lot 14	S. Lot 15	/ / /
24' F-E	Temp Pvmt	Modesto Ave.	S. Lot 15	24' Existing Pvmt	/ / /
	WATER				
8"	Waterline	Glendale Ave.	Exist 12" WL San Pedro	E. Lot 6 Prop Line	/ / /
8"	Waterline	Lansdowne Place N.E.	Glendale Ave.	Yawkey Way N.E.	/ / /
6"	Waterline	Hammerhead	Lot 1	Lot 6	/ / /
8"	Waterline	Yawkey Way N.E.	Lansdowne Place N.E.	Modesto Ave.	/ / /
4"	Waterline	Yawkey Way N.E.	Lot 23	Lansdowne Place N.E.	/ / /
8"	Waterline	Modesto Ave.	Yawkey Way N.E.	Exist 8" WL Modesto Ave. (near SE corner Lot 15)	/ / /
	SANITARY SEWER				
8"	SAS	Lansdowne Place N.E.	Glendale Av. Exist 18" SAS	Yawkey Way N.E.	/ / /
8"	SAS	Hammerhead	Lot 1	Lot 6	/ / /
8"	SAS	Yawkey Way N.E.	Lot 23	Modesto Ave	/ / /
8"	SAS	Modesto Ave.	Yawkey Way N.E.	South PL Lot 15	/ / /
	DRAINAGE				
Per Design	Roadside Swale - Shotcrete	Glendale Ave.	San Pedro Ave.	East PL Lot 6	/ / /
16'	Channel - Concrete	Tract A	Hammerhead	Glendale Ave.	/ / /
36" RCP	Storm Drain	San Pedro Ave.	Glendale Ave.	Existing SD Stub near La Cueva Channel	/ / /
Dble D Inlet	Storm Drain	Glendale Ave.	At San Pedro Ave.		/ / /

The items listed below are on the CCIP and approved for Impact Fee credits. Signatures from the Impact Fee Administrator and the City User Department is required prior to DRB approval

Financially Guaranteed DRC #	Constructed Under DRC #	Size	Type of Improvement	Location	From	To

Construction Certification

Private Inspector P.E.		City Cnst Engineer
/	/	/
/	/	/

Approval of Creditable Items:

Impact Fee Admistrator Signature Date

Approval of Creditable Items:

City User Dept. Signature Date

1. Sidewalks to be Deferred per Exhibit
2. Street Lights Per DPM
3. Water Infrastructure includes Valves, Fittings, Valve Boxes, Fire Hydrants, and Appurtenances.
4. Sanitary Sewer includes manholes and service connection to property line
5. Grading & Drainage certification per DPM for releast of SIA & Financial Guaranty's. Financial Guaranty's are not required for grading.

AGENT / OWNER

Diane Hoelzer, P.E.
NAME (print)

MARK GOODWIN & ASSOCIATES
FIRM

7-6-17
SIGNATURE - date

MAXIMUM TIME ALLOWED TO CONSTRUCT THE IMPROVEMENTS WITHOUT A DRB EXTENSION: N/A

DEVELOPMENT REVIEW BOARD MEMBER APPROVALS

DRB CHAIR - date

PARKS & GENERAL SERVICES - date

TRANSPORTATION DEVELOPMENT - date

AMAFCA - date

UTILITY DEVELOPMENT - date

_____ - date

CITY ENGINEER - date

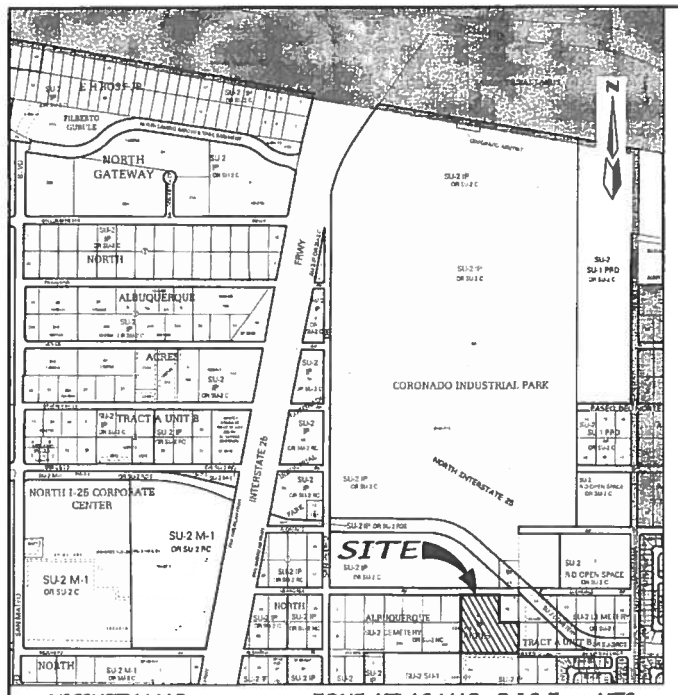
_____ - date

DESIGN REVIEW COMMITTEE REVISIONS

REVISION	DATE	DRC CHAIR	USER DEPARTMENT	AGENT /OWNER

AMENDED PRELIMINARY PLAT FOR GLENDESTO SUBDIVISION

WITHIN THE
ELENA GALLEGOS GRANT
PROJECTED SECTION 12
TOWNSHIP 11 NORTH, RANGE 3 EAST, NMPM
CITY OF ALBUQUERQUE
BERNALILLO COUNTY, NEW MEXICO
JUNE, 2017



VICINITY MAP ZONE ATLAS MAP: B-18-Z NTS

PURPOSE OF PLAT

1. SUBDIVIDE LOT 8-A, BLOCK 25, TRACT A, UNIT B, NORTH ALBUQUERQUE ACRES. INTO 27 RESIDENTIAL LOTS AND 1 TRACT.
2. GRANT NEW EASEMENTS AS SHOWN.

NOTES

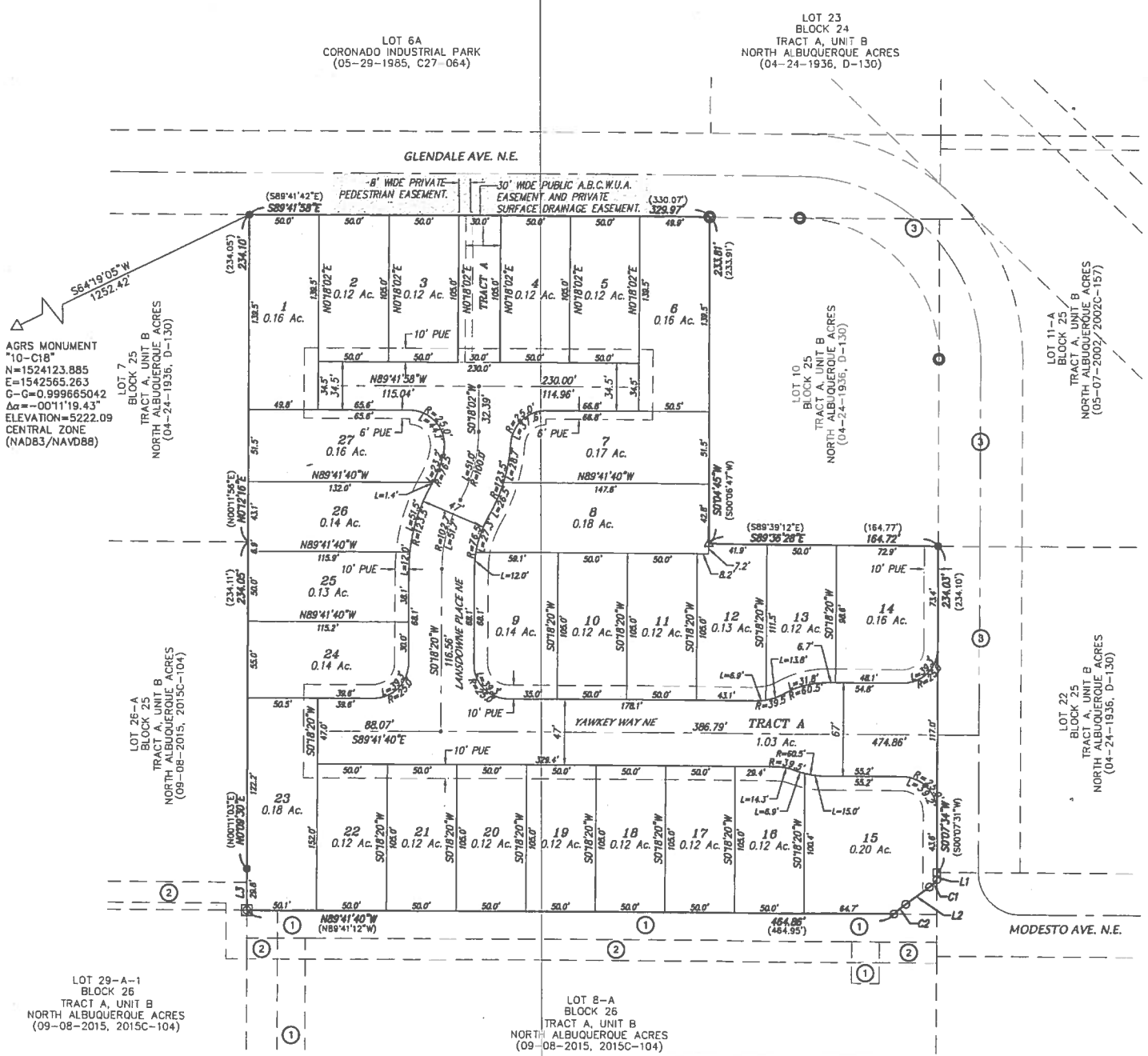
1. BEARINGS ARE GRID BASED ON THE NEW MEXICO STATE PLANE COORDINATE SYSTEM (CENTRAL ZONE).
2. ALL DISTANCES ARE GROUND DISTANCES.
3. BEARINGS AND DISTANCES IN PARENTHESIS ARE RECORD.
4. BASIS OF BOUNDARY ARE THE FOLLOWING PLATS OF RECORD ENTITLED:

   "NORTH ALBUQUERQUE ACRES, TRACT A, UNIT B", (09-08-2015, 2015C-104)
   "NORTH ALBUQUERQUE ACRES, TRACT A, UNIT B", (04-23-2012, 2012C-045)
   "NORTH ALBUQUERQUE ACRES, TRACT A, UNIT B", (05-07-2002, 2002C-157)
   "NORTH ALBUQUERQUE ACRES, TRACT A, UNIT B", (02-12-1993, 93C-038)
   "NORTH ALBUQUERQUE ACRES, TRACT A, UNIT B", (04-24-1936, D-130)

5. FIELD SURVEY PERFORMED IN MAY, 2016.
6. CITY OF ALBUQUERQUE, NEW MEXICO ZONE: SU-2.
7. 100 YEAR FLOOD ZONE DESIGNATION: ZONE X, AS SHOWN ON PANEL 129 OF 825, FLOOD INSURANCE RATE MAP, CITY OF ALBUQUERQUE, BERNALILLO COUNTY, NEW MEXICO, DATED AUGUST 16, 2012. THIS PROPERTY DOES NOT LIE IN THE 100 YEAR FLOOD ZONE.
8. TITLE REPORT: NONE PROVIDED.
9. MANHOLES WILL BE OFFSET AT ALL POINTS OF CURVATURE, POINTS OF TANGENCY, STREET INTERSECTIONS, AND ALL OTHER ANGLE POINTS TO ALLOW USE OF CENTERLINE MONUMENTATION.

LINE	BEARING	DISTANCE
L1	S 00°05'03" W	5.12'
L2	S 53°40'01" W	20.86'
L3	N 00°12'49" E	29.85'

CURVE	ARC LENGTH	RADIUS	DELTA ANGLE	CHORD BEARING	CHORD LENGTH
C1	7.50'	25.00'	17°10'40"	S 45°04'41" W	7.47'
C2	11.03'	96.00'	06°34'57"	S 50°22'33" W	11.02'



TRACT A IS A PRIVATE ROADWAY AND DRAINAGE EASEMENT FOR THE BENEFIT OF THE HOMEOWNERS TO BE MAINTAINED BY THE H.O.A. AND A PUBLIC WATER AND SANITARY SEWER EASEMENT GRANTED TO THE ALBUQUERQUE BERNALILLO COUNTY WATER AUTHORITY.

LEGAL DESCRIPTION

A TRACT OF LAND SITUATE WITHIN THE ELENA GALLEGO GRANT, PROJECTED SECTION 12, TOWNSHIP 11 NORTH, RANGE 3 EAST, NEW MEXICO PRINCIPAL MERIDIAN, CITY OF ALBUQUERQUE, BERNALILLO COUNTY, NEW MEXICO, BEING ALL OF LOT 8-A, BLOCK 25, TRACT A, UNIT B, NORTH ALBUQUERQUE ACRES, AS THE SAME IS SHOWN AND DESIGNATED ON SAID PLAT, FILED FOR RECORD IN THE OFFICE OF THE COUNTY CLERK OF BERNALILLO COUNTY, NEW MEXICO, ON SEPTEMBER 8, 2015, IN PLAT BOOK 2015C, PAGE 104, AND CONTAINING 4.7676 ACRES MORE OR LESS.

SUBDIVISION DATA

GROSS ACREAGE	4.7676 AC
ZONE ATLAS NO.	B-18-Z
NO. OF EXISTING TRACTS/LOTS	1 TRACT/0 LOTS
NO. OF TRACTS CREATED	1 TRACT
NO. OF LOTS CREATED	27 LOTS
DATE OF SURVEY	MAY, 2016

Scott B. Clark managing member 6/13/2017
SCOTT B. CLARK, MANAGING PARTNER (40% OWNER) DATE
CCG PARTNERSHIP, A NEW MEXICO PARTNERSHIP

Scott B. Clark 6/13/2017
SCOTT B. CLARK (40% OWNER) DATE

Pamela M. Clark by Scott B. Clark 6/13/2017
PAMELA M. CLARK, HIS WIFE (40% OWNER) DATE
her attorney in fact

Carlton P. Davenport 6/13/17
CARLTON P. DAVENPORT (20% OWNER) DATE

Nancy B. Whitman-Davenport 6/13/17
NANCY B. WHITMAN-DAVENPORT, HIS WIFE (20% OWNER) DATE

Soren N. Rasmussen P.S. 6/14/17
CITY SURVEYOR, CITY OF ALBUQUERQUE, N.M. DATE

PROPERTY CORNERS

- ● FOUND 5/8" REBAR WITH 2" ALUMINUM CAP "LS 12651" (TYP.)
- ◎ FOUND 1/2" REBAR WITH CAP "LS 8686" (TYP.)
- □ FOUND 5/8" REBAR
- △ FOUND 5/8" REBAR WITH 3" ALUMINUM CAP "LS 11883" (TYP.)
- ▣ FOUND 5/8" REBAR WITH CAP "LS 11808" (TYP.)
- ○ SET 1/2" REBAR WITH CAP "LS 7719" (TYP.)

EASEMENTS

1. EXISTING 20' ABCWUA PUBLIC WATERLINE EASEMENT (09-08-2015, 2015C-104)
2. EXISTING 15' NMGC GAS LINE EASEMENT (09-08-2015, 2015C-104)
3. EXISTING 60' COA PERMANENT EASEMENT FOR GLENDALE EXTENSION ROADWAY IMPROVEMENTS (12-21-2001, 2001152060)

OWNERS

CCG PARTNERSHIP,
A NEW MEXICO GENERAL PARTNERSHIP
ATTN: SCOTT B. CLARK
P.O. BOX 14641 ADDRESS
ALBUQUERQUE, N.M. 87191
(505) 883-1414

ENGINEERS

D. MARK GOODWIN & ASSOCIATES, P.A.
CONSULTING ENGINEERS
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ALBUQUERQUE, NEW MEXICO 87199
(505) 828-2200

SURVEYOR

ALDRICH LAND SURVEYING.
P.O. BOX 30701
ALBUQUERQUE, N.M. 87190
(505) 884-1990

SITE BENCHMARK

A.G.R.S. MONUMENT
"10-C18"
ELEVATION=5222.09
(NAVD 1988)

50' 25' 0 50' 100'
SCALE: 1" = 50'

DRB PROJECT No. 1004472

SHEET 1 OF 1

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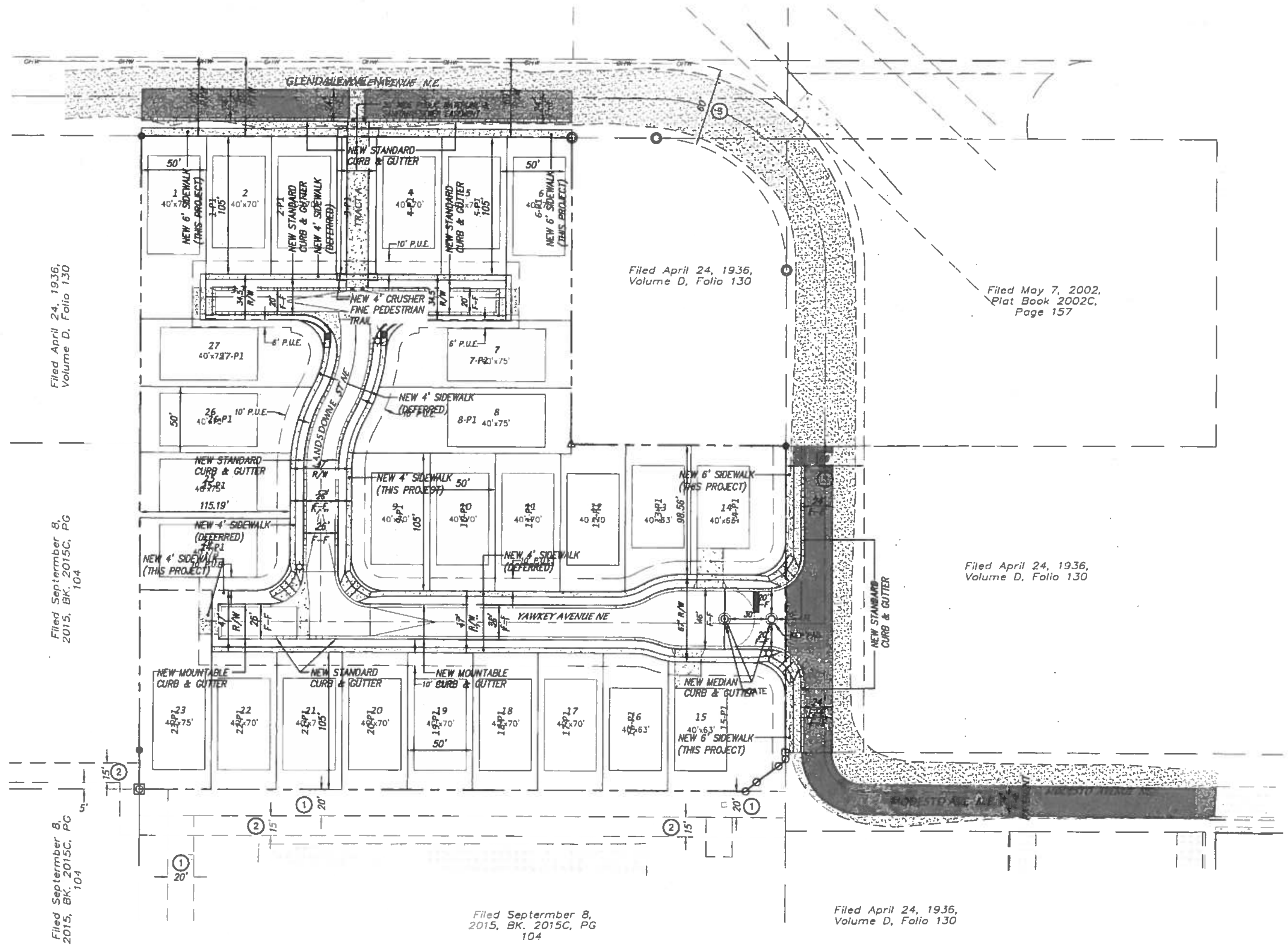

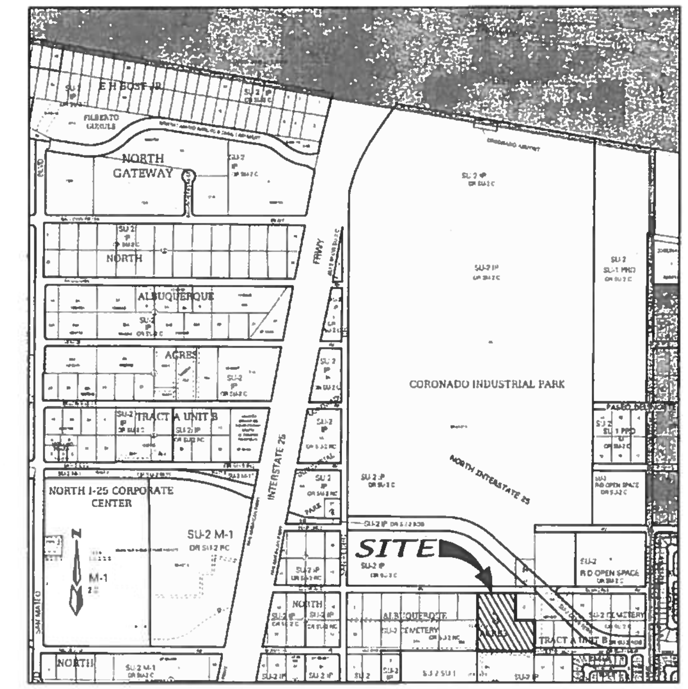

VICINITY MAP

ZONE MAP: B-18-Z

CONSTRUCTION NOTES

1. ALL SIDEWALKS PER CITY OF ALBUQUERQUE STANDARD DWG #2430.
2. ALL 6' VALLEY GUTTERS PER CITY OF ALBUQUERQUE STANDARD DWG #2420.
3. ALL HANDICAP RAMPS ARE PER CITY OF ALBUQUERQUE STANDARD DWG #2441, CASE II, UNLESS OTHERWISE NOTED, WITH ADA COMPLIANT DETECTABLE WARNING SURFACES TO BE CAST IN PLACE AND REPLACEABLE. SUBMIT SPECIFICATIONS TO ENGINEER PRIOR TO CONSTRUCTION.
4. TRANSITION SECTION FROM FULL CROWN TO NO CROWN TO BE A MINIMUM OF 50 FEET PER CITY OF ALBUQUERQUE STANDARD DWG #2401.
5. TRANSITION FROM MOUNTABLE ROLL CURB AND GUTTER TO STANDARD CURB AND GUTTER SHALL BE 10 FEET WHEN NOT TRANSITIONING AT INTERSECTIONS. TRANSITION FROM MOUNTABLE ROLL CURB TO STANDARD CURB AND GUTTER AT INTERSECTIONS SHALL OCCUR THROUGH THE HC RAMP PER CITY OF ALBUQUERQUE STANDARD DWG #2418.
6. SOIL NOT HAVING THE MINIMUM R-VALUE > 50 FOR STREETS SHALL BE REMOVED TO A DEPTH OF 2 FEET AND REPLACED, BY THE CONTRACTOR, WITH SUITABLE MATERIAL OR A PAVEMENT SECTION SHALL BE DESIGNED BY THE CONSULTANT ACCOMMODATING THE EXISTING R-VALUE PER CITY OF ALBUQUERQUE STANDARD STANDARD SPECIFICATIONS.
7. ALL MOUNTABLE ROLL CURB & GUTTER PER CITY OF ALBUQUERQUE STANDARD DWG #2415A.
8. ALL STANDARD CURB & GUTTER PER CITY OF ALBUQUERQUE STANDARD DWG #2415A.
9. ALL DEPRESSED STANDARD CURB & GUTTER PER CITY OF ALBUQUERQUE STANDARD DWG #2415A.

EASEMENTS

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2. EXISTING 15' NMGC GAS LINE EASEMENT (09-08-2015, 2015C-104)
3. EXISTING 60' COA PERMANENT EASEMENT FOR GLENDALE EXTENSION ROADWAY IMPROVEMENTS (12-21-2001, 2001152060)

LEGEND

- NEW BOUNDARY LINE
- NEW RIGHT OF WAY LINE
- NEW LOT LINE
- EXISTING ADJOINER LINE
- NEW CENTER LINE
- NEW EASEMENT LINE
- NEW SIDEWALK (THIS PROJECT)- ALL OTHERS DEFERRED
- NEW SIDEWALK (DEFERRED)
- NEW MOUNTABLE ROLL CURB & GUTTER
- NEW STANDARD CURB & GUTTER
- NEW DEPRESSED STANDARD CURB & GUTTER
- CROWN TO NO CROWN TRANSITION
- NEW WHEELCHAIR RAMP WITH TRUNCATED DOME
- NEW STREET LIGHT
- 2 NEW LOT ID
- NEW TEMPORARY PAVEMENT
- NEW 4" A.C. PAVEMENT (2- 2" LIFTS)
- NEW CONCRETE
- EXISTING GRAVEL ROAD

N

50' 25' 0 50' 100'

SCALE: 1" = 50'

AS BUILT INFORMATION	
CONTRACTOR	
WORK STAKED BY	DATE
INSPECTOR'S ACCEPTANCE BY	DATE
FIELD VERIFICATION BY	DATE
DRAWINGS CORRECTED BY	DATE
MICRO FILM INFORMATION	
RECORDED BY	DATE
NO.	

BENCH MARKS

AGRS Brass Cap stamped "10-C18"
Elevation, in feet (NAVD88)=55222.090

SURVEY INFORMATION		
FIELD NOTES		
NO.	BY	DATE

ENGINEER'S SEAL

NO.	DATE	REMARKS	BY
		REVISIONS	
		DESIGN	
DESIGNED BY	DLH	DATE	06/16
DRAWN BY	DER	DATE	06/16
CHECKED BY	DMG	DATE	06/16

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ALBUQUERQUE, NEW MEXICO 87199
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CITY OF ALBUQUERQUE
PUBLIC WORKS DEPARTMENT

TITLE

GLENDESTO SUBDIVISION
MASTER PAVING PLAN

DESIGN REVIEW COMMITTEE	CITY ENGINEER APPROVAL	MO./DAY/YR.	MO./DAY/YR.
		LAST DESIGN UPDATE	

CITY PROJECT NO	ZONE MAP NO	SHEET	OF
-	B-18-Z	X	X

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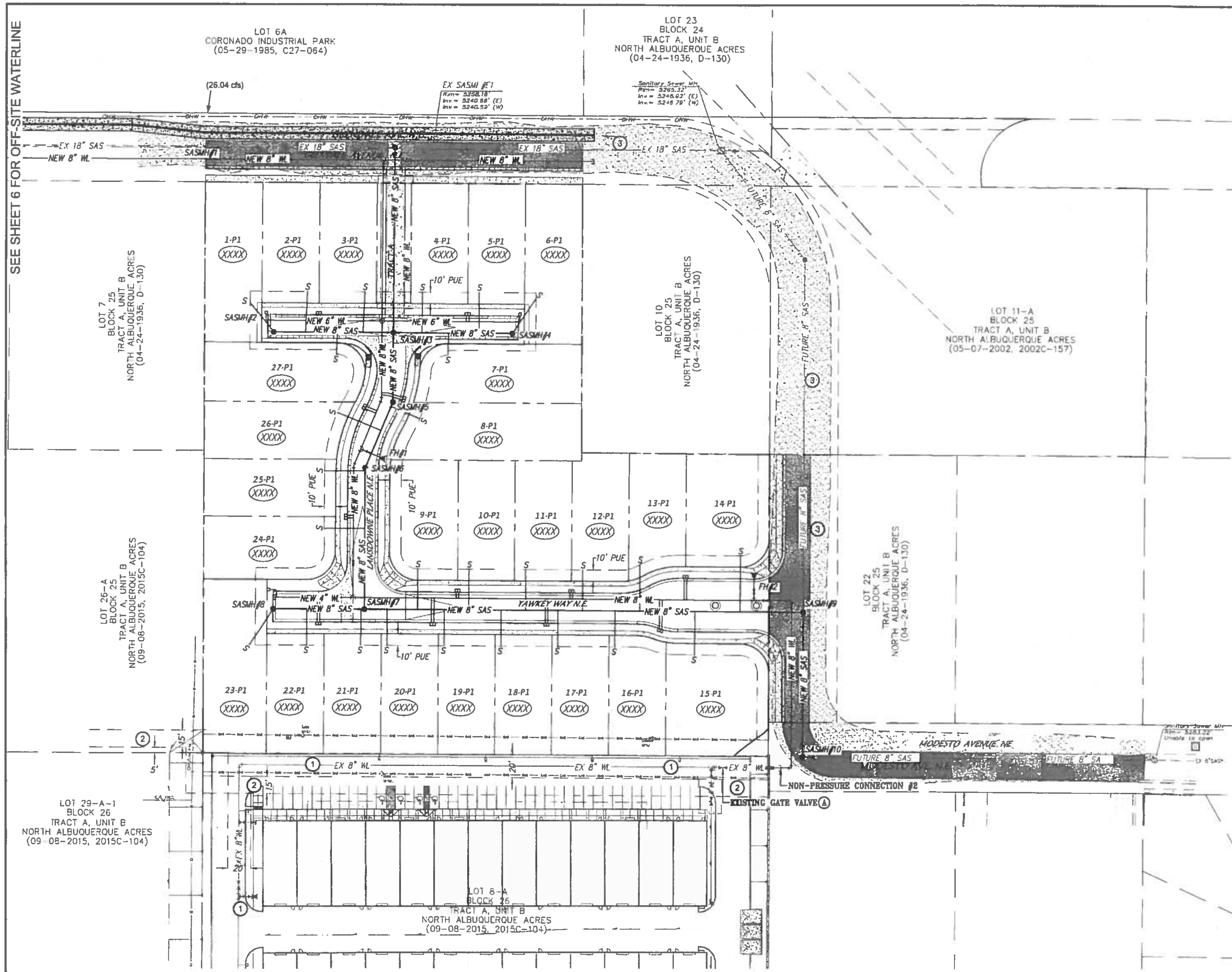

LEGEND

	EXISTING ASPHALT PAVEMENT
	EXISTING ELECTRIC TRANSFORMER
	EXISTING OVERHEAD ELECTRIC LINE
	EXISTING POWER POLE
	EXISTING LIGHT POLE
	EXISTING TRAFFIC SIGNAL PULLBOX
	EXISTING CATV PEDESTAL
	EXISTING UTILITY PEDESTAL
	EXISTING DROP INLET
30"SD	EXISTING STORM DRAIN
	EXISTING STORM DRAIN MANHOLE
	EXISTING SANITARY SEWER MANHOLE
8"SAS	EXISTING SANITARY SEWER LINE
8"WL	EXISTING WATER LINE
	EXISTING GATE VALVE & BOX
	EXISTING FIRE HYDRANT
	NEW RIGHT-OF-WAY
	NEW CENTERLINE
	NEW LOT LINES
	NEW EASEMENTS
	NEW STORM DRAIN INLET
24" RCP	NEW STORM DRAIN
	NEW STORM DRAIN MANHOLE
8"SAS	NEW SANITARY SEWER LINE
MH#1	NEW SANITARY SEWER MANHOLE
	NEW SANITARY SEWER FLOW DIRECTION
	NEW SANITARY SEWER SERVICE LINE
10"WL	NEW WATERLINE
	NEW FIRE HYDRANT
	NEW GATE VALVE & BOX
	NEW WATERLINE TEE
	NEW WATERLINE CAP
	NEW WATERLINE BEND
DOUBLE SINGLE	NEW WATER METER SERVICES *
	NEW STREET LIGHT
2401	NEW LOT ADDRESS

RESTRAINED JOINT LENGTH FOR TEES (FT.)		
PIPE SIZE	RUN	BRANCH
8"x8"x8"	13'	3'
8"x8"x6"	7'	5'

RESTRAINED JOINT LENGTH FOR BENDS, VALVES, DEAD ENDS (FT.)					
PIPE SIZE	90°	45°	22.5°	11.25°	VALVE
12"	30'	12'	6'	3'	85'
8"	22'	9'	4'	2'	60'
6"	17'	7'	3'	2'	46'
4"	12'	5'	2'	1'	32'

LENGTH OF RESTRAINED PIPE ON EACH SIDE OF THE BEND. RESTRAIN BRANCH AT FITTING ONLY MINIMUM ATTACHED PIPE LENGTH, RUN SIDE OF TEE: 10.00 FEET.

WATER SHUT-OFF PLAN

1. CONTRACTOR SHALL MAKE SHUT OFF REQUEST ONLINE AT http://abcwua.org/content/view/463/729/.
2. VALVES SHALL ONLY BE OPERATED BY WATER AUTHORITY EMPLOYEES.
3. FOR PRESSURE CONNECTION #1, ON SAN PEDRO, SHUT-OFF GATE VALVES "B", "C" & "D".
4. FOR NON-PRESSURE CONNECTION #2, SHUT-OFF EXISTING GATE VALVE "A".

RESTRAINED JOINT LENGTHS FOR REDUCERS (FT.)		
PIPE SIZE	LARGE SIDE	SMALL SIDE
12" X 8"	45'	68'
8" X 6"	25'	33'

NOTES

1. ALL FIRE HYDRANTS ARE 4' BURY UNLESS OTHERWISE SPECIFIED.
2. ALL METER BOXES ADJACENT TO MOUNTABLE CURB SHALL HAVE DUCTILE IRON METER BOX COVER & LID PER ABCWUA STD. DWG 2368; WATER METER BOX PER ABCWUA STD. DWG #2366; WATER SERVICE PER ABCWUA STD. DWG #2362.
3. ALL FIRE HYDRANTS PER ABCWUA STD. 2340.
4. ALL FIRE HYDRANTS TO HAVE A 6" GATE VALVE & BOX PER ABCWUA STD. DWG #2326; CONTRACTOR TO USE POLY COATED CMP FOR VALVE BOXES.
5. CONTRACTOR IS NOT TO INSTALL THE STEPS IN THE SAS MANHOLES.
6. ALL SANITARY SEWER MANHOLES PER ABCWUA STD. DWG #2101, #2107 AND #2109.

N

50' 25' 0 50' 100'
SCALE: 1" = 50'

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AS BUILT INFORMATION
CONTRACTOR — DATE
WORK STAKED BY — DATE
INSPECTOR'S ACCEPTANCE BY — DATE
FIELD VERIFICATION BY — DATE
DRAWINGS CORRECTED BY — DATE

MICRO-FILM INFORMATION
RECORDED BY — DATE
NO.

BENCH MARKS
AGRS Brass Cap stamped "10-C18"
Elevation, in feet (NAVD88)=55222.090

SURVEY INFORMATION
FIELD NOTES
NO. | BY | DATE

ENGINEER'S SEAL

NO.	DATE	REMARKS	BY
		REVISIONS	
		DESIGN	
DESIGNED BY	DLH	DATE	06/16
DRAWN BY	DER	DATE	06/16
CHECKED BY	DMG	DATE	06/16

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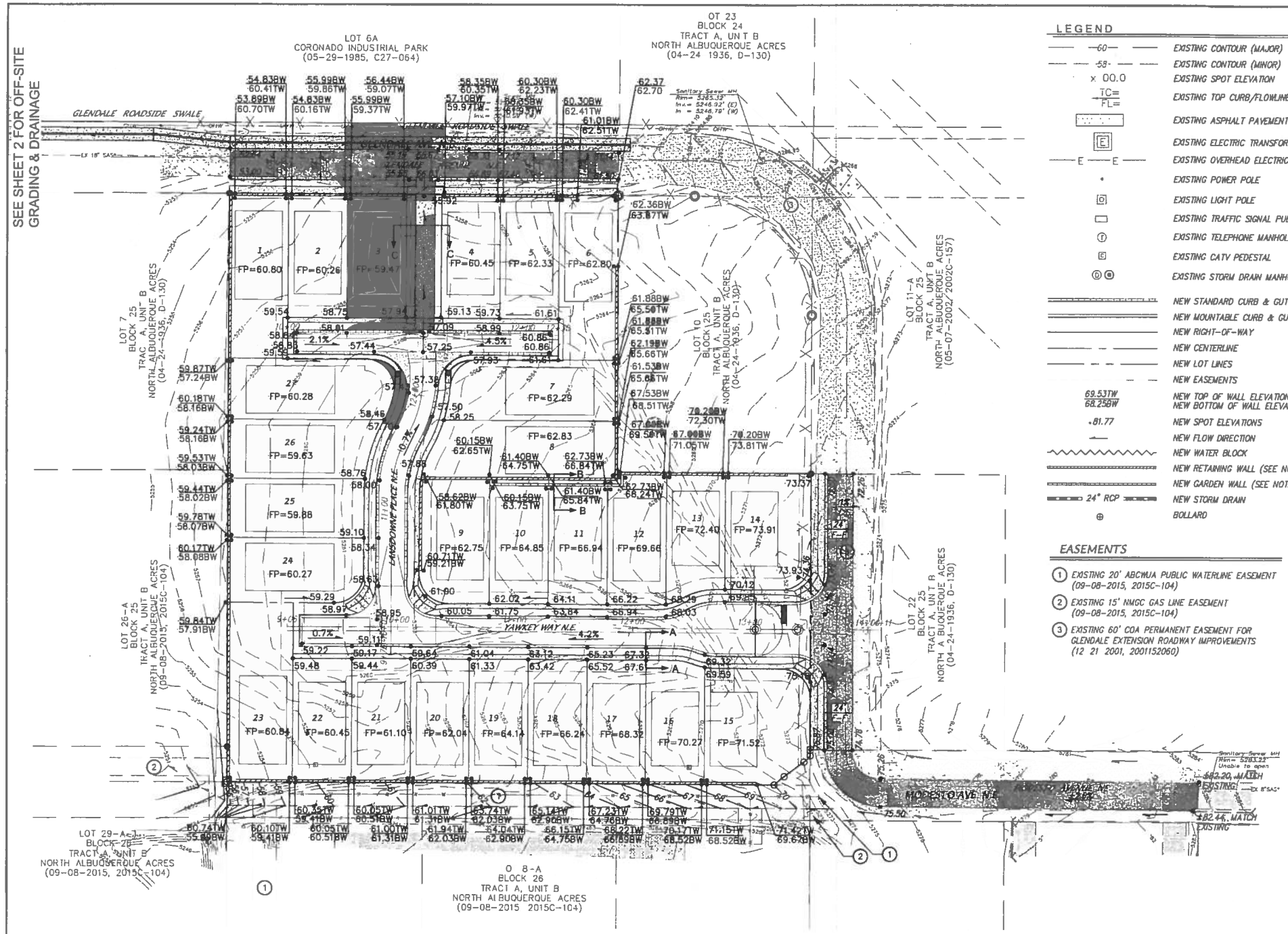
CITY OF ALBUQUERQUE
PUBLIC WORKS DEPARTMENT

TITLE: GLENDESTO SUBDIVISION
MASTER UTILITY PLAN

DESIGN REVIEW COMMITTEE	CITY ENGINEER APPROVAL	LAST DESIGN UPDATE	MO./DAY/YR.	MO./DAY/YR.

CITY PROJECT NO. — ZONE MAP NO. B-18-Z — SHEET X OF X

SEE SHEET 2 FOR OFF-SITE GRADING & DRAINAGE



LEGEND

- EXISTING CONTOUR (MAJOR)
- EXISTING CONTOUR (MINOR)
- EXISTING SPOT ELEVATION
- EXISTING TOP CURB/FLOWLINE ELEVATION
- EXISTING ASPHALT PAVEMENT
- EXISTING ELECTRIC TRANSFORMER
- EXISTING OVERHEAD ELECTRIC LINE
- EXISTING POWER POLE
- EXISTING LIGHT POLE
- EXISTING TRAFFIC SIGNAL PULLBOX
- EXISTING TELEPHONE MANHOLE
- EXISTING CATV PEDESTAL
- EXISTING STORM DRAIN MANHOLE
- NEW STANDARD CURB & GUTTER
- NEW MOUNTABLE CURB & GUTTER
- NEW RIGHT-OF-WAY
- NEW CENTERLINE
- NEW LOT LINES
- NEW EASEMENTS
- NEW TOP OF WALL ELEVATION / NEW BOTTOM OF WALL ELEVATION
- NEW SPOT ELEVATIONS
- NEW FLOW DIRECTION
- NEW WATER BLOCK
- NEW RETAINING WALL (SEE NOTE 7 & 8)
- NEW GARDEN WALL (SEE NOTE 7)
- NEW STORM DRAIN
- BOLLARD

EASEMENTS

1. EXISTING 20' ABCWUA PUBLIC WATERLINE EASEMENT (09-08-2015, 2015C-104)
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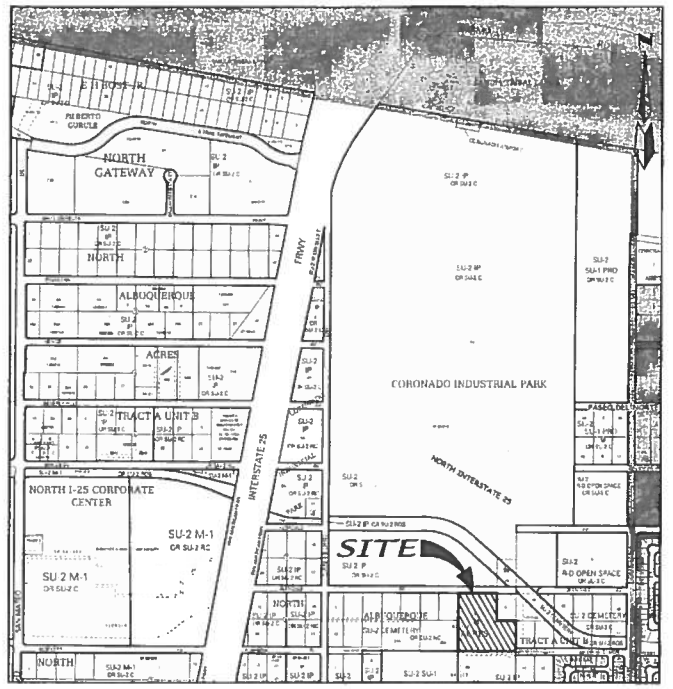


VICINITY MAP (NTS)

ZONE ATLAS MAP B-18

LEGAL DESCRIPTION

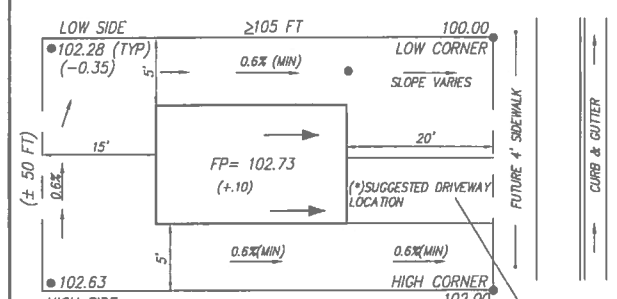
A TRACT OF LAND SITUATE WITHIN THE ELENA GALLEGO GRANT, PROJECTED SECTION 12, TOWNSHIP 11 NORTH, RANGE 3 EAST, NEW MEXICO PRINCIPAL MERIDIAN, CITY OF ALBUQUERQUE, BERNALILLO COUNTY, NEW MEXICO, BEING ALL OF LOT 8-A, BLOCK 25, TRACT A, UNIT B, NORTH ALBUQUERQUE ACRES, AS THE SAME IS SHOWN AND DESIGNATED ON SAID PLAT, FILED FOR RECORD IN THE OFFICE OF THE COUNTY CLERK OF BERNALILLO COUNTY, NEW MEXICO, ON SEPTEMBER 8, 2015, IN PLAT BOOK 2015C, PAGE 104, AND CONTAINING 4.7676 ACRES MORE OR LESS.

NOTES

1. CONTRACTOR MUST OBTAIN A TOPSOIL DISTURBANCE PERMIT FROM THE ENVIRONMENTAL HEALTH DIVISION PRIOR TO CONSTRUCTION.
2. CITY OF ALBUQUERQUE STANDARD SPECIFICATIONS FOR PUBLIC WORKS CONSTRUCTION, LATEST EDITION SHALL GOVERN ALL WORK.
3. THE CONTRACTOR SHALL CONFORM TO ALL CITY, COUNTY, STATE AND FEDERAL DUST CONTROL MEASURES AND REQUIREMENTS AND WILL BE RESPONSIBLE FOR PREPARING AND OBTAINING ALL NECESSARY APPLICATIONS AND APPROVALS.
4. THE CONTRACTOR SHALL ENSURE THAT NO SOIL ERODES FROM THE LOTS INTO PUBLIC RIGHT-OF-WAY. THIS CAN BE ACHIEVED BY CONSTRUCTING TEMPORARY BERMS AND WETTING THE SOIL TO KEEP IT FROM BLOWING.
5. THE EARTHWORK CONTRACTOR SHALL STOCKPILE ENOUGH MATERIAL ADJACENT TO RETAINING WALL LOCATIONS TO BE UTILIZED FOR WALL BACKFILL.
6. SITE DOES NOT LIE IN A 100 YEAR FLOOD ZONE.
7. ALL SITE WALLS SHALL CONFORM TO THE GENERAL HEIGHT AND DESIGN REGULATIONS CONTAINED IN SECTION 14-16-3-19 OF THE CITY ZONING CODE.
8. COMBINATION GARDEN WALL/RETAINING WALL CANNOT EXCEED 8 FEET. IF THE RETAINING WALL IS GREATER THAN 4 FEET, THE DEVELOPER CAN ADD 3:1 SLOPES IN BACKYARD OR A SECOND RETAINING WALL OFFSET FROM THE FIRST ONE.

LANDSCAPE BUFFER NOTES:

1. SWALE TO BE 6" DEEP WHEN THE DISTANCE BETWEEN BACK OF CURB AND THE SIDEWALK IS 5 FEET.
2. FINAL GRADE OF DIRT TO BE 1 TO 2 INCHES BELOW TOP OF CURB AND TOP OF SIDEWALK GRADE.
3. SURFACE BETWEEN BACK OF CURB AND SIDEWALK TO BE COVERED WITH GRAVEL MULCH (MINIMUM 3/4"), COBBLES OF RIP-RAP. DO NOT FILL ENTIRE SWALE.
4. LANDSCAPE FABRIC IS RECOMMENDED, BUT NOT REQUIRED, BETWEEN THE DIRT AND THE STONE. IF LANDSCAPE FABRIC IS TO BE USED IT IS TO BE PERMEABLE.

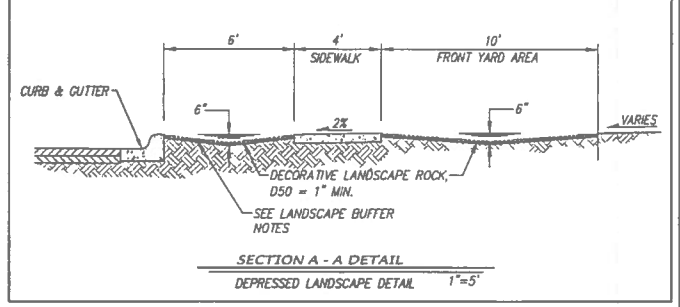


TYPICAL LOT LAYOUT PLAN

N.T.S.

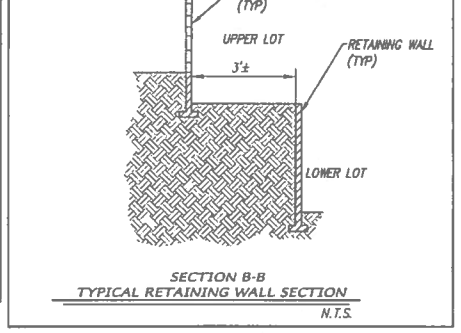
(*) SPECIAL NOTE FOR DRIVEWAYS:

TO MINIMIZE DRIVEWAY SLOPE ON ALL LOTS WHERE THE FINISHED PAD IS > 2.0 FEET HIGHER THAN THE LOW CORNER OF THE LOT, THE DRIVEWAY SHOULD BE CONSTRUCTED ON THE HIGH SIDE OF THE PAD/LOT.



SECTION A - A DETAIL

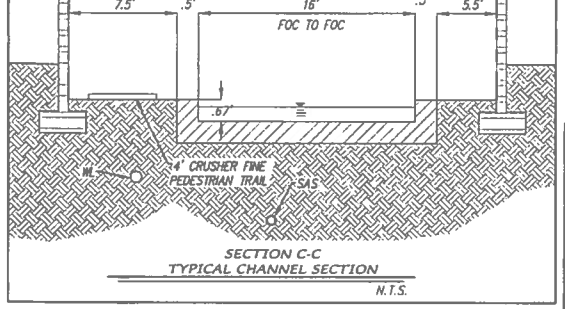
DEPRESSED LANDSCAPE DETAIL 1"=5'



SECTION B-B

TYPICAL RETAINING WALL SECTION

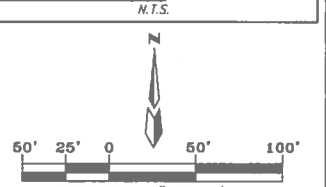
N.T.S.



SECTION C-C

TYPICAL CHANNEL SECTION

N.T.S.



SCALE: 1" = 50'

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AS BUILT INFORMATION

BENCH MARKS

ACRS Brass Cap stamped "10-C18"
Elevation, in feet (NAVD88)=5222.080

CONTRACTOR — DATE
WORK STARTED BY — DATE
INSPECTORS ACCEPTANCE BY — DATE
CERTIFICATION BY — DATE
DRAWINGS CORRECTED BY — DATE
MICRO-FILM INFORMATION
RECORDED BY — DATE
NO

SURVEY INFORMATION

FIELD NOTES

NO.	BY	DATE

ENGINEER'S SEAL

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CONSULTING ENGINEERS
P.O. BOX 90606
ALBUQUERQUE, NEW MEXICO 87199
OFFICE (505) 828-2200, FAX (505) 797-9539

NO.	DATE	REMARKS / REVISIONS	
		DESIGN	
		DESIGNED BY	DLH 05/16
		DRAWN BY	DER 05/16
		CHECKED BY	DMG 05/16

CITY OF ALBUQUERQUE
PUBLIC WORKS DEPARTMENT

TITLE.

GLENDESTO SUBDIVISION
GRADING & DRAINAGE PLAN

DESIGN REVIEW COMMITTEE	CITY ENGINEER APPROVAL	LAST DESIGN UPDATE	MO./DAY/YR.	MO./DAY/YR.

CITY PROJECT NO. -

ZONE MAP NO. B-18-Z

SHEET 1 OF 2

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CITY OF ALBUQUERQUE
PUBLIC WORKS DEPARTMENT

TITLE:

GLENDESTO SUBDIVISION
GRADING & DRAINAGE PLAN

DESIGN REVIEW COMMITTEE	CITY ENGINEER APPROVAL	LAST DESIGN UPDATE	MO./DAY/YR.	MO./DAY/YR.

CITY PROJECT NO.	ZONE MAP NO.	SHEET	OF
.	B-18-Z	2	2

NO.	DATE	REMARKS	BY
		REVISIONS	
		DESIGN	

DESIGNED BY	DLH	DATE	05/16
DRAWN BY	DER	DATE	05/16
CHECKED BY	DMG	DATE	05/16

ENGINEER'S SEAL

DIANE HOELZER
NEW MEXICO
11987
LICENSED PROFESSIONAL ENGINEER
2-7-17

SURVEY INFORMATION

FIELD NOTES		
NO.	BY	DATE

BENCH MARKS

ACRS Brass Cap stamped "10-C18"
Elevation, in feet (NAVD88)=55222.090

AS BUILT INFORMATION

CONTRACTOR	
WORK STAKED BY	DATE
INSPECTOR'S ACCEPTANCE BY	DATE
FIELD VERIFICATION BY	DATE
DRAWINGS CORRECTED BY	DATE

MICRO-FILM INFORMATION

RECORDED BY	DATE
NO.	

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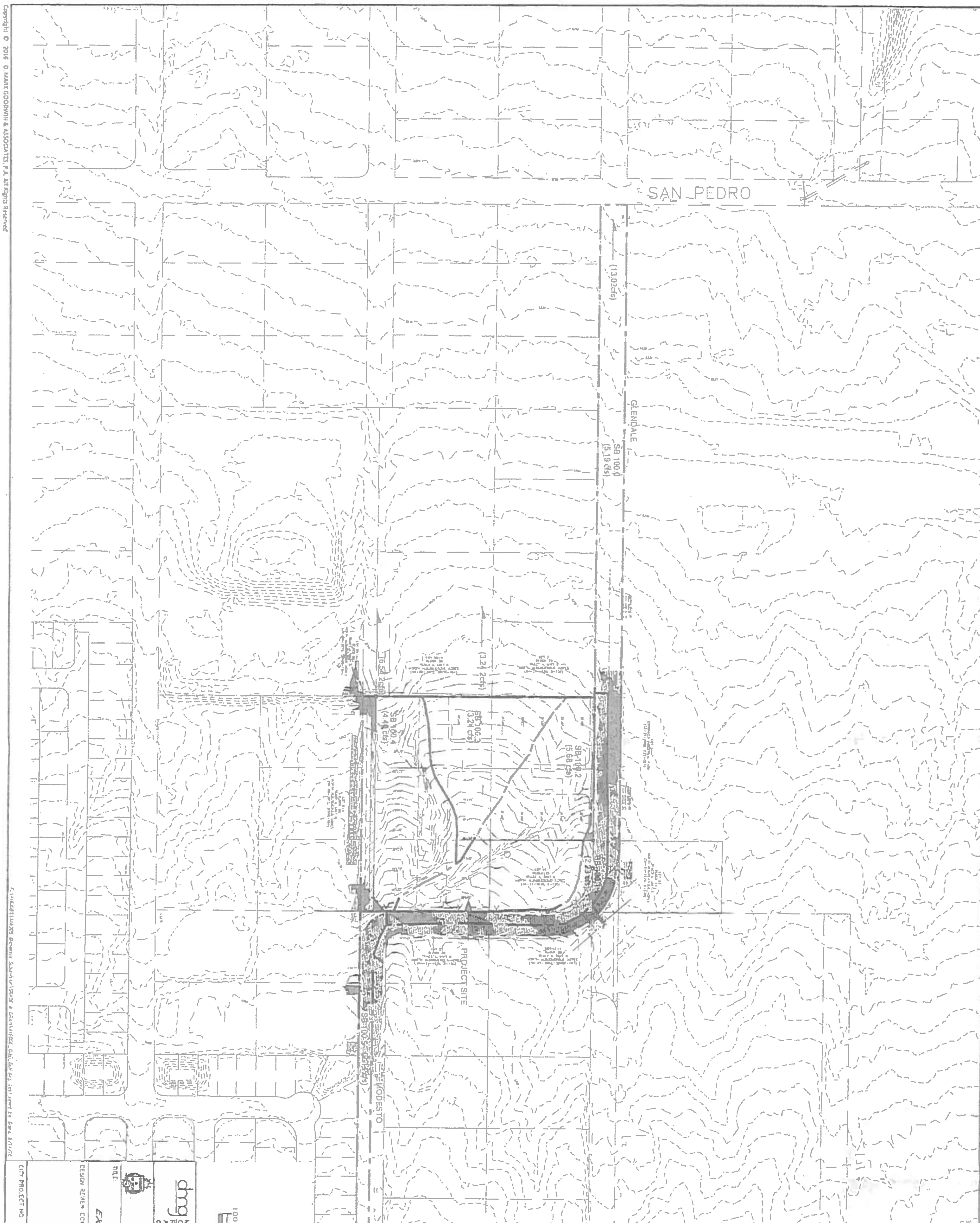

dmg
MARK GOODWIN & ASSOCIATES, P.A.
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ALBUQUERQUE, NEW MEXICO 87199
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CITY OF ALBUQUERQUE
PUBLIC WORKS DEPARTMENT

TITLE:
GLENDESTO SUBDIVISION
EXISTING DRAINAGE CONDITIONS

DESIGN REVIEW COMMITTEE	CITY ENGINEER APPROVAL	MO./DAY/YR.	MO./DAY/YR.

CITY PROJECT NO.

ZONE MAP NO. B-18-Z

SHEET OF

LAST DESIGN UPDATE

NO.	DATE	REMARKS	BY

REVISIONS

DESIGN

DESIGNED BY	DLH	DATE	05/16
DRAWN BY	DER	DATE	05/16
CHECKED BY	DMG	DATE	05/16

ENGINEER'S SEAL

DIANE HOELZER NEW MEXICO 17937 LICENSED PROFESSIONAL ENGINEER
8-12-16

SURVEY INFORMATION

FIELD NOTES

NO	BY	DATE

BENCH MARKS

ACRS Brass Cap stamped "10-C18"
Elevation, in feet (NAVD88)=5222.090

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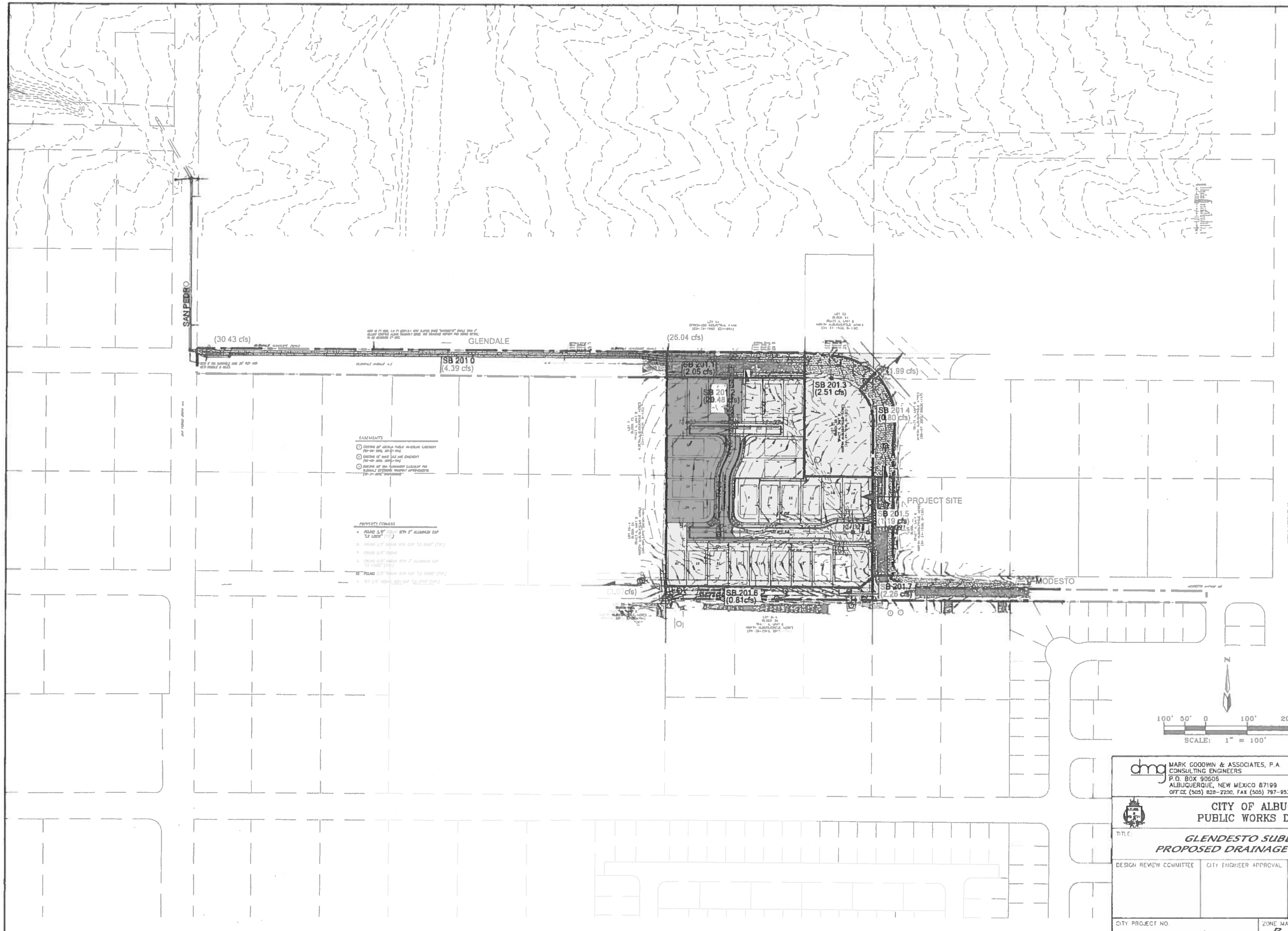

AS BUILT INFORMATION	
CONTRACTOR	
WORK STAKED BY	DATE
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BENCH MARKS

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Elevation, in feet (NAVD88)=55222.090

SURVEY INFORMATION

FIELD NOTES		
NO.	BY	DATE

ENGINEER'S SEAL

DIANE HOELZER, NEW MEXICO, 11987, LICENSED PROFESSIONAL ENGINEER — 6-15-17

NO.	DATE	REMARKS	BY
		REVISIONS	
		DESIGN	
DESIGNED BY	DLH	DATE	05/16
DRAWN BY	DER	DATE	05/16
CHECKED BY	DMG	DATE	05/16

dmg MARK GOODWIN & ASSOCIATES, P.A.
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ALBUQUERQUE, NEW MEXICO 87199
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CITY OF ALBUQUERQUE
PUBLIC WORKS DEPARTMENT

TITLE:
GLENDESTO SUBDIVISION
PROPOSED DRAINAGE CONDITIONS

DESIGN REVIEW COMMITTEE	CITY ENGINEER APPROVAL	MO./DAY/YR.	MO./DAY/YR.
LAST DESIGN UPDATE			

CITY PROJECT NO.	ZONE MAP NO.	SHEET	OF
	B-18-Z		

F:\A17\C001\A17019 Glendesto Revisions\GRADE & DRAIN\A17019_GAD.dwg, Last saved by: ..., 6/14/17

APPENDIX A - HYDROLOGY

EXISTING DRAINAGE CONDITIONS – SUMMARY TABLE

AHYMO PROGRAM SUMMARY TABLE (AHYMO-S4) - Ver. S4.01a, Rel: 01a RUN DATE (MON/DAY/YR) =06/15/2016
INPUT FILE = C:\Program Files (x86)\AHYMO-S4\GLENEX.DAT USER NO.= M-GoodwinNMSiteA90075759

PAGE = 1

COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	NOTATION
*S**										
*S	Tierra Serena Subdivison									
*S	100 YEAR 24 HOUR STORM EVENT									
*S										
*S	FILE: GLENEX.DAT									
*S	LAST REVISED: 6-15-16									
*S	NOAA ATLAS 2, VOL IV ZONE B-18									
START										TIME= 0.00
LOCATION	NEW MEXICO									
RAINFALL TYPE= 2 NOAA 14										RAIN24= 2.850
*S**										
*S EXSTING DRAINAGE CONDITIONS										
COMPUTE NM HYD	100.00	-	1	0.00289	5.19	0.152	0.98842	1.532	2.806	PER IMP= 0.00
COMPUTE NM HYD	100.10	-	1	0.00124	2.23	0.065	0.98842	1.532	2.813	PER IMP= 0.00
COMPUTE NM HYD	100.20	-	1	0.00388	5.68	0.170	0.82168	1.532	2.287	PER IMP= 0.00
COMPUTE NM HYD	100.30	-	1	0.00221	3.24	0.097	0.82168	1.532	2.290	PER IMP= 0.00
COMPUTE NM HYD	100.40	-	1	0.00301	4.40	0.132	0.82168	1.532	2.288	PER IMP= 0.00
COMPUTE NM HYD	100.50	-	1	0.00092	2.14	0.078	1.60396	1.532	3.649	PER IMP= 40.00
FINISH										

EXISTING DRAINAGE CONDITIONS

```
*S*************************************************************
*S                    GLENDESTO SUBDIVISION
*S                    100 YEAR 24 HOUR STORM EVENT
*S
*S                    FILE: GLENEX.DAT
*S                    LAST REVISED: 6-15-16
*S                    NOAA ATLAS 2, VOL IV ZONE B-18
START                 TIME=0.0 HR PUNCH CODE=0 PRINT LINES=-6
LOCATION              NEW MEXICO
RAINFALL              TYPE=2 RAIN QUARTER=0.0
                      RAIN ONE=2.10 IN RAIN SIX=2.50 IN
                      RAIN DAY=2.85 IN DT=0.0333 HRS
*S*************************************************************
*S  EXSTING DRAINAGE CONDITIONS
*** ********************
*** SUB BASIN 100.0
*** ********************
COMPUTE NM HYD        ID=1  HYD NO=100.0 AREA= 0.002888 SQ MI
                      PER A=20  PER B=80  PER C=0  PER D=0
                      TP=-.1333 HR  MASS RAIN=-1
PRINT HYD             ID=1 CODE=1
*** ********************
*** SUB BASIN 100.1
*** ********************
COMPUTE NM HYD        ID=1  HYD NO=100.1 AREA= 0.001236 SQ MI
                      PER A=20  PER B=80  PER C=0  PER D=0
                      TP=-.1333 HR  MASS RAIN=-1
PRINT HYD             ID=1 CODE=1
*** ********************
*** SUB BASIN 100.2
*** ********************
COMPUTE NM HYD        ID=1  HYD NO=100.2 AREA= 0.003884 SQ MI
                      PER A=100  PER B=0  PER C=0  PER D=0
                      TP=-.1333 HR  MASS RAIN=-1
PRINT HYD             ID=1 CODE=1
*** ********************
*** SUB BASIN 100.3
*** ********************
COMPUTE NM HYD        ID=1  HYD NO=100.3 AREA= 0.002210 SQ MI
                      PER A=100  PER B=0  PER C=0  PER D=0
                      TP=-.1333 HR  MASS RAIN=-1
PRINT HYD             ID=1 CODE=1
*** ********************
*** SUB BASIN 100.4
*** ********************
COMPUTE NM HYD        ID=1  HYD NO=100.4 AREA= 0.003007 SQ MI
                      PER A=100  PER B=0  PER C=0  PER D=0
                      TP=-.1333 HR  MASS RAIN=-1
PRINT HYD             ID=1 CODE=1
*** ********************
*** SUB BASIN 100.5
*** ********************
COMPUTE NM HYD        ID=1  HYD NO=100.5 AREA= 0.000915 SQ MI
                      PER A=25  PER B=35  PER C=0  PER D=40
                      TP=-.1333 HR  MASS RAIN=-1
PRINT HYD             ID=1 CODE=1
FINISH
```

PROPOSED CONDITIONS

AHYMO PROGRAM SUMMARY TABLE (AHYMO-S4) - Ver. S4.01a, Rel: 01a RUN DATE (MON/DAY/YR) =08/10/2016
INPUT FILE = C:\Program Files (x86)\AHYMO-S4\glenp6.dat USER NO.= M-GoodwinNMSiteA90075759

COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.		TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	NOTATION	
*S**********												
*S	GLENDESTO SUBDIVISION											
*S	PROPOSED DRAINAGE CONDITIONS											
*S	100 YEAR 24 HOUR STORM EVENT											
*S	FILE: GLENP6.DAT											
*S	LAST REVISED: 8-8-16											
*S	NOAA ATLAS 2, VOL IV ZONE B-18											
START											TIME=	0.00
LOCATION	NEW MEXICO											
RAINFALL	TYPE= 1 NOAA 14										RAIN6=	2.500
*S**********												
*S	PROPOSED DRAINAGE CONDITIONS											
COMPUTE NM HYD	201.00	-		1	0.00244	4.39	0.129	0.98842	1.532	2.807	PER IMP=	0.00
COMPUTE NM HYD	201.10	-		1	0.00071	2.05	0.073	1.91043	1.532	4.505	PER IMP=	72.00
COMPUTE NM HYD	201.20	-		1	0.00744	20.48	0.707	1.78078	1.532	4.299	PER IMP=	57.00
*s**********												
s	ROUTE THRU ONSISTE POND											
*s**********												
ROUTE RESERVOIR	POND.12	1		12	0.00744	10.24	0.437	1.10157	1.698	2.150	AC-FT=	0.424
COMPUTE NM HYD	201.30	-		1	0.00162	2.51	0.075	0.86460	1.532	2.426	PER IMP=	0.00
COMPUTE NM HYD	201.40	-		1	0.00044	0.80	0.023	0.98842	1.532	2.837	PER IMP=	0.00
COMPUTE NM HYD	201.50	-		1	0.00041	1.19	0.042	1.91043	1.532	4.538	PER IMP=	72.00
COMPUTE NM HYD	201.60	-		1	0.00055	0.81	0.024	0.82168	1.532	2.312	PER IMP=	0.00
COMPUTE NM HYD	201.70	-		1	0.00091	2.26	0.075	1.54680	1.532	3.890	PER IMP=	40.00
COMPUTE NM HYD	201.20	-		1	0.00744	10.88	0.326	0.82168	1.532	2.285	PER IMP=	0.00
FINISH												

PROPOSED CONDITIONS

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      AHYMO PROGRAM (AHYMO-S4)                    - Version: S4.01a - Rel: 01a
           RUN DATE (MON/DAY/YR) = 08/10/2016
           START TIME (HR:MIN:SEC) = 14:17:12     USER NO.= M-GoodwinNMSiteA90075759
           INPUT FILE = C:\Program Files (x86)\AHYMO-S4\glenp6.dat

*S************************************************************
*S                    Tierra Serena Subdivison
*S                   PROPOSED DRAINAGE CONDITIONS
*S                   100 YEAR 24 HOUR STORM EVENT
*S                   FILE: GLENP6.DAT
*S                   LAST REVISED: 8-8-16
*S                   NOAA ATLAS 2, VOL IV ZONE B-18
START               TIME=0.0 HR PUNCH CODE=0 PRINT LINES=-6
LOCATION            NEW MEXICO
     State of New Mexico soil infiltration values (LAND FACTORS) used for computations.
     Land Treatment     Initial Abstr.(in)    Unif. Infilt.(in/hour)
          A             0.65                  1.67
          B             0.50                  1.25
          C             0.35                  0.83
          D             0.10                  0.04

RAINFALL            TYPE=1 RAIN QUARTER=0.0
                    RAIN ONE=2.10 IN RAIN SIX=2.50 IN
                    RAIN DAY=2.85 IN DT=0.0333 HRS

               6-HOUR RAINFALL DIST. - BASED ON NOAA ATLAS 14 FOR CONVECTIVE AREAS (NM & AZ) - D1
               DT =   0.033300 HOURS       END TIME =     5.994000 HOURS
                 0.0000  0.0018  0.0036  0.0054  0.0074  0.0094  0.0116
                 0.0138  0.0161  0.0186  0.0212  0.0265  0.0319  0.0377
                 0.0438  0.0499  0.0565  0.0631  0.0699  0.0769  0.0839
                 0.0912  0.0985  0.1063  0.1145  0.1227  0.1320  0.1413
                 0.1558  0.1760  0.1962  0.2231  0.2502  0.2824  0.3203
                 0.3582  0.4145  0.4714  0.5475  0.6458  0.7441  0.9994
                 1.2612  1.4697  1.6149  1.7600  1.8362  1.9091  1.9698
                 2.0158  2.0617  2.0943  2.1261  2.1541  2.1775  2.2008
                 2.2187  2.2364  2.2501  2.2588  2.2675  2.2753  2.2831
                 2.2902  2.2966  2.3029  2.3088  2.3146  2.3204  2.3260
                 2.3316  2.3345  2.3372  2.3399  2.3425  2.3451  2.3475
                 2.3499  2.3523  2.3547  2.3570  2.3593  2.3616  2.3638
                 2.3660  2.3681  2.3702  2.3723  2.3743  2.3763  2.3784
                 2.3803  2.3822  2.3841  2.3860  2.3878  2.3897  2.3915
                 2.3933  2.3950  2.3967  2.3984  2.4001  2.4018  2.4035
                 2.4051  2.4068  2.4084  2.4100  2.4116  2.4132  2.4147
                 2.4163  2.4178  2.4193  2.4209  2.4224  2.4239  2.4253
                 2.4268  2.4283  2.4297  2.4311  2.4326  2.4340  2.4354
                 2.4368  2.4381  2.4395  2.4409  2.4422  2.4436  2.4449


*S************************************************************
*S  PROPOSED DRAINAGE CONDITIONS
*** ********************
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*** ********************
*** SUB BASINS 201.0
*** ********************
COMPUTE NM HYD       ID=1  HYD NO=201.0 AREA= 0.002442 SQ MI
                     PER A=20  PER B=80  PER C=0  PER D=0
                     TP=-.1333 HR  MASS RAIN=-1

     K =  0.138116HR    TP =  0.133300HR    K/TP RATIO =  1.036128     SHAPE CONSTANT, N =  3.406848
     UNIT PEAK =   5.7427    CFS   UNIT VOLUME =   0.9977       B =   313.47      P60 = 2.1000
     AREA =     0.002442 SQ MI    IA =   0.53000 INCHES    INF =   1.33400 INCHES PER HOUR
     RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT =  0.033300

PRINT HYD            ID=1 CODE=1

                                   PARTIAL HYDROGRAPH   201.00


    RUNOFF VOLUME =     0.98842 INCHES     =       0.1287 ACRE-FEET
    PEAK DISCHARGE RATE =       4.39 CFS  AT   1.532 HOURS   BASIN AREA =  0.0024 SQ. MI.



*** ********************
*** SUB BASINS 201.1
*** ********************
COMPUTE NM HYD       ID=1  HYD NO=201.1 AREA= 0.000712 SQ MI
                     PER A=0  PER B=28  PER C=0  PER D=72
                     TP=-.1333 HR  MASS RAIN=-1

     K =  0.072649HR    TP =  0.133300HR    K/TP RATIO =  0.545000     SHAPE CONSTANT, N =  7.106428
     UNIT PEAK =   2.0239    CFS   UNIT VOLUME =   0.9941       B =   526.28      P60 = 2.1000
     AREA =     0.000513 SQ MI    IA =   0.10000 INCHES    INF =   0.04000 INCHES PER HOUR
     RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT =  0.033300


     K =  0.133173HR    TP =  0.133300HR    K/TP RATIO =  0.999050     SHAPE CONSTANT, N =  3.533543
     UNIT PEAK =  0.48273    CFS   UNIT VOLUME =   0.9712       B =   322.78      P60 = 2.1000
     AREA =     0.000199 SQ MI    IA =   0.50000 INCHES    INF =   1.25000 INCHES PER HOUR
     RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT =  0.033300

PRINT HYD            ID=1 CODE=1

                                   PARTIAL HYDROGRAPH   201.10


    RUNOFF VOLUME =     1.91043 INCHES     =       0.0725 ACRE-FEET
    PEAK DISCHARGE RATE =       2.05 CFS  AT   1.532 HOURS   BASIN AREA =  0.0007 SQ. MI.



*** ********************
*** SUB BASINS 201.2
*** ONSISTE SUB BASIN
*** ********************
COMPUTE NM HYD       ID=1  HYD NO=201.2 AREA= 0.007444 SQ MI
                     PER A=0  PER B=21.5  PER C=21.5  PER D=57
                     TP=-.1333 HR  MASS RAIN=-1
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      K =  0.072649HR    TP =  0.133300HR    K/TP RATIO =  0.545000     SHAPE CONSTANT, N =  7.106428
      UNIT PEAK =   16.752    CFS   UNIT VOLUME =   0.9986       B =   526.28      P60 = 2.1000
      AREA =     0.004243 SQ MI    IA =   0.10000 INCHES    INF =   0.04000 INCHES PER HOUR
      RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT =  0.033300


      K =  0.120817HR    TP =  0.133300HR    K/TP RATIO =  0.906356     SHAPE CONSTANT, N =  3.908407
      UNIT PEAK =   8.3810    CFS   UNIT VOLUME =   0.9989       B =   349.02      P60 = 2.1000
      AREA =     0.003201 SQ MI    IA =   0.42500 INCHES    INF =   1.04000 INCHES PER HOUR
      RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT =  0.033300

PRINT HYD            ID=1 CODE=1

                                  PARTIAL HYDROGRAPH   201.20


    RUNOFF VOLUME =     1.78078 INCHES     =       0.7070 ACRE-FEET
    PEAK DISCHARGE RATE =      20.48 CFS  AT   1.532 HOURS   BASIN AREA =  0.0074 SQ. MI.



*s**********************************************************************
*s* ROUTE THRU ONSISTE POND
*s**********************************************************************
ROUTE RESERVOIR      ID=12  HYD=POND.12   INFLOW=1   CODE=24
                     OUTFLOW(CFS)   STORAGE(ACFT)     ELEV(FT)
                     0.00              0.0000          49.0
                     0.01              0.36049         54.5
                     10.88             0.4279          55.0

  *  *  *  *  *  *  *  *  *  *  *  *  *  *  *  *  *

    TIME      INFLOW    ELEV      VOLUME    OUTFLOW
    (HRS)     (CFS)     (FEET)    (AC-FT)   (CFS)

     0.00       0.00     49.00      0.000      0.00
     0.80       0.03     49.00      0.000      0.00
     1.60      16.81     54.67      0.383      3.60
     2.40       0.74     54.54      0.366      0.94
     3.20       0.08     54.50      0.361      0.09
     4.00       0.05     54.50      0.361      0.05
     4.80       0.05     54.50      0.361      0.05
     5.59       0.07     54.50      0.361      0.07
     6.39       0.01     54.50      0.361      0.01

 PEAK DISCHARGE =       10.245 CFS - PEAK OCCURS AT HOUR    1.70
 MAXIMUM WATER SURFACE ELEVATION =       54.971
 MAXIMUM STORAGE =        0.4240 AC-FT      INCREMENTAL TIME=    0.033300HRS

PRINT HYD            ID=12 CODE=50

                                  HYDROGRAPH FROM AREA POND.12

0.0
      25.974       0.0         52.614       0.0         79.254       0.0        105.894       0.0        132.534       0.0
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    RUNOFF VOLUME =     1.10157 INCHES      =       0.4373 ACRE-FEET
    PEAK DISCHARGE RATE =      10.24 CFS  AT   1.698 HOURS   BASIN AREA =  0.0074 SQ. MI.


*** ********************
*** SUB BASINS 201.3
*** ********************
COMPUTE NM HYD       ID=1  HYD NO=201.3 AREA= 0.001617 SQ MI
                     PER A=78  PER B=22  PER C=0  PER D=0
                     TP=-.1333 HR  MASS RAIN=-1

     K =  0.152449HR    TP =  0.133300HR    K/TP RATIO =  1.143653     SHAPE CONSTANT, N =  3.096457
     UNIT PEAK =   3.5130    CFS   UNIT VOLUME =   0.9958       B =   289.60      P60 = 2.1000
     AREA =     0.001617 SQ MI    IA =   0.61700 INCHES    INF =   1.57760 INCHES PER HOUR
     RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT =  0.033300

PRINT HYD            ID=1 CODE=1

                                  PARTIAL HYDROGRAPH   201.30


    RUNOFF VOLUME =     0.86460 INCHES      =       0.0746 ACRE-FEET
    PEAK DISCHARGE RATE =       2.51 CFS  AT   1.532 HOURS   BASIN AREA =  0.0016 SQ. MI.


*** ********************
*** SUB BASINS 201.4
*** ********************
COMPUTE NM HYD       ID=1  HYD NO=201.4 AREA= 0.000439 SQ MI
                     PER A=20  PER B=80  PER C=0  PER D=0
                     TP=-.1333 HR  MASS RAIN=-1

     K =  0.138116HR    TP =  0.133300HR    K/TP RATIO =  1.036128     SHAPE CONSTANT, N =  3.406848
     UNIT PEAK =   1.0324    CFS   UNIT VOLUME =   0.9869       B =   313.47      P60 = 2.1000
     AREA =     0.000439 SQ MI    IA =   0.53000 INCHES    INF =   1.33400 INCHES PER HOUR
     RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT =  0.033300

PRINT HYD            ID=1 CODE=1

                                  PARTIAL HYDROGRAPH   201.40


    RUNOFF VOLUME =     0.98842 INCHES      =       0.0231 ACRE-FEET
    PEAK DISCHARGE RATE =       0.80 CFS  AT   1.532 HOURS   BASIN AREA =  0.0004 SQ. MI.


*** ********************
*** SUB BASINS 201.5
*** ********************
COMPUTE NM HYD       ID=1  HYD NO=201.5 AREA= 0.000409 SQ MI
                     PER A=0  PER B=28 PER C=0  PER D=72
                     TP=-.1333 HR  MASS RAIN=-1
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```
     K =  0.072649HR    TP =  0.133300HR    K/TP RATIO =  0.545000     SHAPE CONSTANT, N =  7.106428
     UNIT PEAK =   1.1626    CFS   UNIT VOLUME =   0.9896       B =   526.28      P60 = 2.1000
     AREA =     0.000294 SQ MI    IA =   0.10000 INCHES    INF =   0.04000 INCHES PER HOUR
     RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT =  0.033300


     K =  0.133173HR    TP =  0.133300HR    K/TP RATIO =  0.999050     SHAPE CONSTANT, N =  3.533543
     UNIT PEAK =  0.27730    CFS   UNIT VOLUME =   0.9522       B =   322.78      P60 = 2.1000
     AREA =     0.000115 SQ MI    IA =   0.50000 INCHES    INF =   1.25000 INCHES PER HOUR
     RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT =  0.033300

PRINT HYD            ID=1 CODE=1

                                  PARTIAL HYDROGRAPH   201.50


    RUNOFF VOLUME =     1.91043 INCHES     =       0.0417 ACRE-FEET
    PEAK DISCHARGE RATE =       1.19 CFS  AT   1.532 HOURS   BASIN AREA =  0.0004 SQ. MI.


*** ********************
*** SUB BASINS 201.6
*** ********************
COMPUTE NM HYD       ID=1  HYD NO=201.6 AREA= 0.000545 SQ MI
                     PER A=100  PER B=0  PER C=0  PER D=0
                     TP=-.1333 HR  MASS RAIN=-1

     K =  0.157886HR    TP =  0.133300HR    K/TP RATIO =  1.184438     SHAPE CONSTANT, N =  2.996859
     UNIT PEAK =   1.1512    CFS   UNIT VOLUME =   0.9873       B =   281.57      P60 = 2.1000
     AREA =     0.000545 SQ MI    IA =   0.65000 INCHES    INF =   1.67000 INCHES PER HOUR
     RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT =  0.033300

PRINT HYD            ID=1 CODE=1

                                  PARTIAL HYDROGRAPH   201.60


    RUNOFF VOLUME =     0.82168 INCHES     =       0.0239 ACRE-FEET
    PEAK DISCHARGE RATE =       0.81 CFS  AT   1.532 HOURS   BASIN AREA =  0.0005 SQ. MI.


*** ********************
*** SUB BASINS 201.7
*** ********************
COMPUTE NM HYD       ID=1  HYD NO=201.7 AREA= 0.000909 SQ MI
                     PER A=25  PER B=0  PER C=35  PER D=40
                     TP=-.1333 HR  MASS RAIN=-1

     K =  0.072649HR    TP =  0.133300HR    K/TP RATIO =  0.545000     SHAPE CONSTANT, N =  7.106428
     UNIT PEAK =   1.4355    CFS   UNIT VOLUME =   0.9910       B =   526.28      P60 = 2.1000
     AREA =     0.000364 SQ MI    IA =   0.10000 INCHES    INF =   0.04000 INCHES PER HOUR
     RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT =  0.033300


     K =  0.129055HR    TP =  0.133300HR    K/TP RATIO =  0.968152     SHAPE CONSTANT, N =  3.648501
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     UNIT PEAK =   1.3544     CFS   UNIT VOLUME =   0.9899       B =   331.02       P60 = 2.1000
     AREA =      0.000545 SQ MI     IA =   0.47500 INCHES     INF =   1.18000 INCHES PER HOUR
     RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT =  0.033300

PRINT HYD            ID=1 CODE=1

                                   PARTIAL HYDROGRAPH   201.70


    RUNOFF VOLUME =      1.54680 INCHES      =       0.0750 ACRE-FEET
    PEAK DISCHARGE RATE =       2.26 CFS  AT   1.532 HOURS   BASIN AREA =  0.0009 SQ. MI.



*** ************************************************************
*** EXISTING CONDITIONS FOR THE ONSITE PROJECT AREA
*** ************************************************************
*** SUB BASINS 100.2
*** ONSISTE SUB BASIN
*** ********************
COMPUTE NM HYD       ID=1  HYD NO=201.2 AREA= 0.007444 SQ MI
                     PER A=100  PER B=0  PER C=0  PER D=0
                     TP=-.1333 HR  MASS RAIN=-1

     K =  0.157886HR    TP =  0.133300HR    K/TP RATIO =  1.184438     SHAPE CONSTANT, N =  2.996859
     UNIT PEAK =   15.724     CFS   UNIT VOLUME =   0.9989       B =   281.57       P60 = 2.1000
     AREA =      0.007444 SQ MI     IA =   0.65000 INCHES     INF =   1.67000 INCHES PER HOUR
     RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT =  0.033300

PRINT HYD            ID=1 CODE=1

                                   PARTIAL HYDROGRAPH   201.20


    RUNOFF VOLUME =      0.82168 INCHES      =       0.3262 ACRE-FEET
    PEAK DISCHARGE RATE =      10.88 CFS  AT   1.532 HOURS   BASIN AREA =  0.0074 SQ. MI.


FINISH

    NORMAL PROGRAM FINISH          END TIME (HR:MIN:SEC) = 14:17:12
```

APPENDIX B - HYDRAULICS

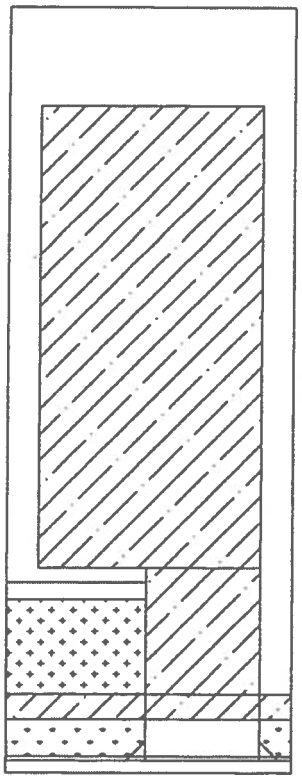
IMPERVIOUS AREA

PAD AREA
2555 SF
FF=72 CF

DRIVEWAY
360 SF
FF=10.2 CF

SIDEWALK
180 SF
FF=5.1 CF

(first flush = SFx 0.34"/12")



DEPRESSED AREA PROVIDED

Concept: Depressed areas are provided to capture the first flush from impervious areas on each individual lot. These depressed areas will be landscaped with rock or grass which will allow drainage to pass through retaining some volume but in some cases will not retain all the flow in the volume provided in this calculation because varying street slopes will effectively reduce the amount of runoff that can be interecepted and retained on any given individual lot.

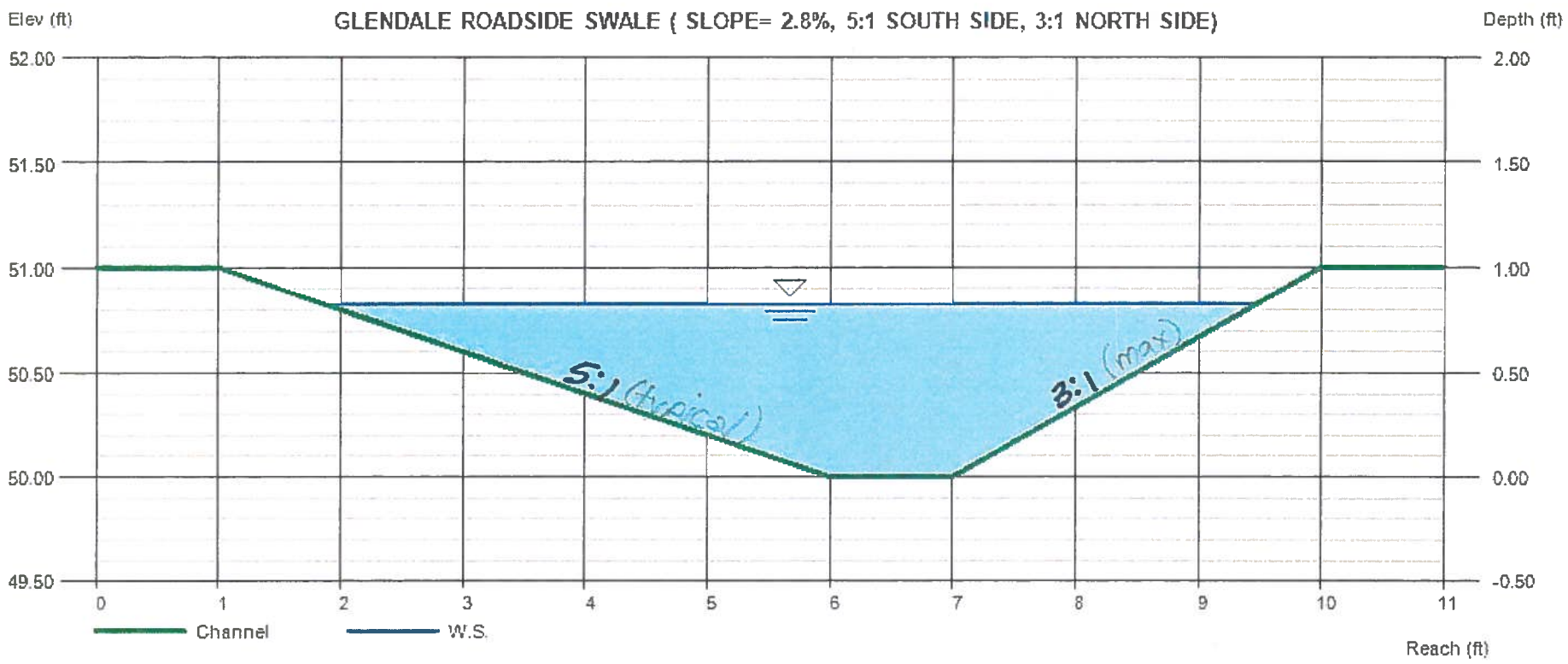
front yard volume=
(19' x 15' x 0.5')/2=
71.25 CF

Curb-sdwk volume=
(19'x5.88'x0.5')/2=
27.9 CF

GLENDESTO SUBDIVISION

FIRST FLUSH CALCULATIONS FOR TYPICAL LOT

(scale 1 inch=30 feet)



Depth	Q	Area	Veloc	Wp	Yc	TopWidth	Energy
(ft)	(cfs)	(sqft)	(ft/s)	(ft)	(ft)	(ft)	(ft)
0.83	30.43	3.586	8.49	7.86	1.00	7.64	1.95

GLENDESTO SUBDIVISION

CALCULATIONS FOR INLET AT END OF SWALE (SAN PEDRO)

*Capacity is measured using the orifice equation: Q= c x a x((2xgxH)**.5)*

FOR SINGLE TYPE D INLET

*Assume 2 feet of head: Q = (0.6) x (**4.56 sq.ft.**) x ((2 x 32.4 x 2 ft)**0.5) = 31.15 cfs*

FOR DOUBLE TYPE D INLET

*Assume 2 feet of head Q = (0.6) x (**9.12 sq.ft.**) x ((2 x 32.4 x 2 ft)**0.5) = 62.30 cfs*

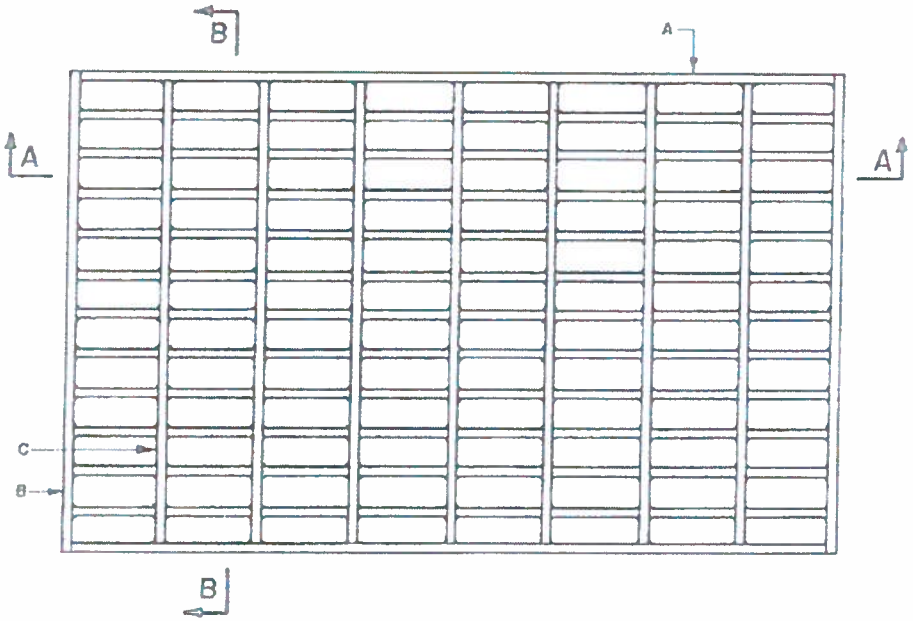
The 100 year flow to the sump in San Pedro is 30.43 cfs.

Assume a 50% clogging factor ~ Use a double D Inlet.

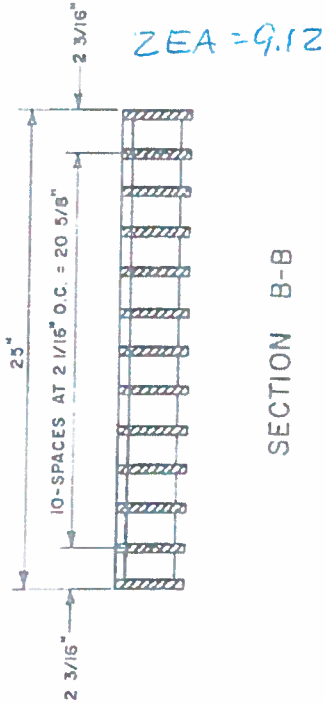
Note: Refer to Standard drawing **2220 DRAINAGE STORM INLET ALBUQUERQUE GRATE**

REVISED 8-12-16

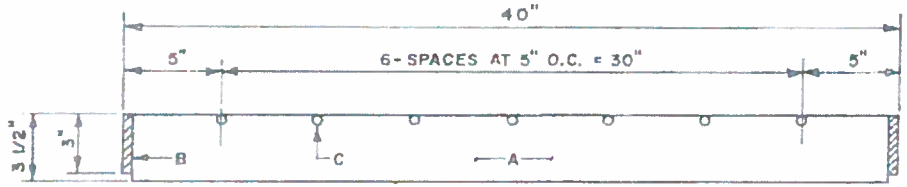
40" - 4.5" = 35.5"
25" - 6.5" = 18.5"
⟩ 656.75 sq in (OPENING)
= 4.56 sq. ft (OPENING) = 1 EA.
2 EA = 9.12 sq. ft.



PLAN



SECTION B-B



SECTION A-A

GENERAL NOTES:

1. ALL BARS SHALL BE STRUCTURAL GRADE STEEL, GRADE A36.
2. THE GRATE SHALL BE WELDED WITH 1/8" FILLET WELD AROUND BOTH SIDES OF CROSS BARS, 1/4". FILLET WELD BOTH SIDES OF BEARING BARS TO END BARS.
3. AFTER CLEANING SURFACE OF SCALE, RUST, OILS, ETC., PAINT GRATE WITH ONE SHOP COAT RED OXIDE, TWO FINISH COATS ALUMINUM PAINT (AASHTO M 69).
4. TOP OF CROSS BARS SHALL BE FLUSH WITH TOP OF GRATE.
5. GRIND WELDS FLUSH WITH BEARING BARS.
6. WHEN INSTALLED IN FRAME, PUSH TIGHT TO ONE SIDE, OTHER SIDE SHALL HAVE 1/2" MAX. OPENING. SPACERS WELDED TO FRAME MAY BE USED IF REQUIRED TO KEEP 1/2" SPACE OR LESS.

CONSTRUCTION NOTES:

A. BEARING BARS, (13) 1/2" X 3 1/2" X 39".

B. END BARS, (2) 1/2" X 3" X 25".

C. CROSS BARS, (7) 1/2" DIA. X 24.

CITY OF ALBUQUERQUE

REVISIONS

DRAINAGE
STORM INLET
ALBUQUERQUE GRATE

DWG. 2220 AUG. 1986

Channel Report

Hydraflow Express Extension for Autodesk® AutoCAD® Civil 3D® by Autodesk, Inc. Thursday, Jul 6 2017

Glendesto Channel

Rectangular

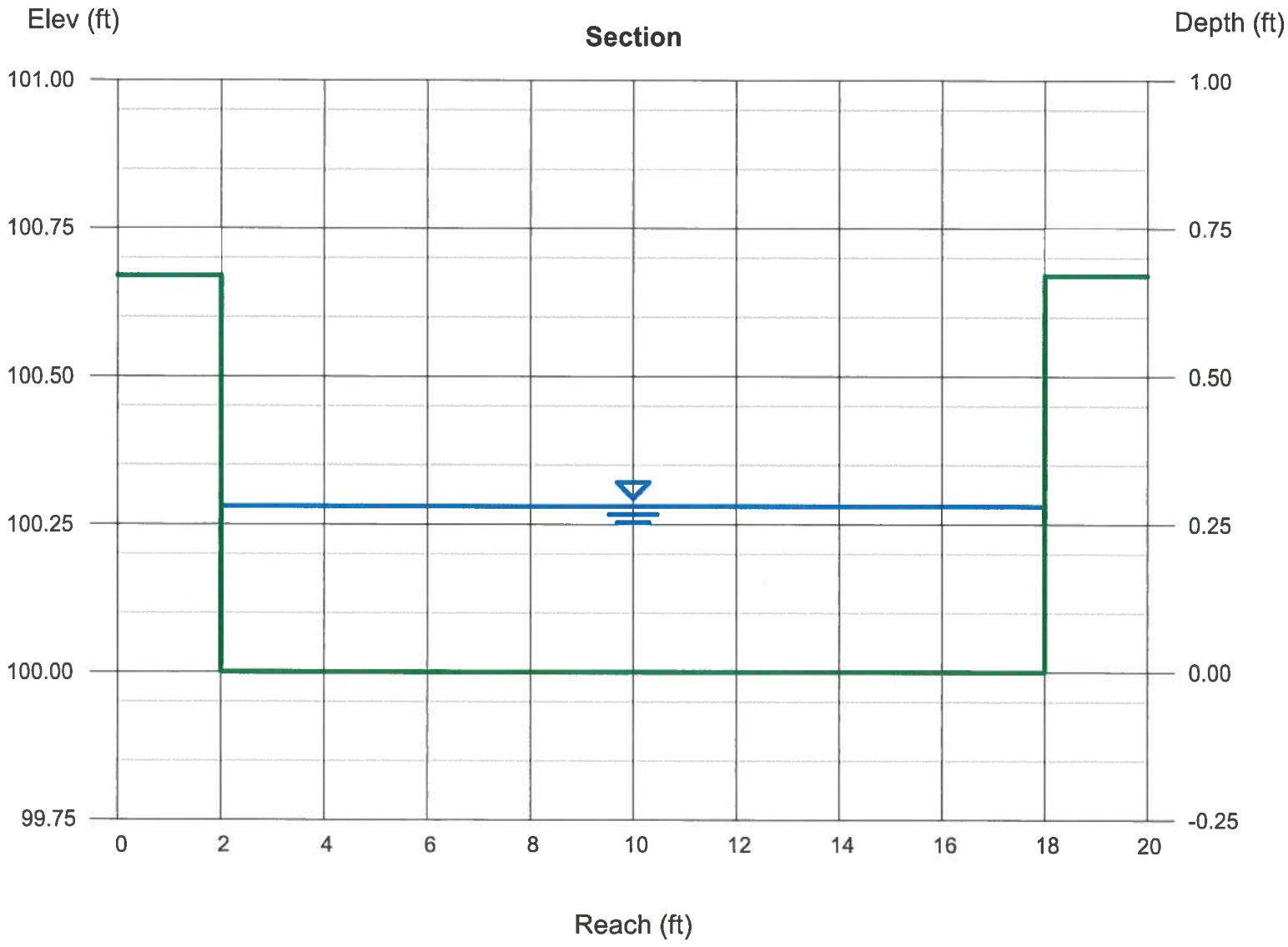
Bottom Width (ft)	= 16.00
Total Depth (ft)	= 0.67
Invert Elev (ft)	= 100.00
Slope (%)	= 1.00
N-Value	= 0.013

Calculations

Compute by:	Known Q
Known Q (cfs)	= 20.48

Highlighted

Depth (ft)	= 0.28
Q (cfs)	= 20.48
Area (sqft)	= 4.48
Velocity (ft/s)	= 4.57
Wetted Perim (ft)	= 16.56
Crit Depth, Yc (ft)	= 0.38
Top Width (ft)	= 16.00
EGL (ft)	= 0.60



Channel Report

Hydraflow Express Extension for Autodesk® AutoCAD® Civil 3D® by Autodesk, Inc. Thursday, Jul 6 2017

Lansdowne Place NE

User-defined

Invert Elev (ft)	= 99.50
Slope (%)	= 0.70
N-Value	= 0.013

Calculations

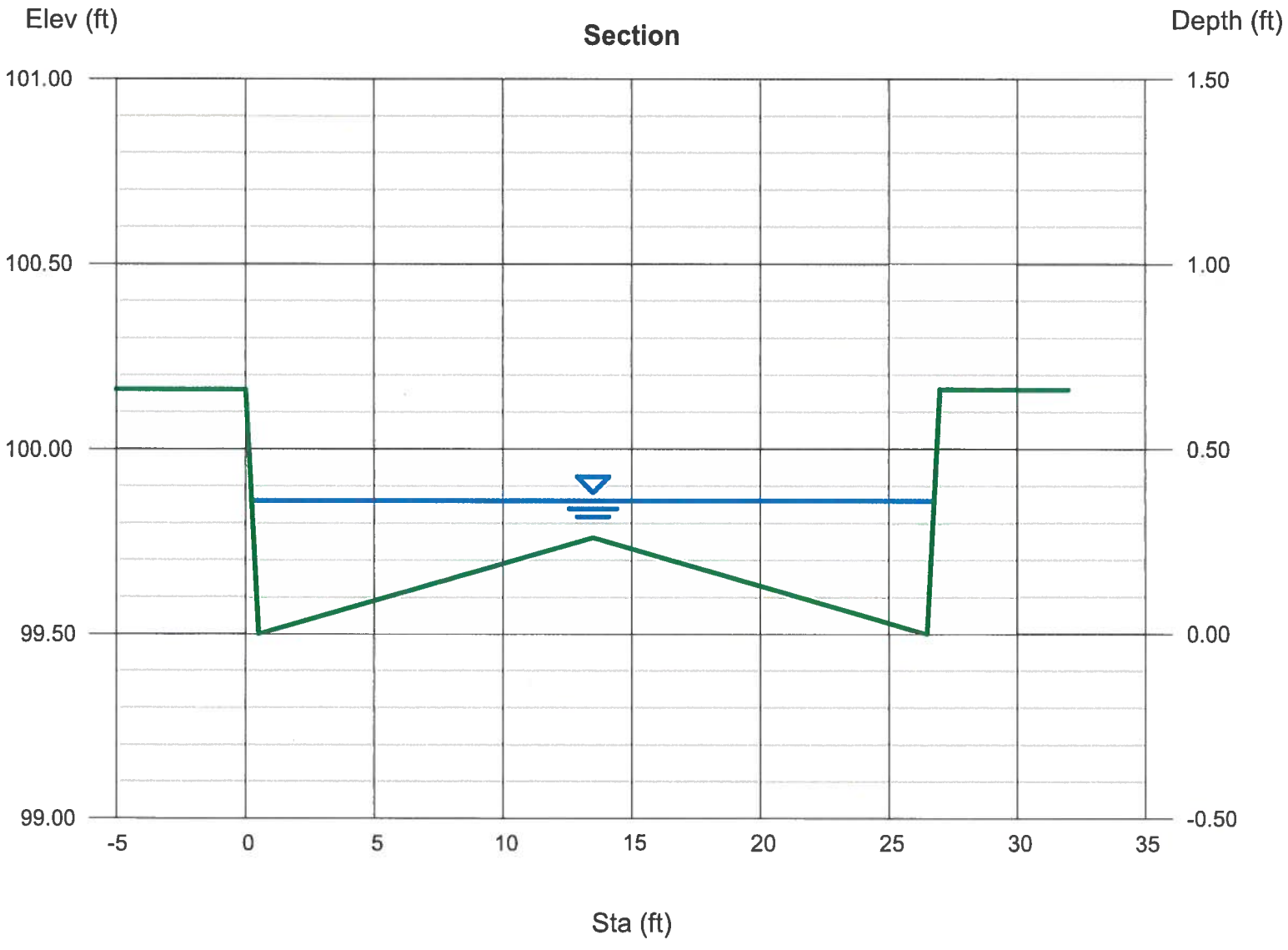
Compute by:	Known Q
Known Q (cfs)	= 20.48

Highlighted

Depth (ft)	= 0.36
Q (cfs)	= 20.48
Area (sqft)	= 6.08
Velocity (ft/s)	= 3.37
Wetted Perim (ft)	= 26.91
Crit Depth, Yc (ft)	= 0.40
Top Width (ft)	= 26.55
EGL (ft)	= 0.54

(Sta, El, n)-(Sta, El, n)...

(0.00, 100.16)-(0.50, 99.50, 0.013)-(2.50, 99.54, 0.013)-(13.50, 99.76, 0.013)-(24.50, 99.54, 0.013)-(26.50, 99.50, 0.013)-(27.00, 100.16, 0.013)



Channel Report

Hydraflow Express Extension for Autodesk® AutoCAD® Civil 3D® by Autodesk, Inc. Thursday, Jul 6 2017

Yawkey Way NE

User-defined

Invert Elev (ft)	= 99.79
Slope (%)	= 4.20
N-Value	= 0.013

Calculations

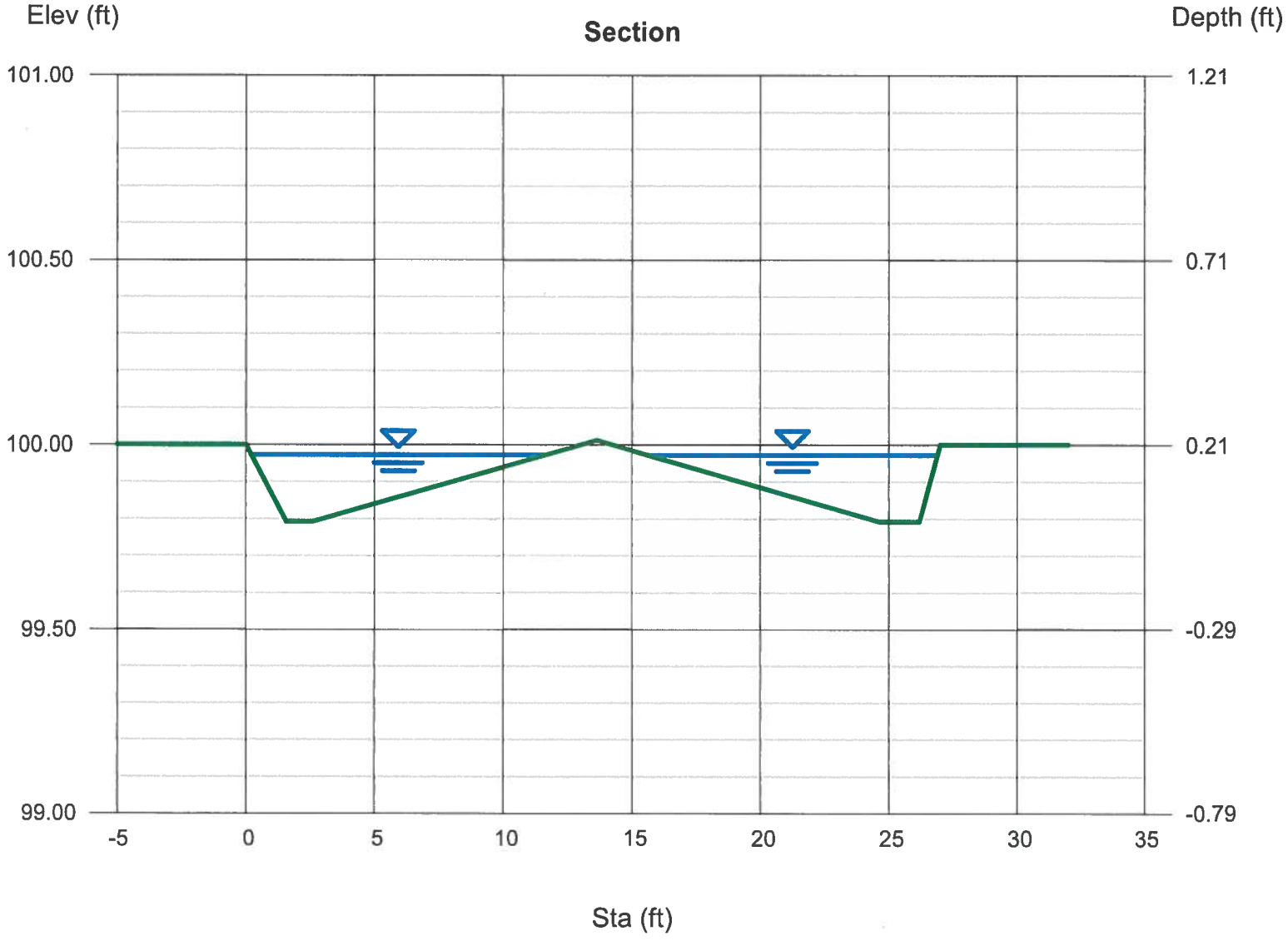
Compute by:	Known Q
Known Q (cfs)	= 10.24

Highlighted

Depth (ft)	= 0.18
Q (cfs)	= 10.24
Area (sqft)	= 2.28
Velocity (ft/s)	= 4.50
Wetted Perim (ft)	= 22.72
Crit Depth, Yc (ft)	= 0.22
Top Width (ft)	= 22.68
EGL (ft)	= 0.49

(Sta, El, n)-(Sta, El, n)...

(0.00, 100.00)-(1.57, 99.79, 0.013)-(2.63, 99.79, 0.013)-(13.63, 100.01, 0.013)-(24.63, 99.79, 0.013)-(26.19, 99.79, 0.013)-(27.00, 100.00, 0.013)



APPENDIX C – FINAL N.A.A. MASTER DRAINAGE PLAN

ID 299

FINAL
NORTH ALBUQUERQUE ACRES
MASTER DRAINAGE PLAN

Prepared For:

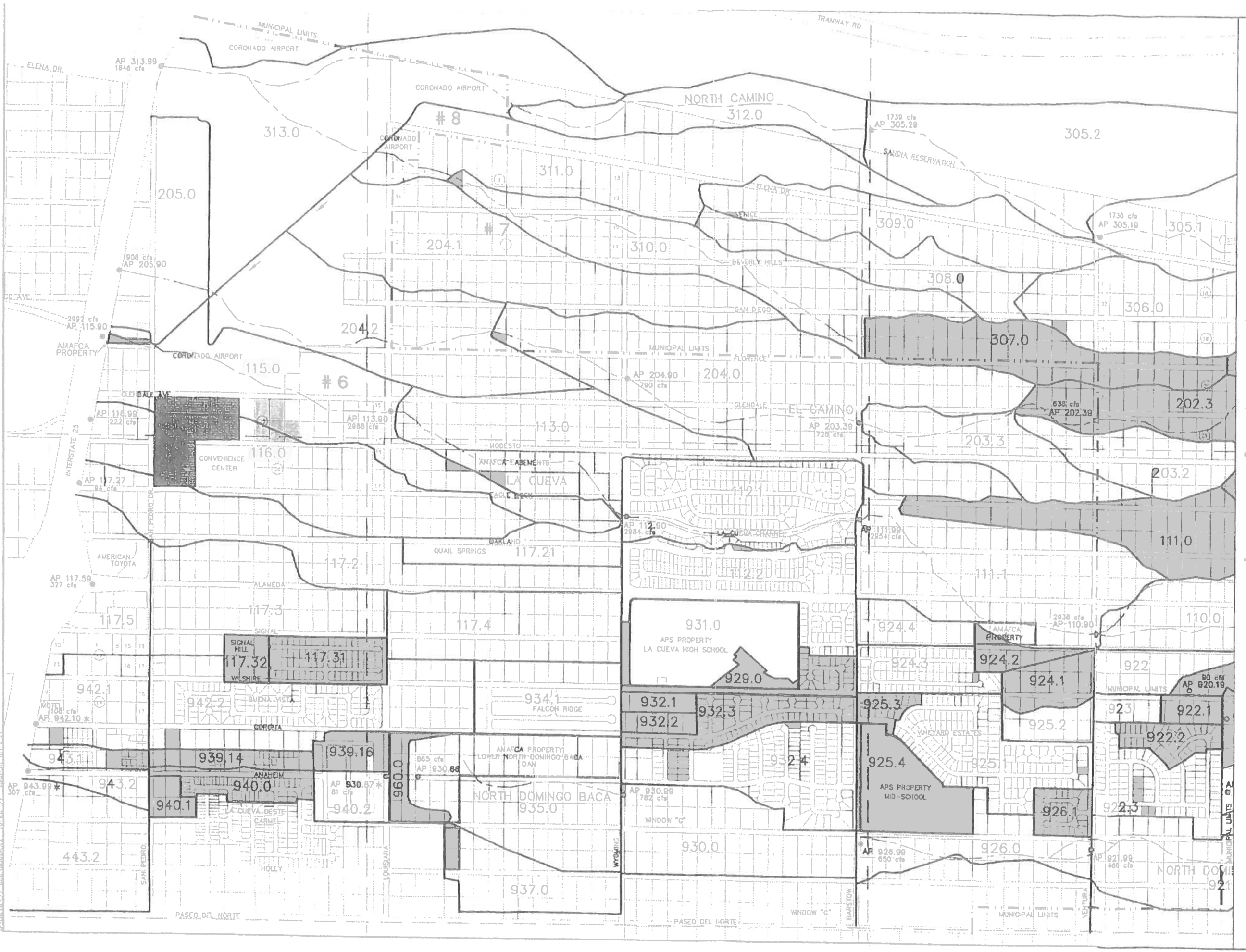
City of Albuquerque

Prepared By:

Resource Technology, Inc.

ENGINEERS AND ENVIRONMENTAL SCIENTISTS
1720-B Randolph Road SE, Albuquerque, NM 87106
Telephone (505) 243-7300
Fax (505) 243-7400
rti@nmia com

October 1998



N

1000 0 500 1000

(IN FEET)

LEGEND

- 107.1 SUBBASIN DESIGNATION
- SUBBASIN BOUNDARY
- EXISTING PLATTING
- EXISTING ARROYO FLOW PATH
- AP 107.29 / 2750 cfs — ANALYSIS POINT AND EXISTING CONDITION FLOW RATE
- * FLOW RATE NOT BULKED FOR SEDIMENT
- #2 POTENTIAL AVULSION LOCATION
- MUNICIPAL LIMITS

NORTH ALBUQUERQUE ACRES MASTER DRAINAGE PLAN

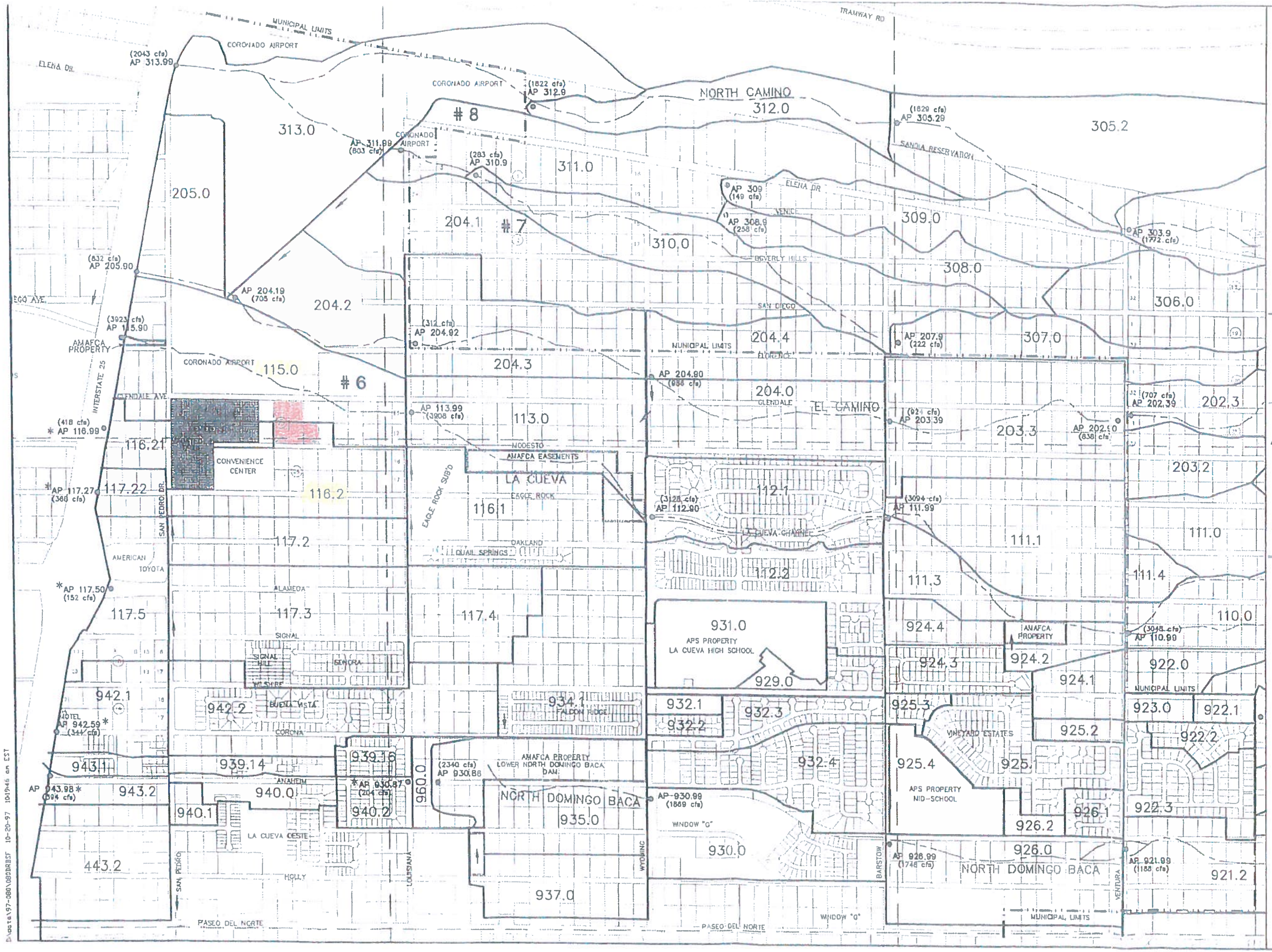
EXISTING CONDITION

FIGURE 3A

CITY OF ALBUQUERQUE
PUBLIC WORKS DEPARTMENT

CITY OF Albuquerque

Resource Technology, Inc.
Civil Engineering
Environmental Sciences
Water Resources
Landscape Architecture
Planning
1720 – B Randolph Road SE
Albuquerque, New Mexico 87106
E-mail: rti@rmia.com
Telephone: (505) 243-7300
Facsimile: (505) 243-7400



LEGEND

- 107.1 SUBBASIN DESIGNATION
- SUBBASIN BOUNDARY
- EXISTING PLATTING
- EXISTING ARROYO FLOW PATH
- AP 107.29 (@58 cfs) ANALYSIS POINT AND FUTURE CONDITION FLOW RATE
- * FLOW RATE NOT BULKED FOR SEDIMENT
- # 2 POTENTIAL AVULSION LOCATION
- MUNICIPAL LIMITS

NORTH ALBUQUERQUE ACRES
MASTER DRAINAGE PLAN

FUTURE CONDITION

FIGURE 4A

CITY OF ALBUQUERQUE
PUBLIC WORKS DEPARTMENT

Resource Technology, Inc.
Civil Engineering
Environmental Sciences
Water Resources
Landscape Architecture
Planning
1720 - B Randolph Road SE
Albuquerque, New Mexico 87106
E-mail: rti@rtinc.com
Telephone: (505) 243-7300
Facsimile: (505) 243-7400

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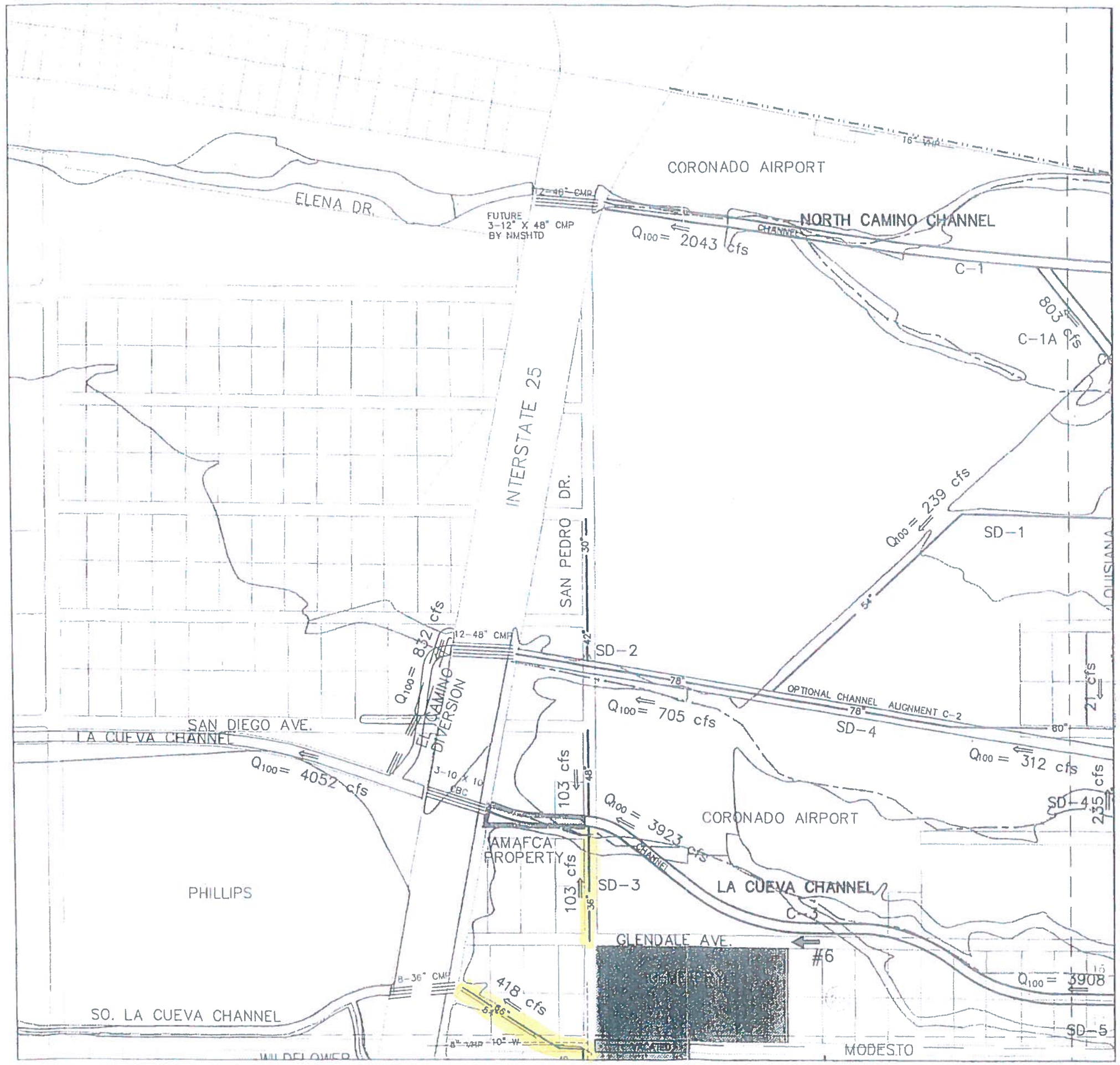

N

500 0 250 500

(IN FEET)

LEGEND

- MUNICIPAL LIMITS
- EXISTING PLATTING
- EXISTING ARROYO FLOW PATH
- 10" W — EXISTING WATER LINE
- 10" SAS — EXISTING SANITARY SEWER
- 12" WIP — EXISTING GAS LINE
- 78" — EXISTING STORM DRAIN
- 54" — PROPOSED STORM DRAIN
- CHANNEL — PROPOSED CHANNEL
- PROPOSED STRUCTURE OR ROAD
- PROPOSED DIKE
- #5 / AV-5 — POTENTIAL AVULSION

NOTE:
All flow rates shown are future condition 100-year.

NORTH ALBUQUERQUE ACRES
MASTER DRAINAGE PLAN

STORM DRAIN FACILITIES B-18

FIGURE 5A

CITY OF ALBUQUERQUE
PUBLIC WORKS DEPARTMENT

CITY OF Albuquerque

Resource Technology, Inc.
Civil Engineering
Environmental Sciences
Water Resources
Landscape Architecture
Planning
1720 - B Randolph Road SE
Albuquerque, New Mexico 87106
E-mail rti@rmia.com
Telephone (505) 243-7300
Facsimile (505) 243-7400

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TABLE A-9 LA CUEVA ARROYO FUTURE CONDITIONS					
Sub-basin	Area (sq. mi.)	10-yr Vol (ac-ft)	10-yr Qp (cfs)	100-yr Vol (ac-ft)	100-yr Qp (cfs)
110.0	.1634	5.774	138.24	11.738	275.61
111.0	.0533	1.823	57.02	3.739	108.83
111.1	.0500	2.054	57.41	7.699	195.97
111.3	.0420	2.498	64.56	4.348	107.90
111.4	.0141	0.482	15.09	0.989	28.80
112.1	.0894	5.152	129.98	8.942	219.11
112.2	.0826	4.760	120.22	8.262	202.31
113.0	.1000	6.393	159.65	10.797	262.65
115.0	.1202	7.581	189.15	12.750	312.21
116.1	.1028	6.570	164.05	11.100	270.05
116.2	.0719	4.529	113.32	7.629	185.54
116.21	.0344	1.682	45.58	3.024	79.13
117.2	.0500	2.788	72.23	4.836	121.61
117.22	.0156	1.108	27.22	1.820	43.06
117.3	.1172	6.536	167.85	11.336	286.33
117.4	.0512	3.225	80.83	5.432	132.07
117.5	.0550	3.907	95.92	6.417	151.76

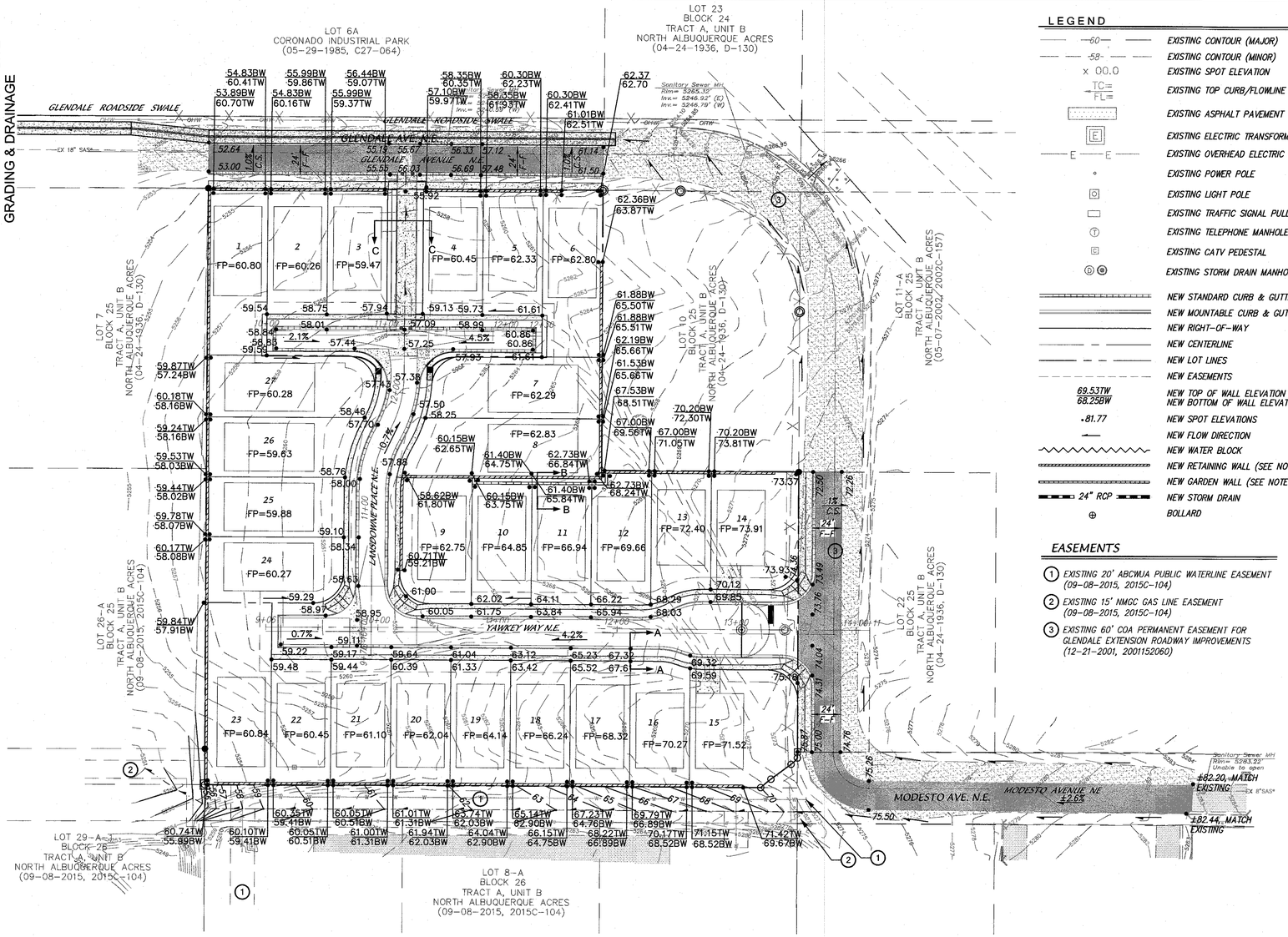
TABLE A-2 (cont.)

LA CUEVA ARROYO SUB-BASIN CHARACTERISTICS

Basin ID	Hydrologic Condition	Basin Area (mi^2)	Land Treatment (%)				TP (hrs)
			A	B	C	D	
113*	Existing	.1136	80	0	15	5	.133
	Future	.1000	0	25	15	60	.133
115*	Existing	.1337	80	0	15	5	.133
	Future	.1202	0	26	12	62	.133
116*	Existing	.1309	80	0	5	15	.133
116.1	Future	.1000	0	25	15	50	.133
116.2	Future	.0719	0	25	15	50	.133
116.21	Future	.0344	0	40	20	40	.133
117.2*	Existing	.1391	73	0	7	20	.22
	Future	.0500	0	34	16	50	.133
117.21*	Existing	.0234	0	34	16	50	.133
117.22*	Future	.0156	0	20	10	70	.133
117.3*	Existing	.0863	65	5	15	15	.133
	Future	.1172	0	34	16	50	.133
117.31*	Existing	.0250	0	34	16	50	.133
117.32*	Existing	.0090	0	34	16	50	.133
117.4*	Existing	.0750	85	0	5	10	.133
	Future	.0512	0	25	15	60	.133
117.5*	Existing	.0550	0	10	20	70	.133
	Future	.0550	0	10	20	70	.133
118	Existing	.0649	0	20	10	70	.133
	Future	.0649	0	20	10	70	.133
118.1	Existing	.0306	75	5	10	10	.133
	Future	.0306	0	20	30	50	.133
119	Existing	.0549	0	20	10	70	.133
	Future	.0549	0	20	10	70	.133
120	Existing	.0268	50	0	0	50	.133
	Future	.0268	0	20	10	70	.133
121	Existing	.0489	80	0	15	5	.133
	Future	.0489	0	20	10	70	.133

*Modified for COA NAA MDP 9/97

SEE SHEET 2 FOR OFF-SITE GRADING & DRAINAGE

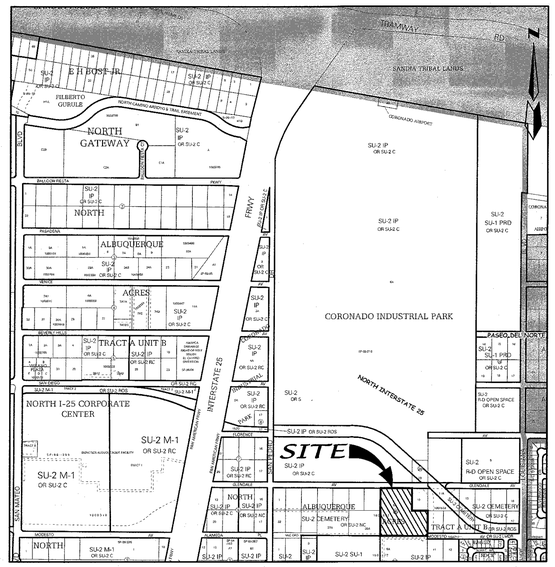


LEGEND

- EXISTING CONTOUR (MAJOR)
- EXISTING CONTOUR (MINOR)
- EXISTING SPOT ELEVATION
- EXISTING TOP CURB/FLOWLINE ELEVATION
- EXISTING ASPHALT PAVEMENT
- EXISTING ELECTRIC TRANSFORMER
- EXISTING OVERHEAD ELECTRIC LINE
- EXISTING POWER POLE
- EXISTING LIGHT POLE
- EXISTING TRAFFIC SIGNAL PULLBOX
- EXISTING TELEPHONE MANHOLE
- EXISTING CATV PEDESTAL
- EXISTING STORM DRAIN MANHOLE
- NEW STANDARD CURB & GUTTER
- NEW MOUNTABLE CURB & GUTTER
- NEW RIGHT-OF-WAY
- NEW CENTERLINE
- NEW LOT LINES
- NEW EASEMENTS
- NEW TOP OF WALL ELEVATION / NEW BOTTOM OF WALL ELEVATION
- NEW SPOT ELEVATIONS
- NEW FLOW DIRECTION
- NEW WATER BLOCK
- NEW RETAINING WALL (SEE NOTE 7 & 8)
- NEW GARDEN WALL (SEE NOTE 7)
- NEW STORM DRAIN
- BOLLARD

EASEMENTS

1. EXISTING 20' ABCWUA PUBLIC WATERLINE EASEMENT (09-08-2015, 2015C-104)
2. EXISTING 15' NMGC GAS LINE EASEMENT (09-08-2015, 2015C-104)
3. EXISTING 60' COA PERMANENT EASEMENT FOR GLENDALE EXTENSION ROADWAY IMPROVEMENTS (12-21-2001, 2001152060)



VICINITY MAP (NTS)

ZONE ATLAS MAP B-18

LEGAL DESCRIPTION

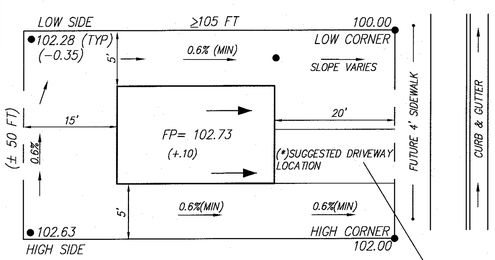
A TRACT OF LAND SITUATE WITHIN THE ELENA GALLEGO GRANT, PROJECTED SECTION 12, TOWNSHIP 11 NORTH, RANGE 3 EAST, NEW MEXICO PRINCIPAL MERIDIAN, CITY OF ALBUQUERQUE, BERNALILLO COUNTY, NEW MEXICO, BEING ALL OF LOT 8-A, BLOCK 25, TRACT A, UNIT B, NORTH ALBUQUERQUE ACRES, AS THE SAME IS SHOWN AND DESIGNATED ON SAID PLAT, FILED FOR RECORD IN THE OFFICE OF THE COUNTY CLERK OF BERNALILLO COUNTY, NEW MEXICO, ON SEPTEMBER 8, 2015, IN PLAT BOOK 2015C, PAGE 104, AND CONTAINING 4.7676 ACRES MORE OR LESS.

NOTES

1. CONTRACTOR MUST OBTAIN A TOPSOIL DISTURBANCE PERMIT FROM THE ENVIRONMENTAL HEALTH DIVISION PRIOR TO CONSTRUCTION.
2. CITY OF ALBUQUERQUE STANDARD SPECIFICATIONS FOR PUBLIC WORKS CONSTRUCTION, LATEST EDITION SHALL GOVERN ALL WORK.
3. THE CONTRACTOR SHALL CONFORM TO ALL CITY, COUNTY, STATE AND FEDERAL DUST CONTROL MEASURES AND REQUIREMENTS AND WILL BE RESPONSIBLE FOR PREPARING AND OBTAINING ALL NECESSARY APPLICATIONS AND APPROVALS.
4. THE CONTRACTOR SHALL ENSURE THAT NO SOIL ERODES FROM THE LOTS INTO PUBLIC RIGHT-OF-WAY. THIS CAN BE ACHIEVED BY CONSTRUCTING TEMPORARY BERMS AND WETTING THE SOIL TO KEEP IT FROM BLOWING.
5. THE EARTHWORK CONTRACTOR SHALL STOCKPILE ENOUGH MATERIAL ADJACENT TO RETAINING WALL LOCATIONS TO BE UTILIZED FOR WALL BACKFILL.
6. SITE DOES NOT LIE IN A 100 YEAR FLOOD ZONE.
7. ALL SITE WALLS SHALL CONFORM TO THE GENERAL HEIGHT AND DESIGN REGULATIONS CONTAINED IN SECTION 14-16-3-19 OF THE CITY ZONING CODE.
8. COMBINATION GARDEN WALL/RETAINING WALL CANNOT EXCEED 8 FEET. IF THE RETAINING WALL IS GREATER THAN 4 FEET, THE DEVELOPER CAN ADD 3:1 SLOPES IN BACKYARD OR A SECOND RETAINING WALL OFFSET FROM THE FIRST ONE.

LANDSCAPE BUFFER NOTES:

1. SWALE TO BE 6" DEEP WHEN THE DISTANCE BETWEEN BACK OF CURB AND THE SIDEWALK IS 5 FEET.
2. FINAL GRADE OF DIRT TO BE 1 TO 2 INCHES BELOW TOP OF CURB AND TOP OF SIDEWALK GRADE.
3. SURFACE BETWEEN BACK OF CURB AND SIDEWALK TO BE COVERED WITH GRAVEL MULCH (MINIMUM 3/4"), COBBLES OF RIP-RAP. DO NOT FILL ENTIRE SWALE.
4. LANDSCAPE FABRIC IS RECOMMENDED, BUT NOT REQUIRED, BETWEEN THE DIRT AND THE STONE. IF LANDSCAPE FABRIC IS TO BE USED IT IS TO BE PERMEABLE.

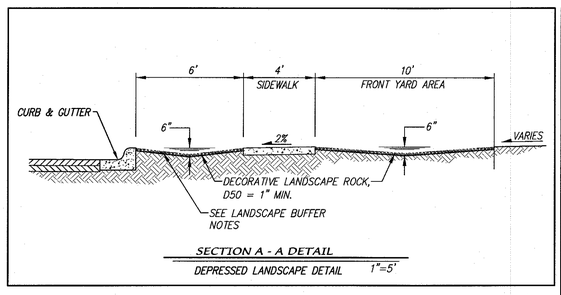


TYPICAL LOT LAYOUT PLAN

N.T.S.

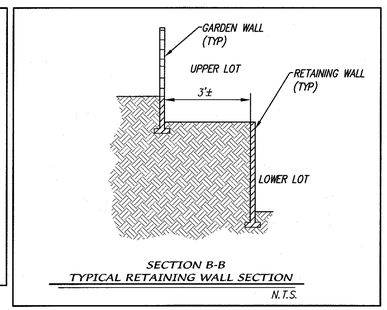
(*) SPECIAL NOTE FOR DRIVEWAYS:

TO MINIMIZE DRIVEWAY SLOPE ON ALL LOTS WHERE THE FINISHED PAD IS > 2.0 FEET HIGHER THAN THE LOW CORNER OF THE LOT, THE DRIVEWAY SHOULD BE CONSTRUCTED ON THE HIGH SIDE OF THE PAD/LOT.



SECTION A - A DETAIL

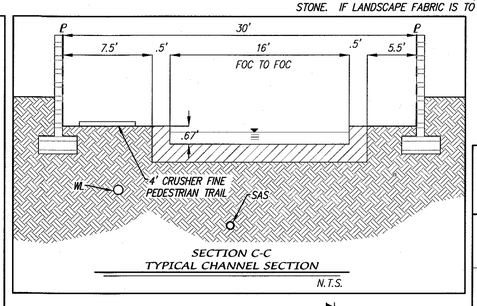
DEPRESSED LANDSCAPE DETAIL 1"=5'



SECTION B-B

TYPICAL RETAINING WALL SECTION

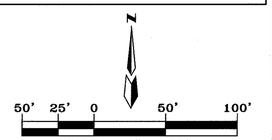
N.T.S.



SECTION C-C

TYPICAL CHANNEL SECTION

N.T.S.



SCALE: 1" = 50'

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AS BUILT INFORMATION

CONTRACTOR — DATE
WORK STAKED BY
INSPECTOR'S INSPECTION BY
FIELD VERIFICATION BY — DATE
DRAWING CORRECTED BY
MICRO-FILM INFORMATION
RECORDED BY — DATE
NO.

BENCH MARKS

AGRS Brass Cap stamped "10-C18"
Elevation, in feet (NAVD88)=5522.090

SURVEY INFORMATION

FIELD NOTES

NO.	BY	DATE

ENGINEER'S SEAL

ARNE HOELZER NEW MEXICO 17967 REGISTERED PROFESSIONAL ENGINEER

7-7-17

NO.	DATE	REMARKS / REVISIONS	BY	DATE
		DESIGN	DLH	05/16
		DRAWN BY	DER	05/16
		CHECKED BY	DMG	05/16

dmg
MARK GOODWIN & ASSOCIATES, P.A.
CONSULTING ENGINEERS
P.O. BOX 90606
ALBUQUERQUE, NEW MEXICO 87199
OFFICE (505) 828-2200, FAX (505) 797-9539

CITY OF ALBUQUERQUE
PUBLIC WORKS DEPARTMENT

TITLE:

GLENDESTO SUBDIVISION GRADING & DRAINAGE PLAN

DESIGN REVIEW COMMITTEE	CITY ENGINEER APPROVAL	LAST DESIGN UPDATE	MO./DAY/YR.	MO./DAY/YR.

CITY PROJECT NO. .

ZONE MAP NO. B-18-Z

SHEET 1 OF 2

54" RCP STORM PIPE

EXISTING INLET AND STORM MANHOLE

NEW 36" STORM PIPE

NEW 36" STORM PIPE

NEW 10 FT. WIDE, 1.0 FT DEEP,5:1 SIDE SLOPES (MIN) "SHOTCRETE" SWALE WITH 4" YELLOW STRIPING ALONG ROADWAY EDGE; SEE DRAINAGE REPORT FOR SWALE DETAIL; TO BE DESIGNED AT DRC.

GLENDALE ROADSIDE SWALE

Sanitary Sewer MH
Rim= 5245.81'
Inv.= 5239.45' (E)
Inv.= 5229.39' (W)

GLENDALE ROADSIDE SWALE

NEW 6' DIA MANHOLE AND 36" RCP AND NEW DOUBLE D INLET.

GLENDALE AVENUE N.E.

EX 18" SAS

EX 18" SAS

SAN PEDRO DRIVE N.E.

CORONADO (05-29-

54.83BW
60.41TW
53.89BW
60.70TW
54.83BW
60.16TW
55.99BW
59.86TW

52.64
53.00
24' F-F

1
FP=60.80
2
FP=60.26

59.54
58.84
59.84
58.01
2.1%

27
FP=60.2

26
FP=59.6

25
FP=59.8

24
FP=60.2

59.87TW
57.24BW
60.18TW
58.16BW
59.24TW
58.16BW
59.53TW
58.03BW
59.44TW
58.02BW
59.78TW
58.07BW
60.17TW
58.08BW
59.84TW
57.91BW

LOT 7
BLOCK 25
TRACT A, UNIT B
NORTH ALBUQUERQUE ACRES
(04-24-1936, D-130)

26-A
BLOCK 25
TRACT A, UNIT B
NORTH ALBUQUERQUE ACRES
(04-24-1936, D-130)
LOT 5, 2015C-104)

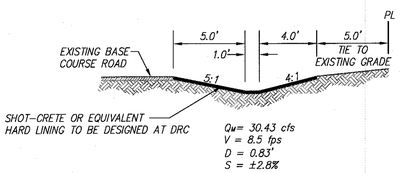
SEE SHEET 1 FOR ON-SITE GRADING & DRAINAGE

N

50' 25' 0 50' 100'

SCALE: 1" = 50'

EASEMENTS



SHOT-CRETE SWALE DETAIL
1"=5'

LEGEND

- EXISTING CONTOUR (MAJOR)
- EXISTING CONTOUR (MINOR)
- EXISTING SPOT ELEVATION
- EXISTING TOP CURB/FLOWLINE ELEVATI
- EXISTING ASPHALT PAVEMENT
- EXISTING ELECTRIC TRANSFORMER
- EXISTING OVERHEAD ELECTRIC LINE
- EXISTING POWER POLE
- EXISTING LIGHT POLE
- EXISTING TRAFFIC SIGNAL PULLBOX
- EXISTING TELEPHONE MANHOLE
- EXISTING CATV PEDESTAL
- EXISTING STORM DRAIN MANHOLE
- NEW STANDARD CURB & GUTTER
- NEW MOUNTABLE CURB & GUTTER
- NEW RIGHT-OF-WAY
- NEW CENTERLINE
- NEW LOT LINES
- NEW EASEMENTS
- 69.53TW / 68.25BW — NEW TOP OF WALL ELEVATION / NEW BOTTOM OF WALL ELEVATION
- 81.77 — NEW SPOT ELEVATIONS
- NEW FLOW DIRECTION
- NEW WATER BLOCK
- NEW RETAINING WALL (SEE NOTE 7 &
- NEW GARDEN WALL (SEE NOTE 7)

SURVEY INFORMATION

FIELD NOTES

NO.	BY	DATE

BENCH MARKS

AGRS Brass Cap stamped "10-C18"
Elevation, in feet (NAVD88)=5222.080

AS BUILT INFORMATION

CONTRACTOR

WORK STAKED BY: DATE
INSPECTIONS: DATE
ACCEPTANCE BY: DATE
FIELD LOCATION BY: DATE
SURVEY DRAWINGS CORRECTED BY: DATE

MICRO-FILM INFORMATION

RECORDED BY: DATE
NO.

ENGINEER'S SEAL

DANIEL HOELZER
NEW MEXICO
19867
REGISTERED PROFESSIONAL ENGINEER
LICENSED PROFESSIONAL ENGINEER
4-17-17

NO.	DATE	REMARKS	BY
		REVISIONS	
		DESIGN	

DESIGNED BY DLH 05/16
DRAWN BY DER 05/16
CHECKED BY DMG 05/16

dmg
MARK GOODWIN & ASSOCIATES, P.A.
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CITY OF ALBUQUERQUE
PUBLIC WORKS DEPARTMENT

TITLE:
GLENDESTO SUBDIVISION GRADING & DRAINAGE PLAN

DESIGN REVIEW COMMITTEE	CITY ENGINEER APPROVAL	LAST DESIGN UPDATE	MO./DAY/YR.	MO./DAY/YR.

CITY PROJECT NO. .
ZONE MAP NO. B-18-Z
SHEET 2 OF 2

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SAN PEDRO

(30.43 cfs)

NEW 10 FT WIDE, 1.0 FT DEEP, 0:1 SIDE SLOPES (MIN) "SHOTCRETE" SWALE WITH 4" YELLOW STRIPING ALONG ROADWAY EDGE. SEE GRADING REPORT FOR SWALE DETAIL TO BE DESIGNED AT DRC.

GLENDALE

GLENDALE ROUGHCUT SWALE

NEW 5' DIA MANHOLE AND 36" RCP AND NEW DOUBLE D INLET.

GLENDALE AVENUE NE

SB 201.0
(4.39 cfs)

SB 201.1
(2.05 cfs)

(26.04 cfs)

LOT 6A
CORONADO INDUSTRIAL PARK
(10-29-1985, C27-064)

LOT 23
BLOCK 24
TRACT A, UNIT B
NORTH ALBUQUERQUE ACRES
(04-24-1936, D-130)

SB 201.2
(20.48 cfs)

SB 201.3
(2.51 cfs)

(1.99 cfs)

SB 201.4
(0.80 cfs)

PROJECT SITE

SB 201.5
(1.19 cfs)

SB 201.7
(2.26 cfs)

MODESTO

MODESTO AVENUE NE

(3.07cfs)

SB 201.6
(0.81cfs)

LOT 29-A
BLOCK 28
TRACT B, UNIT B
NORTH ALBUQUERQUE ACRES
(09-08-2015, 2015C-104)

LOT 5-A
BLOCK 24
TRACT A, UNIT B
NORTH ALBUQUERQUE ACRES
(09-08-2015, 2015C-104)

SAN PEDRO DRIVE NE

EASEMENTS

1. EXISTING 20' ABCWUA PUBLIC WATERLINE EASEMENT (09-08-2015, 2015C-104)
2. EXISTING 15' NMGC GAS LINE EASEMENT (09-08-2015, 2015C-104)
3. EXISTING 60' COA PERMANENT EASEMENT FOR GLENDALE EXTENSION ROADWAY IMPROVEMENTS (12-21-2005, 2005Z-00482)

PROPERTY CORNERS

- FOUND 5/8" REBAR WITH 2" ALUMINUM CAP "LS 12651" (TYP.)
- FOUND 1/2" REBAR WITH CAP "LS 8686" (TYP.)
- FOUND 5/8" REBAR
- FOUND 5/8" REBAR WITH 3" ALUMINUM CAP "LS 11805" (TYP.)
- FOUND 5/8" REBAR WITH CAP "LS 11808" (TYP.)
- SET 1/2" REBAR WITH CAP "LS 7716" (TYP.)

N

100' 50' 0 100' 200'

SCALE: 1" = 100'

AS BUILT INFORMATION

CONTRACTOR — DATE
WORK STAKED BY — DATE
INSPECTOR'S ACCEPTANCE BY — DATE
AS BUILT VERIFICATION BY — DATE
DRAWING CHANGED BY — DATE

MICRO-FILM INFORMATION

RECORDED BY — DATE
NO.

BENCH MARKS

AGRS Brass Cap stamped "10-C18"
Elevation, in feet (NAVD88)=5522.090

SURVEY INFORMATION

NO.	BY	DATE

FIELD NOTES

ENGINEER'S SEAL

JANE HOELZER
NEW MEXICO
14980
REGISTERED PROFESSIONAL ENGINEER

NO.	REMARKS	BY	DATE
	REVISIONS		
	DESIGN	DLH	05/16
	DRAWN BY	DER	05/16
	CHECKED BY	DMG	05/16

dmg
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CITY OF ALBUQUERQUE
PUBLIC WORKS DEPARTMENT

TITLE: *GLENDESTO SUBDIVISION*
PROPOSED DRAINAGE CONDITIONS

DESIGN REVIEW COMMITTEE	CITY ENGINEER APPROVAL	LAST DESIGN UPDATE	MO./DAY/YR.	MO./DAY/YR.

CITY PROJECT NO. .

ZONE MAP NO. B-18-Z

SHEET OF

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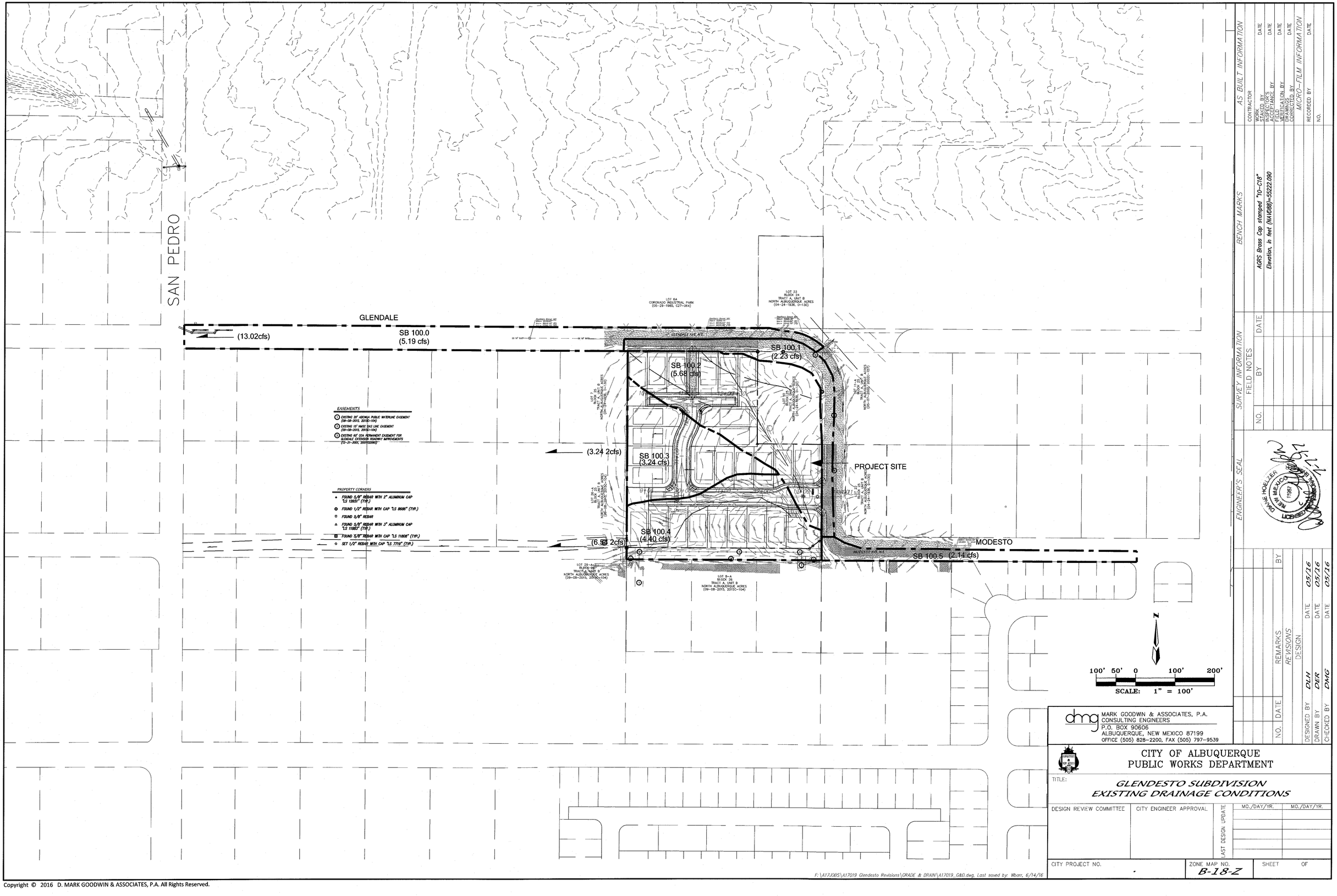

SURVEY INFORMATION

FIELD NOTES

NO.	BY	DATE

BENCH MARKS

AGRS Brass Cap stamped "10-C18"
Elevation, in feet (NAVD88)=5222.090

AS BUILT INFORMATION

CONTRACTOR

	DATE
WORK STARTED BY	
INSPECTOR'S ACCEPTANCE BY	DATE
AS-BUILT VERIFICATION BY	DATE
DRAWINGS CORRECTED BY	DATE

MICRO-FILM INFORMATION

RECORDED BY DATE

NO.

ENGINEER'S SEAL

DUANE HOELZER
NEW MEXICO
11967
REGISTERED PROFESSIONAL ENGINEER
7-7-16

NO.	DATE	REMARKS	BY
		REVISIONS	
		DESIGN	

DESIGNED BY DLH 05/16
DRAWN BY DER DATE 05/16
CHECKED BY DMG DATE 05/16

dmg
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CITY OF ALBUQUERQUE
PUBLIC WORKS DEPARTMENT

TITLE:
GLENDESTO SUBDIVISION
EXISTING DRAINAGE CONDITIONS

DESIGN REVIEW COMMITTEE	CITY ENGINEER APPROVAL	LAST DESIGN UPDATE	MO./DAY/YR.	MO./DAY/YR.

CITY PROJECT NO. .

ZONE MAP NO. B-18-Z

SHEET OF

F:\A17JOBS\A17019 Glendesto Revisions\GRADE & DRAIN\A17019_G&D.dwg, Last saved by: Wbarr, 6/14/16