CITY OF ALBUQUERQUE

Planning Department
Brennon Williams, Director

Mayor Timothy M. Keller

September 8, 2020

Ron Hensley, P.E.
THE Group
300 Branding Iron Rd. SE
Rio Rancho, NM 87124

RE: Glendale Subdivision
8321 Glendale Ave. NE
Grading and Drainage Plan
Engineer's Stamp Date: 08/25/20
Hydrology File: B20D068

Dear Mr. Hensley:

PO Box 1293

Albuquerque

NM 87103

www.cabq.gov

Based upon the information provided in your resubmittal received 08/26/2020, the Grading and Drainage Plan is approved for action by the DRB for Preliminary Plat.

As a reminder, if the project total area of disturbance (including the staging area and any work within the adjacent Right-of-Way) is 1 acre or more, then an Erosion and Sediment Control (ESC) Plan and Owner's certified Notice of Intent (NOI) is required to be submitted to the Stormwater Quality Engineer (Doug Hughes, PE, jhughes@cabq.gov, 924-3420) 14 days prior to any earth disturbance.

If you have any questions, please contact me at 924-3995 or rbrissette@cabq.gov.

Sincerely,

Renée C. Brissette

Renée C. Brissette, P.E. CFM
Senior Engineer, Hydrology
Planning Department

City of Albuquerque

Planning Department

Development & Building Services Division

DRAINAGE AND TRANSPORTATION INFORMATION SHEET (REV 11/2018)

Project Title: GLENDALE SUBDIVISION Building Permit #: _____ Hydrology File #: B20D068

DRB#: 2020-003661 EPC#: _____ Work Order#: _____

Legal Description: LOT 31. BLOCK 3, UNIT 2, TRACT 2 NORTH ALBUQUERQUE ACRES

City Address: 8321 GLENDALE AV NE

Applicant: THE Group Contact: Ron Hensley

Address: 300 Branding Iron Rd. SE, Rio Rancho, NM 87124

Phone#: 505-410-1622 Fax#: _____ E-mail: ron@thegroup.cc

Owner: CLEARBROOK LLC Contact: Scott Henry

Address: 8801 Jefferson NE Bldg. A, ALBUQUERQUE, NM 87113

Phone#: 505-858-1800 Fax#: _____ E-mail: scotth@stillbrooke.com

TYPE OF SUBMITTAL: ✓ PLAT (2 # OF LOTS) ____ RESIDENCE ____ DRB SITE ____ ADMIN SITE

IS THIS A RESUBMITTAL?: ✓ Yes _____ No

DEPARTMENT: _____ TRAFFIC/ TRANSPORTATION ✓ HYDROLOGY/ DRAINAGE

Check all that Apply:

TYPE OF SUBMITTAL:

____ ENGINEER/ARCHITECT CERTIFICATION
____ PAD CERTIFICATION
____ CONCEPTUAL G & D PLAN
✓ GRADING PLAN
____ DRAINAGE MASTER PLAN
____ DRAINAGE REPORT
____ FLOODPLAIN DEVELOPMENT PERMIT APPLIC
____ ELEVATION CERTIFICATE
____ CLOMR/LOMR
____ TRAFFIC CIRCULATION LAYOUT (TCL)
____ TRAFFIC IMPACT STUDY (TIS)
✓ OTHER (SPECIFY) WALL CALCULATIONS
____ PRE-DESIGN MEETING?

TYPE OF APPROVAL/ACCEPTANCE SOUGHT:

____ BUILDING PERMIT APPROVAL
____ CERTIFICATE OF OCCUPANCY
✓ PRELIMINARY PLAT APPROVAL
____ SITE PLAN FOR SUB'D APPROVAL
____ SITE PLAN FOR BLDG. PERMIT APPROVAL
____ FINAL PLAT APPROVAL
____ SIA/ RELEASE OF FINANCIAL GUARANTEE
____ FOUNDATION PERMIT APPROVAL
____ GRADING PERMIT APPROVAL
____ SO-19 APPROVAL
____ PAVING PERMIT APPROVAL
____ GRADING/ PAD CERTIFICATION
____ WORK ORDER APPROVAL
____ CLOMR/LOMR
____ FLOODPLAIN DEVELOPMENT PERMIT
____ OTHER (SPECIFY) _____

DATE SUBMITTED: 8/25/20 By: THE Group / Ron Hensley

COA STAFF: ELECTRONIC SUBMITTAL RECEIVED: _____

FEE PAID: _____

THE group

The HENSLEY ENGINEERING GROUP

August 25, 2020

Hydrology Development
City of Albuquerque
PO Box 1293
Albuquerque, NM 87103

Re: 8321 Glendale Ave. N.E. – Grading and Drainage Plan

We are requesting a review of the attached revised plan and wall calculations in support of the Preliminary Plat of Glendale subdivision. The site is located at 8321 Glendale Ave. N.E. and described as "LOT 31. BLOCK 3, UNIT 2, TRACT 2 NORTH ALBUQUERQUE ACRES", and the submittal covers the impact of the development.

Please contact me at 410-1622 or via email if you have any questions or comments.

Sincerely,

Ron E. Hensley

Ron E. Hensley P.E.
ron@thegroup.cc

This Wall in File: f:\stillbrooke\glendale\glendale walls.rpx

RetainPro (c) 1987-2019, Build 11.20.03.31
License : KW-06062143
License To : THE Group

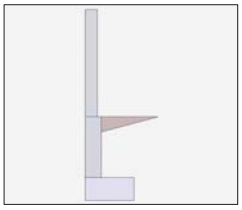
Cantilevered Retaining Wall

Code: IBC 2015,ACI 318-14,ACI 530-13

Criteria

Retained Height	=	2.67 ft
Wall height above soil	=	4.67 ft
Slope Behind Wall	=	0.00
Height of Soil over Toe	=	0.00 in
Water height over heel	=	0.0 ft

RON E. HENSLEY NEW MEXICO 21850 PROFESSIONAL ENGINEER
8/25/20



Load Factors

Building Code	IBC 2015,ACI
Dead Load	1.200
Live Load	1.600
Earth, H	1.600
Wind, W	1.000
Seismic, E	1.000

Soil Data and Lateral Earth Pressure

Allow Soil Bearing	=	1,500.0 psf	Soil Density, Heel	=	110.00 pcf
Coulomb Soil Pressure calculation			Soil Density, Toe	=	110.00 pcf
Soil Friction Angle	=	30.0 deg	Footing||Soil Friction	=	0.400
Active Pressure: Ka*Gamma (horiz)	=	28.3 psf/ft	Soil height to ignore for passive pressure	=	12.00 in
Passive Pressure:Kp*Gar	=	528.8 psf/ft			

Surcharge Loads

Surcharge Over Heel Used To Resist Sliding & Overturning	=	0.0 psf	Surcharge Over Toe Used for Sliding & Overturning	=	0.0 psf

Axial Load Applied to Stem

Axial Dead Load	=	0.0 lbs	Axial Load Eccentricity	=	0.0 in
Axial Live Load	=	0.0 lbs			

Lateral Load Applied to Stem

Lateral Load	=	0.0 #/ft
...Height to Top	=	0.00 ft
...Height to Bottom	=	0.00 ft
Load Type	=	Wind (W) (Service Level)

Wind on Exposed Stem

Wind on Exposed Stem (Service Level)	=	0.0 psf

Adjacent Footing Load

Adjacent Footing Load	=	0.0 lbs	Footing Type		Line Load
Footing Width	=	0.00 ft	Base Above/Below Soil		
Eccentricity	=	0.00 in	at Back of Wall	=	-1.5 ft
Wall to Ftg CL Dist	=	4.00 ft	Poisson's Ratio	=	0.300

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Cantilevered Retaining Wall

Code: IBC 2015,ACI 318-14,ACI 530-13

Wall Design Summary

Stability Ratios

Overturning	=	3.96 OK
Sliding	=	2.20 OK

Soil Bearing

Total Bearing Load	=	1,049 lbs
...resultant ecc.	=	4.10 in
Soil Pressure @ Toe	=	1,062 psf OK
Soil Pressure @ Heel	=	0 psf OK
Allowable	=	1,500 psf
Soil Pressure Less Than Allowable		
ACI Factored @ Toe	=	1,486 psf
ACI Factored @ Heel	=	0 psf
Footing Shear @ Toe	=	0.0 psi OK
Footing Shear @ Heel	=	5.9 psi OK
Allowable	=	82.2 psi

Sliding

Resisting Forces

Vertical Forces	Force
Soil Over Heel (above water table, if any)	391.6 lbs
Soil Over Heel (below water table, if any)	0.0
Water Over Heel	0.0
Buoyant Force	0.0
Sloped Soil Over Heel	0.0
Surcharge Over Heel	0.0
Adjacent Footing Load	0.0
Axial Dead Load on Stem	0.0
Axial Live Load on Stem *	Omit
Soil Over Toe	0.0
Surcharge Over Toe	0.0
Stem Weight(s)	357.0
Earth @ Stem Transitions	0.0
Footing Weight	300.0
Key Weight	0.0
Vert. Component **	0.0
Total Vertical Loads	1,048.6 lbs

Sliding Forces

Lateral Forces	Force
Heel Active Pressure (above water table, if any)	190.6 lbs
Heel Active Pressure (below water table, if any)	0.0
Hydrostatic Force	0.0
* Heel Active Pressure	190.6
Surcharge over Heel	0.0
Adjacent Footing	0.0
Surcharge Over Toe	0.0
Load @ Stem Above Soil	0.0
Added Lateral Load	0.0
Seismic Load	0.0
Seismic-Self-weight	0.0
Lateral on Key	0.0
Totals =	190.6 lbs

*Includes water table effect

* Axial live load NOT included in total displayed , or used for overturning or sliding resistance, but is included for soil pressure calculations.

Sliding Calcs

Lateral Sliding Force	=	190.6 lbs	
less 100% Passive Force	=	- 0.0 lbs	
less 100% Friction Force	=	- 419.4 lbs	
Added Force Req'd	=	0.0 lbs	OK
....for 1.5 Stability	=	0.0 lbs	OK

Vertical component of active lateral soil pressure IS NOT considered in the calculation of soil bearing pressures.

Overturning

Resisting Moments

Resisting Moments	Force		Distance		Moment
Soil Over Heel (above water table, if any)	391.6	lbs	1.33	ft	522.1ft-#
Soil Over Heel (below water table, if any)	0.0				
Water Table	0.0				
Soil Over Heel	391.6		1.33		522.1
Sloped Soil Over Heel	0.0				
Surcharge Over Heel	0.0				
Adjacent Footing Load	0.0				
Axial Dead Load on Stem	0.0				
Axial Live Load on Stem *	0.0				
Soil Over Toe	0.0				
Surcharge Over Toe	0.0				
Stem Weight(s)	357.0		0.28		101.5
Earth @ Stem Transitions	0.0				
Footing Weight	300.0		1.00		300.0
Key Weight	0.0		0.50		
Vert. Component	0.0				
Total Vertical Loads	1,048.6	lbs			
Resisting Moment					**923.6** ft-#
Eccentricity					**-4.1** in

* Axial live load NOT included in total displayed, or used for overturning or sliding resistance, but is included for soil pressure calculations.

Overturning

Overturning Moments

Overturning Moments	Force		Distance		Moment
Heel Active Pressure (above water table, if any)	190.6	lbs	1.22	ft	233.2 ft-#
Heel Active Pressure (below water table, if any)	0.0				
Hydrostatic Force	0.0				
Buoyant Force	0.0				
Surcharge over Heel	0.0				
Adjacent Footing	0.0				
Surcharge Over Toe	0.0				
Load @ Stem Above Soil	0.0				
Added Lateral Load	0.0				
Seismic Load	0.0				
Seismic-Self-weight	0.0				
Totals =	190.6	lbs			
Overturning Moment					233.2 ft-#

Stem Design Summary

			2nd	Bottom
			Stem OK	Stem OK
Design Height Above Ftg	ft	=	2.67	0.00
Wall Material Above "Ht"		=	Masonry	Masonry
Design Method		=	LRFD	LRFD
Thickness		=	6.00	8.00
Rebar Size		=	# 4	# 4
Rebar Spacing		=	48.00	48.00
Rebar Placed at		=	Edge	Edge
Design Data				
fb/FB + fa/Fa		=	**0.000**	**0.124**
Total Force @ Section				
Service Level	lbs	=		
Strength Level	lbs	=		161.5
Moment....Actual				
Service Level	ft-#	=		
Strength Leve	ft-#	=		143.7
Moment.....Allowable	ft-#	=	592.7	1,155.2
Shear.....Actual				
Service Level	psi	=		
Strength Leve	psi	=		10.2
Shear.....Allowable	psi	=	69.7	69.7
Anet	in2	=	11.69	15.85
Rebar Depth 'd'	in	=	2.75	5.25
Masonry Data				
f'm	psi	=	1,500	1,500
Fy	psi	=	60,000	60,000
Solid Grouting		=	No	No
Modular Ratio 'n'		=	21.48	21.48
Wall Weight	psf	=	45.0	55.0
Equiv. Solid Thick.	in	=	3.70	4.60
Masonry Block Type		=	Normal Weight	
Masonry Design Method		=	LRFD	
Concrete Data				
f'c	psi	=		
Fy	psi	=		

Cantilevered Retaining Wall

Code: IBC 2015,ACI 318-14,ACI 530-13

Footing Data

Toe Width	=	0.00 ft	f'c	=	3,000	psi
Heel Width	=	2.00	Fy	=	60,000	psi
Total Footing Width	=	2.00 ft	Footing Concrete Density	=	150.00	pcf
Footing Thickness	=	12.00 in	Min. As %	=	0.0018	
Key Width	=	12.00 in	Rebar Cover @ Top	=	2.00	in
Key Depth	=	0.00 in	@ Bottom	=	3.00	in
Key Distance from Toe	=	0.00 ft				

Footing Design Results

		Toe	Heel	
Factored Pressure	=	1,486	0	psf
Mu' : Upward	=	0	0	ft-#
Mu' : Downward	=	0	592	ft-#
Mu: Design	=	0	-592	ft-#
Actual 1-Way Shear	=	0.00	5.92	psi
Allow 1-Way Shear	=	0.00	43.82	psi

Toe Reinforcing	=	# 4 @ 48.00 in
Heel Reinforcing	=	None Spec'd
Key Reinforcing	=	None Spec'd

Other Acceptable Sizes & Spacings

Toe: phiMn = phi'5'lambda'sqrt(fc)'Sm

Heel: phiMn = phi'5'lambda'sqrt(fc)'Sm

Key: No key defined

Min footing T&S reinf Area	0.52	in2
Min footing T&S reinf Area per fo	0.26	in2 /ft

If one layer of horizontal bars:
#4@ 9.26 in
#5@ 14.35 in
#6@ 20.37 in

If two layers of horizontal bars:
#4@ 18.52 in
#5@ 28.70 in
#6@ 40.74 in

Footing Torsion, Tu	=	0.00	ft-lbs
Footing Allow. Torsion, phi Tu	=	0.00	ft-lbs

If torsion exceeds allowable, provide supplemental design for footing torsion.

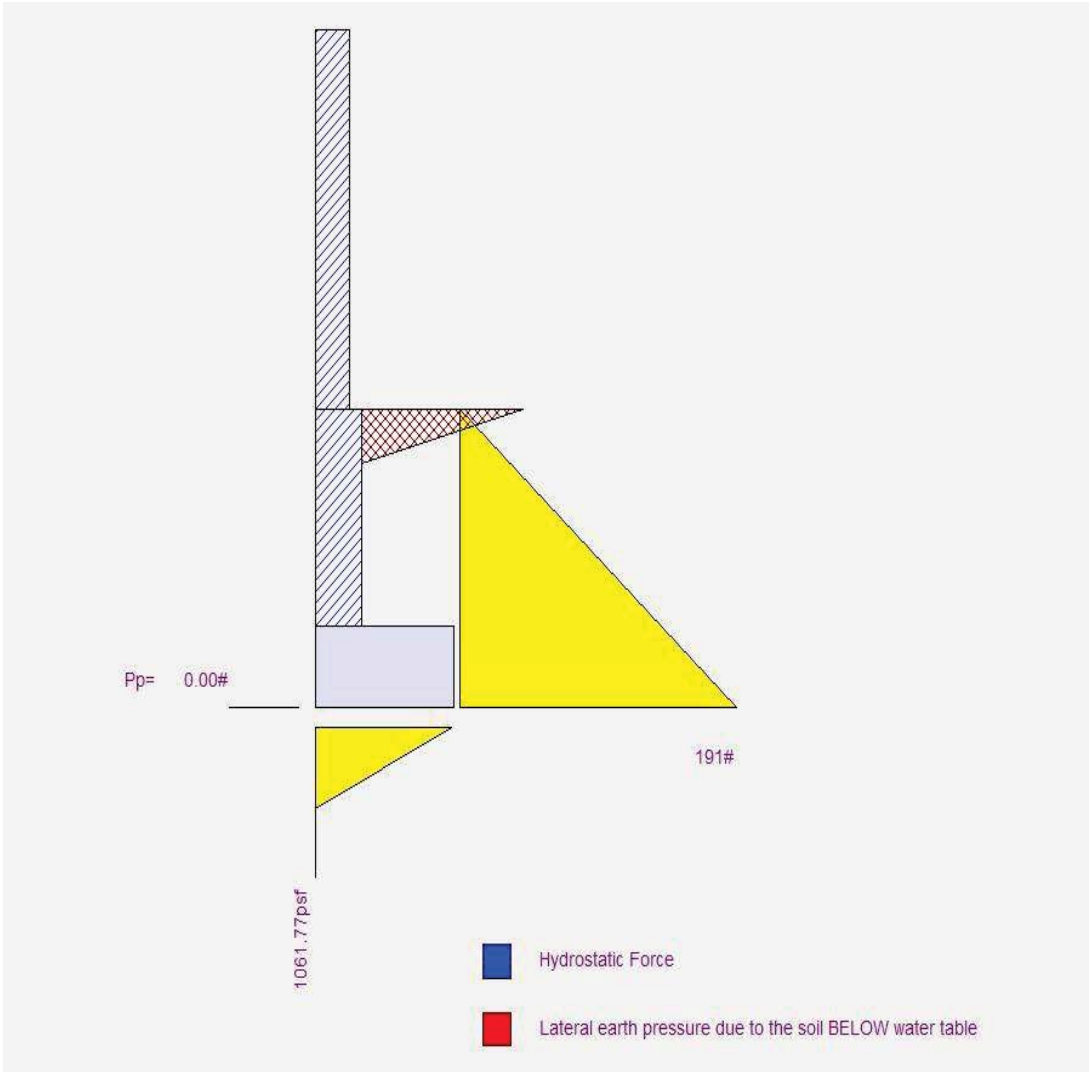
Tilt

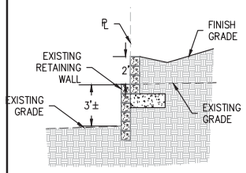
Horizontal Deflection at Top of Wall due to settlement of soil

(Deflection due to wall bending not considered)

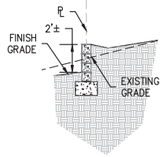
Soil Spring Reaction Modulus	250.0	pci
Horizontal Defl @ Top of Wall (approximate only)	0.108	in

The above calculation is not valid if the heel soil bearing pressure exceeds that of the toe, because the wall would then tend to rotate into the retained soil.

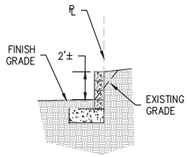

Pp= 0.00#
191#
1061.77psf
Hydrostatic Force
Lateral earth pressure due to the soil BELOW water table



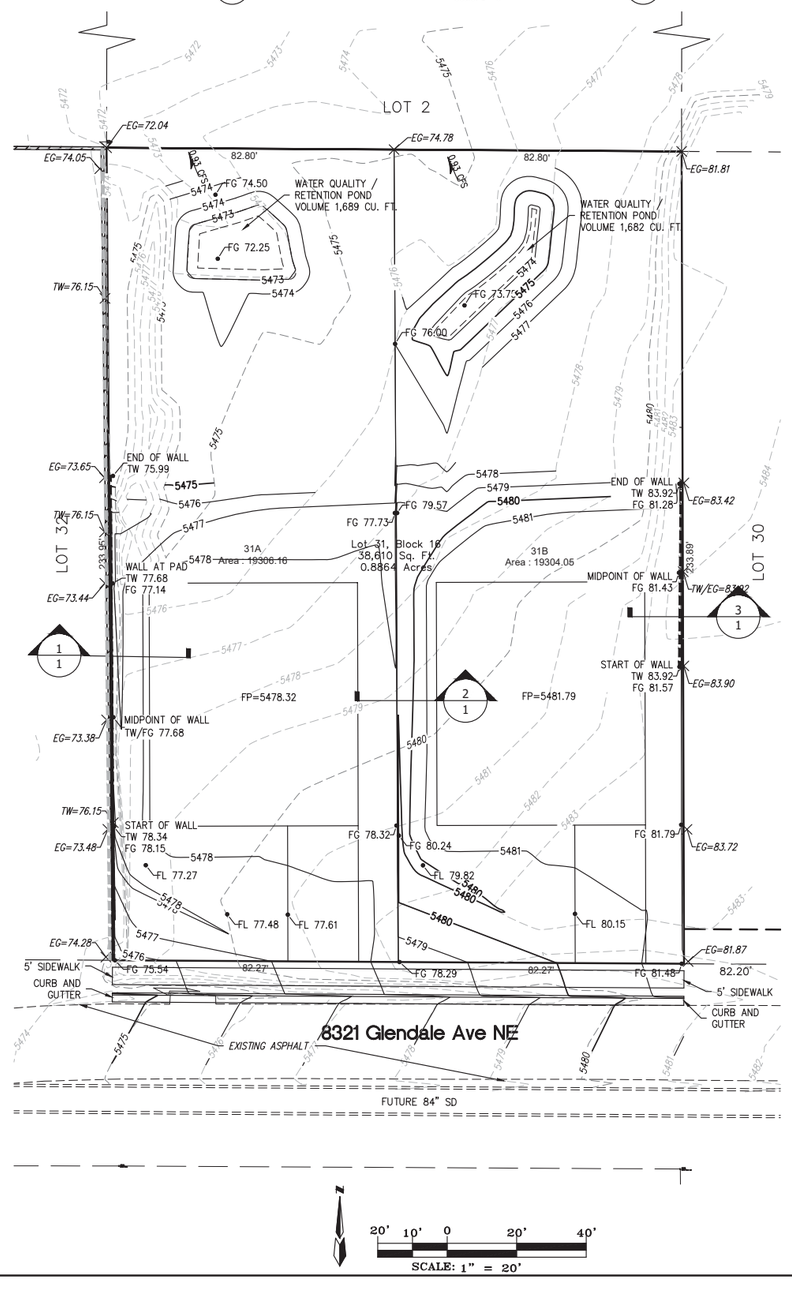
1 SECTION AT PROPERY LINE
Scale: 1"=5'

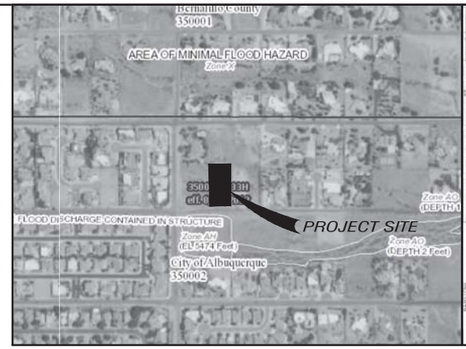


2 SECTION AT PROPERY LINE
Scale: 1"=5'

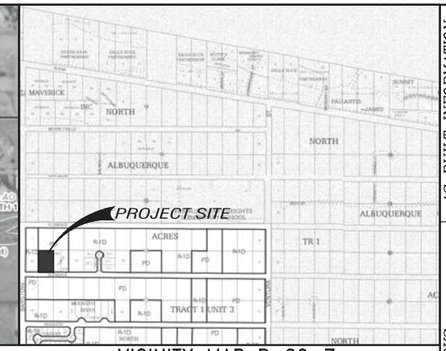


3 SECTION AT PROPERY LINE
Scale: 1"=5'





FIRM MAP NO. 35001C0133H



VICINITY MAP B-20-Z

DRAINAGE INFORMATION

LOCATION & DESCRIPTION

THE PROPOSED SITE IS AN EXISTING NORTH ALBUQUERQUE ACRES LOT LOCATED ON THE NORTH SIDE OF GLENDALE AVENUE EAST OF BARSTOW BLVD. AS SEEN ON THE VICINITY MAP. TO THE WEST IS A SINGLE FAMILY LOT AND THE HISTORIC LOT TO THE EAST IS BEING DEVELOPED AS TWO SINGLE FAMILY LOTS. TO THE NORTH IS AN UNDEVELOPED NORTH ALBUQUERQUE ACRES LOT

FLOODPLAIN STATUS

THIS PROJECT, AS SHOWN ON FEMA'S FLOOD INSURANCE RATE MAP 35001C0133H, DATED AUGUST 16, 2012 IS NOT WITHIN A DESIGNATED 100-YEAR FLOODPLAIN. AN EXHIBIT WITH THE SITE SHOWN ON THE FIRM PANEL IS INCLUDED ON THE GRADING PLAN.

EXISTING DRAINAGE

HISTORIC DRAINAGE WHICH ENTERED THE SITE FROM THE EAST HAS BEEN SIGNIFICANTLY MODIFIED BY GRADING ACTIVITIES AND ARE LIMITED. NO OTHER ADJACENT PROPERTIES DRAIN TO THIS SITE. THIS SITE DRAINS TO THE LOT TO THE NORTH AND TO THE EXISTING CHANNEL VIA HISTORIC DRAINAGE PATTERNS.

DEVELOPED CONDITION

THE APPROPRIATE OUTFALL FOR A PROPOSED DEVELOPMENT IS THE HISTORIC FLOW PATH THROUGH THE ADJACENT PRIVATE PROPERTY (LOT 2), THE HISTORIC FLOW CHARACTERISTICS AND PATH WILL BE MAINTAINED AND THE ADJOINING PROPERTY WILL NOT SEE A CHANGE IN PEAK FLOW OR TOTAL VOLUME. RETENTION PONDS OF ADEQUATE VOLUME TO RETAIN 64% OF THE TOTAL DEVELOPED FLOW FROM EACH PROPOSED LOT FOR A 24 HOUR EVENT. THE VOLUME IS ADEQUATE TO DISCHARGE ONLY HISTORIC FLOW RATES AFTER POINT OF PEAK RUNOFF. HISTORIC DRAINAGE CONDITIONS WILL BE MAINTAINED.

METHODOLOGY

THE HYDROLOGY FOR THIS PROJECT WAS ANALYZED USING THE WEIGHTED E METHOD..

PRECIPITATION

THE 100-YR 24-HR DURATION STORM WAS USED AS THE DESIGN STORM FOR THIS ANALYSIS. THIS SITE IS WITHIN ZONE 4 AS IDENTIFIED IN THE CITY OF ALBUQUERQUE DEVELOPMENT PROCESS MANUAL, CHAPTER 6.

EQUATIONS:
WEIGHTED E = Ea*Aa + Eb*Ab + Ec*Ac + Ed*Ad / (Total Area)

FLOW = Qa * Aa + Qb * Ab + Qc * Ac + Qd * Ad

WHERE FOR 100-YEAR, 24-HOUR STORM(ZONE4)
Ea= 0.76 Qa= 2.09
Eb= 0.95 Qb= 2.73
Ec= 1.20 Qc= 3.41
Ed= 3.34 Qd= 4.78

V_{360} = WEIGHTED E* (Aa + Ab + Ac + Ad)
24 RUNOFF VOLUME = $V_{1440} = V_{360} + Ad * (P_{1440} - P_{360}) / 12$ in/ft

BASIN	AREA (sf)	TREATMENT A %	TREATMENT A sf	TREATMENT B %	TREATMENT B sf	TREATMENT C %	TREATMENT C sf	TREATMENT D %	TREATMENT D sf	WEIGHTED E	VOLUME (cu.-ft.)	FLOW (cfs)
EXISTING	19305	100%	19305	0%	0	0%	0	0%	0	0.7600	1223	0.926
PROPOSED	19305	56%	10746	19%	3759	0%	0	25%	4800	1.4385	2314	1.28

24 RUNOFF VOLUME = V_{1440} = 2,314+ 4,800 * (3.60 – 2.64) / 12 in/ft = 2,614

REQUIRED WATER QUALITY VOLUME

SITE DRAINAGE AS DEPICTED ON THE GRADING PLAN SHALL BE MAINTAINED.

EACH LOT DEPICTED HEREON SHALL BE RESPONSIBLE FOR MAINTAINING FIRST FLUSH RUNOFF RETENTION ON THE LOT IMMEDIATELY PRIOR TO DISCHARGE. THE VOLUME SHALL BE EQUAL TO: IMPERVIOUS AREA * (0.44–0.10)/12 IN CUBIC FEET.

IMPERVIOUS AREA / LOT = 4,800 SQ.FT.

REQUIRED VOLUME = 4,800 * (0.44–0.10)/12 = 136 CU.FT. / LOT
VOLUME PROVIDED = LOT A 1,689 CU.FT. / LOT B 1,682 CU.FT.

NOTES

THE ENGINEER HAS UNDERTAKEN LIMITED FIELD VERIFICATION OF THE LOCATION, DEPTH, SIZE, OR TYPE OF EXISTING UNDERGROUND UTILITY LINES, MAKES NO REPRESENTATION PERTAINING THERETO AND ASSUMES NO RESPONSIBILITY OR LIABILITY THEREFORE. THE CONTRACTOR SHALL INFORM ITSELF OF THE LOCATION OF ANY UTILITY LINE IN OR NEAR THE AREA OF THE WORK IN ADVANCE OF AND DURING EXCAVATION WORK. THE CONTRACTOR IS FULLY RESPONSIBLE FOR ANY AND ALL DAMAGE CAUSED BY ITS FAILURE TO LOCATE, IDENTIFY, AND PRESERVE ANY AND ALL EXISTING UTILITIES. THE CONTRACTOR SHALL COMPLY WITH STATE STATUTES, MUNICIPAL AND LOCAL ORDINANCES, RULES AND REGULATIONS PERTAINING TO THE LOCATION OF THESE LINES AND FACILITIES IN PLANNING AND CONDUCTING EXCAVATION, WHETHER BY CALLING OR NOTIFYING THE UTILITIES, COMPLYING WITH "BLUE STAKES" PROCEDURES, OR OTHERWISE.

THESE DRAWINGS DO NOT INCLUDE NECESSARY COMPONENTS FOR CONSTRUCTION SAFETY WHICH SHALL REMAIN THE RESPONSIBILITY OF THE CONTRACTOR.

THIS DOCUMENT, AND THE IDEAS AND DESIGNS INCORPORATED HEREIN, ARE INTENDED FOR USE ON THIS PROJECT AND IS NOT TO BE USED, IN WHOLE OR IN PART, FOR ANY OTHER PROJECT WITHOUT THE WRITTEN AUTHORIZATION OF GND ENGINEERING, LLC IN THE EVENT OF UNAUTHORIZED USE, THE USER ASSUMES ALL RESPONSIBILITY AND LIABILITY WHICH RESULTS.

GENERAL NOTES:

1. ALL WORK DETAILED ON THESE PLANS TO BE PERFORMED UNDER THIS CONTRACT SHALL, EXCEPT AS OTHERWISE STATED OR PROVIDED HEREIN, BE CONSTRUCTED IN ACCORDANCE WITH THE CITY OF ALBUQUERQUE STANDARD SPECIFICATIONS FOR PUBLIC WORKS CONSTRUCTION, 1986 EDITION, UPDATE NO 8.

2. THE EROSION PROTECTION SPECIFIED ON THIS PLAN IS THE MINIMUM RECOMMENDED. THE OWNER IS ENCOURAGED TO INCORPORATE EROSION RESISTANT LANDSCAPING ON AREAS WHERE EROSION MAY OCCUR SUCH AS SLOPES AND SWALES. THE OWNER IS RESPONSIBLE FOR THE INSTALLATION AND MAINTENANCE OF ALL EROSION CONTROL FEATURES NECESSARY TO PRESERVE THE DESIGN INTENT OF THE GRADING PLAN.

3. THE DRAINAGE INFRASTRUCTURE SHOWN ON THIS PLAN IS THE RESPONSIBILITY OF THE PROPERTY OWNER.

4. ALL DISTURBED AREAS OUTSIDE THE BUILDING PAD MUST BE RESEEDED OR LANDSCAPED

5. TWO (2) WORKING DAYS PRIOR TO ANY EXCAVATION, CONTRACTOR MUST CONTACT LINE LOCATING SERVICE, (260–1990) FOR LOCATION OF EXISTING UTILITIES.

6. PRIOR TO CONSTRUCTION, THE CONTRACTOR SHALL EXCAVATE AND VERIFY THE HORIZONTAL AND VERTICAL LOCATIONS OF ALL OBSTRUCTIONS AND EXISTING PAVEMENT. SHOULD A CONFLICT EXIST, THE CONTRACTOR SHALL NOTIFY THE ENGINEER OR SURVEYOR SO THAT THE CONFLICT CAN BE RESOLVED WITH A MINIMUM OF DELAY.

EROSION CONTROL NOTES

1. CONTRACTOR IS RESPONSIBLE FOR OBTAINING A TOPSOIL DISTURBANCE PERMIT PRIOR TO BEGINNING WORK.
2. CONTRACTOR IS RESPONSIBLE FOR MAINTAINING RUN-OFF ON SITE DURING CONSTRUCTION.
3. CONTRACTOR IS RESPONSIBLE FOR CLEANING ALL SEDIMENT THAT GETS INTO EXISTING RIGHT-OF-WAY.
4. REPAIR OF DAMAGED FACILITIES AND CLEANUP OF SEDIMENT ACCUMULATIONS ON ADJACENT PROPERTIES AND IN PUBLIC FACILITES IS THE RESPONSIBILITY OF THE CONTRACTOR.
5. ALL EXPOSED EARTH SURFACES MUST BE PROTECTED FROM WIND AND WATER EROSION PRIOR TO FINAL (CITY) ACCEPTANCE OF ANY PROJECT.

LEGEND

- FLOW ARROW
- SLOPE ARROW
- EL=11.28 PROPOSED ELEVATION
- 66.33 EXISTING ELEVATION
- GRADE BREAK
- PROPOSED CONTOUR
- EXISTING CONTOUR
- PROPOSED EASEMENT
- 4.00% PROPOSED GRADE
- EXISTING WALL
- PROPOSED WALL

AS BUILT INFORMATION
CONTRACTOR
WORK STAKED BY — DATE
INSPECTOR'S FIELD DISTANCE BY — DATE
VERIFICATION BY
DRAWINGS CORRECTED BY — DATE
MICRO-FILM INFORMATION
RECORDED BY — DATE
NO.

BENCH MARKS

SURVEY INFORMATION
FIELD NOTES BY — DATE
NO.

ENGINEER'S SEAL
RON E. HENSLEY NEW MEXICO 21850 REGISTERED PROFESSIONAL ENGINEER
8/25/20

NO.	REMARKS / REVISIONS	BY	DATE
	DESIGN		
		DESIGNED BY REH	DATE JULY 2020
		DRAWN BY REH	DATE JULY 2020
		CHECKED BY REH	DATE JULY 2020

THE Group
300 Branding Iron Rd. SE
Rio Rancho, New Mexico 87124
Phone:(505)410-1622

CITY OF ALBUQUERQUE
PUBLIC WORKS DEPARTMENT
ENGINEERING DEVELOPMENT GROUP

GLENDALE SUBDIVISION
BEING A REPLAT OF LOT 31, BLOCK 3, UNIT 2, TRACT 2, NAA
GRADING & DRAINAGE PLAN

DESIGN REVIEW COMMITTEE	CITY ENGINEER APPROVAL	LAST DESIGN UPDATE	Mo./DAY/YR.	Mo./DAY/YR.

DRB No. 2020-003661
CITY PROJECT No.
ZONE MAP NO. B-20-Z
SHEET 1 OF 1