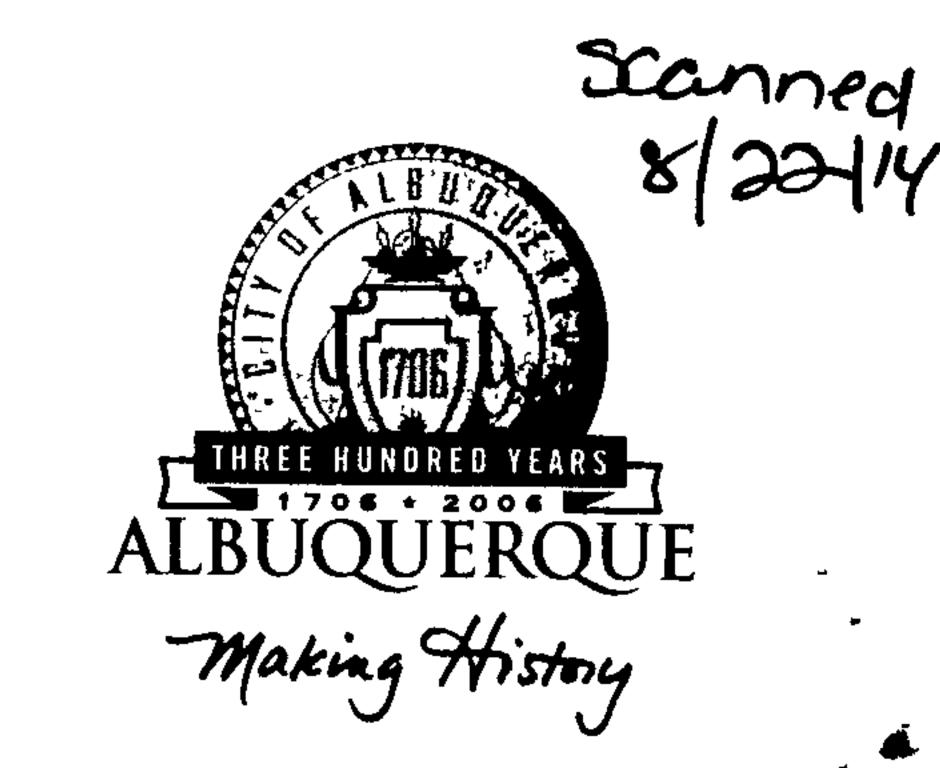
CITY OF ALBUQUERQUE



August 11, 2004

Genevieve Donart, P.E. Isaacson & Arfman, P.A. 128 Monroe St. NE Albuquerque, NM 87108

Re: The Reserve at the Trails Subdivision, Preliminary Plat Engineer's Stamp dated 7-26-04 (C9-D3)

Dear Ms. Donart,

Based upon the information provided in your submittal received 7-27-04, the above referenced plan is approved for Preliminary Plat action by the DRB. Once the DRB has approved the plan, please submit a mylar copy to me in order to obtain rough grading approval.

P.O. Box 1293

This project requires a National Pollutant Discharge Elimination System (NPDES) permit. If you have any questions regarding this permit please feel free to call the DMD Storm Drainage Design section at 768-3654 (Charles Caruso).

Albuquerque

If you have any questions, you can contact me at 924-3981.

New Mexico 87103

www.cabq.gov

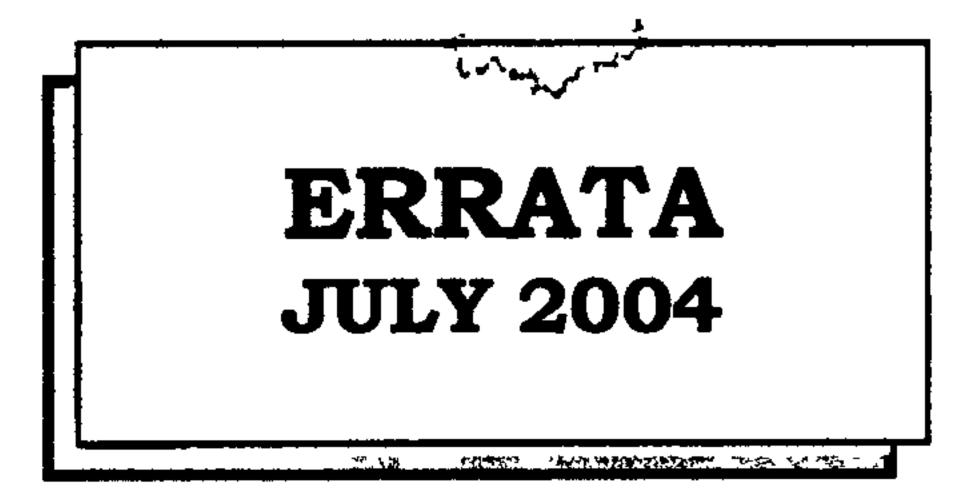
/_____

Kristal D. Metro

Sincerely,

Engineering Associate, Planning Dept. Development and Building Services

C: Charles Caruso, DMD Storm Drainage Design Bradley Bingham, DRB file



DRAINAGE REPORT

FOR

THE RESERVE AT THE TRAILS

A SINGLE-FAMILY RESIDENTIAL SUBDIVISION

ALBUQUERQUE, NEW MEXICO APRIL 2004

Prepared by:

ISAACSON & ARFMAN, P.A. 128 Monroe Street NE

Albuquerque, NM 87108

(505) 268-8828

Genevieve L. Donart, PE

PROFESSION Date

Date

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IV. PROPOSED CONDITIONS

Most of The Reserve at the Trails is contained within Basin G of the BHI Study. The northern row of lots that faces Tree Line Ave and the south half of Tree Line Ave (Basins F10 & F11) drain to Basin F of the BHI Study. No flows from Rainbow Blvd.

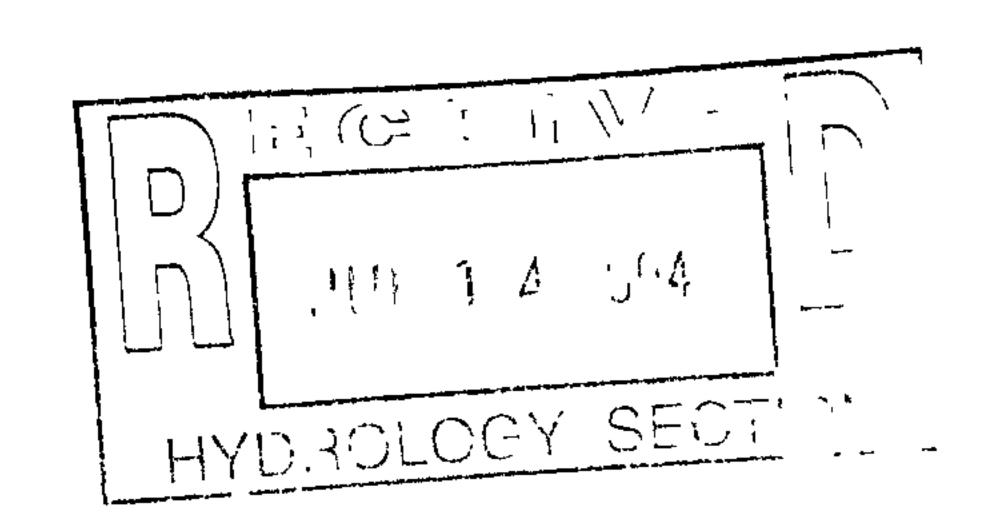
enter the site. Proposed flows for individual sub-basins are determined based on an area percentage of the overall Basin G and Basin F flows. The overall Basin G has an area of 34.1 acres and a fully-developed 100-year flow of 113.2 cfs. This plan develops 46.4% (15.8 acres) of the 34.1 acres. The overall Basin F has an area of 96.5 acres and a fully-developed 100-year flow of 319.7 cfs. This plan develops 3.6% (3.5 acres) of the 96.5 acres. See the Proposed Runoff Calculations in Appendix A.

This report divides Basins F and G into sub-basins. (See the Proposed Basin Map in Appendix D.) Drainage basin boundaries were determined based on the grades established in the Grading & Drainage Plan, and by street flow capacity and storm drain requirements. (See the Grading & Drainage Plans in Appendix E and Street Flow Capacity Calculations in Appendix B.) Grading was strongly affected by the subsurface basalt layer, as well as sewer and storm drain bury depths.

Proposed flows from Basins G1 through G9 are directed east through a storm drain system from Hallston Trail to Pond G. The storm drain system is sized to also carry future developed flows from the adjacent portions of Woodmont Rd. (See the HydraFlow Storm Drain Calculations and Inlet Capacity Calculations in Appendix C.)

Proposed flows from Basins F10 and F11 are directed to Tree Line Ave, where they are captured by inlets that drain to the system as part of the Taos at the Trails subdivision. This storm system also outlets to Pond G.

Pond G was sized as a detention facility at 8.9 Ac-ft by the BHI Study, but the downstream storm drain and pond system required to discharge stormwater from the Pond is not yet designed or constructed. The pond acts as a retention pond until such time as the downstream improvements are constructed. This report sizes the pond at 3.55 Ac-ft to accept the volume of a 100-year, 10-day storm for the developed flows from this project, which is less than the 8.9 Ac-ft required for the final detention pond size. (See the Volume Calculations for Proposed Conditions in Appendix A.) A Covenant & Agreement will be required for this pond.



v. SUMMARY & CONCLUSIONS

Based on information in previous sections, it is recommended that the following items be constructed with The Reserve at the Trails:

- 1. Mountable curb on Winncrest Trail, Winterset Trail, and the portions of Crosswinds Trail west of Lot 40 and Glyndon Trail west of Lot 67.
- 2. Standard curb on Woodmont Rd, Tree Line Avenue, Hallston Trail, and the portions of Crosswinds Trail east of Lot 41 and Glyndon Trail east of Lot 68.
- 3. 2 type 'A' single-grate storm inlets at the east end of Crosswinds Trail.
- 4. 2 type 'C' double-grate sump inlets at the low point in Hallston Trail north of the intersection with Crosswinds Trail.
- 5. 2 type 'C' single-grate sump inlets at the low point in Woodmont Rd.
- 6. 1 type 'A' single-grate storm inlet and 1 type 'C' double-grate inlet at the east end of the adjacent portion of Tree Line Ave that connects to a storm drain manhole designed as part of Taos at the Trails.
- 7. Storm drain as shown on the Grading & Drainage Plan.
- 8. A temporary retention pond east of the project with appropriate Covenant & Agreements and temporary drainage easement.

APPENDIX A

Runoff & Volume Calculations

PROPOSED RUNOFF CALCULATIONS FOR							
THE RESERVE AT THE TRAILS*							
BASIN ID	AREA (Ac)	AREA (mi ²)	% AREA OF	Q ₁₀₀			
TOTAL Q100	BASIN G* =	113.2	TOTAL	(Calc'd			
TOTAL AREA	BASIN G* =	34.1	BASIN G	by %)			
Proposed Rese	erve at the Tr	ails in Basin (3				
G1	1.8414	0.0029	5.4%	6.11			
G2	1.0852	0.0017	3.2%	3.60			
G3	1.8757	0.0029	5.5%	6.23			
G4	1.2863	0.0020	3.8%	4.27			
G5	2.2240	0.0035	6.5%	7.38			
G6	1.0768	0.0017	3.2%	3.57			
G7	3.4780	0.0055	10.2%	11.55			
G8	0.7540	0.0012	2.2%	2.50			
G9	2.2000	0.0046	<u>6.5%</u>	7.30			
<u> </u>	15.8215	0.0260	46.4%	52.52			
	······································						
BASIN ID	AREA (Ac)	AREA (mi ²)	% AREA OF	Q ₁₀₀			
TOTAL Q100	BASIN F* =	319.7	TOTAL	(Calc'd			
TOTAL AREA BASIN F* =		96.5	BASIN F	by %)			
Proposed Rese	erve at the Tr	ails in Basin F					
F10	3.0120	0.0042		9.98			
F11	0.4880	0.0042		1.62			
	3.5000	0.0085	3.6%	11.60			
TOTAL AREA =	19.3215	7	OTAL FLOW =	64.12			
* Q ₁₀₀ quantities	s and basin ar	reas based on	a percentage of	the			
flows calculated for Basins F & G in the "Master Drainage Study							
for The Trails Subdivision" dated 12/10/03 prepared by							
i tor ine italis	Subdivision" (dated 12/10/03	prepared by				

VOLUME CALCULATIONS FOR DEVELOPED CONDITIONS (V₁₀₀)

100-YEAR, 6-HOUR STORM

Per the City of Albuquerque D.P.M. Section 22.2

PROJECT NAME: Reserve at the Trails

JOB NUMBER: 1325

PRECIP	E ₃₆₀ EXCESS PRECIPITATION (in.)					
ZONE	Α	В	C	D		
1	0.44	0.67	0.99	1.97		
2	0.53	0.78	1.13	2.12		
3	0.66	0.92	1.29	2.36		
4	0.80	1.08	1.46	2.64		

% LAND TREATMENTS					
	TREAT	TREAT	TREAT	TREAT	
	TYPE 1	TYPE 2	TYPE 3	TYPE 4	
Α	0	0	0	0	
В	25	25	25	0	
С	25	25	25	0	
D	50	50	50	0	
∑% =	100	100	100	0	

PRECIPITATION ZONE: 1

	TREATMENT TYPE 1							
540114		LAND TRE	ATMENT A	REAS (Ac)		V ₁₀₀	V ₁₀₀	REMARKS
BASIN#	A _{TOTAL}	A _A	A _B	A _C	A _D	(Ac-ft)	(cu.ft.)	REIVIANO
G1	1.8414	0	0.46	0.46	0.92	0.2148	9358.0	
G2	1.0852	0	0.27	0.27	0.54	0.1266	5515.0	(
G3	1.8757	0	0.47	0.47	0.94	0.2188	9532.3	
G4	1.2863	0	0.32	0.32	0.64	0.1501	6537.0	
G5	2.224	0	0.56	0.56	1.11	0.2595	11302.4	
G6	1.0768	0	0.27	0.27	0.54	0.1256	5472.3	
G7	3.478	0	0.87	0.87	1.74	0.4058	17675.2	
G8	0.754	0	0.19	0.19	0.38	0.0880	3831.8	
G9	2.200	0	0.55	0.55	1.10	0.2567	11180.4	
F10	3.012	0	0.75	0.75	1.51	0.3514	15307.0	
F11	0.488	0	0.12	0.12	0.24	0.0569	2480.0	

19.3214 9.66 2.2542 98191.4

RETENTION POND VOLUMES

FOR 100-YEAR, 10-DAY STORM

PROJECT NAME: Reserve at the Trails

JOB NUMBER:

11325

POND G

$$P_{360} = \frac{1}{2.25}$$
 in

(from Fig. C-2, COA DPM)

$$P_{1440} = \frac{1}{1}$$
 2.72 in

(from Fig. C-3, COA DPM)

$$V_{360} = \frac{1}{100}$$
 2.25 Ac-ft (from Volume calcs)
 $A_D = \frac{1}{100}$ 9.66 Ac

$$P_{10day} = 10 - [24.9/(P_{1440})^{1.4}]$$

$$P_{10day} = 3.865165 in$$

$$V_{10day} = V_{360} + A_D(P_{10day} - P_{360})/12$$

$$V_{10day} = 3.5502 \text{ Ac-ft}$$

=> 154647 cu. ft.

APPENDIX B

Street Flow Capacity

THE RESERVE AT THE TRAILS

STREET FLOW DEPTH SUMMARY

STREET	LOCATION	STREET WIDTH	CURB TYPE	SLOPE (ft/ft)	Q ₁₀₀ (cfs)	DEPTH (ft)	
GLYNDON TR	AP1	28' F-F	MTBL	0.0650	6.1	0.17	
GLYNDON TR	AP2	28' F-F	MTBL	0.0400	9.7	0.22	
GLYNDON TR	AP3	28' F-F	STD	0.0400	15.9	0.34	
HALLSTON TR	AP4	28' F-F	STD	0.0050	32.2	0.59	
WINNCREST TR	AP5	28' F-F	MTBL	0.0050	7.4	0.29	
CROSSWINDS TR	AP5	28' F-F	MTBL	0.0400	7.4	0.20	
CROSSWINDS TR	AP6	28' F-F	MTBL	0.0400	11	0.23	
CROSSWINDS TR	AP7	28' F-F	STD	0.0247	22.6	0.41	
WOODMONT RD	AP8	76' F-F	STD	0.0050	7.3	0.37	
TREE LINE AVE	AP9	48' F-F	STD	0.0564	20	0.35	
TREE LINE AVE	AP10	48' F-F	STD	0.0072	7.2	0.35	

3.7 CFS NORTH SIDE 10 CFS SOUTH SIDE 3.6 CFS SOUTH SIDE

1325 STREET FLOW-rev.xls 7/8/2004

Storm Drain Inlet Table						
Inlet ID	Q Upstream (cfs)	Street Grade (%)	Flow Depth (ft)	Inlet Type	Inlet Capacity (cfs)	Q Downstream (cfs)
1 & 2Crosswinds @ Hallston	22.6	2.47	0.41	'A'	2 @ 6.5 **	9.6
3 & 4Hallston	32.2	Low Pt	0.59	DBL 'C'	2@16.2*	0
5 & 6Woodmont Road (Inlet 6 on south sidefuture)	7.3 (3.7 north side 3.7 south sidefuture)	Low Pt	0.37	SGL 'C'	1 @ 3.7 * 1 future @ 3.7 *	0
7Tree Line West inlet	10	5.64	0.35	'A'	1 @ 6.4 **	3.6
8Tree Line East inlet	3.6	0.72	0.35	DBL 'C'	1 @ 5.0 **	0

7/8/2004 1325 INLET TABULATION.xls

^{*} See Sump Inlet Calculations

** Capacity from COA DPM Plate 22.3 D-5

SUMP INLET CALCULATIONS

GRATE OPEN AREA:

(per COA std dwg #2220, single grate)

GROSS AREA FOR ONE GRATE = (25 in/12)(40 in/12) = 6.94 SF LESS BEARING BARS = (0.5 in/12)(3.33 ft)(13) = 1.80 SF LESS CROSS BARS = (0.5 in/12)(7)[(25 in/12)-(13)(0.5 in/12)] = 0.45 SF

NET GRATE OPEN AREA = 4.69 SF

GRATE OPEN AREA (assuming 50% clogging factor) = 2.35 SF

ORIFICE EQUATION:

$$Q = CA(2gh)^{1/2}$$

Where:

C = 0.67 $A = 2.35 \text{ ft}^2$

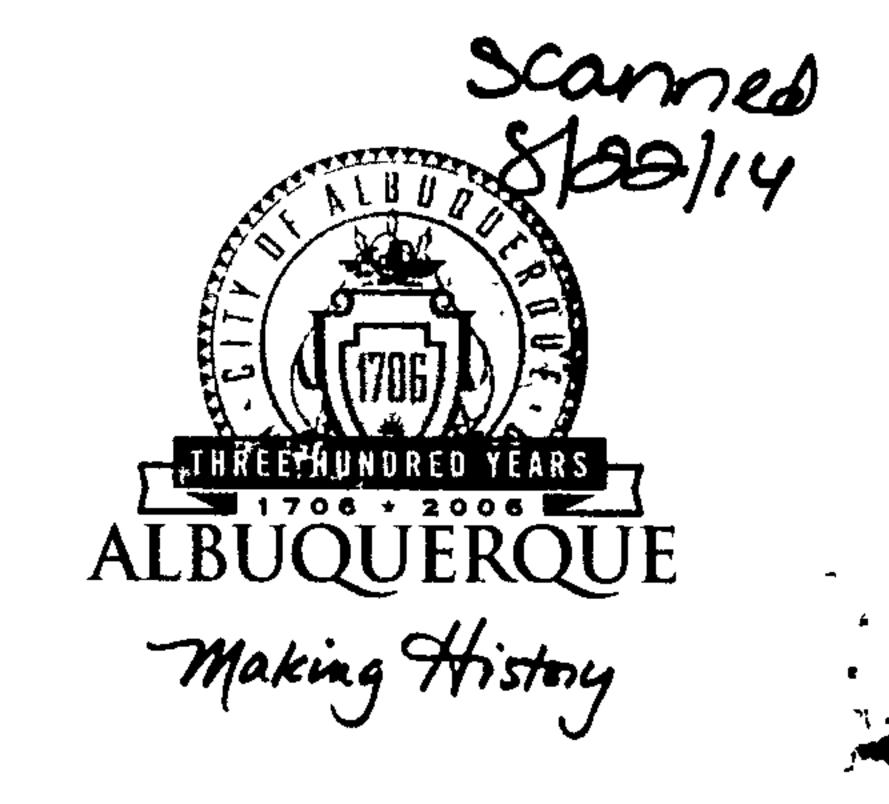
 $g = 32.2 \text{ ft/sec}^2$

h = height of the water surface above the grate

CAPACITY CALCULATIONS:

INLET# 5 & 6 (6=future) LOCATION: Woodmont Rd	
$h = \begin{bmatrix} 0.67 \text{ ft} \\ Q_{\text{(capacity)}} = 10.32 \text{ cfs} \\ \text{NUMBER OF GRATES REQUIRED} = \begin{bmatrix} 0.67 \text{ ft} \\ 0.67 f$	REQUIRED Q = 3.7 cfs1

CITY OF ALBUQUERQUE



February 1, 2006

Ms. Genny Donart, PE ISAACSON & ARFMAN, PA 128 Monroe St. NE Albuquerque, NM 87108

RE: THE RESERVE AT THE TRAILS, (C-9/D3)

Engineers Certification for Release of Financial Guaranty

Engineers Stamp dated 07/26/2004

Engineers Certification dated 02/01/2006

Dear Genny:

New Mexico 87103

www.cabq.gov

Based upon the information provided in your Engineer's Certification Submittal P.O. Box 1293

dated 01/31/2006, the above referenced plan is adequate to satisfy the Grading and

Drainage Certification for Release of Financial Guaranty.

Albuquerque If you have any questions, you can contact me at 924-3982

Sincerely,

Irlene V. Portillo Arlene V. Portillo

Plan Checker, Planning Dept.- Hydrology

Development and Building Services

Marilyn Maldonado, COA #738483

File