

**VOLCANO HEIGHTS
DRAINAGE COMPILATION REPORT**

OCTOBER 2011

Prepared For

City of Albuquerque
Albuquerque, NM

Prepared By

**WILSON
& COMPANY**

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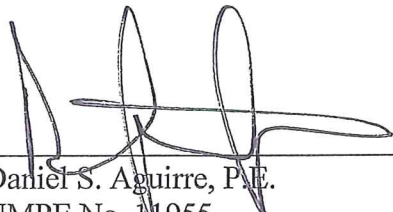
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I, Daniel S. Aguirre, P.E., do hereby certify that this report was prepared by me or under my direction and that I am a duly registered Professional Engineer under the laws of the State of New Mexico



Daniel S. Aguirre, P.E.
NMPE No. 11955



Date

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SECTION 1 – PURPOSE

The purpose of this drainage report is to provide a compilation of the storm drain infrastructure constructed and proposed to be constructed within the portions of the Lyon storm drain watershed, the Piedras Marcadas Dam watershed and the Mariposa Detention Basin watershed. The report identifies allowable flows from the sub-basins within these watersheds for the 100-year 6-hour event.

1.1 Introduction

This report summarizes proposed hydrologic conditions; provides the design for fully developed peak flows; provides recommendations; and identifies downstream drainage capacities.

1.2 Existing Reports

Existing drainage reports providing information used in this report include:

“Boca Negra – Mariposa Arroyo Drainage Management Plan”, dated April 2005, by Resource Technology, Inc.

“La Cuentista Subdivision Drainage Report”, dated November 2003, by Wilson & Company, Inc.

“Paseo del Norte Drainage Report”, dated August 16, 2004, by Wilson & Company, Inc.

“Amendment to the Trails Unit II Drainage Master Plan”, dated August 2007, by Wilson & Company, Inc.

Guidelines and recommendations from the above reports were incorporated into this drainage report where possible.

SECTION 2 - HYDROLOGIC ANALYSIS

There are three ultimate discharge points affected by the infrastructure described in this report. The first area discharges a portion to the Lyon Boulevard storm drain system with a 100-year 24-hour overflow to Petroglyph National Monument discharging to the Piedras Marcadas Dam. This area is bound by the Trails Unit IV and Unser Boulevard to the south and the Chamisa basin to the north. The allowable discharge for this area is described below and shown on Plate 1.

Discharge Point	Allowable Discharge	Ultimate Discharge Point
Chamisa Storm Drain	45 cfs	Lyon Boulevard Storm Drain
Unser Detention Basin	285 cfs	Low Flows diverted to Lyon Boulevard Storm Drain
	120 cfs	Maximum Overflow to Petroglyph National Monument

The second Area ultimately discharges to the Piedras Marcadas Dam. This area is bound by Paseo del Norte and the Trails Unit IV to the south, the aforementioned basin to the northwest, and the Chamisa basin line to the North. A portion of the watershed is collected in a storm drain system below Unser Blvd. and is directed to the Unser Detention Basin. We recommend the Unser Detention Basin and the Chamisa Detention basins be connected to allow low flows to discharge to the Lyon Storm Drain. This will limit the high frequency erosive discharge to the Petroglyph National Monument. The Unser Detention basin would include an overflow weir that would have a maximum capacity of 120 cfs discharge to the monument and ultimately to the Piedras Marcadas Arroyo. The remainder of the watershed drains to a storm drain system beneath Paseo del Norte and also discharges into the Piedras Marcadas Arroyo.

The third discharge point is the Mariposa Basin. The areas contributing to the Mariposa Basin includes storm water runoff west of Unser Boulevard and south of Paseo del Norte Including Basins N1 and N2 on the east side of Unser. These areas discharge to the Boca Negra Detention Dam through various infrastructure including the Unser storm drain, the Universe storm drain and the Boca Negra channel. Additional areas include La Cuentista Subdivision and SAD 228. The flow from these additional areas discharges at several locations to the Petroglyph National Monument where it is conveyed to the Mariposa basin through overland flow. The southeast portion of SAD 228 is drained by a storm drain in Unser Boulevard to join with the discharge from the Boca Negra Dam and conveyed to the Boca Negra Arroyo in the Atrisco storm drain as shown on Plate 1, in Appendix B. See Figure 1 for Vicinity map.

2.1 Methodology

Hydrologic modeling used for the existing infrastructure designs and calculations shown in this report are performed using the 1997 version of The Arid-Lands Hydrologic Model (AHYMO) in accordance with the City of Albuquerque Development process Manual (DPM), section 22.2, December 1999 for the given 100-year, 6-hour storm event. The AHYMO input, summary and output files for the developed conditions are included in Appendix A. Hydraflow Storm Sewers 2005 software by Intelisolve was used to size the underground storm systems for modeling purposes and are for information only project specific reports and calculations are required for actual design of future infrastructure. See Appendix A for calculations summary.

2.2 Design Storm Precipitation

AHYMO was used to calculate proposed runoff. The rainfall values used were for the 1 and 6 - hour precipitation for a 100-year storm event. Rainfall values were obtained from precipitation frequency data server NOAA Atlas 14.

Table 1: Precipitation Values		
Return Period (yrs)	1 hr Rainfall (in)	6 hr Rainfall (in)
100	1.7	2.2

2.3 Land Treatments

Proposed land treatment percentages used in the AHYMO Computer model are based on the original Paseo del Norte drainage report dated August 2004. See Table 2 below for land treatment percentages.

Table 2: Land Treatment Percentages				
Land Use	Type A	Type B	Type C	Type D
Residential/Town homes	0%	15%	35%	50%
Residential	0%	10%	40%	50%
Commercial	0%	10%	10%	80%
Open Space	100%	0%	0%	0%
Roadway	0%	10%	0%	90%

2.1 Existing and Proposed Conditions

2.4.1 Existing Conditions

The existing topography slopes generally from the northwest to the northeast, east, and southeast. The Chamisa Storm Drain and Chamisa Detention Basin are constructed, the Trails Subdivision west of Universe and portions of the La Cuentista subdivision are constructed, Vista Vieja is constructed and the downstream section of the storm drain system beneath Paseo del Norte is constructed. The remainder of the infrastructure is future although several projects in the area are currently in the design phase.

2.4.2 Proposed Conditions

Each of the ultimate discharge points identified have limited peak flow capacities driving the need to look at the activities in this area as a whole. This planning document recommends ponds located throughout the basin to restrict flows to meet downstream capacities. The analysis points shown on Plate 1 represent allowable flows to meet the infrastructure capacities.

Pond 11 detains flows originating in sub-basins K1, K2, K3, K4, ST11, and E2.1. These flows will be discharged into the Chamisa Basin and are to be limited to 45 cfs.

The Chamisa Detention Basin and the Unser Detention Basin act as a system and their design will be detailed in a future study. The Chamisa Detention Basin accepts flow from sub-basins 1, 2, 3, E1, E2, F, F1, U1, and U2 in addition to storm water originating outside of this report's scope. The Chamisa Detention Basin and Unser Detention Basin will be joined to allow flows in the smaller more frequent events to discharge (5 year event minimum) to the Lyon Boulevard storm drain. Then in larger events a portion of the discharge will overflow a weir structure to the Piedras Marcadas Arroyo in the Petroglyph National Monument with a maximum discharge of 120 cfs during the 100-year 6-hour event.

Pond 2 detains flows from the sub-basins north of Paseo del Norte and east of Unser Blvd. The outlet pipe from Pond 2 and runoff from Basin A and south half of Paseo Del Norte Blvd Basin are connected into the existing 72" pipe in Paseo Del Norte with an allowable discharge of 620 cfs. The 72" storm drain ultimately discharges into the existing concrete culvert boxes conveying flows to the Piedras Marcadas Dam.

Basins identified in this report as 4A and 4B located immediately south of Paseo del Norte and east of Unser Boulevard discharge to proposed Pond 10 Diverting these flows south to the La Cuentista Subdivision storm drain system.

The Universe Boulevard storm drain accepts flows generated from the roadway, the Trails Subdivision pond system and the area contained in basins labeled M1 and M2-B, ultimately discharging to the proposed Boca Negra Dam. The Unser storm drain system shall not receive flows from areas located below elevation 5339.

Flows generated from areas below elevation 5339 including areas a portion of the area designated as SAD 228 discharge to a new 54" storm drain parallel to Unser Boulevard with an 80 cfs capacity. This storm drain discharges to the Atrisco storm drain below at a confluence with the primary principal spillway from the proposed Boca Negra Dam. The storm drain system conveys these flows to the Boca Negra Arroyo with a ultimate discharge to the Mariposa Basin.

Ponds 6, 7, 8 and 9 detain flows before discharging allowable amounts through overland flow to the Mariposa Basin. Refer to Plate 1 in Appendix B for pond locations and allowable discharge rates. Exhibit 1 in Appendix B shows basin boundaries and major discharge points in color. See Table 3 for Basin Summary.

Table 3: Basin Summary								
BASIN	AREA (SQ MI)	AREA (ACRE)	LAND TREATMENT (%)				Q ₁₀₀ (CFS)	VOL ₁₀₀ (AC-FT)
			A	B	C	D		
BASINS DRAINING TO THE CHAMISA BASIN THROUGH POND 11¹								
E2.1	0.0124	7.93	0	15	35	50	26	0.91
K1	0.0238	15.23	0	10	10	80	55	2.17
K2	0.0059	3.78	0	10	10	80	14	0.54
K3	0.0148	9.47	0	10	10	80	34	1.35
K4	0.0196	12.54	0	10	10	80	46	1.78
ST11	0.0068	4.33	0	10	0	90	14	0.65
BASINS DRAINING TO THE UNSER DETENTION BASIN								
1	0.0132	8.47	0	10	15	75	27	0.97
2	0.0113	7.23	0	10	15	75	26	1.00
3	0.0151	9.66	0	10	15	75	35	1.33
11A	0.0066	4.20	0	10	10	80	15	0.60
E1	0.0118	7.52	0	15	35	50	24	0.86
E2	0.0453	28.97	0	15	35	50	85	0.94
F	0.0043	2.77	0	15	35	50	9	0.26
PDN1	0.0196	12.51	0	10	0	90	37	1.89
U1	0.0158	10.11	0	10	0	90	38	1.53
U2	0.0259	16.60	0	10	0	90	49	2.34
BASINS DRAINING INTO POND 10								
4A	0.0388	24.83	0	10	10	80	90	3.53
4B	0.0080	5.12	0	10	10	80	19	0.73
PDN2	0.0148	9.50	0	10	0	90	36	1.43
BASINS DRAINING INTO POND 2								
5	0.0275	17.62	0	10	10	80	64	2.51
6	0.0355	22.70	0	10	10	80	83	3.23
7	0.0354	22.66	0	10	10	80	77	3.22
8	0.0198	12.67	0	10	10	80	46	1.80
9	0.0316	20.20	0	10	10	80	74	2.87
10	0.0567	36.29	0	10	10	80	122	5.16
13	0.0626	40.06	0	10	10	80	140	5.70
11B	0.0553	35.37	0	10	0	90	116	5.03
11C	0.0332	21.23	0	10	10	80	77	3.02
12A	0.0308	19.71	0	10	10	80	72	2.80
12B	0.0144	9.22	0	10	10	80	34	1.31
6A	0.0153	9.77	0	10	10	80	33	1.39
PDN3 ⁴	0.0151	9.66	0	10	0	90	30	1.46

Table 3-Continued: Basin Summary

BASIN	AREA (SQ MI)	AREA (ACRE)	LAND TREATMENT (%)				Q ₁₀₀ (CFS)	VOL ₁₀₀ (AC-FT)
			A	B	C	D		
PDN4 ⁴	0.0111	7.13	0	10	0	90	25	1.08
ST1	0.0141	9.04	0	10	0	90	31	1.37
ST2	0.0109	7.00	0	10	0	90	24	1.06
ST3	0.0069	4.39	0	10	0	90	15	0.66
ST4	0.0077	4.94	0	10	0	90	17	0.75
BASINS DRAINING INTO PASEO DEL NORTE STORM DRIAN SYSTEM								
PDN3 ⁴	0.0151	9.66	0	10	0	90	30	1.46
PDN4 ⁴	0.0111	7.13	0	10	0	90	25	1.08
A	0.0351	22.46	0	15	35	50	61	2.58
BASINS DRAINING INTO PIEDRAS MARCADAS								
B	0.0211	13.53	100	0	0	0	16	0.46
F1	0.0204	13.08	0	60	40	0	28	0.78
G	0.1032	66.05	100	0	0	0	80	2.22
H	0.3826	244.84	100	0	0	0	288	8.24
PDN5	0.0198	12.66	0	10	0	90	48	1.91
PDN6	0.0185	11.82	0	10	0	90	45	1.79
BASINS DRAINING INTO BOCA NEGRA DAM								
UNIVERSE BLVD								
P1	0.0313	20.00	25	26	27	22	44	1.52
P2	0.1094	70.02	0	25	25	50	153	7.85
P3	0.0515	32.96	0	25	25	50	63	3.70
UNSER BLVD								
M1	0.1381	88.38	0	10	40	50	234	10.25
M2-B	0.0201	21.79	0	10	40	50	41	1.49
*N1 ²	0.0814	52.10	0	10	40	50	146	6.05
N2	0.0246	15.74	0	10	40	50	51	1.83
T1	0.0890	56.96	0	10	40	50	149	6.61
*U0 ³	0.0319	20.42	0	10	40	50	49	2.37
BASIN DRAINING INTO ATRISCO STORM DRAIN								
M2-A	0.1145	64.35	5	30	35	30	142	6.52
M3	0.1793	114.75	0	10	40	50	303	13.32
BASIN DRAINING INTO POND-6								
M3-1	0.0534	34.17	0	10	40	50	108	3.97
BASIN DRAINING INTO POND-7								
M4	0.0172	11.01	0	10	40	50	36	1.28
BASIN DRAINING INTO POND-8								
M5	0.0707	45.25	0	10	40	50	137	5.25

Table 3-Continued: Basin Summary								
BASIN	AREA (SQ MI)	AREA (ACRE)	LAND TREATMENT (%)				Q ₁₀₀ (CFS)	VOL ₁₀₀ (AC-FT)
			A	B	C	D		
BASIN M6 DRAINING INTO POND-9 AND ESCARPMENT								
M6-1 ⁵	0.0002	0.15	0	10	40	50	3	0.02
M6-2 ⁶	0.0063	4.01	0	10	40	50	11	0.47
NOTES:								
* DIVIDED FLOW								
1 - 45 CFS FROM BASIN E2 DRAINS INTO CHAMISA STORM DRAIN (5 CFS/LOT)								
2- 90 CFS DRAINS INTO LA CUENTISTA SUBDIVISION								
3 - 19 CFS DRAINS INTO THE 48" OUTLET FROM BOCA NEGRA DAM								
4- BASIN INCLUDES HALF OF PASEO DEL NOTRE BLVD WIDTH.								
5- FREE DISCAHRGE.								
6- DRAINS INTO POND 9.								

SECTION 3 – HYDRAULIC ANALYSIS

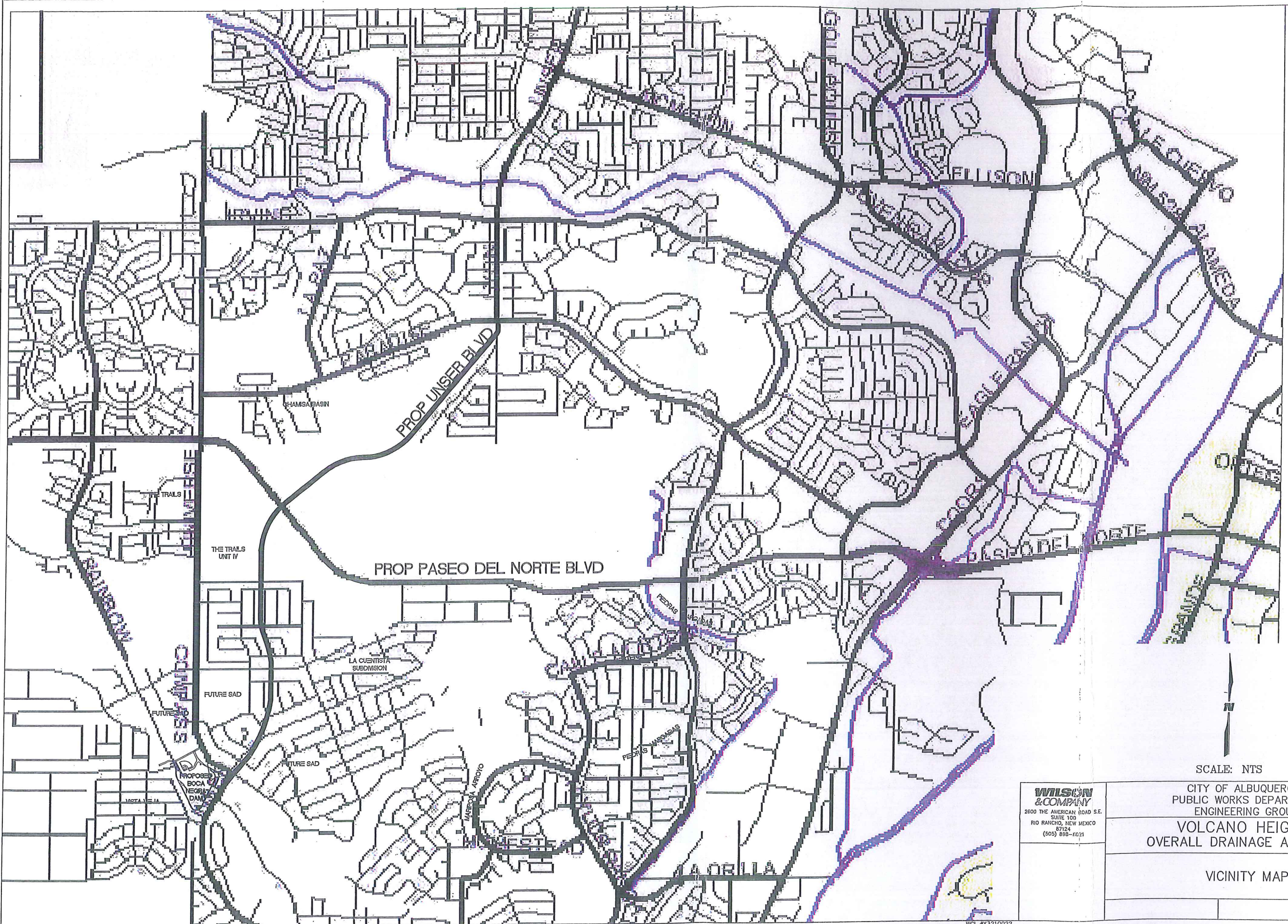
3.1 Storm Drain Analysis

Analyses for these systems were modeled using HydraFlow Storm Sewers 2005 by Intelisolve. The conceptual storm drain systems was modeled for planning and study purposes detailed design calculations are required for final infrastructure design. The proposed system may be found on Plate 1 in Appendix B.

SECTION 4 - CONCLUSION AND RECOMMENDATIONS

Proposed storm drain facilities have been evaluated and modeled to convey the developed runoff from the sub basins identified in this report. Plate 1 summarizes the proposed storm drain system for development of Volcano Heights as delineated in this report. The following table is a summary of the 100 year design flows recommended to be used for the design of infrastructure located within the boundaries identified on Plate 1.

Table 4 - Analysis Point Summary		
Analysis Point	Location	Q ₁₀₀ (cfs)
AP1	PdN at the Petoglyph national Monument	620
AP2	PdN at Sub-Basin B Entry	643
AP3	PdN Xing of Piedras Marcadas	288
AP4	Piedras Marcadas Arroyo below PdN	824
AP5	The Trails at Proposed Unser Blvd	227
AP6	Proposed Unser Blvd at Rosa Parks	330
AP7	Proposed Unser Blvd at SAD 228	439
AP8	Universe Blvd at Albuquerque Public School	176



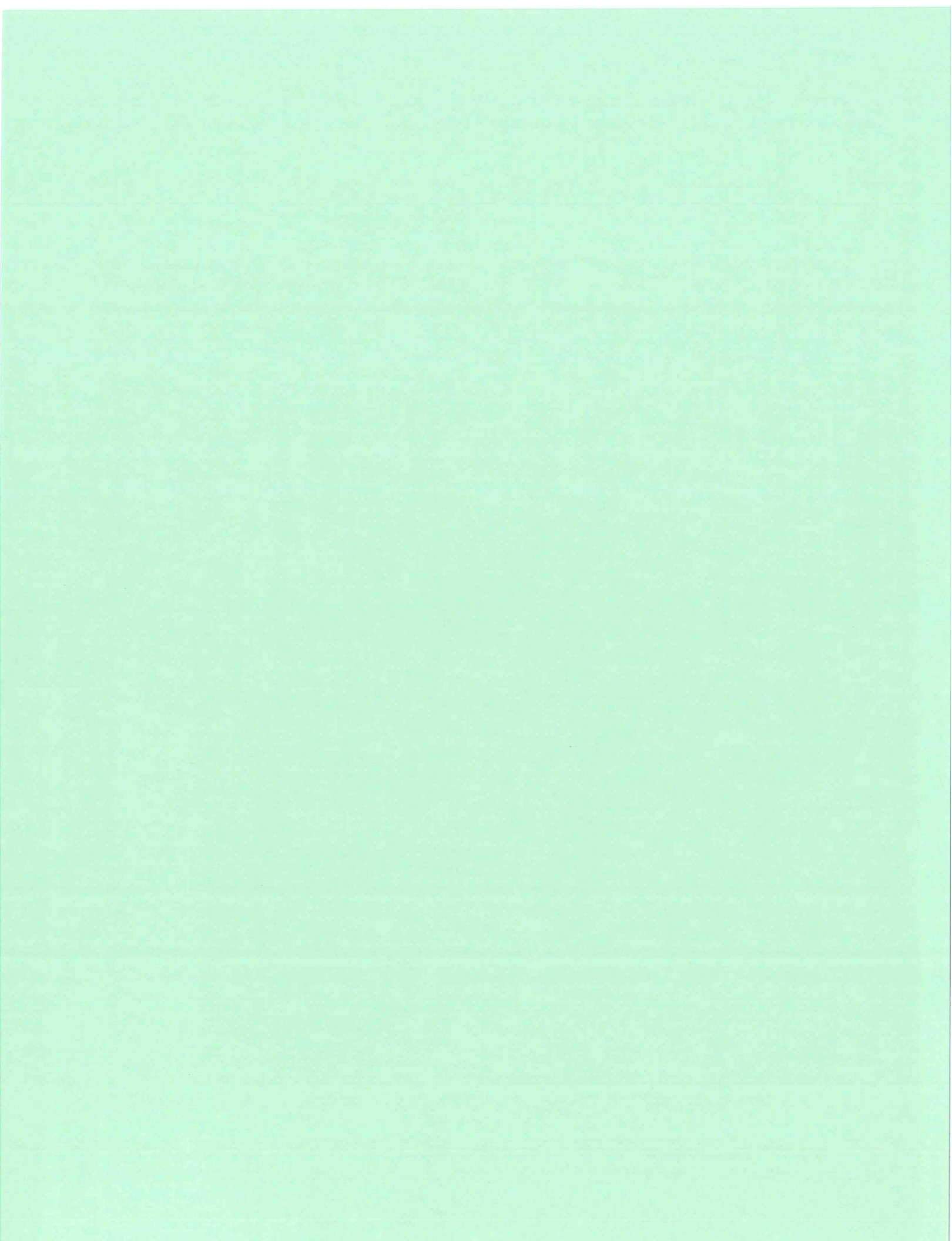
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CITY OF ALBUQUERQUE
 PUBLIC WORKS DEPARTMENT
 ENGINEERING GROUP
**VOLCANO HEIGHTS
 OVERALL DRAINAGE AREA MAP**

VICINITY MAP

FIGURE 1



APPENDIX A

AHYMO PROGRAM SUMMARY TABLE (AHYMO_97) - AHYMO.SUM
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COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	PAGE =	NOTATION
*S	100 YEAR 6 HOUR STORM - PROPOSED RUNOFF ANALYSIS									1	
*S	RAINFALL DATA FROM NOAA ATLAS 14										
*S	BERNALILLO COUNTY										
*S	SUB-BASINS NORTH AND WEST OF UNSER BLVD. TO POND 11,										
*S	ULTIMATELY DISCHARGING TO CHAMISA BASIN										
*S	COMPUTE BASIN "E2.1"	200.10	99	.01243	25.55	.913	1.37668	1.500	3.211	PER IMP=	50.00
*S	THE TRAILS UNIT 4 - NORTH BASINS										
*S	COMPUTE BASIN "K1"	200.20	**	.02380	55.46	2.166	1.70664	1.500	3.641	PER IMP=	80.00
*S	COMPUTE BASIN "K2"	200.30	**	.00590	13.76	.537	1.70664	1.500	3.644	PER IMP=	80.00
*S	COMPUTE BASIN "K3"	200.40	**	.01480	34.49	1.347	1.70664	1.500	3.641	PER IMP=	80.00
*S	COMPUTE BASIN "K4"	200.50	**	.01959	45.65	1.783	1.70664	1.500	3.641	PER IMP=	80.00
*S	COMPUTE BASIN "ST11"	200.60	**	.00676	13.96	.654	1.81350	1.567	3.228	PER IMP=	90.00
*S	ADD SUB-BASINS "K1" AND "K2"	200.70	**&**	.02970	69.22	2.703	1.70662	1.500	3.642		
*S	ADD SUB-BASINS "K3" AND "K4"	200.80	**&**	.03439	80.14	3.130	1.70663	1.500	3.641		
*S	ADD SUB-BASINS "K1", "K2" AND "E2.1"	200.90	99&**	.04213	94.76	3.616	1.60927	1.500	3.515		
*S	ADD SUB-BASINS "K1", "K2", "E2.1" AND "K4"	200.10	**&**	.07652	174.91	6.746	1.65302	1.500	3.572		
*S	SUB-BASINS NORTH AND WEST OF UNSER BLVD W/ OVERFLOW OF STORMS (5 YR EVENT MIN										

TIME= .00
 RAIN6= 2.200

FROM TO ID NO. AREA DISCHARGE (CFS) RUNOFF VOLUME (AC-FT) TIME TO PEAK (HOURS) CFS PER ACRE PAGE = 2 NOTATION

Page 1

AHYMO.SUM

```

*S TO THE CHAMISA DETENTION. THE UNSER DETENTION BASIN OUTFLOW IS LIMITED
*S TO 120 CFS. FLOWS ULTIMATELY DISCHARGE TO AP3
-----
*S COMPUTE BASIN "1" *****
COMPUTE NM HYD      1.10 - 1      .01324      27.21      .972
*S
*S COMPUTE BASIN "2" *****
COMPUTE NM HYD      1.20 - 2      .01130      25.87      .997
*S
*S ADD SUB-BASINS "1" AND "2" *****
ADD HYD             1.30 1& 2 11    .02454      53.09      1.969
*S
*S COMPUTE BASIN "U1" (UNSER TO PDN) *****
COMPUTE NM HYD      1.40 - 4      .01580      38.16      1.528
*S
*S ADD SUB-BASINS "1", "2" AND "U1" *****
ADD HYD             1.50 11& 4 13    .04034      91.25      3.497
*S
*S
*S COMPUTE BASIN "PDN1" *****
COMPUTE NM HYD      1.90 - **     .01955      37.08      1.891
*S
*S ADD SUB-BASINS "1", "2", "U1" AND "PDN1" *****
ADD HYD             2.00 11 13& **    .05989     120.82     5.388
*S
*S
*S COMPUTE BASIN "3" *****
COMPUTE NM HYD      2.60 - 3      .01510      34.57      1.332
*S
*S ADD SUB-BASINS "1", "2", "U1", "PDN1" AND "3" *****
ADD HYD             2.70 *& 3 14     .07499     154.62     6.720
*S
*S COMPUTE BASIN "E1" *****
COMPUTE NM HYD      2.80 - 10     .01175      24.15      .863
*S
*S ADD SUB-BASINS "E1", "1", "2", "3" AND "U1" *****
ADD HYD             2.90 10&14 15    .08674     178.77     7.583
*S
*S COMPUTE BASIN "E2" *****
COMPUTE NM HYD      3.00 - 11     .04526      85.32      3.323
*S
*S ADD SUB-BASINS "E2", "E1", "1", "2", "3" AND "U1" *****
ADD HYD             3.20 15&11 16    .13200     262.23    10.906
*S
*S COMPUTE BASIN "U2" (UNSER NORTH OF PDN) *****
COMPUTE NM HYD      3.30 - 20     .02593      48.61      2.508
*S
*S ADD SUB-BASINS "E1", "E1", "1", "2", "3", "U1" AND "U2" *****
ADD HYD             3.40 20&16 17    .15793     305.56    13.414
*S
*S COMPUTE BASIN "F" *****

```

COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	PAGE =	NOTATION
COMPUTE NM HYD		3.50	21	.00433	8.91	.318	1.37668	1.500	3.215	3	PER IMP= 50.00
*S ADD SUB-BASINS "E1", "E1", "1", "2", "3", "U1", "U2" AND "F" *****											

AHYMO.SUM

```

*S-----
*S BASINS NORTH OF PASEO DEL NORTE BLVD TO POND 2
*S-----
*S
*S COMPUTE BASIN "5" *****
*S
*COMPUTE NM HYD 5.10 - 50 .02753 64.15 2.506 1.70664 1.500 3.641 PER IMP= 80.00
*S COMPUTE BASIN "7" *****
*COMPUTE NM HYD 5.70 - 80 .03540 77.49 3.222 1.70664 1.533 3.420 PER IMP= 80.00
*S ADD SUB-BASINS "5" AND "7" *****
*ADD HYD 5.20 50&80 44 .06293 140.74 5.728 1.70663 1.500 3.494
*S COMPUTE BASIN "12B" *****
*COMPUTE NM HYD 5.90 - 90 .01440 33.56 1.311 1.70664 1.500 3.642 PER IMP= 80.00
*S ADD SUB-BASINS "5", "7" AND "12B" *****
*ADD HYD 6.00 90&44 45 .07733 174.30 7.039 1.70663 1.500 3.522
*S COMPUTE BASIN "PDN3" *****
*COMPUTE NM HYD 6.10 - ** .01509 30.10 1.460 1.81351 1.567 3.117 PER IMP= 90.00
*S COMPUTE BASIN "PDN4" *****
*COMPUTE NM HYD 6.30 - ** .01114 24.99 1.077 1.81350 1.533 3.505 PER IMP= 90.00
*S ADD SUB-BASINS "PDN3" AND "PDN4" *****
*ADD HYD 6.20 **&** 46 .02623 53.75 2.537 1.81348 1.533 3.202
*S *****
*S DIVIDE PDN NORTH & SOUTH FLOWS
*DIVIDE HYD 46.22 46 46 .01312 26.87 1.268 1.81346 1.533 3.202
* 46.22 and 46 .01312 26.87 1.268 1.81346 1.533 3.202
*S COMPUTE BASIN "A" *****
*COMPUTE NM HYD 9.30 - 72 .03509 61.16 2.576 1.37668 1.533 2.723 PER IMP= 50.00
*S FLOWS INTO PDN STORM SYSTEM
*S ADD SUB-BASINS SOUTH OF PDN AND "A" *****
*ADD HYD 9.40 72&46 73 .04821 88.04 3.845 1.49550 1.533 2.854
*S
COMMAND HYDROGRAPH FROM TO PEAK DISCHARGE RUNOFF VOLUME TIME TO CFS
IDENTIFICATION NO. NO. (SQ MI) (CFS) (AC-FT) (INCHES) PEAK (HOURS) PER
NOTATION
*S FLOWS INTO POND 2 *****
*S ADD SUB-BASINS NORTH "PDN3", "PDN4", "7" AND "12B" *****
*ADD HYD 7.20 46&45 47 .09045 199.02 8.307 1.72212 1.500 3.438
*S
*S ADD INTERNAL STREET TO PDN SUB-BASINS
*S COMPUTE BASIN "6A" *****
*COMPUTE NM HYD 7.00 - 60 .01527 33.25 1.390 1.70664 1.533 3.402 PER IMP= 80.00
*S

```


* PASEO DEL NORTE DRAINAGE ANALYSIS
* WILSON & COMPANY PROJECT X3210022
* DEVELOPED CONDITIONS MODEL
* 100 YR 6 HR STORM EVENT
* DATE: OCTOBER 2011
* FILE: X:\Public\PROJECTS\X3210022\Drainage Report\DOCS\AHYMO\PROP VH.TXT

*S*****
*S
*S 100 YEAR 6 HOUR STORM - PROPOSED RUNOFF ANALYSIS
*S RAINFALL DATA FROM NOAA ATLAS 14
*S
*S*****

START 0.0 HOURS
LOCATION BERNALILLO COUNTY
RAINFALL TYPE=1 RAIN QUARTER=0.0 IN RAIN ONE=1.70 IN
RAIN SIX=2.20 IN RAIN DAY=0 IN DT=0.033333 HR

*S
*S -----
*S SUB-BASINS NORTH AND WEST OF UNSER BLVD. TO POND 11,
*S ULTIMATELY DISCHARGING TO CHAMISA BASIN
*S -----

*S
*S
*S COMPUTE BASIN "E2.1" *****

COMPUTE LT TP LCODE=1 NK=2 ISLOPE=-1
LENGTH=400 FT SLOPE=0.020 K=0.7
LENGTH=480 FT SLOPE=0.020 K=2.0

COMPUTE NM HYD ID=99 HYD NO=200.1 DA=0.01243 SQ MI
PER A=0 PER B=15 PER C=35 PER D=50
TP=0.0 MASS RAIN=-1

PRINT HYD ID=99 CODE=1

*S
*S THE TRAILS UNIT 4 - NORTH BASINS
*S COMPUTE BASIN "K1" *****
*

COMPUTE LT TP LCODE=1 NK=3 ISLOPE=-1
LENGTH=400 FT SLOPE=0.02 K=0.7
LENGTH=600 FT SLOPE=0.02 K=2.0
LENGTH=100 FT SLOPE=0.02 K=3.0

COMPUTE NM HYD ID=200 HYD NO=200.2 DA=0.0238 SQ MI
PER A=0 PER B=10 PER C=10 PER D=80
TP=0.0 MASS RAIN=-1

PRINT HYD ID=200 CODE=1

*S COMPUTE BASIN "K2" *****

COMPUTE LT TP LCODE=1 NK=1 ISLOPE=-1
LENGTH=400 FT SLOPE=0.020 K=0.7

COMPUTE NM HYD ID=201 HYD NO=200.3 DA=0.0059 SQ MI
PER A=0 PER B=10 PER C=10 PER D=80
TP=0.0 MASS RAIN=-1

PRINT HYD ID=201 CODE=1

*S
*S COMPUTE BASIN "K3" *****

COMPUTE LT TP LCODE=1 NK=3 ISLOPE=-1
LENGTH=400 FT SLOPE=0.020 K=0.7
LENGTH=600 FT SLOPE=0.020 K=2.0
LENGTH=100 FT SLOPE=0.020 K=3.0

COMPUTE NM HYD ID=202 HYD NO=200.4 DA=0.0148 SQ MI
PER A=0 PER B=10 PER C=10 PER D=80
TP=0.0 MASS RAIN=-1

PRINT HYD ID=202 CODE=1

*S

PROP VH.txt

*S COMPUTE BASIN "K4" *****

COMPUTE LT TP LCODE=1 NK=2 ISLOPE=-1
LENGTH=400 FT SLOPE=0.020 K=0.7
LENGTH=590 FT SLOPE=0.020 K=2.0

COMPUTE NM HYD ID=203 HYD NO=200.5 DA=0.01959 SQ MI
PER A=0 PER B=10 PER C=10 PER D=80
TP=0.0 MASS RAIN=-1

PRINT HYD ID=203 CODE=1

*S

*S

*S COMPUTE BASIN "ST11" *****

*

COMPUTE LT TP LCODE=1 NK=2 ISLOPE=-1
LENGTH=400 FT SLOPE=0.020 K=0.7
LENGTH=1730 FT SLOPE=0.020 K=2.0

COMPUTE NM HYD ID=204 HYD NO=200.6 DA=0.006758 SQ MI
PER A=0 PER B=10 PER C=0 PER D=90
TP=0.0 MASS RAIN=-1

PRINT HYD ID=204 CODE=1

*S

*S ADD SUB-BASINS "K1" AND "K2" ++++++

ADD HYD ID=205 HYD NO=200.7 ID I=200 ID II=201
PRINT HYD ID=205 CODE=1

*S

*S ADD SUB-BASINS "K3" AND "K4" ++++++

ADD HYD ID=206 HYD NO=200.8 ID I=202 ID II=203
PRINT HYD ID=206 CODE=1

*S

*S ADD SUB-BASINS "K1", "K2" AND "E2.1" ++++++

ADD HYD ID=207 HYD NO=200.9 ID I=99 ID II=205
PRINT HYD ID=207 CODE=1

*S

*S ADD SUB-BASINS "K1", "K2", "E2.1" AND "K3", "K4" ++++++

ADD HYD ID=208 HYD NO=200.10 ID I=206 ID II=207
PRINT HYD ID=208 CODE=1

*S

*S

*S -----

*S SUB-BASINS NORTH AND WEST OF UNSER BLVD W/ OVERFLOW OF STORMS (5 YR EVENT MIN)

*S TO THE CHAMISA DETENTION. THE UNSER DETENTION BASIN OUTFLOW IS LIMITED

*S TO 120 CFS. FLOWS ULTIMATELY DISCHARGE TO AP3

*S -----

*S COMPUTE BASIN "1" *****

*

COMPUTE LT TP LCODE=1 NK=2 ISLOPE=-1
LENGTH=400 FT SLOPE=0.020 K=0.7
LENGTH=275 FT SLOPE=0.020 K=2.0

COMPUTE NM HYD ID=1 HYD NO=1.1 DA=0.01324 SQ MI
PER A=0 PER B=15 PER C=35 PER D=50
TP=0.0 MASS RAIN=-1

PRINT HYD ID=1 CODE=1

*S

*S COMPUTE BASIN "2" *****

*

COMPUTE LT TP LCODE=1 NK=2 ISLOPE=-1
LENGTH=400 FT SLOPE=0.020 K=0.7
LENGTH=280 FT SLOPE=0.020 K=2.0

COMPUTE NM HYD ID=2 HYD NO=1.2 DA=0.0113 SQ MI
PER A=0 PER B=10 PER C=15 PER D=75
TP=0.0 MASS RAIN=-1

PRINT HYD ID=2 CODE=1

*S

*S ADD SUB-BASINS "1" AND "2" ++++++

ADD HYD ID=11 HYD NO=1.3 ID I=1 ID II=2
PRINT HYD ID=11 CODE=1

*S

*S COMPUTE BASIN "U1" (UNSER TO PDN)*****

*

COMPUTE LT TP LCODE=1 NK=2 ISLOPE=-1
LENGTH=400 FT SLOPE=0.020 K=0.7
LENGTH=280 FT SLOPE=0.020 K=2.0

COMPUTE NM HYD ID=4 HYD NO=1.4 DA=0.0158 SQ MI
PER A=0 PER B=10 PER C=0 PER D=90
TP=0.0 MASS RAIN=-1

PRINT HYD ID=4 CODE=1

*S

*S ADD SUB-BASINS "1", "2" AND "U1" ++++++

ADD HYD ID=13 HYD NO=1.5 ID I=11 ID II=4
PRINT HYD ID=13 CODE=1

*S

*S COMPUTE BASIN "PDN1" *****

*

COMPUTE LT TP LCODE=1 NK=2 ISLOPE=-1
LENGTH=400 FT SLOPE=0.020 K=0.7
LENGTH=2510 FT SLOPE=0.020 K=2.0

COMPUTE NM HYD ID=209 HYD NO=1.9 DA=0.01955 SQ MI
PER A=0 PER B=10 PER C=0 PER D=90
TP=0.0 MASS RAIN=-1

PRINT HYD ID=209 CODE=1

*S

*S ADD SUB-BASINS "1", "2", "U1" AND "PDN1" ++++++

ADD HYD ID=210 HYD NO=200.11 ID I=13 ID II=209
PRINT HYD ID=210 CODE=1

*S

*S COMPUTE BASIN "3" *****

*

COMPUTE LT TP LCODE=1 NK=2 ISLOPE=-1
LENGTH=400 FT SLOPE=0.020 K=0.7
LENGTH=470 FT SLOPE=0.020 K=2.0

COMPUTE NM HYD ID=3 HYD NO=2.6 DA=0.0151 SQ MI
PER A=0 PER B=10 PER C=15 PER D=75
TP=0.0 MASS RAIN=-1

PRINT HYD ID=3 CODE=1

*S

*S ADD SUB-BASINS "1", "2", "U1", "PDN1" AND "3" ++++++

ADD HYD ID=14 HYD NO=2.7 ID I=210 ID II=3
PRINT HYD ID=14 CODE=1

*S

*S COMPUTE BASIN "E1" *****

*

COMPUTE LT TP LCODE=1 NK=2 ISLOPE=-1
LENGTH=400 FT SLOPE=0.030 K=0.7
LENGTH=1060 FT SLOPE=0.030 K=2.0

COMPUTE NM HYD ID=10 HYD NO=2.8 DA=0.01175 SQ MI
PER A=0 PER B=15 PER C=35 PER D=50
TP=0.0 MASS RAIN=-1

PRINT HYD ID=10 CODE=1

*S
*S ADD SUB-BASINS "E1", "1", "2", "3" AND "U1" ++++++

ADD HYD ID=15 HYD NO=2.9 ID I=10 ID II=14
PRINT HYD ID=15 CODE=1

*S
*S COMPUTE BASIN "E2" *****

COMPUTE LT TP LCODE=1 NK=3 ISLOPE=-1
LENGTH=400 FT SLOPE=0.030 K=0.7
LENGTH=600 FT SLOPE=0.030 K=2.0
LENGTH=1800 FT SLOPE=0.030 K=3.0

COMPUTE NM HYD ID=11 HYD NO=3.0 DA=0.045259 SQ MI
PER A=0 PER B=15 PER C=35 PER D=50
TP=0.0 MASS RAIN=-1

PRINT HYD ID=11 CODE=1

*S
*S ADD SUB-BASINS "E2", "E1", "1", "2", "3" AND "U1" ++++++

ADD HYD ID=16 HYD NO=3.2 ID I=15 ID II=11
PRINT HYD ID=16 CODE=1

*S COMPUTE BASIN "U2" (UNSER NORTH OF PDN)*****
*S

COMPUTE LT TP LCODE=1 NK=3 ISLOPE=-1
LENGTH=400 FT SLOPE=0.02 K=0.7
LENGTH=600 FT SLOPE=0.02 K=2.0
LENGTH=2550 FT SLOPE=0.02 K=3.0

COMPUTE NM HYD ID=20 HYD NO=3.3 DA=0.025933 SQ MI
PER A=0 PER B=10 PER C=0 PER D=90
TP=0.0 MASS RAIN=-1

PRINT HYD ID=20 CODE=1

*S
*S ADD SUB-BASINS "E1", "E1", "1", "2", "3", "U1" AND "U2" ++++++

ADD HYD ID=17 HYD NO=3.4 ID I=20 ID II=16
PRINT HYD ID=17 CODE=1

*S
*S COMPUTE BASIN "F" *****
*S

COMPUTE LT TP LCODE=1 NK=1 ISLOPE=-1
LENGTH=320 FT SLOPE=0.020 K=0.7

COMPUTE NM HYD ID=21 HYD NO=3.5 DA=0.00433 SQ MI
PER A=0 PER B=15 PER C=35 PER D=50
TP=0.0 MASS RAIN=-1

PRINT HYD ID=21 CODE=1

*S
*S ADD SUB-BASINS "E1", "E1", "1", "2", "3", "U1", "U2" AND "F" ++++++

ADD HYD ID=18 HYD NO=3.6 ID I=17 ID II=21
PRINT HYD ID=18 CODE=1

*S
*S COMPUTE BASIN "11A" *****
*S

COMPUTE LT TP LCODE=1 NK=2 ISLOPE=-1
LENGTH=400 FT SLOPE=0.020 K=0.7
LENGTH=80 FT SLOPE=0.020 K=2.0

COMPUTE NM HYD ID=22 HYD NO=3.7 DA=0.00656 SQ MI
PER A=0 PER B=10 PER C=10 PER D=80
TP=0.0 MASS RAIN=-1

*S
*S COMPUTE BASIN "5" *****
*S

COMPUTE LT TP LCODE=1 NK=2 ISLOPE=-1
LENGTH=400 FT SLOPE=0.020 K=0.7
LENGTH=650 FT SLOPE=0.020 K=2.0

COMPUTE NM HYD ID=50 HYD NO=5.1 DA=0.02753 SQ MI
PER A=0 PER B=10 PER C=10 PER D=80
TP=0.0 MASS RAIN=-1

PRINT HYD ID=50 CODE=1

*S COMPUTE BASIN "7" *****
*S

COMPUTE LT TP LCODE=1 NK=2 ISLOPE=-1
LENGTH=400 FT SLOPE=0.020 K=0.7
LENGTH=1150 FT SLOPE=0.020 K=2.0

COMPUTE NM HYD ID=80 HYD NO=5.7 DA=0.0354 SQ MI
PER A=0 PER B=10 PER C=10 PER D=80
TP=0.0 MASS RAIN=-1

PRINT HYD ID=80 CODE=1

*S
*S ADD SUB-BASINS "5" AND "7"+++++

ADD HYD ID=44 HYD NO=5.2 ID I=50 ID II=80
PRINT HYD ID=44 CODE=1

*S
*S COMPUTE BASIN "12B" *****
*S

COMPUTE LT TP LCODE=1 NK=2 ISLOPE=-1
LENGTH=400 FT SLOPE=0.020 K=0.7
LENGTH=800 FT SLOPE=0.020 K=2.0

COMPUTE NM HYD ID=90 HYD NO=5.9 DA=0.0144 SQ MI
PER A=0 PER B=10 PER C=10 PER D=80
TP=0.0 MASS RAIN=-1

PRINT HYD ID=90 CODE=1

*S
*S ADD SUB-BASINS "5", "7" AND "12B"+++++

ADD HYD ID=45 HYD NO=6.0 ID I=90 ID II=44
PRINT HYD ID=45 CODE=1

*S
*S COMPUTE BASIN "PDN3" *****
*S

COMPUTE LT TP LCODE=1 NK=3 ISLOPE=-1
LENGTH=400 FT SLOPE=0.020 K=0.7
LENGTH=500 FT SLOPE=0.020 K=2.0
LENGTH=2130 FT SLOPE=0.020 K=3.0

COMPUTE NM HYD ID=100 HYD NO=6.1 DA=0.01509 SQ MI
PER A=0 PER B=10 PER C=0 PER D=90
TP=0.0 MASS RAIN=-1

PRINT HYD ID=100 CODE=1

*S
*S COMPUTE BASIN "PDN4" *****
*S

COMPUTE LT TP LCODE=1 NK=3 ISLOPE=-1
LENGTH=400 FT SLOPE=0.020 K=0.7
LENGTH=500 FT SLOPE=0.020 K=2.0
LENGTH=1130 FT SLOPE=0.020 K=3.0

COMPUTE NM HYD ID=110 HYD NO=6.3 DA=0.01114 SQ MI
PER A=0 PER B=10 PER C=0 PER D=90
TP=0.0 MASS RAIN=-1

```
PRINT HYD      ID=110  CODE=1
*S
*S ADD SUB-BASINS "PDN3" AND "PDN4"+++++
ADD HYD        ID=46   HYD NO=6.2   ID I=100   ID II=110
PRINT HYD      ID=46   CODE=1
*S
*****
*S DIVIDE PDN NORTH & SOUTH FLOWS
DIVIDE HYD     ID=46   RATIO=-0.50   ID=46.1 HYD NO=46.11
                                           ID=46.2 HYD NO=46.22

PRINT HYD      ID=46.1 CODE=1
PRINT HYD      ID=46.2 CODE=1
*
*S
*
*S COMPUTE BASIN "A" *****
*S
COMPUTE LT TP  LCODE=1 NK=2 ISLOPE=-1
                LENGTH=400 FT SLOPE=0.03 K=0.7
                LENGTH=2500 FT SLOPE=0.03 K=2.0

COMPUTE NM HYD ID=72   HYD NO=9.3   DA=0.03509 SQ MI
                PER A=0   PER B=15   PER C=35   PER D=50
                TP=0.0   MASS RAIN=-1

PRINT HYD      ID=72   CODE=1
*
*S FLOWS INTO PDN STORM SYSTEM
*S ADD SUB-BASINS SOUTH OF PDN AND "A" +++++
*S
ADD HYD        ID=73   HYD NO=9.4   ID I=72 ID II=46.2
PRINT HYD      ID=73   CODE=1
*S
*S
*S FLOWS INTO POND 2*****
*
*S ADD SUB-BASINS NORTH "PDN3", "PDN4", "7"AND "12B"+++++
ADD HYD        ID=47   HYD NO=7.2   ID I=46.1   ID II=45
PRINT HYD      ID=47   CODE=1
*S
*S ADD INTERNAL STREET TO PDN SUB-BASINS
*S
*S COMPUTE BASIN "6A" *****
*S
COMPUTE LT TP  LCODE=1 NK=3 ISLOPE=-1
                LENGTH=400 FT SLOPE=0.020 K=0.7
                LENGTH=500 FT SLOPE=0.020 K=2.0
                LENGTH=1040 FT SLOPE=0.020 K=3.0

COMPUTE NM HYD ID=60   HYD NO=7.0   DA=0.01527 SQ MI
                PER A=0   PER B=10   PER C=10   PER D=80
                TP=0.0   MASS RAIN=-1

PRINT HYD      ID=60   CODE=1
*
*S
*S COMPUTE BASIN "6" *****
*S
COMPUTE LT TP  LCODE=1 NK=3 ISLOPE=-1
                LENGTH=400 FT SLOPE=0.02 K=0.7
                LENGTH=600 FT SLOPE=0.02 K=2.0
                LENGTH=150 FT SLOPE=0.02 K=3.0

COMPUTE NM HYD ID=61   HYD NO=7.1   DA=0.03547 SQ MI
                PER A=0   PER B=10   PER C=10   PER D=80
                TP=0.0   MASS RAIN=-1

PRINT HYD      ID=61   CODE=1
```



```

*
*S -----
*S REMAINDER OF SUB-BASINS IN INTERNAL STREET TO POND 2
*S -----
*S
*S COMPUTE BASIN "13" *****
*S

```

```

COMPUTE LT TP  LCODE=1 NK=3 ISLOPE=-1
                LENGTH=400 FT SLOPE=0.020 K=0.7
                LENGTH=600 FT SLOPE=0.020 K=2.0
                LENGTH=680 FT SLOPE=0.020 K=3.0

```

```

COMPUTE NM HYD ID=120  HYD NO=6.5      DA=0.0626 SQ MI
                PER A=0   PER B=10   PER C=10   PER D=80
                TP=0.0   MASS RAIN=-1

```

```

PRINT HYD      ID=120  CODE=1

```

```

*S
*S COMPUTE BASIN "11B" *****
*S

```

```

COMPUTE LT TP  LCODE=1 NK=2 ISLOPE=-1
                LENGTH=400 FT SLOPE=0.02 K=0.7
                LENGTH=1400 FT SLOPE=0.02 K=2.0

```

```

COMPUTE NM HYD ID=68  HYD NO=8.5      DA=0.05527 SQ MI
                PER A=0   PER B=10   PER C=10   PER D=80
                TP=0.0   MASS RAIN=-1

```

```

PRINT HYD      ID=68  CODE=1

```

```

*S
*S ADD BASINS "11B" AND "13"+++++++

```

```

ADD HYD        ID=681  HYD NO=6.6      ID I=120      ID II=68
PRINT HYD      ID=681  CODE=1

```

```

*S
*S COMPUTE BASIN "ST3" *****
*S

```

```

COMPUTE LT TP  LCODE=1 NK=2 ISLOPE=-1
                LENGTH=400 FT SLOPE=0.02 K=0.7
                LENGTH=1400 FT SLOPE=0.02 K=2.0

```

```

COMPUTE NM HYD ID=69  HYD NO=8.7      DA=0.00686 SQ MI
                PER A=0   PER B=10   PER C=0   PER D=90
                TP=0.0   MASS RAIN=-1

```

```

PRINT HYD      ID=69  CODE=1

```

```

*S
*S ADD SUB-BASINS "11B" AND "ST3"+++++++

```

```

ADD HYD        ID=58  HYD NO=8.8      ID I=681      ID II=69
PRINT HYD      ID=58  CODE=1

```

```

*S
*S COMPUTE BASIN "ST4" *****
*S

```

```

COMPUTE LT TP  LCODE=1 NK=2 ISLOPE=-1
                LENGTH=400 FT SLOPE=0.02 K=0.7
                LENGTH=1400 FT SLOPE=0.02 K=2.0

```

```

COMPUTE NM HYD ID=588  HYD NO=8.9      DA=0.00772 SQ MI
                PER A=0   PER B=10   PER C=0   PER D=90
                TP=0.0   MASS RAIN=-1

```

```

PRINT HYD      ID=588  CODE=1

```

```

*S
*S ADD SUB-BASINS "11B", "ST3" AND "ST4"+++++++

```

```

ADD HYD        ID=59  HYD NO=9.0      ID I=58  ID II=588
PRINT HYD      ID=59  CODE=1

```


PROP VH.txt
LENGTH=1390 SLOPE=0.02 K=3.0

COMPUTE NM HYD ID=15 HYD=10.4 DA=0.0814 SQ MI
A=0 B=10 C=40 D=50
TP=0.0 MASSRAIN=-1

PRINT HYD ID=15 CODE=5

*S
*S

*S DIVIDE BASIN N1 FOR ALLOWABLE EXISTING FLOWS THROUGH LA CUENTISTA //////////////////////////////////

DIVIDE HYD ID=15 Q=90 ID I=9 HYD=10.5
ID II=16 HYD=10.6

PRINT HYD ID=9 CODE=1
PRINT HYD ID=16 CODE=1

*S
*S

*S ADD SUB-BASINS M1 AND N1(DIVIDED)++

ADD HYD ID=17 HYD=10.7 ID I=16 II=14
PRINT HYD ID=17 CODE=5

*S
*

*S COMPUTE SUB-BASIN N2 - EAST OF UNSER*****

COMPUTE LT TP LCODE=1 NK=2 ISLOPE=-1
LENGTH=400 SLOPE=0.02 K=0.7
LENGTH=700 SLOPE=0.02 K=2.0

COMPUTE NM HYD ID=18 HYD=10.8 DA=0.0246 SQ MI
A=0 B=10 C=40 D=50
TP=0.0 MASSRAIN=-1

PRINT HYD ID=18 CODE=5

*S
*S

*S ADD SUB-BASINS "N1", "M1" AND N2++

ADD HYD ID=19 HYD=10.9 ID I=18 II=17
PRINT HYD ID=19 CODE=5

*S
*S

*S COMPUTE SUB-BASIN UNSER U0*****

COMPUTE LT TP LCODE=1 NK=3 ISLOPE=-1
LENGTH=400 SLOPE=0.02 K=0.7
LENGTH=700 SLOPE=0.02 K=2.0
LENGTH=4125 SLOPE=0.02 K=3.0

COMPUTE NM HYD ID=20 HYD=11.1 DA=0.0319 SQ MI
A=0 B=10 C=40 D=50
TP=0.0 MASSRAIN=-1

PRINT HYD ID=20 CODE=5

*S
*S

*S DIVIDE SUB BASIN UNSER- SEPARATE BYPASS FLOWS FROM BOCA NEGRA DAM////////////////////////////////

DIVIDE HYD ID=20 Q=19 ID I=30 HYD=30.1
ID II=31 HYD=30.2

PRINT HYD ID=30 CODE=1
PRINT HYD ID=31 CODE=1

*S
*S

*S ADD SUB-BASINS EAST AND WEST OF UNSER++

ADD HYD ID=21 HYD=11.2 ID I=31 II=19
PRINT HYD ID=21 CODE=5

*S
*S

*S COMPUTE SUB-BASIN M2-A - WEST OF UNSER*****

COMPUTE LT TP LCODE=1 NK=3 ISLOPE=-1
LENGTH=400 SLOPE=0.02 K=0.7
LENGTH=600 SLOPE=0.02 K=2.0
LENGTH=2600 SLOPE=0.02 K=3.0

COMPUTE NM HYD ID=22 HYD=11.3 DA=0.1145 SQ MI
A=5 B=35 C=35 D=30
TP=0.0 MASSRAIN=-1

PRINT HYD ID=22 CODE=5

*S
*S

APPENDIX B

APPENDIX C



Parks and Recreation Department OPEN SPACE DIVISION

Richard J. Berry, Mayor

Barbara Baca, Director

October 21, 2011

Mr. Steven Metro
Wilson & Company
4900 Lang Ave. NE
Albuquerque, NM 87109

**Regarding: Open Space Division Letter of Concurrence with SAD 228
Drainage and Trails Planning Adjacent to Boca Negra Canyon,
Petroglyph National Monument**

Dear Mr. Metro;

The City of Albuquerque Open Space Division (OSD), in coordination with staff from the National Park Service, Petroglyph National Monument Unit (PETR), has reviewed the Drainage Report "Volcano Heights Drainage Compilation Report" dated October, 2011, as prepared by Wilson & Company.

OSD and PETR approve and concur with the proposed controlled outlet flows from the Drainage Ponds as summarized below:

Pond 6 Allowable Outlet Flow = 40 c.f.s.
Pond 7 Allowable Outlet Flow = 10 c.f.s.
Pond 8 Allowable Outlet Flow = 30 c.f.s.
Pond 9 Allowable Outlet Flow = 10 c.f.s.

The attached map shows the proposed ponds and the compilation of drainage outlets for the areas above the PETR escarpment. In general, the allowable drainage flows over the escarpment are less than half of the historic flows from the subject-area with no development, as we negotiated and agreed upon.

It is my understanding that, per OSD and PETR staff comments (during a meeting on October 17, 2011, at the Wilson & Company office), the Pond 6 emergency overflow location will be moved to the east and additional native/local basalt rip-rap will be added to the overflow in order to ensure the flows at the escarpment are spread out and dissipated so as to prevent channelization and erosion in the canyon below.

In addition, we concur with the proposed escarpment trail alignment, both horizontal and vertical. The proposed trail alignment follows the bladed roadways for Compass and Vista del Prado streets which will reduce grading impacts to the Petroglyph National Monument and Boca Negra canyon.

PO Box 1293

Albuquerque

NM 87103

www.cabq.gov

Further, it is my understanding that proposed the fencing surrounding Pond #6 and the adjacent lift-station will be eliminated and replaced with native vegetation per OSD and PETR recommendation.

Thank you for your cooperation and responsiveness to the interests of the public in this matter.

Should you have any questions or need additional information, please feel free to contact me.

Sincerely,

A handwritten signature in blue ink, appearing to read 'M. F. Schmader', with a long horizontal flourish extending to the right.

Matthew F. Schmader, Ph.D.
Superintendent, Open Space Division

Attachment - Volcano Heights Drainage Compilation Report Exhibit 1

Copy: Barbara C. Baca, Director, Parks and Recreation Department
Joseph P. Sanchez, Superintendent, NPS PETR

