

VOLCANO HEIGHTS DRAINAGE COMPILATION REPORT

REVISED
DECEMBER 2011

Prepared For

City of Albuquerque
Albuquerque, NM

Prepared By

WILSON
& COMPANY

Wilson & Company, Inc., Engineers & Architects
4900 Lang Ave, NE
Albuquerque, NM 87109
505-348-4000

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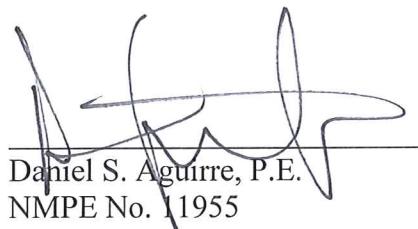
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I, Daniel S. Aguirre, P.E., do hereby certify that this report was prepared by me or under my direction and that I am a duly registered Professional Engineer under the laws of the State of New Mexico



Daniel S. Aguirre, P.E.
NMPE No. 11955

12/12/11
Date

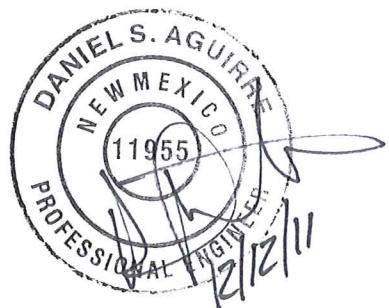


Table of Contents

SECTION 1 – PURPOSE.....	1
1.1 Introduction.....	1
1.2 Existing Reports.....	1
SECTION 2 - HYDROLOGIC ANALYSIS.....	1
2.1 Methodology	2
2.2 Design Storm Precipitation	2
2.3 Land Treatments	3
2.1 Existing and Proposed Conditions	3
<i>2.4.1 Existing Conditions.....</i>	<i>3</i>
<i>2.4.2 Proposed Conditions.....</i>	<i>3</i>
SECTION 3 – HYDRAULIC ANALYSIS.....	7
3.1 Storm Drain Analysis.....	7
SECTION 4 - CONCLUSION AND RECOMMENDATIONS	7

List of Figures

Figure 1 – Vicinity Map

List of Tables

Table 1 – Precipitation Values
Table 2 – Land Treatment Percentages
Table 3 – Basin Summary
Table 4 – Analysis Point Summary

List of Plates/Exhibits

Plate 1 – Conceptual Drainage Compilation Plan
Exhibit 1– Conceptual Drainage Compilation Plan

Appendices

Appendix A – AHYMO Calculations

Appendix B – Plate 1
 Exhibit 1

Appendix C – Park and Recreation Department, Open Space Division Approval Letter

SECTION 1 – PURPOSE

The purpose of this drainage report is to provide a compilation of the storm drain infrastructure constructed and proposed to be constructed within the portions of the Lyon storm drain watershed, the Piedras Marcadas Dam watershed and the Mariposa Detention Basin watershed. The report identifies allowable flows from the sub-basins within these watersheds for the 100-year 6-hour event.

1.1 Introduction

This report summarizes proposed hydrologic conditions; provides the design for fully developed peak flows; provides recommendations; and identifies downstream drainage capacities.

1.2 Existing Reports

Existing drainage reports providing information used in this report include:

“Boca Negra – Mariposa Arroyo Drainage Management Plan”, dated April 2005, by Resource Technology, Inc.

“La Cuentista Subdivision Drainage Report”, dated November 2003, by Wilson & Company, Inc.

“Paseo del Norte Drainage Report”, dated August 16, 2004, by Wilson & Company, Inc.

“Amendment to the Trails Unit II Drainage Master Plan”, dated August 2007, by Wilson & Company, Inc.

Guidelines and recommendations from the above reports were incorporated into this drainage report where possible.

SECTION 2 - HYDROLOGIC ANALYSIS

There are three ultimate discharge points affected by the infrastructure described in this report. The first area discharges a portion to the Lyon Boulevard storm drain system with a 100-year 24-hour overflow to Petroglyph National Monument discharging to the Piedras Marcadas Dam. This area is bound by the Trails Unit IV and Unser Boulevard to the south and the Chamisa basin to the north. The allowable discharge for this area is described below and shown on Plate 1.

Discharge Point	Allowable Discharge	Ultimate Discharge Point
Chamisa Storm Drain	45 cfs	Lyon Boulevard Storm Drain
Unser Detention Basin	285 cfs	Low Flows diverted to Lyon Boulevard Storm Drain
	120 cfs	Maximum Overflow to Petroglyph National Monument

The second Area ultimately discharges to the Piedras Marcadas Dam. This area is bound by Paseo del Norte and the Trails Unit IV to the south, the aforementioned basin to the northwest, and the Chamisa basin line to the North. A portion of the watershed is collected in a storm drain system below Unser Blvd. and is directed to the Unser Detention Basin. We recommend the Unser Detention Basin and the Chamisa Detention basins be connected to allow low flows to discharge to the Lyon Storm Drain. This will limit the high frequency erosive discharge to the Petroglyph National Monument. The Unser Detention basin would include an overflow weir that would have a maximum capacity of 120 cfs discharge to the monument and ultimately to the Piedras Marcadas Arroyo. The remainder of the watershed drains to a storm drain system beneath Paseo del Norte and also discharges into the Piedras Marcadas Arroyo.

The third discharge point is the Mariposa Basin. The areas contributing to the Mariposa Basin includes storm water runoff west of Unser Boulevard and south of Paseo del Norte Including Basins N1 and N2 on the east side of Unser. These areas discharge to the Boca Negra Detention Dam through various infrastructures including the Unser storm drain, the Universe storm drain and the Boca Negra channel. Additional areas include La Cuentista Subdivision and SAD 228. The flow from these additional areas discharges at several locations to the Petroglyph National Monument where it is conveyed to the Mariposa basin through overland flow. The southeast portion of SAD 228 is drained by a storm drain in Unser Boulevard to join with the discharge from the Boca Negra Dam and conveyed to the Boca Negra Arroyo in the Atrisco storm drain as shown on Plate 1, in Appendix B. See Figure 1 for Vicinity map.

2.1 Methodology

Hydrologic modeling used for the existing infrastructure designs and calculations shown in this report are performed using the 1997 version of The Arid-Lands Hydrologic Model (AHYMO) in accordance with the City of Albuquerque Development process Manual (DPM), section 22.2, December 1999 for the given 100-year, 6-hour storm event. The AHYMO input, summary and output files for the developed conditions are included in Appendix A. Hydraflow Storm Sewers 2005 software by Intelisolve was used to size the underground storm systems for modeling purposes and are for information only project specific reports and calculations are required for actual design of future infrastructure. See Appendix A for calculations summary.

2.2 Design Storm Precipitation

AHYMO was used to calculate proposed runoff. The rainfall values used were for the 1 and 6 - hour precipitation for a 100-year storm event. Rainfall values were obtained from precipitation frequency data server NOAA Atlas 14.

Table 1: Precipitation Values

Return Period (yrs)	1 hr Rainfall (in)	6 hr Rainfall (in)
100	1.7	2.2

2.3 Land Treatments

Proposed land treatment percentages used in the AHYMO Computer model are based on the original Paseo del Norte drainage report dated August 2004. See Table 2 below for land treatment percentages.

Table 2: Land Treatment Percentages				
Land Use	Type A	Type B	Type C	Type D
Residential/Town homes	0%	15%	35%	50%
Residential	0%	10%	40%	50%
Commercial	0%	10%	10%	80%
Open Space	100%	0%	0%	0%
Roadway	0%	10%	0%	90%

2.1 Existing and Proposed Conditions

2.4.1 Existing Conditions

The existing topography slopes generally from the northwest to the northeast, east, and southeast. The Chamisa Storm Drain and Chamisa Detention Basin are constructed, the Trails Subdivision west of Universe and portions of the La Cuentista subdivision are constructed, Vista Vieja is constructed and the downstream section of the storm drain system beneath Paseo del Norte is constructed. The remainder of the infrastructure is future although several projects in the area are currently in the design phase.

2.4.2 Proposed Conditions

Each of the ultimate discharge points identified have limited peak flow capacities driving the need to look at the activities in this area as a whole. This planning document recommends ponds located throughout the basin to restrict flows to meet downstream capacities. The analysis points shown on Plate 1 represent allowable flows to meet the infrastructure capacities.

Pond 11 detains flows originating in sub-basins K1, K2, K3, K4, ST11, and E2.1. These flows will be discharged into the Chamisa Basin and are to be limited to 45 cfs.

The Chamisa Detention Basin and the Unser Detention Basin act as a system and their design will be detailed in a future study. The Chamisa Detention Basin accepts flow from sub-basins 1, 2, 3, E1, E2, F, F1, U1, and U2 in addition to storm water originating outside of this report's scope. The Chamisa Detention Basin and Unser Detention Basin will be joined to allow flows in the smaller more frequent events to discharge (5 year event minimum) to the Lyon Boulevard storm drain. Then in larger events a portion of the discharge will overflow a weir structure to the Piedras Marcadas Arroyo in the Petroglyph National Monument with a maximum discharge of 120 cfs during the 100-year 6-hour event.

Basins identified in this report as 4A, 4B and part of Paseo del Norte Blvd located immediately south of Paseo del Norte and east of Unser Blvd discharge to proposed Pond 10. A proposed storm system in Paseo Del Norte Blvd will convey routed flows from Pond 10, runoff from sub-basins north of Paseo Del Norte Blvd and Bain A to the existing 72" storm drain pipe in Paseo Del Norte Blvd.

Pond 2 detains flows from the sub-basins south and east of Unser Blvd. The outlet pipe from this pond and the storm system in Paseo Del Norte Blvd tie into the existing 72" storm drain. The allowable discharge at this point will be 620 cfs. The existing 72" storm drain ultimately discharges into the existing concrete culvert boxes conveying flows to the Piedras Marcadas Dam.

The Universe Boulevard storm drain accepts flows generated from the roadway, the Trails Subdivision pond system and the area contained in basins labeled M1 and M2-B, ultimately discharging to the proposed Boca Negra Dam. The Unser storm drain system shall not receive flows from areas located below elevation 5339.

Flows generated from areas below elevation 5339 including areas a portion of the area designated as SAD 228 discharge to a new 54" storm drain parallel to Unser Boulevard with an 80 cfs capacity. This storm drain discharges to the Atrisco storm drain below at a confluence with the primary principal spillway from the proposed Boca Negra Dam. The storm drain system conveys these flows to the Boca Negra Arroyo with a ultimate discharge to the Mariposa Basin.

Ponds 6, 7, 8 and 9 detain flows before discharging allowable amounts through overland flow to the Mariposa Basin. Refer to Plate 1 in Appendix B for pond locations and allowable discharge rates. Exhibit 1 in Appendix B shows basin boundaries and major discharge points in color. See Table 3 for Basin Summary.

Table 3: Basin Summary

BASIN	AREA (SQ MI)	AREA (ACRE)	LAND TREATMENT (%)				Q ₁₀₀ (CFS)	VOL ₁₀₀ (AC-FT)
			A	B	C	D		
BASINS DRAINING TO THE CHAMISA BASIN THROUGH POND 11¹								
E2.1	0.0124	7.93	0	15	35	50	26	0.91
K1	0.0238	15.23	0	10	10	80	55	2.17
K2	0.0059	3.78	0	10	10	80	14	0.54
K3	0.0148	9.47	0	10	10	80	34	1.35
K4	0.0196	12.54	0	10	10	80	46	1.78
ST11	0.0068	4.33	0	10	0	90	14	0.65
BASINS DRAINING TO THE UNSER DETENTION BASIN								
1	0.0132	8.47	0	10	15	75	27	0.97
2	0.0113	7.23	0	10	15	75	26	1.00
3	0.0151	9.66	0	10	15	75	35	1.33
11A	0.0066	4.20	0	10	10	80	15	0.60
E1	0.0118	7.52	0	15	35	50	24	0.86
E2	0.0453	28.97	0	15	35	50	85	0.94
F	0.0043	2.77	0	15	35	50	9	0.26
PDN1	0.0196	12.51	0	10	0	90	37	1.89
U1	0.0158	10.11	0	10	0	90	38	1.53
U2	0.0259	16.60	0	10	0	90	49	2.34
BASINS DRAINING INTO POND 2								
5	0.0298	19.07	0	10	10	80	69	2.71
6	0.0355	22.70	0	10	10	80	83	3.23
8	0.0198	12.67	0	10	10	80	46	1.80
9	0.0316	20.20	0	10	10	80	74	2.87
10	0.0567	36.29	0	10	10	80	122	5.16
13	0.0626	40.06	0	10	10	80	140	5.70
11B	0.0553	35.37	0	10	0	90	116	5.03
11C	0.0332	21.23	0	10	10	80	77	3.02
12A	0.0308	19.71	0	10	10	80	72	2.80
6A	0.0153	9.77	0	10	10	80	33	1.39
ST1	0.0141	9.04	0	10	0	90	31	1.37
ST2	0.0109	7.00	0	10	0	90	24	1.06
ST3	0.0069	4.39	0	10	0	90	15	0.66
ST4	0.0077	4.94	0	10	0	90	17	0.75

Table 3-Continued: Basin Summary

BASIN	AREA (SQ MI)	AREA (ACRE)	LAND TREATMENT (%)				Q ₁₀₀ (CFS)	VOL ₁₀₀ (AC-FT)
			A	B	C	D		
BASINS DRAINING INTO PASEO DEL NORTE STORM DRIAN SYSTEM								
4A ⁴	0.0388	24.83	0	10	10	80	90	3.53
4B ⁴	0.0080	5.12	0	10	10	80	19	0.73
PDN2 ⁴	0.0126	8.06	0	10	0	90	30	1.22
7	0.0354	22.66	0	10	10	80	77	3.22
12B	0.0144	9.22	0	10	10	80	34	1.31
PDN3	0.0151	9.66	0	10	0	90	30	1.46
PDN4	0.0111	7.13	0	10	0	90	25	1.08
A	0.0275	17.60	0	5	10	85	66	2.60
BASINS DRAINING INTO PIEDRAS MARCADAS								
B	0.0211	13.53	100	0	0	0	16	0.46
F1	0.0204	13.08	0	60	40	0	28	0.78
G	0.1032	66.05	100	0	0	0	80	2.22
H	0.3826	244.84	100	0	0	0	288	8.24
PDN5	0.0198	12.66	0	10	0	90	48	1.91
PDN6	0.0185	11.82	0	10	0	90	45	1.79
BASINS DRAINING INTO BOCA NEGRA DAM								
UNIVERSE BLVD								
P1	0.0313	20.00	25	26	27	22	44	1.52
P2	0.1094	70.02	0	25	25	50	153	7.85
P3	0.0515	32.96	0	25	25	50	63	3.70
UNSER BLVD								
M1	0.1381	88.38	0	10	40	50	234	10.25
M2-B	0.0201	21.79	0	10	40	50	41	1.49
*N1 ²	0.0814	52.10	0	10	40	50	146	6.05
N2	0.0246	15.74	0	10	40	50	51	1.83
T1	0.0890	56.96	0	10	40	50	149	6.61
*U0 ³	0.0319	20.42	0	10	40	50	49	2.37
BASIN DRAINING INTO ATRISCO STORM DRAIN								
M2-A	0.1145	64.35	5	30	35	30	142	6.52
M3	0.1793	114.75	0	10	40	50	303	13.32
BASIN DRAINING INTO POND-6								
M3-1	0.0534	34.17	0	10	40	50	108	3.97
BASIN DRAINING INTO POND-7								
M4	0.0172	11.01	0	10	40	50	36	1.28
BASIN DRAINING INTO POND-8								
M5	0.0707	45.25	0	10	40	50	137	5.25

Table 3-Continued: Basin Summary

BASIN	AREA (SQ MI)	AREA (ACRE)	LAND TREATMENT (%)				Q ₁₀₀ (CFS)	VOL ₁₀₀ (AC-FT)
			A	B	C	D		
BASIN M6 DRAINING INTO POND-9 AND ESCARPMENT								
M6-15	0.0003	1.01	0	10	40	50	3	0.02
M6-26	0.0075	4.01	0	10	40	50	13	0.56

NOTES:

- * DIVIDED FLOW
- 1 - 45 CFS FROM BASIN E2 DRAINS INTO CHAMISA STORM DRAIN (5 CFS/LOT)
- 2- 90 CFS DRAINS INTO LA CUENTISTA SUBDIVISION
- 3 - 19 CFS DRAINS INTO THE 48" OUTLET FROM BOCA NEGRA DAM
- 4- ROUTED THROUGH POND 10
- 5- FREE DISCAHRGE.
- 6- DRAINS INTO POND 9.

SECTION 3 – HYDRAULIC ANALYSIS

3.1 Storm Drain Analysis

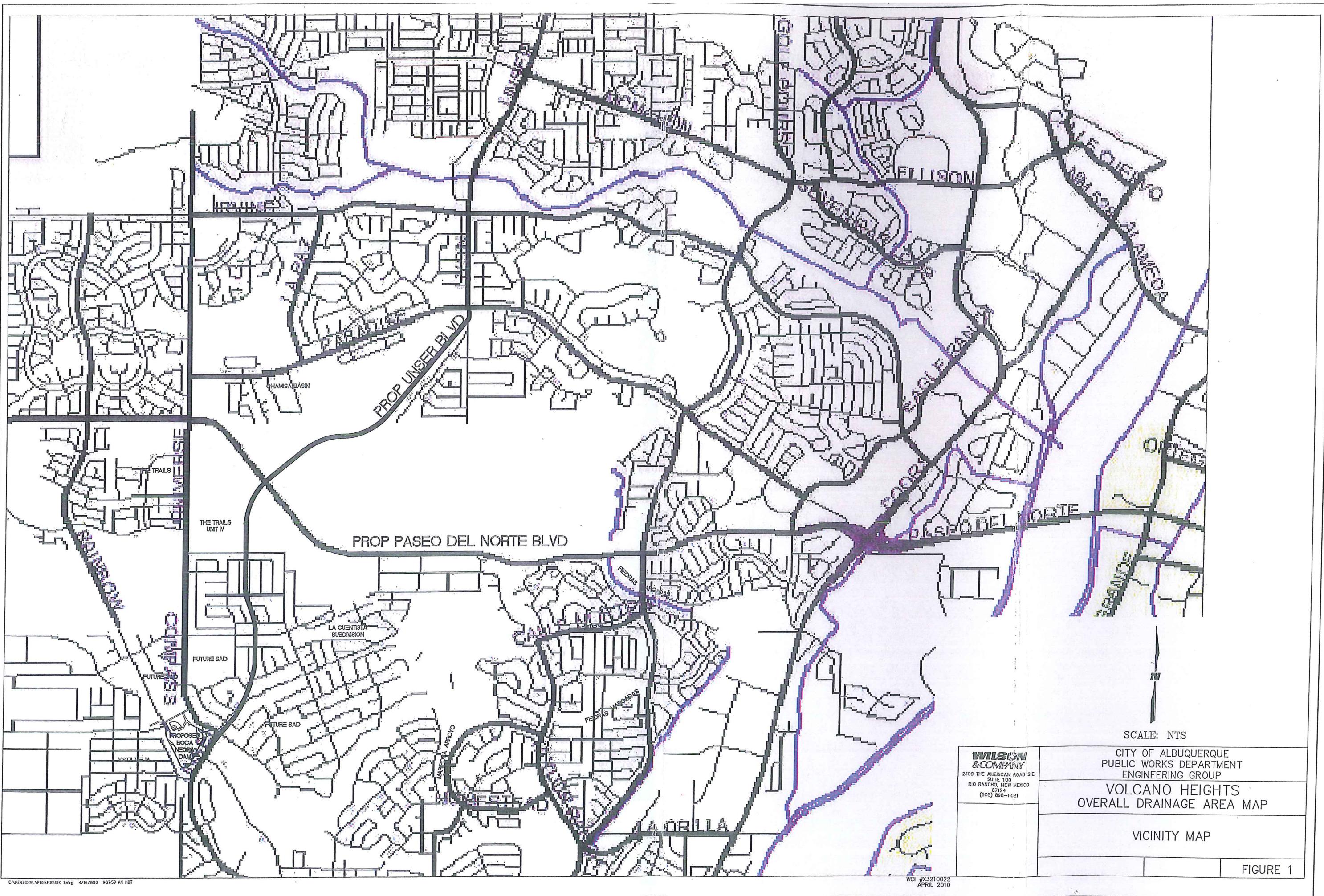
Analyses for these systems were modeled using HydraFlow Storm Sewers 2005 by Intelisolve. The conceptual storm drain systems was modeled for planning and study purposes detailed design calculations are required for final infrastructure design. The proposed system may be found on Plate 1 in Appendix B.

SECTION 4 - CONCLUSION AND RECOMMENDATIONS

Proposed storm drain facilities have been evaluated and modeled to convey the developed runoff from the sub basins identified in this report. Plate 1 summarizes the proposed storm drain system for development of Volcano Heights as delineated in this report. The following table is a summary of the 100 year design flows recommended to be used for the design of infrastructure located within the boundaries identified on Plate 1.

Table 4 - Analysis Point Summary

Analysis Point	Location	Q ₁₀₀ (cfs)
AP1	PdN at the Petoglyph national Monument	620
AP2	PdN at Sub-Basin B Entry	643
AP3	PdN Xing of Piedras Marcadas	288
AP4	Piedras Marcadas Arroyo below PdN	824
AP5	The Trails at Proposed Unser Blvd	227
AP6	Proposed Unser Blvd at Rosa Parks	330
AP7	Proposed Unser Blvd at SAD 228	439
AP8	Universe Blvd at Albuquerque Public School	176



APPENDIX A

AHYMO PROGRAM SUMMARY TABLE (AHYMO_97) - AHYMO.SUM
 INPUT FILE = M:\RR\0887DF~1\AE_DATA\CALCS\AHYMO\PROPVH~1.TXT
 RUN DATE (MON/DAY/YR) =12/09/2011
 USER NO.= AHYMO-C-9803C01UNMLIB-AH
 VERSION: 1997.02c
 PAGE = 1
 CFS PER ACRE
 NOTATION

COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)
START RAINFALL	LOCATION TYPE= 1							
*S	---							
*S	SUB-BASINS NORTH AND WEST OF UNSER BLVD. TO POND 11,							
*S	ULTIMATELY DISCHARGING TO CHAMISA BASIN							
*S	-----							
*S	COMPUTE BASIN "E2.1" 200.10 - 99	.01243		25.55	.913	1.37668	1.500	3.211 PER IMP= 50.00
*S	THE TRAILS UNIT 4 - NORTH BASINS							
*S	COMPUTE BASIN "K1" 200.20 - ** .02380			55.46		1.70664	1.500	3.641 PER IMP= 80.00
*S	COMPUTE BASIN "K2" 200.30 - ** .00590			2.166		1.70664	1.500	3.644 PER IMP= 80.00
*S	COMPUTE BASIN "K3" 200.40 - ** .01480			13.76	.537	1.70664	1.500	3.641 PER IMP= 80.00
*S	COMPUTE BASIN "K4" 200.50 - ** .01959			34.49	1.347	1.70664	1.500	3.641 PER IMP= 80.00
*S	COMPUTE NM HYD 200.60 - ** .00676			45.65	1.783	1.70664	1.500	3.641 PER IMP= 80.00
*S	COMPUTE BASIN "ST11" 200.80 ** &** .03439			13.96	.654	1.81350	1.567	3.228 PER IMP= 90.00
*S	ADD SUB-BASINS "K1" AND "K2" 200.70 ** &** .02970			69.22	2.703	1.70662	1.500	3.642
*S	ADD SUB-BASINS "K3" AND "K4" 200.90 99&** .04213			80.14	3.130	1.70663	1.500	3.641
*S	ADD SUB-BASINS "K1" "K2" AND "E2.1" 200.10 ** &** .07652			94.76	3.616	1.60927	1.500	3.515
*S	-----			174.91	6.746	1.65302	1.500	3.572
*S	SUB-BASINS NORTH AND WEST OF UNSER BLVD W/ OVERFLOW OF STORMS (5 YR EVENT MIN							
COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)

Page 1

AHYMO • SUM

*S TO THE CHAMISA DETENTION. THE UNSER DETENTION BASIN OUTFLOW IS LIMITED
 *S TO 120 CFS. FLOWS ULTIMATELY DISCHARGE TO AP3

*S COMPUTE BASIN "1" ****
 COMPUTE NM HYD 1.10 - 1 .01324 27.21 .972 1.37668 1.500 3.211 PER IMP= 50.00
 *S COMPUTE BASIN "2" ****
 COMPUTE NM HYD 1.20 - 2 .01130 25.87 .997 1.65421 1.500 3.578 PER IMP= 75.00
 *S ADD SUB-BASINS "1" AND "2" ****
 ADD HYD 1.30 1& 2 .02454 53.09 1.969 1.50445 1.500 3.380
 *S COMPUTE BASIN "U1" (UNSER TO PDN)****
 COMPUTE NM HYD 1.40 - 4 .01580 38.16 1.528 1.81350 1.500 3.774 PER IMP= 90.00
 *S ADD SUB-BASINS "1" , "2" AND "U1" ****
 ADD HYD 1.50 11& 4 13 .04034 91.25 3.497 1.62549 1.500 3.534
 *S COMPUTE BASIN "PDN1" ****
 COMPUTE NM HYD 1.90 - ** .01955 37.08 1.891 1.81350 1.600 2.964 PER IMP= 90.00
 *S ADD SUB-BASINS "1" , "2" "U1" AND "PDN1" ****
 ADD HYD 200.11 13&** .05989 120.82 5.388 1.68686 1.533 3.152
 *S COMPUTE BASIN "3" ****
 COMPUTE NM HYD 2.60 - 3 .01510 34.57 1.332 1.65421 1.500 3.577 PER IMP= 75.00
 *S ADD SUB-BASINS "1" , "2" "U1" , "PDN1" AND "3" ****
 ADD HYD 2.70 *& 3 14 .07499 154.62 6.720 1.68028 1.500 3.222
 *S COMPUTE BASIN "E1" ****
 COMPUTE NM HYD 2.80 - 10 .01175 24.15 .863 1.37668 1.500 3.211 PER IMP= 50.00
 *S ADD SUB-BASINS "E1" , "1" "2" "3" AND "U1" ****
 ADD HYD 2.90 10&14 15 .08674 178.77 7.583 1.63915 1.500 3.220
 *S COMPUTE BASIN "E2" ****
 COMPUTE NM HYD 3.00 - 11 .04526 85.32 3.323 1.37668 1.533 2.946 PER IMP= 50.00
 *S ADD SUB-BASINS "E2" , "E1" "1" "2" "3" AND "U1" ****
 ADD HYD 3.20 15&11 16 .13200 262.23 10.906 1.54915 1.533 3.104
 *S COMPUTE BASIN "U2" (UNSER NORTH OF PDN)****
 COMPUTE NM HYD 3.30 - 20 .02593 48.61 2.508 1.81350 1.600 2.929 PER IMP= 90.00
 *S ADD SUB-BASINS "E1" , "E1" "1" "2" "3" , "U1" AND "U2" ****
 ADD HYD 3.40 20&16 17 .15793 305.56 13.414 1.59256 1.533 3.023
 *S COMPUTE BASIN "F" ****
 COMPUTE NM HYD 3.50 - 21 .00433 8.91 .318 1.37668 1.500 3.215 PER IMP= 50.00
 *S ADD SUB-BASINS "E1" , "E1" , "1" , "2" , "3" , "U1" , "U2" AND "F" ++++++
 *S

COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	PAGE = 3 NOTATION
COMPUTE NM HYD		3.50	-	21	.00433	8.91	.318	1.37668	1.500	

```

ADD HYD          3.60 17&21 18      .16226   AHYMO.SUM    1.58679   1.533   3.025
*S COMPUTE BASIN "11A" *****
*S COMPUTE NM HYD 3.70 - 22     .00656   15.30    .597    1.70664   1.500   3.643 PER IMP= 80.00
*S ADD SUB-BASTNS "E1" "E1", "1", "2", "3", "U1", "U2", "F" AND "11A" ++++++
*S THIS IS THE FLOW THAT IS BEING CONTRIBUTED TO THE UNSER DETENTION BASIN
ADD HYD          3.60 18&22 19      .16882   328.67   14.329   1.59145   1.533   3.042
*S THE OVERFLOW FOR THE CHAMISA DETENTION BASIN SHALL BE DONE IN A FUTURE STUDY.
*S THE UNSER DETENTION BASIN LIMITS FLOW TO 120 CFS
*S ROUTE THROUGH UNSER DETENTION BASIN >>>>>>>>>>>>>>>>>>>>>>
*S ROUTE RESERVOIR POND1 19 ***
*S ROUTE BASINS TO AP-3 THROUGH PNM BOUNDARY
*S COMPUTE BASIN "F1" *****
*S COMPUTE NM HYD 3.90 - 23     .02044   28.58    .779    .71481   1.500   2.185 PER IMP= .00
*S ROUTE THROUGH SUB-BASIN "H" IN CHANNEL "H" TO AP-3
ROUTE MCUNGE    4.10 ** 25     .16882   114.53   14.250   1.58269   2.567   1.060 CCODE = .1
*S COMPUTE BASIN "H" *****
*S COMPUTE NM HYD 4.20 - 26     .38256   288.36   8.238   .40375   1.533   1.178 PER IMP= .00
*S ADD SUB-BASTNS "F1", DISCHARGE FROM POND-1 AND "H"+++++(AP-3)+++++288.37 22.488
ADD HYD          4.30 26&25 27      .55138   288.37   22.488   .76472   1.533   .817
*S -----
*S BASINS EAST OF UNSER AND SOUTH OF PASEO DEL NORTE BLVD
*S -----
*S COMPUTE BASIN "4A" *****
*S COMPUTE NM HYD 5.00 - 40     .03880   90.40    3.532   1.70664   1.500   3.641 PER IMP= 80.00
*S COMPUTE BASIN "4B" *****
*S COMPUTE NM HYD 5.50 - 70     .00800   18.65    .728   1.70664   1.500   3.643 PER IMP= 80.00
*S ADD SUB-BASTNS "4A" AND "4B"+++++43 .04680   109.05   4.260   1.70663   1.500   3.641
ADD HYD          5.60 70&40 43
*S COMPUTE BASIN "PDN2" *****
*S COMPUTE NM HYD 5.30 - 60     .01260   30.43    1.219   1.81350   1.500   3.774 PER IMP= 90.00

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□

COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	PAGE =
*S ADD HYD	ADD SUB-BASTNS "4A", "4B" AND "PDN2"	42.10 60&43	42	.05940	139.49	5.478	1.72930	1.500	3.669	
*S ROUTE RESERVOIR	ROUTE THROUGH POND-10	42	**	.05940	18.59	5.375	1.69665	2.133	.489 AC-FT=	3.945
*S										

AHYMO.SUM

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*S ----- BASINS NORTH OF PASEO DEL NORTE BLVD TO EXISTING 72" IN PDN
*S COMPUTE BASIN "7" ****
*S COMPUTE BASIN "7" ****
*S COMPUTE NM HYD      5.70   -    80     .03540    77.49    3.222    1.70664    1.533    3.420 PER IMP= 80.00
*S ADD SUB-BASINS "POND 10" AND "7" ****
*S ADD HYD      5.20 ***&80  44     .09480    82.43    8.597    1.70038    1.533    1.359
*S COMPUTE BASIN "12B" ****
*S COMPUTE NM HYD      5.90   -    90     .01440    33.56    1.311    1.70664    1.500    3.642 PER IMP= 80.00
*S ADD SUB-BASINS "POND 10", "7" AND "12B" ****
*S ADD HYD      6.00  90&44  45     .10920   114.41    9.908    1.70120    1.533    1.637
*S COMPUTE BASIN "PDN3" ****
*S COMPUTE NM HYD      6.10   -    **     .01509    30.10    1.460    1.81351    1.567    3.117 PER IMP= 90.00
*S *** ****
*S ADD SUB-BASINS "POND 10", "7", "12B" AND PDN3 ****
*S ADD HYD      6.00 ***&45  48     .12429   143.17   11.367    1.71483    1.533    1.800
*S COMPUTE BASIN "PDN4" ****
*S COMPUTE NM HYD      6.30   -    **     .01114    24.99    1.077    1.81350    1.533    3.505 PER IMP= 90.00
*S ADD SUB-BASINS "POND 10", "7", "12B", PDN3 AND "PDN4" ****
*S ADD HYD      6.20  48&**  46     .13543   168.16   12.445    1.72295    1.533    1.940
*S ROUTE SUB-BASINS THROUGH PIPE IN "PDN4" >>>>>>>>>>>>>>>>>>>>>>>>>>>>>>>>
*S ROUTE MCUNGE      6.21   46   **     .13543   167.01   12.443    1.72272    1.533    1.927 CCODE = .2
*S COMPUTE BASIN "A" ****
*S COMPUTE NM HYD      9.30   -    72     .02750    65.96    2.603    1.77452    1.500    3.748 PER IMP= 85.00
*S FLOWS INTO EXISTING 72" PDN SYSTEM
*S ADD SUB-BASINS "POND 10", "7", "12B", "PDN3", "PDN4" AND A ****
*S ADD HYD      9.40  72&**  73     .16293   229.78   15.046    1.73146    1.533    2.204
*S *** ****
*S HYDROGRAPH IDENTIFICATION FROM TO PEAK RUNOFF TIME TO CFS PAGE =
COMMAND ID NO. ID AREA DISCHARGE VOLUME RUNOFF PEAK (SQ MI) (CFS) (AC-FT) (INCHES) (HOURS) PER NOTATION
*S *** ****
*S FLOWS INTO POND 2 ****
*S COMPUTE BASIN "5" ****
*S COMPUTE NM HYD      5.10   -    50     .02980    69.43    2.712    1.70664    1.500    3.641 PER IMP= 80.00
*S COMPUTE BASIN "6A" ****

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□

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COMPUTE NM HYD    7.00   -   60   .01527          AHYMO.SUM      1.390   1.70664   1.533   3.402 PER IMP= 80.00
*S ADD SUB-BASINS "5" AND "6A"+++++++.49.10 60&50 49   .04507          102.01   4.102   1.70663   1.500   3.536
*S COMPUTE BASIN "6" ****
*S COMPUTE NM HYD    7.10   -   61   .03547          82.64   3.229   1.70664   1.500   3.641 PER IMP= 80.00
*S ADD SUB-BASINS "6", "5" AND "6A"+++++++.7.20 49&61 50   .08054          184.65   7.331   1.70663   1.500   3.582
*S COMPUTE BASIN "8" ****
*S COMPUTE NM HYD    7.30   -   62   .01980          46.14   1.802   1.70664   1.500   3.641 PER IMP= 80.00
*S ADD SUB-BASINS "6", "6A" AND "8"+++++++.7.40 50&62 51   .10034          230.79   9.133   1.70663   1.500   3.594
*S COMPUTE BASIN "9" ****
*S COMPUTE NM HYD    7.50   -   63   .03156          73.54   2.873   1.70664   1.500   3.641 PER IMP= 80.00
*S ADD SUB-BASINS "6", "6A" "8" AND "9"+++++++.6.0 51&63 52   .13190          304.33   12.006   1.70663   1.500   3.605
*S ROUTE SUB-BASINS THROUGH PIPE IN UPPER HALF "ST1">>>>>>>>>>>>>>>>>>>>
*S ROUTE MCUNGE     7.51   52   1   .13190          303.29   12.005   1.70658   1.533   3.593 CCODE = .2
*S COMPUTE BASIN "12A" ****
*S COMPUTE NM HYD    7.70   -   64   .03080          71.76   2.803   1.70664   1.500   3.641 PER IMP= 80.00
*S ADD SUB-BASINS "6", "6A" "8" "9" AND "12A"+++++++.7.80 1&64 53   .16270          371.66   14.809   1.70659   1.533   3.569
*S COMPUTE BASIN "10" ****
*S COMPUTE NM HYD    7.90   -   65   .05670          122.05   5.161   1.70664   1.533   3.363 PER IMP= 80.00
*S

□
COMMAND   IDENTIFICATION   FROM ID   TO ID   AREA (SQ MI)   PEAK DISCHARGE (CFS)   RUNOFF VOLUME (AC-FT)   RUNOFF (INCHES)   TIME TO PEAK (HOURS)   CFS PER ACRE   PAGE = 6
NOTATION
*S ADD SUB-BASINS "6", "6A" "8" "9" "12A" AND "10"+++++++.8.00 53&65 54   .21940          493.72   19.969   1.70660   1.533   3.516
*S COMPUTE BASIN "ST1" ****
*S COMPUTE NM HYD    8.10   -   66   .01413          30.89   1.367   1.81350   1.533   3.416 PER IMP= 90.00
*S ADD SUB-BASINS "6", "6A" "8" "9" "12A" "10" AND "ST1"+++++++.8.20 66&54 55   .23553          524.61   21.336   1.71307   1.533   3.510
*S COMPUTE BASIN "ST2" ****

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* S TO EXISTING 72" PDN STORM SYSTEM (AP-1)                                AHYMO.SUM
* S ADD HYD      9.51 73&** **.57302   620.23   52.559   1.71979   1.667   1.691
* S EXISTING PDN STORM SYSTEM
* S ROUTE AP-1 THROUGH PIPE IN "PDNS" (AP-1 TO AP-2)>>>>>>>>>>>>>>>>>>>>
* S ROUTE MCUNGE "PDNS" *** 1 57302   619.44   52.540   1.71917   1.700   1.689 CCODE = .2
* S COMPUTE BASIN "PDNS" *** 1 57302   619.44   52.540   1.71917   1.700   1.689 CCODE = .2
* S COMPUTE NM HYD 9.70 - 75 .01978   47.77   1.913   1.81350   1.500   3.774 PER IMP= 90.00
* S ADD ALL SUB-BASINS NORTH OF PDN, SOUTH OF UNSER, "A" AND "PDNS"+++++*****+
* S ADD HYD      9.80 1&75 76 .59280   640.82   54.453   1.72231   1.700   1.689
* S COMPUTE BASIN "B" *** 1 59280   640.82   54.453   1.72231   1.700   1.689
* S COMPUTE NM HYD 9.90 - 77 .02114   15.98   .455   .40375   1.500   1.181 PER IMP= 0.00
* S ADD ALL SUB-BASINS N. OF PDN, S. UNSER, "A" AND "B"+++++(AP-2)+++++
* S ADD HYD      10.00 76&77 78 .61394   647.99   54.908   1.67691   1.700   1.649
* S ROUTE THROUGH "PDN6" IN PIPE (AP-2)>>>>>>>>>>>>>>>>>>>>>>>>>>>>>
* S ROUTE MCUNGE    788.10 78 **.61394   644.55   54.887   1.67627   1.700   1.640 CCODE = .2
* S COMPUTE BASIN "G" (AP-3)***+++++*****+
* S COMPUTE NM HYD 10.11 - 28 .10320   79.53   2.222   .40375   1.500   1.204 PER IMP= .00
* S ADD ALL SUB-BASINS NORTH OF PDN, SOUTH OF UNSER, "A", "B", "PDNS" AND "G"+++++
* S ADD HYD      10.12 28&** **.71714   696.48   57.109   1.49314   1.567   1.517
* S COMPUTE BASIN "PDN6" *** 1 71714   696.48   57.109   1.49314   1.567   1.517
* S COMPUTE NM HYD 10.20 - 79 .01847   44.61   1.786   1.81350   1.500   3.774 PER IMP= 90.00
* S ADD ALL SUB-BASINS TO "PDN6"+++++*****+
* S ADD HYD      10.30 **&79 80 .73561   733.18   58.895   1.50119   1.567   1.557
* S ADD FLOWS TO AP-4+++ (PIEDRAS MARCADAS)+++++*****+
* S ADD HYD      10.40 27&80 81 1.28699   1012.90   81.383   1.18566   1.567   1.230
* S -----UNIVERSE STORM DRAIN TO BOCA NEGRA DAM-----
* S COMPUTE STATE LAND OFFICE BASIN, P1 (SPLIT FROM SCHOOL SITE)*****+
* S COMPUTE NM HYD P2B - 50 .03125   44.34   1.515   .90878   1.533   2.217 PER IMP= 22.00
* S ROUTE THROUGH PIPE IN UNIVERSE BLVD>>>>>>>>>>>>>>>>>>>>>>>>>>>>>>>

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ROUTE MCUNGE      TRSLO1   50    71     .03125           AHYMO.SUM
                                                    43.94     1.510     .90626     1.567     2.197 CCODE =   .1
*S BASIN P2 (HIGH SCHOOL) FROM UNIVERSE STORM DRAIN STUDY*****.*****.*****
COMPUTE NM HYD P2A - 72   .10940 152.78   7.852     1.34570 1.633     2.182 PER IMP= 50.00
*S ADD STATE LAND OFFICE & HIGH SCHOOL+++++#+#+#+#+#+#+#+#+#+#+#+#+#+#+#+#+#+#+#+#+#+#+#+#+#+#
ADD HYD TR.HS 71&72 73   .14065 194.24   9.362     1.24806 1.600     2.158
*S ROUTE THROUGH PIPE IN UNIVERSE BLVD>>>>>>>>>>>>>>>>>>>>>>>>>>
ROUTE MCUNGE      TRHS 73    74     .14065           179.11     9.240     1.23175 1.700     1.990 CCODE =   .2
*S BASIN P3 (FUTURE SAD) FROM UNIVERSE STORM DRAIN STUDY*****.*****.*****
COMPUTE NM HYD P3 - 75   .05150 63.39     3.696     1.34570 1.667     1.923 PER IMP= 50.00
*S ADD UNIVERSE SUB-BASINS TO BOCA NEGRA DAM+++++#+#+#+#+#+#+#+#+#+#+#+#+#+#+#+#+#+#+#+#+#+#
ADD HYD TR.HS 74&75 76   .19215 242.47   12.936     1.26229 1.700     1.972
*S -----
*S UNSER BLVD SOUTH OF THE GRANT LINE TO BOCA NEGRA DAM
-----
*S -----
*S COMPUTE SUB-BASIN THE TRAILS UNIT 4 (SUB-BASIN T1)*****.*****.*****
*S ASSUME AN AREA OF 10 ACRES FOR THE THREE PONDS IN THIS SUB-BASIN
*S
*S COMPUTE NM HYD 10.10   - 10    .08900 148.83     6.612     1.39293 1.567     2.613 PER IMP= 50.00
*S RECALL POND K DISCHARGE FROM THE TRAILS SUBDIVISION*****.*****.*****
*S

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PROP VH.txt

* PASEO DEL NORTE DRAINAGE ANALYSIS
 * WILSON & COMPANY PROJECT X3210022
 * DEVELOPED CONDITIONS MODEL
 * 100 YR 6 HR STORM EVENT
 * DATE: DECEMBER 2011
 * FILE: X:\Public\PROJECTS\x3210022\Drainage Report\DOCS\AHYMO\PROP VH.TXT

*S*****
 *S
 *S 100 YEAR 6 HOUR STORM - PROPOSED RUNOFF ANALYSIS
 *S RAINFALL DATA FROM NOAA ATLAS 14
 *S
 *S*****

START 0.0 HOURS
 LOCATION BERNALILLO COUNTY
 RAINFALL TYPE=1 RAIN QUARTER=0.0 IN RAIN ONE=1.70 IN
 RAIN SIX=2.20 IN RAIN DAY=0 IN DT=0.033333 HR

*S
 *S
 *S -----
 *S SUB-BASINS NORTH AND WEST OF UNSER BLVD. TO POND 11,
 *S ULTIMATELY DISCHARGING TO CHAMISA BASIN
 *S -----

*S
 *S COMPUTE BASIN "E2.1" *****

COMPUTE LT TP LCODE=1 NK=2 ISLOPE=-1
 LENGTH=400 FT SLOPE=0.020 K=0.7
 LENGTH=480 FT SLOPE=0.020 K=2.0

COMPUTE NM HYD ID=99 HYD NO=200.1 DA=0.01243 SQ MI
 PER A=0 PER B=15 PER C=35 PER D=50
 TP=0.0 MASS RAIN=-1

PRINT HYD ID=99 CODE=1

*S
 *S THE TRAILS UNIT 4 - NORTH BASINS
 *S COMPUTE BASIN "K1" *****

COMPUTE LT TP LCODE=1 NK=3 ISLOPE=-1
 LENGTH=400 FT SLOPE=0.02 K=0.7
 LENGTH=600 FT SLOPE=0.02 K=2.0
 LENGTH=100 FT SLOPE=0.02 K=3.0

COMPUTE NM HYD ID=200 HYD NO=200.2 DA=0.0238 SQ MI
 PER A=0 PER B=10 PER C=10 PER D=80
 TP=0.0 MASS RAIN=-1

PRINT HYD ID=200 CODE=1

*S COMPUTE BASIN "K2" *****

COMPUTE LT TP LCODE=1 NK=1 ISLOPE=-1
 LENGTH=400 FT SLOPE=0.020 K=0.7

COMPUTE NM HYD ID=201 HYD NO=200.3 DA=0.0059 SQ MI
 PER A=0 PER B=10 PER C=10 PER D=80
 TP=0.0 MASS RAIN=-1

PRINT HYD ID=201 CODE=1

*S
 *S COMPUTE BASIN "K3" *****

COMPUTE LT TP LCODE=1 NK=3 ISLOPE=-1
 LENGTH=400 FT SLOPE=0.020 K=0.7
 LENGTH=600 FT SLOPE=0.020 K=2.0
 LENGTH=100 FT SLOPE=0.020 K=3.0

COMPUTE NM HYD ID=202 HYD NO=200.4 DA=0.0148 SQ MI
 PER A=0 PER B=10 PER C=10 PER D=80
 TP=0.0 MASS RAIN=-1

PRINT HYD ID=202 CODE=1

*S

PROP VH.txt

*S COMPUTE BASIN "K4" ****

COMPUTE LT TP LCODE=1 NK=2 ISLOPE=-1
 LENGTH=400 FT SLOPE=0.020 K=0.7
 LENGTH=590 FT SLOPE=0.020 K=2.0

COMPUTE NM HYD ID=203 HYD NO=200.5 DA=0.01959 SQ MI
 PER A=0 PER B=10 PER C=10 PER D=80
 TP=0.0 MASS RAIN=-1

PRINT HYD ID=203 CODE=1

*S
 *S
 *S COMPUTE BASIN "ST11" ****
 *

COMPUTE LT TP LCODE=1 NK=2 ISLOPE=-1
 LENGTH=400 FT SLOPE=0.020 K=0.7
 LENGTH=1730 FT SLOPE=0.020 K=2.0

COMPUTE NM HYD ID=204 HYD NO=200.6 DA=0.006758 SQ MI
 PER A=0 PER B=10 PER C=0 PER D=90
 TP=0.0 MASS RAIN=-1

PRINT HYD ID=204 CODE=1

*S
 *S ADD SUB-BASINS "K1" AND "K2" ++++++

ADD HYD ID=205 HYD NO=200.7 ID I=200 ID II=201
 PRINT HYD ID=205 CODE=1

*S
 *S ADD SUB-BASINS "K3" AND "K4" ++++++

ADD HYD ID=206 HYD NO=200.8 ID I=202 ID II=203
 PRINT HYD ID=206 CODE=1

*S
 *S ADD SUB-BASINS "K1", "K2" AND "E2.1" ++++++

ADD HYD ID=207 HYD NO=200.9 ID I=99 ID II=205
 PRINT HYD ID=207 CODE=1

*S
 *S ADD SUB-BASINS "K1", "K2", "E2.1" AND "K3", "K4" ++++++

ADD HYD ID=208 HYD NO=200.10 ID I=206 ID II=207
 PRINT HYD ID=208 CODE=1

*S
 *S
 *S -----
 *S SUB-BASINS NORTH AND WEST OF UNSER BLVD W/ OVERFLOW OF STORMS (5 YR EVENT MIN)
 *S TO THE CHAMISA DETENTION. THE UNSER DETENTION BASIN OUTFLOW IS LIMITED
 *S TO 120 CFS. FLOWS ULTIMATELY DISCHARGE TO AP3
 *S -----

*S COMPUTE BASIN "1" ****
 *

COMPUTE LT TP LCODE=1 NK=2 ISLOPE=-1
 LENGTH=400 FT SLOPE=0.020 K=0.7
 LENGTH=275 FT SLOPE=0.020 K=2.0

COMPUTE NM HYD ID=1 HYD NO=1.1 DA=0.01324 SQ MI
 PER A=0 PER B=15 PER C=35 PER D=50
 TP=0.0 MASS RAIN=-1

PRINT HYD ID=1 CODE=1

*S
 *S COMPUTE BASIN "2" ****

COMPUTE LT TP LCODE=1 NK=2 ISLOPE=-1
 LENGTH=400 FT SLOPE=0.020 K=0.7
 LENGTH=280 FT SLOPE=0.020 K=2.0

COMPUTE NM HYD ID=2 HYD NO=1.2 DA=0.0113 SQ MI
 PER A=0 PER B=10 PER C=15 PER D=75
 TP=0.0 MASS RAIN=-1

PROP VH.txt

PRINT HYD ID=2 CODE=1

*S
 *S ADD SUB-BASINS "1" AND "2" ++++++
 ADD HYD ID=11 HYD NO=1.3 ID I=1 ID II=2
 PRINT HYD ID=11 CODE=1
 *S
 *S COMPUTE BASIN "U1" (UNSER TO PDN)*****
 *S COMPUTE LT TP LCODE=1 NK=2 ISLOPE=-1
 LENGTH=400 FT SLOPE=0.020 K=0.7
 LENGTH=280 FT SLOPE=0.020 K=2.0
 COMPUTE NM HYD ID=4 HYD NO=1.4 DA=0.0158 SQ MI
 PER A=0 PER B=10 PER C=0 PER D=90
 TP=0.0 MASS RAIN=-1
 PRINT HYD ID=4 CODE=1
 *S
 *S ADD SUB-BASINS "1", "2" AND "U1" ++++++
 ADD HYD ID=13 HYD NO=1.5 ID I=11 ID II=4
 PRINT HYD ID=13 CODE=1
 *S
 *S COMPUTE BASIN "PDN1" *****
 *S COMPUTE LT TP LCODE=1 NK=2 ISLOPE=-1
 LENGTH=400 FT SLOPE=0.020 K=0.7
 LENGTH=2510 FT SLOPE=0.020 K=2.0
 COMPUTE NM HYD ID=209 HYD NO=1.9 DA=0.01955 SQ MI
 PER A=0 PER B=10 PER C=0 PER D=90
 TP=0.0 MASS RAIN=-1
 PRINT HYD ID=209 CODE=1
 *S
 *S ADD SUB-BASINS "1", "2", "U1" AND "PDN1" ++++++
 ADD HYD ID=210 HYD NO=200.11 ID I=13 ID II=209
 PRINT HYD ID=210 CODE=1
 *S
 *S COMPUTE BASIN "3" *****
 *S COMPUTE LT TP LCODE=1 NK=2 ISLOPE=-1
 LENGTH=400 FT SLOPE=0.020 K=0.7
 LENGTH=470 FT SLOPE=0.020 K=2.0
 COMPUTE NM HYD ID=3 HYD NO=2.6 DA=0.0151 SQ MI
 PER A=0 PER B=10 PER C=15 PER D=75
 TP=0.0 MASS RAIN=-1
 PRINT HYD ID=3 CODE=1
 *S
 *S ADD SUB-BASINS "1", "2", "U1", "PDN1" AND "3"++++++
 ADD HYD ID=14 HYD NO=2.7 ID I=210 ID II=3
 PRINT HYD ID=14 CODE=1
 *S
 *S COMPUTE BASIN "E1" *****
 *S COMPUTE LT TP LCODE=1 NK=2 ISLOPE=-1
 LENGTH=400 FT SLOPE=0.030 K=0.7
 LENGTH=1060 FT SLOPE=0.030 K=2.0
 COMPUTE NM HYD ID=10 HYD NO=2.8 DA=0.01175 SQ MI
 PER A=0 PER B=15 PER C=35 PER D=50
 TP=0.0 MASS RAIN=-1

PROP VH.txt

PRINT HYD ID=10 CODE=1

*S
*S ADD SUB-BASINS "E1", "1", "2","3" AND "U1" ++++++*****+++++*****+

ADD HYD ID=15 HYD NO=2.9 ID I=10 ID II=14
PRINT HYD ID=15 CODE=1

*S
*S COMPUTE BASIN "E2" *****

COMPUTE LT TP LCODE=1 NK=3 ISLOPE=-1
LENGTH=400 FT SLOPE=0.030 K=0.7
LENGTH=600 FT SLOPE=0.030 K=2.0
LENGTH=1800 FT SLOPE=0.030 K=3.0

COMPUTE NM HYD ID=11 HYD NO=3.0 DA=0.045259 SQ MI
PER A=0 PER B=15 PER C=35 PER D=50
TP=0.0 MASS RAIN=-1

PRINT HYD ID=11 CODE=1

*S
*S ADD SUB-BASINS "E2","E1", "1", "2","3" AND "U1" ++++++*****+++++*****+

ADD HYD ID=16 HYD NO=3.2 ID I=15 ID II=11
PRINT HYD ID=16 CODE=1

*S COMPUTE BASIN "U2" (UNSER NORTH OF PDN)*****
*S

COMPUTE LT TP LCODE=1 NK=3 ISLOPE=-1
LENGTH=400 FT SLOPE=0.02 K=.7
LENGTH=600 FT SLOPE=0.02 K=2.0
LENGTH=2550 FT SLOPE=0.02 K=3.0

COMPUTE NM HYD ID=20 HYD NO=3.3 DA=0.025933 SQ MI
PER A=0 PER B=10 PER C=0 PER D=90
TP=0.0 MASS RAIN=-1

PRINT HYD ID=20 CODE=1

*S
*S ADD SUB-BASINS "E1","E1", "1", "2","3","U1" AND "U2" ++++++*****+++++*****

ADD HYD ID=17 HYD NO=3.4 ID I=20 ID II=16
PRINT HYD ID=17 CODE=1

*S
*S COMPUTE BASIN "F" *****
*S

COMPUTE LT TP LCODE=1 NK=1 ISLOPE=-1
LENGTH=320 FT SLOPE=0.020 K=0.7

COMPUTE NM HYD ID=21 HYD NO=3.5 DA=0.00433 SQ MI
PER A=0 PER B=15 PER C=35 PER D=50
TP=0.0 MASS RAIN=-1

PRINT HYD ID=21 CODE=1

*S
*S ADD SUB-BASINS "E1","E1", "1", "2","3","U1", "U2" AND "F"+++++*****+++++*****

ADD HYD ID=18 HYD NO=3.6 ID I=17 ID II=21
PRINT HYD ID=18 CODE=1

*S
*S COMPUTE BASIN "11A" *****
*S

COMPUTE LT TP LCODE=1 NK=2 ISLOPE=-1
LENGTH=400 FT SLOPE=0.020 K=0.7
LENGTH=80 FT SLOPE=0.020 K=2.0

COMPUTE NM HYD ID=22 HYD NO=3.7 DA=0.00656 SQ MI
PER A=0 PER B=10 PER C=10 PER D=80
TP=0.0 MASS RAIN=-1

PROP VH.txt

PRINT HYD ID=22 CODE=1
 *S
 *S ADD SUB-BASINS "E1", "E1", "1", "2", "3", "U1", "U2", "F" AND "11A" ++++++
 *S THIS IS THE FLOW THAT IS BEING CONTRIBUTED TO THE UNSER DETENTION BASIN

ADD HYD ID=19 HYD NO=3.6 ID I=18 ID II=22
 PRINT HYD ID=19 CODE=1
 *S
 *S THE OVERFLOW FOR THE CHAMISA DETENTION BASIN SHALL BE DONE IN A FUTURE STUDY.
 *S THE UNSER DETENTION BASIN LIMITS FLOW TO 120 CFS
 *S ROUTE THROUGH UNSER DETENTION BASIN
 >>>>>>>>>>>>>>>>>>>>>>>>>>>>>>>>>>

ROUTE RESERVOIR	ID=220 HYD=POND1	INFLOW ID=19 CODE=10
	OUTFLOW(CFS)	STORAGE(AC-FT) ELEV(FT)
	0	0 0
	10	2.0 1
	20	3.5 2
	50	5.0 3
	80	6.0 4
	110	7.0 5
	130	8.0 6
	150	9.5 7
	170	11.0 8

PRINT HYD ID=220 CODE=10
 *S ROUTE BASINS TO AP-3 THROUGH PNM BOUNDARY
 *S
 *S COMPUTE BASIN "F1" *****
 *S

COMPUTE LT TP LCODE=1 NK=1 ISLOPE=-1
 LENGTH=400 FT SLOPE=0.020 K=0.7

COMPUTE NM HYD ID=23 HYD NO=3.9 DA=0.02044 SQ MI
 PER A=0 PER B=60 PER C=40 PER D=0
 TP=0.0 MASS RAIN=-1

PRINT HYD ID=23 CODE=1
 *S ROUTE THROUGH SUB-BASIN "H" IN CHANNEL "H" TO AP-3

COMPUTE RATING CURVE CID=1 VS NO=1 NO SEGS=1
 MIN ELEV=0 MAX ELEV=4.50
 CH SLOPE=0.020 FP SLOPE=0.020
 N=0.038 DIST=52.0
 DIST ELEV DIST ELEV
 0.0 5.0 6.0 0.0
 46.0 0.0 52.0 5.0

ROUTE MCUNGE ID=25 HYD NO=4.1 INFLOW ID=220
 DT=0.0 L=6500 FT NS=0 SLOPE=0.020
 MATCODE=0 REGCODE=0 CCODE=0 MM CODE=0

PRINT HYD ID=25 CODE=1
 *S
 *S COMPUTE BASIN "H" *****
 *S

COMPUTE LT TP LCODE=1 NK=3 ISLOPE=-1
 LENGTH=400 FT SLOPE=0.040 K=0.7
 LENGTH=2100 FT SLOPE=0.060 K=2.0
 LENGTH=4000 FT SLOPE=0.060 K=3.0

COMPUTE NM HYD ID=26 HYD NO=4.2 DA=0.38256 SQ MI
 PER A=100 PER B=0 PER C=0 PER D=0
 TP=0.0 MASS RAIN=-1

PRINT HYD ID=26 CODE=1
 *S
 *S ADD SUB-BASINS "F1", DISCHARGE FROM POND-1 AND "H"+++(AP-3)+++++

ADD HYD ID=27 HYD NO=4.3 ID I=26 ID II=25

PROP VH.txt

```

PRINT HYD      ID=27    CODE=1
*S
*S
*S -----
*S BASINS EAST OF UNSER AND SOUTH OF PASEO DEL NORTE BLVD
*S -----
*S
*S COMPUTE BASIN "4A" ****
*S
COMPUTE LT TP  LCODE=1 NK=2 ISLOPE=-1
LENGTH=400 FT SLOPE=0.020 K=0.7
LENGTH=760 FT SLOPE=0.020 K=2.0

COMPUTE NM HYD ID=40    HYD NO=5.0      DA=0.0388 SQ MI
PER A=0      PER B=10     PER C=10      PER D=80
TP=0.0       MASS RAIN=-1

PRINT HYD      ID=40    CODE=1

*S COMPUTE BASIN "4B" ****
*S
COMPUTE LT TP  LCODE=1 NK=2 ISLOPE=-1
LENGTH=400 FT SLOPE=0.020 K=0.7
LENGTH=410 FT SLOPE=0.020 K=2.0

COMPUTE NM HYD ID=70    HYD NO=5.5      DA=0.0080 SQ MI
PER A=0      PER B=10     PER C=10      PER D=80
TP=0.0       MASS RAIN=-1

PRINT HYD      ID=70    CODE=1

*S
*S ADD SUB-BASINS "4A"AND "4B"++++++
*S
ADD HYD      ID=43    HYD NO=5.6      ID I=70 ID II=40
PRINT HYD      ID=43    CODE=1
*S
*S
*S COMPUTE BASIN "PDN2" ****
*S
COMPUTE LT TP  LCODE=1 NK=2 ISLOPE=-1
LENGTH=400 FT SLOPE=0.020 K=0.7
LENGTH=750 FT SLOPE=0.020 K=2.0

COMPUTE NM HYD ID=60    HYD NO=5.3      DA=0.0126 SQ MI
PER A=0      PER B=10     PER C=0       PER D=90
TP=0.0       MASS RAIN=-1

PRINT HYD      ID=60    CODE=1

*S
*S ADD SUB-BASINS "4A", "4B" AND "PDN2"++++++
*S
ADD HYD      ID=42    HYD NO=42.1     ID I=60 ID II=43
PRINT HYD      ID=42    CODE=1
*
*S
*S ROUTE THROUGH POND-10 >>>>>>>>>>>>>>>>>>>>>>>>>>>>>>>
ROUTE RESERVOIR      ID=421   HYD=POND10      INFLOW ID=42 CODE=10
OUTFLOW(CFS)      STORAGE(AC-FT)    ELEV(FT)
0                  0            0
5                  2            2
10                 3            4
20                 4.1           6

PRINT HYD      ID=421    CODE=10
*S
*S -----
*S BASINS NORTH OF PASEO DEL NORTE BLVD TO EXISTING 72" IN PDN
*S -----

```

PROP VH.txt

```

*S COMPUTE BASIN "7" ****
*S
COMPUTE LT TP    LCODE=1 NK=2 ISLOPE=-1
                  LENGTH=400 FT SLOPE=0.020 K=0.7
                  LENGTH=1150 FT SLOPE=0.020 K=2.0

COMPUTE NM HYD   ID=80     HYD NO=5.7      DA=0.0354 SQ MI
                  PER A=0   PER B=10  PER C=10  PER D=80
                  TP=0.0   MASS RAIN=-1

PRINT HYD        ID=80     CODE=1

*S
*S ADD SUB-BASINS "POND 10" AND "7"+++++
ADD HYD          ID=44     HYD NO=5.2      ID I=421       ID II=80
PRINT HYD        ID=44     CODE=1

*S
*S COMPUTE BASIN "12B" ****
*S
COMPUTE LT TP    LCODE=1 NK=2 ISLOPE=-1
                  LENGTH=400 FT SLOPE=0.020 K=0.7
                  LENGTH=800 FT SLOPE=0.020 K=2.0

COMPUTE NM HYD   ID=90     HYD NO=5.9      DA=0.0144 SQ MI
                  PER A=0   PER B=10  PER C=10  PER D=80
                  TP=0.0   MASS RAIN=-1

PRINT HYD        ID=90     CODE=1

*S
*S ADD SUB-BASINS "POND 10", "7" AND "12B"+++++
ADD HYD          ID=45     HYD NO=6.0      ID I=90 ID II=44
PRINT HYD        ID=45     CODE=1

*S
*S COMPUTE BASIN "PDN3" ****
*S
COMPUTE LT TP    LCODE=1 NK=3 ISLOPE=-1
                  LENGTH=400 FT SLOPE=0.020 K=0.7
                  LENGTH=500 FT SLOPE=0.020 K=2.0
                  LENGTH=2130 FT SLOPE=0.020 K=3.0

COMPUTE NM HYD   ID=100    HYD NO=6.1      DA=0.01509 SQ MI
                  PER A=0  PER B=10  PER C=0   PER D=90
                  TP=0.0  MASS RAIN=-1

PRINT HYD        ID=100    CODE=1

*S
*S ****
*S ADD SUB-BASINS "POND 10", "7", "12B" AND PDN3+++++
ADD HYD          ID=48     HYD NO=6.0      ID I=100      ID II=45
PRINT HYD        ID=48     CODE=1

*S
*S COMPUTE BASIN "PDN4" ****
*S
COMPUTE LT TP    LCODE=1 NK=3 ISLOPE=-1
                  LENGTH=400 FT SLOPE=0.020 K=0.7
                  LENGTH=500 FT SLOPE=0.020 K=2.0
                  LENGTH=1130 FT SLOPE=0.020 K=3.0

COMPUTE NM HYD   ID=110    HYD NO=6.3      DA=0.01114 SQ MI
                  PER A=0  PER B=10  PER C=0   PER D=90
                  TP=0.0  MASS RAIN=-1

PRINT HYD        ID=110    CODE=1

*S
*S ADD SUB-BASINS "POND 10", "7", "12B", PDN3 AND "PDN4"+++++
ADD HYD          ID=46     HYD NO=6.2      ID I=48 ID II=110

```


PROP VH.txt

COMPUTE LT TP LCODE=1 NK=3 ISLOPE=-1
LENGTH=400 FT SLOPE=0.02 K=0.7
LENGTH=600 FT SLOPE=0.02 K=2.0
LENGTH=150 FT SLOPE=0.02 K=3.0

COMPUTE NM HYD ID=61 HYD NO=7.1 DA=0.03547 SQ MI
PER A=0 PER B=10 PER C=10 PER D=80
TP=0.0 MASS RAIN=-1

PRINT HYD ID=61 CODE=1

*S
*S ADD SUB-BASINS "6", "5" AND "6A"+++++*****+++++*****+++++*****

ADD HYD ID=50 HYD NO=7.2 ID I=49 ID II=61
PRINT HYD ID=50 CODE=1

*S
*S COMPUTE BASIN "8" *****
*S

COMPUTE LT TP LCODE=1 NK=2 ISLOPE=-1
LENGTH=400 FT SLOPE=0.02 K=0.7
LENGTH=480 FT SLOPE=0.02 K=2.0

COMPUTE NM HYD ID=62 HYD NO=7.3 DA=0.0198 SQ MI
PER A=0 PER B=10 PER C=10 PER D=80
TP=0.0 MASS RAIN=-1

PRINT HYD ID=62 CODE=1

*S
*S ADD SUB-BASINS "6", "6A" AND "8"+++++*****+++++*****+++++*****

ADD HYD ID=51 HYD NO=7.4 ID I=50 ID II=62
PRINT HYD ID=51 CODE=1

*S
*S COMPUTE BASIN "9" *****
*S

COMPUTE LT TP LCODE=1 NK=2 ISLOPE=-1
LENGTH=400 FT SLOPE=0.02 K=0.7
LENGTH=890 FT SLOPE=0.02 K=2.0

COMPUTE NM HYD ID=63 HYD NO=7.5 DA=0.03156 SQ MI
PER A=0 PER B=10 PER C=10 PER D=80
TP=0.0 MASS RAIN=-1

PRINT HYD ID=63 CODE=1

*S
*S ADD SUB-BASINS "6", "6A", "8" AND "9"+++++*****+++++*****+++++*****

ADD HYD ID=52 HYD NO=7.6 ID I=51 ID II=63
PRINT HYD ID=52 CODE=1

*S
*S ROUTE SUB-BASINS THROUGH PIPE IN UPPER HALF "ST1">>>>>>>>>>>>>>>>>>>>>>>>>>>>>

COMPUTE RATING CURVE CID=1 VS NO=1 CODE=-1 SLP=0.022
DIA=5.0 FT N=0.013

ROUTE MCUNGE ID=1 HYD NO=7.51 INFLOW ID=52
DT=0.0 L=1566 NS=0 SLP=0.022
MATCODE=0 REGCODE=0 CCODE=0 MM CODE=0

PRINT HYD ID=1 CODE=1

*S
*S COMPUTE BASIN "12A" *****
*S

COMPUTE LT TP LCODE=1 NK=2 ISLOPE=-1
LENGTH=400 FT SLOPE=0.02 K=0.7
LENGTH=820 FT SLOPE=0.02 K=2.0

COMPUTE NM HYD ID=64 HYD NO=7.7 DA=0.0308 SQ MI
PER A=0 PER B=10 PER C=10 PER D=80
TP=0.0 MASS RAIN=-1

PROP VH.txt

```

PRINT HYD      ID=64      CODE=1

*S
*S ADD SUB-BASINS "6","6A","8","9" AND "12A"+++++++++++++++++++++++++++++
ADD HYD      ID=53      HYD NO=7.8      ID I=1  ID II=64
PRINT HYD      ID=53      CODE=1

*S
*S COMPUTE BASIN "10" ****
*S
COMPUTE LT TP  LCODE=1 NK=2 ISLOPE=-1
LENGTH=400 FT SLOPE=0.02 K=0.7
LENGTH=1270 FT SLOPE=0.02 K=2.0

COMPUTE NM HYD ID=65      HYD NO=7.9      DA=0.0567 SQ MI
PER A=0      PER B=10     PER C=10     PER D=80
TP=0.0      MASS RAIN=-1

PRINT HYD      ID=65      CODE=1

*S
*S ADD SUB-BASINS "6","6A","8","9","12A" AND "10"+++++++++++++++++++++
ADD HYD      ID=54      HYD NO=8.0      ID I=53 ID II=65
PRINT HYD      ID=54      CODE=1

*S
*S
*S COMPUTE BASIN "ST1" ****
*S
COMPUTE LT TP  LCODE=1 NK=2 ISLOPE=-1
LENGTH=400 FT SLOPE=0.02 K=0.7
LENGTH=1400 FT SLOPE=0.02 K=2.0

COMPUTE NM HYD ID=66      HYD NO=8.1      DA=0.01413 SQ MI
PER A=0      PER B=10     PER C=0      PER D=90
TP=0.0      MASS RAIN=-1

PRINT HYD      ID=66      CODE=1

*S
*S ADD SUB-BASINS "6","6A","8","9","12A","10" AND "ST1"+++++
ADD HYD      ID=55      HYD NO=8.2      ID I=66 ID II=54
PRINT HYD      ID=55      CODE=1
*S
*S

*S
*S COMPUTE BASIN "ST2"****
*S
COMPUTE LT TP  LCODE=1 NK=2 ISLOPE=-1
LENGTH=400 FT SLOPE=0.02 K=0.7
LENGTH=1400 FT SLOPE=0.02 K=2.0

COMPUTE NM HYD ID=67      HYD NO=8.3      DA=0.01094 SQ MI
PER A=0      PER B=10     PER C=0      PER D=90
TP=0.0      MASS RAIN=-1

PRINT HYD      ID=67      CODE=1

*S
*S ADD SUB-BASINS "6","6A","8","9","12A","10","ST1" AND "ST2"+++++
ADD HYD      ID=56      HYD NO=8.4      ID I=55 ID II=67
PRINT HYD      ID=56      CODE=1
*S
*S
*S

*S -----
*S REMAINDER OF SUB-BASINS IN INTERNAL STREET TO POND 2
*S -----
*S
*S COMPUTE BASIN "13" ****

```

PROP VH.txt

*S

COMPUTE LT TP LCODE=1 NK=3 ISLOPE=-1
 LENGTH=400 FT SLOPE=0.020 K=0.7
 LENGTH=600 FT SLOPE=0.020 K=2.0
 LENGTH=680 FT SLOPE=0.020 K=3.0

COMPUTE NM HYD ID=120 HYD NO=6.5 DA=0.0626 SQ MI
 PER A=0 PER B=10 PER C=10 PER D=80
 TP=0.0 MASS RAIN=-1

PRINT HYD ID=120 CODE=1

*S

*S COMPUTE BASIN "11B" *****
 *S

COMPUTE LT TP LCODE=1 NK=2 ISLOPE=-1
 LENGTH=400 FT SLOPE=0.02 K=0.7
 LENGTH=1400 FT SLOPE=0.02 K=2.0

COMPUTE NM HYD ID=68 HYD NO=8.5 DA=0.05527 SQ MI
 PER A=0 PER B=10 PER C=10 PER D=80
 TP=0.0 MASS RAIN=-1

PRINT HYD ID=68 CODE=1

*S

*S ADD BASINS "11B" AND "13"++++++

ADD HYD ID=681 HYD NO=6.6 ID I=120 ID II=68
 PRINT HYD ID=681 CODE=1

*S

*S COMPUTE BASIN "ST3" *****
 *S

COMPUTE LT TP LCODE=1 NK=2 ISLOPE=-1
 LENGTH=400 FT SLOPE=0.02 K=0.7
 LENGTH=1400 FT SLOPE=0.02 K=2.0

COMPUTE NM HYD ID=69 HYD NO=8.7 DA=0.00686 SQ MI
 PER A=0 PER B=10 PER C=0 PER D=90
 TP=0.0 MASS RAIN=-1

PRINT HYD ID=69 CODE=1

*S

*S ADD SUB-BASINS "11B" AND "ST3"++++++

ADD HYD ID=58 HYD NO=8.8 ID I=681 ID II=69
 PRINT HYD ID=58 CODE=1

*S

*S COMPUTE BASIN "ST4" *****
 *S

COMPUTE LT TP LCODE=1 NK=2 ISLOPE=-1
 LENGTH=400 FT SLOPE=0.02 K=0.7
 LENGTH=1400 FT SLOPE=0.02 K=2.0

COMPUTE NM HYD ID=588 HYD NO=8.9 DA=0.00772 SQ MI
 PER A=0 PER B=10 PER C=0 PER D=90
 TP=0.0 MASS RAIN=-1

PRINT HYD ID=588 CODE=1

*S

*S ADD SUB-BASINS "11B", "ST3" AND "ST4"++++++

ADD HYD ID=59 HYD NO=9.0 ID I=58 ID II=588
 PRINT HYD ID=59 CODE=1

*S

*S COMPUTE BASIN "11C" *****
 *S

COMPUTE LT TP LCODE=1 NK=2 ISLOPE=-1

PROP VH.txt

LENGTH=400 FT SLOPE=0.02 K=0.7
LENGTH=50 FT SLOPE=0.02 K=2.0

COMPUTE NM HYD ID=71 HYD NO=9.1 DA=0.03317 SQ MI
PER A=0 PER B=10 PER C=10 PER D=80
TP=0.0 MASS RAIN=-1

PRINT HYD ID=71 CODE=1

*S
*S ADD SUB-BASINS "13", "11B", "ST3", "ST4" AND "11C"++++++
*S
ADD HYD ID=60 HYD NO=9.2 ID I=71 ID II=59
PRINT HYD ID=60 CODE=1
*S
*S

*S ALL DISCHARGE TO POND 2

*S ADD ALL SUB-BASINS NORTH OF PDN AND REMAINDER OF INTERNAL STREETS
+++++
*S DISCHARGING TO POND 2
*S
ADD HYD ID=74 HYD NO=9.5 ID I=56 ID II=60
PRINT HYD ID=74 CODE=1

*S
*S ROUTE THROUGH POND-2 >>>>>>>>>>>>>>>>>>>>>>>>>>>>>>>>>>>>>

	ID=601 HYD=POND2	INFLOW ID=74 CODE=10
OUTFLOW(CFS)	STORAGE(AC-FT)	ELEV(FT)
0	0	5290
20	5	5291
40	7	5292
300	9	5293
350	13	5295
530	15	5296

PRINT HYD ID=601 CODE=10
*S
*S

*S ALL DISCHARGE FROM POND 2, BASIN A AND BASINS NORTH OF PDN
*S TO EXISTING 72" PDN STORM SYSTEM (AP-1)

*S
ADD HYD ID=744 HYD NO=9.51 ID I=73 ID II=601
PRINT HYD ID=744 CODE=1
*S
*S

*S EXISTING PDN STORM SYSTEM

*S
*S ROUTE AP-1 THROUGH PIPE IN "PDN5" (AP-1 TO AP-2)>>>>>>>>>>>>>>>>>>>>>>>>>>>>>>
*S
COMPUTE RATING CURVE CID=1 VS NO=1 CODE=-1 SLP=0.01
DIA=7.0 FT N=0.013

ROUTE MCUNGE ID=1 HYD NO=9.6 INFLOW ID=744
DT=0.0 L=1500 NS=0 SLP=0.01
MATCODE=0 REGCODE=0 CCODE=0 MM CODE=0

PRINT HYD ID=1 CODE=1

*S COMPUTE BASIN "PDN5" *****
*S
COMPUTE LT TP LCODE=1 NK=3 ISLOPE=-1
LENGTH=50 FT SLOPE=0.02 K=0.7
LENGTH=250 FT SLOPE=0.02 K=2.0
LENGTH=900 FT SLOPE=0.02 K=3.0

COMPUTE NM HYD ID=75 HYD NO=9.7 DA=0.01978 SQ MI
PER A=0 PER B=10 PER C=0 PER D=90


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        PROP VH.txt
COMPUTE NM HYD ID=79    HYD NO=10.2      DA=0.01847 SQ MI
PER A=0    PER B=10   PER C=0    PER D=90
TP=0.0    MASS RAIN=-1

PRINT HYD      ID=79    CODE=1

*S
*S ADD ALL SUB-BASINS TO "PDN6"+++++++++++++
ADD HYD      ID=80    HYD NO=10.3      ID I=281          ID II=79
PRINT HYD      ID=80    CODE=1
*S
*S
*S ADD FLOWS TO AP-4+++ (PIEDRAS MARCADAS)+++++++++
ADD HYD      ID=81    HYD NO=10.4      ID I=27 ID II=80
PRINT HYD      ID=81    CODE=1
*S
*S -----
*S UNIVERSE STORM DRAIN TO BOCA NEGRA DAM
*S -----
*S

*S COMPUTE STATE LAND OFFICE BASIN, P1 (SPLIT FROM SCHOOL
SITE)*****
*
COMPUTE LT TP      LCODE=1 NK=2 ISLOPE=-1
                   LENGTH=400 SLOPE=0.0148 K=1.0
                   LENGTH=1100 SLOPE=0.0148 K=2.0
*
COMPUTE NM HYD      ID=50    HYD=P2B DA=0.03125 SQ MI
                   A=25  B=26 C=27 D=22
                   TP=0.0 MASSRAIN=-1
PRINT HYD      ID=50    CODE=5
*S
*S
*S ROUTE THROUGH PIPE IN UNIVERSE BLVD>>>>>>>>>>>>>>>>>>>>>>>>>>>>>>>>>>
COMPUTE RATING CURVE CID=1 VS NO=1 CODE=-1 SLP=0.02
                   DIA=2.5 N=0.013
ROUTE MCUNGE      ID=71    HYD=TRSL01 INFLOW ID=50
                   DT=0.0  L=1100 NS=0  SLOPE=0.022
                   MATCODE=0 REGCODE=0 CCODE=0 MM CODE=0
PRINT HYD      ID=71    CODE=5
*S
*S BASIN P2 (HIGH SCHOOL) FROM UNIVERSE STORM DRAIN STUDY*****
*
COMPUTE LT TP      LCODE=1 NK=3 ISLOPE=-1
                   LENGTH=400 SLOPE=0.0148 K=1.0
                   LENGTH=1600 SLOPE=0.0148 K=2.0
                   LENGTH=1300 SLOPE=0.0148 K=3.0
*
COMPUTE NM HYD      ID=72    HYD=P2A DA=0.1094 SQ MI
                   A=0  B=25 C=25 D=50
                   TP=0.0 MASSRAIN=-1
PRINT HYD      ID=72    CODE=5
*S
*S ADD STATE LAND OFFICE & HIGH SCHOOL+++++
ADD HYD      ID=73    HYD=TR.HS ID I=71 II=72
PRINT HYD      ID=73    CODE=5
*S
*S ROUTE THROUGH PIPE IN UNIVERSE BLVD>>>>>>>>>>>>>>>>>>>>>>>>>>>>>>>>>>>>>
COMPUTE RATING CURVE CID=1 VS NO=1 CODE=-1 SLP=0.013
                   DIA=4 N=0.013
*
ROUTE MCUNGE      ID=74    HYD=TRHS INFLOW ID=73
                   DT=0.0  L=1700 NS=0  SLOPE=0.013
                   MATCODE=0 REGCODE=0 CCODE=0 MM CODE=0
PRINT HYD      ID=74    CODE=5
*S
*S BASIN P3 (FUTURE SAD) FROM UNIVERSE STORM DRAIN STUDY*****
*
COMPUTE LT TP      LCODE=1 NK=3 ISLOPE=-1
                   LENGTH=400 SLOPE=0.0084 K=0.7
                   LENGTH=1600 SLOPE=0.0084 K=2.0
                   LENGTH=400 SLOPE=0.0084 K=3.0
*
COMPUTE NM HYD      ID=75    HYD=P3 DA=0.0515 SQ MI
                   A=0  B=25 C=25 D=50

```

PROP VH.txt

TP=0.0 MASSRAIN=-1

PRINT HYD ID=75 CODE=5

*S

*S ADD UNIVERSE SUB-BASINS TO BOCA NEGRA DAM+++++*****+

ADD HYD ID=76 HYD=TR.HS ID I=74 II=75

PRINT HYD ID=76 CODE=5

*S

*S -----

*S UNSER BLVD SOUTH OF THE GRANT LINE TO BOCA NEGRA DAM

*S -----

*S

*S COMPUTE SUB-BASIN THE TRAILS UNIT 4 (SUB-BASIN T1)*****

*S ASSUME AN AREA OF 10 ACRES FOR THE THREE PONDS IN THIS SUB-BASIN

*S

COMPUTE LT TP LCODE=1 NK=3 ISLOPE=-1
 LENGTH=400 SLOPE=0.02 K=0.7
 LENGTH=900 SLOPE=0.02 K=2.0
 LENGTH=1500 SLOPE=0.02 K=3.0

COMPUTE NM HYD ID=10 HYD=10.1 DA=0.089 SQ MI
 A=0 B=10 C=40 D=50
 TP=0.0 MASSRAIN=-1

PRINT HYD ID=10 CODE=5

*S

*S RECALL POND K DISCHARGE FROM THE TRAILS SUBDIVISION*****

*S

RECALL HYD ID=70 HYD=TRAIL.POND DT=0.0

PRINT HYD ID=70 CODE=5

*S

*S ADD POND K DISCHARGE FROM THE TRAILS SUBDIVISION+++++*****

*S

ADD HYD ID=12 HYD=TR.HS ID I=10 II=70

PRINT HYD ID=12 CODE=5

*

*S

*S SAD 228 WEST OF UNSER BLVD

*S

*S COMPUTE SUB-BASIN M1 - WEST OF UNSER*****

COMPUTE LT TP LCODE=1 NK=3 ISLOPE=-1
 LENGTH=400 SLOPE=0.02 K=0.7
 LENGTH=800 SLOPE=0.02 K=2.0
 LENGTH=1500 SLOPE=0.02 K=3.0

COMPUTE NM HYD ID=13 HYD=10.2 DA=0.138 SQ MI
 A=0 B=10 C=40 D=50
 TP=0.0 MASSRAIN=-1

PRINT HYD ID=13 CODE=5

*S

*S ROUTE SUB-BASIN M1 THROUGH POND-3>>>>>>>>>>>>>>>>>>>>>>>>>>>>

*S

ROUTE RESERVOIR ID=113 HYD=POND3 INFLOW ID=13 CODE=10
 OUTFLOW (CFS) STORAGE (AC-FT) ELEV (FT)

0	0	0
20	1.0	1
40	2.0	2
60	3.0	3
80	4.0	4.0
100	4.5	5.0

PRINT HYD ID=113 CODE=10

*S

*S

*S ADD BASINS M1 AND TRAILS UNIT IV+++++*****

*S

ADD HYD ID=14 HYD=10.3 ID I=12 II=113

PRINT HYD ID=14 CODE=5

*S

*S COMPUTE SUB-BASIN N1 - EAST OF UNSER*****

COMPUTE LT TP LCODE=1 NK=3 ISLOPE=-1
 LENGTH=400 SLOPE=0.02 K=0.7
 LENGTH=600 SLOPE=0.02 K=2.0
 LENGTH=1390 SLOPE=0.02 K=3.0

COMPUTE NM HYD ID=15 HYD=10.4 DA=0.0814 SQ MI

PROP VH.txt

```

A=0  B=10  C=40  D=50
TP=0.0  MASSRAIN=-1

PRINT HYD      ID=15  CODE=5
*S
*S
*S DIVIDE BASIN N1 FOR ALLOWABLE EXISTING FLOWS THROUGH LA CUENTISTA /////////////////
DIVIDE HYD      ID=15  Q=90      ID I=9  HYD=10.5
                  ID II=16     HYD=10.6

PRINT HYD      ID=9   CODE=1
PRINT HYD      ID=16  CODE=1
*S
*S ADD SUB-BASINS M1 AND N1(DIVIDED)+++++++++++++++++++++++++++++
*S
ADD HYD          ID=17 HYD=10.7  ID I=16 II=14
PRINT HYD      ID=17 CODE=5
*S
*S COMPUTE SUB-BASIN N2 - EAST OF UNSER*****
COMPUTE LT TP    LCODE=1  NK=2  ISLOPE=-1
                  LENGTH=400  SLOPE=0.02  K=0.7
                  LENGTH=700  SLOPE=0.02  K=2.0

COMPUTE NM HYD  ID=18  HYD=10.8  DA=0.0246 SQ MI
A=0  B=10  C=40  D=50
TP=0.0  MASSRAIN=-1

PRINT HYD      ID=18  CODE=5
*S
*S ADD SUB-BASINS "N1", "M1" AND N2+++++++++++++++++++++
*S
ADD HYD          ID=19 HYD=10.9  ID I=18 II=17
PRINT HYD      ID=19 CODE=5
*S
*S COMPUTE SUB-BASIN UNSER U0*****
COMPUTE LT TP    LCODE=1  NK=3  ISLOPE=-1
                  LENGTH=400  SLOPE=0.02  K=0.7
                  LENGTH=700  SLOPE=0.02  K=2.0
                  LENGTH=4125 SLOPE=0.02  K=3.0

COMPUTE NM HYD  ID=20  HYD=11.1  DA=0.0319 SQ MI
A=0  B=10  C=40  D=50
TP=0.0  MASSRAIN=-1

PRINT HYD      ID=20  CODE=5
*S
*S
*S DIVIDE SUB BASIN UNSER- SEPARATE BYPASS FLOWS FROM BOCA NEGRA DAM///////////
*S
DIVIDE HYD      ID=20  Q=19      ID I=30 HYD=30.1
                  ID II=31     HYD=30.2

PRINT HYD      ID=30  CODE=1
PRINT HYD      ID=31  CODE=1
*S
*S ADD SUB-BASINS EAST AND WEST OF UNSER+++++
*S
ADD HYD          ID=21 HYD=11.2  ID I=31 II=19
PRINT HYD      ID=21 CODE=5
*S
*S COMPUTE SUB-BASIN M2-A - WEST OF UNSER*****
COMPUTE LT TP    LCODE=1  NK=3  ISLOPE=-1
                  LENGTH=400  SLOPE=0.02  K=0.7
                  LENGTH=600  SLOPE=0.02  K=2.0
                  LENGTH=2600 SLOPE=0.02  K=3.0

COMPUTE NM HYD  ID=22  HYD=11.3  DA=0.1145 SQ MI
A=5  B=35  C=35  D=30
TP=0.0  MASSRAIN=-1

PRINT HYD      ID=22  CODE=5
*S
*S
*S ROUTE SUB-BASIN M2-A THROUGH POND-4A>>>>>>>>>>>>>>>>>>>>>>
*S
ROUTE RESERVOIR      ID=124  HYD=POND4      INFLOW ID=22 CODE=10

```

```

        PROP VH.txt
        OUTFLOW (CFS)    STORAGE (AC-FT) ELEV (FT)
        0                 0                  0
        3                 1.0                1
        16                2.0                2
        18                3.0                3
        22                4.5                4

PRINT HYD      ID=124  CODE=10
*S
*S COMPUTE SUB-BASIN M2-B - WEST OF UNSER*****
COMPUTE LT TP      LCODE=1  NK=3  ISLOPE=-1
                  LENGTH=400  SLOPE=0.02  K=0.7
                  LENGTH=600  SLOPE=0.02  K=2.0
                  LENGTH=500  SLOPE=0.02  K=3.0

COMPUTE NM HYD      ID=24  HYD=11.4  DA=0.0201  SQ MI
A=0  B=10  C=40  D=50
TP=0.0  MASSRAIN=-1

PRINT HYD      ID=24  CODE=5
*S
*S
*S ADD ALL UNSER BLVD SUB-BASINS TO BOCA NEGRA DAM
*S
ADD HYD      ID=25  HYD=11.5  ID I=21 II=24
PRINT HYD      ID=25  CODE=5
*S

*S SAD 228 EAST OF UNSER BLVD
*S
*S COMPUTE SUB-BASIN M3 - EAST OF UNSER*****
COMPUTE LT TP      LCODE=1  NK=3  ISLOPE=-1
                  LENGTH=400  SLOPE=0.02  K=0.7
                  LENGTH=800  SLOPE=0.02  K=2.0
                  LENGTH=1500  SLOPE=0.02  K=3.0

COMPUTE NM HYD      ID=26  HYD=11.7  DA=0.1793  SQ MI
A=0  B=10  C=40  D=50
TP=0.0  MASSRAIN=-1

PRINT HYD      ID=26  CODE=5
*S
*S
*S ROUTE SUB-BASIN M3 THROUGH POND-5>>>>>>>>>>>>>>>>>>>>>>>>>>>>>>>>>>>
*S
ROUTE RESERVOIR      ID=125  HYD=POND5  INFLOW ID=26  CODE=10
                  OUTFLOW (CFS)  STORAGE (AC-FT) ELEV (FT)
                  0                 0                  0
                  20                1.0                1.5
                  30                1.8                2.5
                  40                6.5                6.5
                  50                8.5                7.5

*S
PRINT HYD      ID=125  CODE=10
*S
*S
*S ADD ROUTED FLOWS FROM PONDS 4 AND 5+++++oooooooooooooo
*S
ADD HYD      ID=27  HYD=11.8  ID I=124 II=125
PRINT HYD      ID=27  CODE=5
*S
*S

*S ADD ROUTED FLOWS FROM POND-4, POND-5 AND 19 CFS UNSER BLVD BYPASS+++++
*S
ADD HYD      ID=29  HYD=11.9  ID I=27 II=30
PRINT HYD      ID=29  CODE=5
*S
*S

*S COMPUTE SUB-BASIN M4 - SOUTH OF LA CUENTISTA SUBD.*****
COMPUTE LT TP      LCODE=1  NK=2  ISLOPE=-1
                  LENGTH=400  SLOPE=0.02  K=0.7
                  LENGTH=600  SLOPE=0.02  K=2.0

COMPUTE NM HYD      ID=241  HYD=12.2  DA=0.0172  SQ MI
A=0  B=10  C=40  D=50

```

PROP VH.txt

TP=0.0 MASSRAIN=-1

PRINT HYD ID=241 CODE=5

*S
 *S
 *S ROUTE SUB-BASIN M4 THROUGH POND-7 >>>>>>>>>>>>>>>>>>>>>>>>>>>>>>>>>>>>

*S
 ROUTE RESERVOIR ID=127 HYD=POND7 INFLOW ID=241 CODE=10
 OUTFLOW (CFS) STORAGE (AC-FT) ELEV (FT)
 0 0 0
 5 0.6 1
 7 1.0 2
 10 1.5 2.2

*S
 PRINT HYD ID=127 CODE=10

*S
 *S
 *S COMPUTE SUB-BASIN M5 - SOUTH OF LA CUENTISTA SUBD.*****
 COMPUTE LT TP LCODE=1 NK=2 ISLOPE=-1
 LENGTH=400 FT SLOPE=0.0125 K=0.7
 LENGTH=1230 FT SLOPE=0.0410 K=2.0

COMPUTE NM HYD ID=251 HYD=12.3 DA=0.0707 SQ MI
 A=0 B=10 C=40 D=50
 TP=0.0 MASSRAIN=-1

PRINT HYD ID=251 CODE=5

*S
 *S
 *S ROUTE SUB-BASIN M5 THROUGH POND-8 >>>>>>>>>>>>>>>>>>>>>>>>>>>>>>>>>>>>>>

*S
 ROUTE RESERVOIR ID=128 HYD=POND8 INFLOW ID=251 CODE=10
 OUTFLOW (CFS) STORAGE (AC-FT) ELEV (FT)
 0 0 0
 20 1 1
 30 3 4

*S
 PRINT HYD ID=128 CODE=10

*S
 *S
 *S COMPUTE SUB-BASIN M6 - SOUTH OF BASIN M5*****
 *S

COMPUTE LT TP LCODE=1 NK=2 ISLOPE=-1
 LENGTH=400 FT SLOPE=0.0287 K=0.7
 LENGTH=370 FT SLOPE=0.0138 K=2.0

COMPUTE NM HYD ID=252 HYD NO=12.4 DA=0.0065 SQ MI
 PER A=0 PER B=10 PER C=40 PER D=50
 TP=0.0 MASS RAIN=-1

PRINT HYD ID=252 CODE=1

*S *****

*S DIVIDE FLOWS INTO POND9 FROM FREE DISCHARGE
 DIVIDE HYD ID=252 Q=11 ID=51 HYD NO=51.1
 ID=52 HYD NO=52.1

PRINT HYD ID=51 CODE=1
 PRINT HYD ID=52 CODE=1

*S *****

*S
 *S ROUTE SUB-BASIN M6 THROUGH POND-9 >>>>>>>>>>>>>>>>>>>>>>>>

*S
 ROUTE RESERVOIR ID=130 HYD=POND9 INFLOW ID=51 CODE=10
 OUTFLOW (CFS) STORAGE (AC-FT) ELEV (FT)
 0 0 5316.4
 1 0.113 5317
 8 0.200 5318

*S
 PRINT HYD ID=130 CODE=10

PROP VH.txt

*S

*S COMPUTE SUB-BASIN M3-1 - EAST OF UNSER*****

COMPUTE LT TP LCODE=1 NK=3 ISLOPE=-1
 LENGTH=400 SLOPE=0.02 K=0.7
 LENGTH=800 SLOPE=0.02 K=2.0
 LENGTH=300 SLOPE=0.02 K=3.0

COMPUTE NM HYD ID=254 HYD=13.3 DA=0.05339 SQ MI
 A=0 B=10 C=40 D=50
 TP=0.0 MASSRAIN=-1

PRINT HYD ID=254 CODE=5

*S

*S

*S ROUTE SUB-BASIN M3-1 THROUGH POND-6 >>>>>>>>>>>>>>>>>>>>>>>>>>>>>>>>

*S

ROUTE RESERVOIR ID=129 HYD=POND6 INFLOW ID=254 CODE=10
 OUTFLOW (CFS) STORAGE (AC-FT) ELEV (FT)
 0 0 0
 2 1.3 1
 40 2.1 1.5

*S

PRINT HYD ID=129 CODE=10

*S

*S

FINISH

APPENDIX B

APPENDIX C

CITY OF ALBUQUERQUE



Parks and Recreation Department OPEN SPACE DIVISION

Richard J. Berry, Mayor

Barbara Baca, Director

October 21, 2011

Mr. Steven Metro
Wilson & Company
4900 Lang Ave. NE
Albuquerque, NM 87109

**Regarding: Open Space Division Letter of Concurrence with SAD 228
Drainage and Trails Planning Adjacent to Boca Negra Canyon,
Petroglyph National Monument**

Dear Mr. Metro;

The City of Albuquerque Open Space Division (OSD), in coordination with staff from the National Park Service, Petroglyph National Monument Unit (PETR), has reviewed the Drainage Report "Volcano Heights Drainage Compilation Report" dated October, 2011, as prepared by Wilson & Company.

PO Box 1293

OSD and PETR approve and concur with the proposed controlled outlet flows from the Drainage Ponds as summarized below:

Albuquerque

Pond 6 Allowable Outlet Flow = 40 c.f.s.
Pond 7 Allowable Outlet Flow = 10 c.f.s.
Pond 8 Allowable Outlet Flow = 30 c.f.s.
Pond 9 Allowable Outlet Flow = 10 c.f.s.

NM 87103

The attached map shows the proposed ponds and the compilation of drainage outlets for the areas above the PETR escarpment. In general, the allowable drainage flows over the escarpment are less than half of the historic flows from the subject-area with no development, as we negotiated and agreed upon.

It is my understanding that, per OSD and PETR staff comments (during a meeting on October 17, 2011, at the Wilson & Company office), the Pond 6 emergency overflow location will be moved to the east and additional native/local basalt rip-rap will be added to the overflow in order to ensure the flows at the escarpment are spread out and dissipated so as to prevent channelization and erosion in the canyon below.

In addition, we concur with the proposed escarpment trail alignment, both horizontal and vertical. The proposed trail alignment follows the bladed roadways for Compass and Vista del Prado streets which will reduce grading impacts to the Petroglyph National Monument and Boca Negra canyon.

Further, it is my understanding that proposed the fencing surrounding Pond #6 and the adjacent lift-station will be eliminated and replaced with native vegetation per OSD and PETR recommendation.

Thank you for your cooperation and responsiveness to the interests of the public in this matter.

Should you have any questions or need additional information, please feel free to contact me.

Sincerely,



Matthew F. Schmader, Ph.D.
Superintendent, Open Space Division

Attachment - Volcano Heights Drainage Compilation Report Exhibit 1

Copy: Barbara C. Baca, Director, Parks and Recreation Department
Joseph P. Sanchez, Superintendent, NPS PETR



4900 Lang Ave NE

Albuquerque, NM 87109

505-348-4000 phone

Arizona
California
Colorado
Florida
Kansas
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Nebraska
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Oklahoma
Texas
Utah

To: Shahab Bazar, P.E.
Development and Building Services
City of Albuquerque

From: Daniel S. Aguirre, P.E.

Date: October 24, 2011

File Number: X3-210-022

Re: Volcano Heights Drainage Compilation Report

Dear Mr. Bazar,

The intent of this report is to share with the City and AMAFCA the planning concepts used over the past 8-10 years in this area and to bring to your attention the system limitations based on the existing and planned infrastructure. This report is not intended to be used for design but to identify the allowable discharge at specific points in the area based on the concepts followed with past projects. I have updated the report per your comments to fix any identified errors and tried to explain the intent below. Please remember that any projects in the area require a detailed analysis and do not need to follow the exact concepts used for the purposes of this report.

Please see below responses to your comments of the review of the drainage report dated August 24, 2011:

1. **Any discharge to the Petroglyph National Monument has to be approved by Open Space.** This report is based on several approved reports that have been completed over the pasted several years. These flows have been approved via the Paseo Del Norte Extension drainage report, La Cuentista Subdivision, Vista Vieja Subdivision and Boca Negra Dam plus others. An additional meeting was held with Open Space on October 17, 2011 to discuss all discharges into open space. The Map has been slightly modified based on this meeting in the SAD 228 area and a copy of a letter of approval will be sent to your office when received from Open Space.
2. **Please confirm the allowable discharge from Pond 11 to Chamisa storm drain system.** AMAFCA required that the Chamisa Storm Drain take the 45 cfs from adjacent lots. The flow is shown in the approved drainage report for the Paseo Del Norte Extension and should be included in the Chamisa storm drain report.

3. **How was the capacity determined to allow the Unser Detention Basin to drain to Chamisa Detention Basin?** This is a proposed system (concept only) to prevent low flows from discharging to the Petroglyph National Monument (PETR) during low flow (erosive) events. We created a pond to store water with a minimal discharge to the Chamisa detention basin ultimately discharging to the Lyon Blvd storm drain for low flows (up to 10 year event) once the pond fills to the 10 year volume it will start to spill to PETR through a weir up to the 120 cfs originally identified as historic flow. This was intended to reduce erosion at the PETR discharge point and can be redesigned and modified with a more detailed analysis or other concepts when final design is completed.
4. **Can some of the flows proposed for Unser Detention Basin drain south to the Paseo Del Norte Storm Drain System?** The existing Paseo Del Norte Storm Drain System was designed and constructed based on the roadway original alignment. The existing storm drain system has a limited capacity and would require additional ponds to accept flows from other subbasins. If you incorporate the right amount of detention you can modify in final design plans.
5. **May need to run the runoff calculations based on the 100-year, 24-hour storm based on the duration of the discharge for the detention ponds. Please stay consistent between all the drainage reports for this area. Most of the runoff calculations for the SAD 228 were based on the 100-year, 24-hour storm.** This report serves as a guide to develop allowable flows at specific discharge points. Hydrology calculations need to be modeled in future studies as each area is developed in the same manner that has been done with SAD 228. The final models will be determined based on the requirements for the design.
6. **Flow numbers for Basins K2, M5, M6, and T1 are not the same on the Plate-1, Table-1/Plate-1 and AHYMO calculations. T1 basin area is different from the AHYMO calculations and what is shown on the Plate-1.** All numbers corrected.