

Project Title:

City of Albuquerque

Planning Department

Development & Building Services Division

DRAINAGE AND TRANSPORTATION INFORMATION SHEET

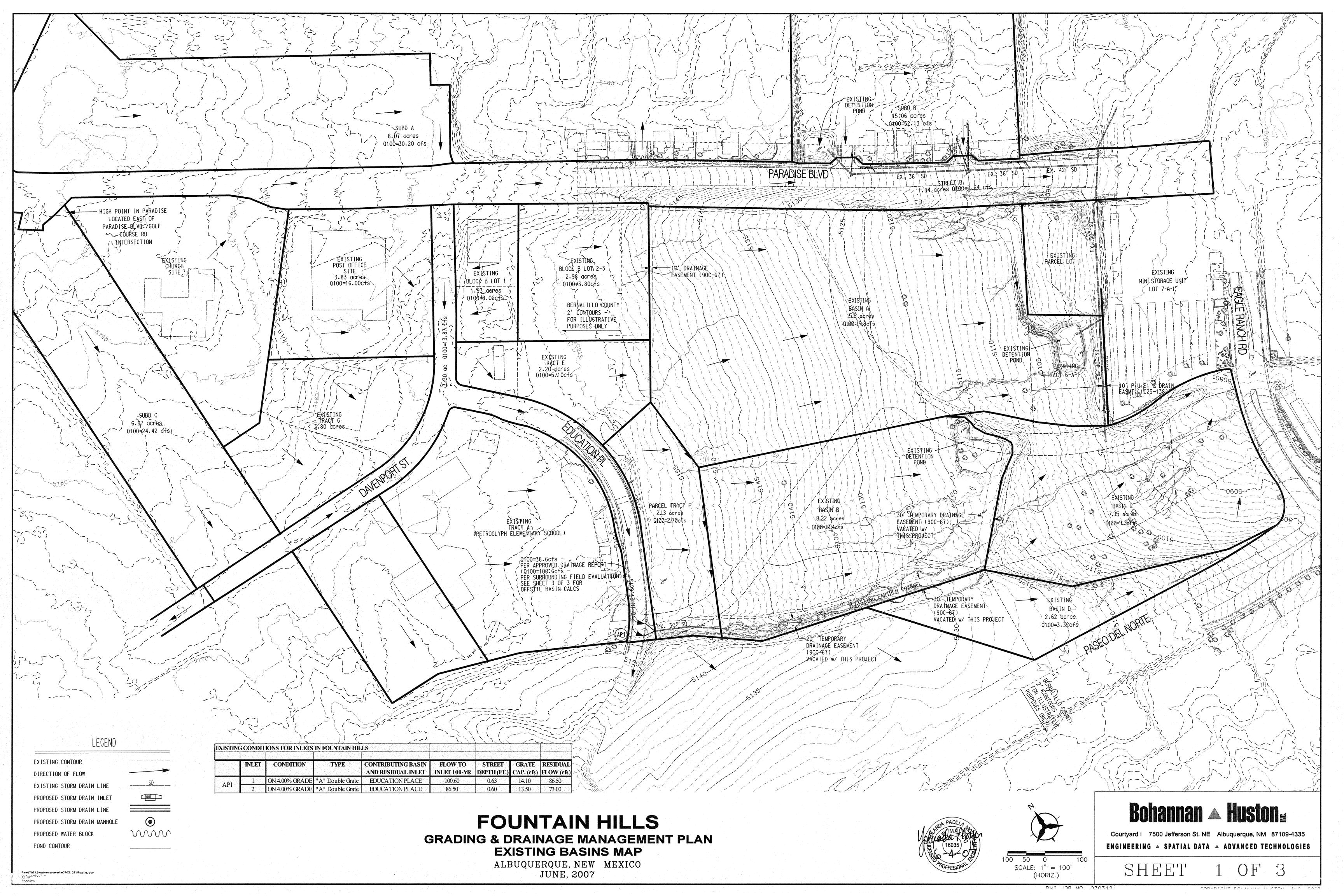
(REV 02/2013)

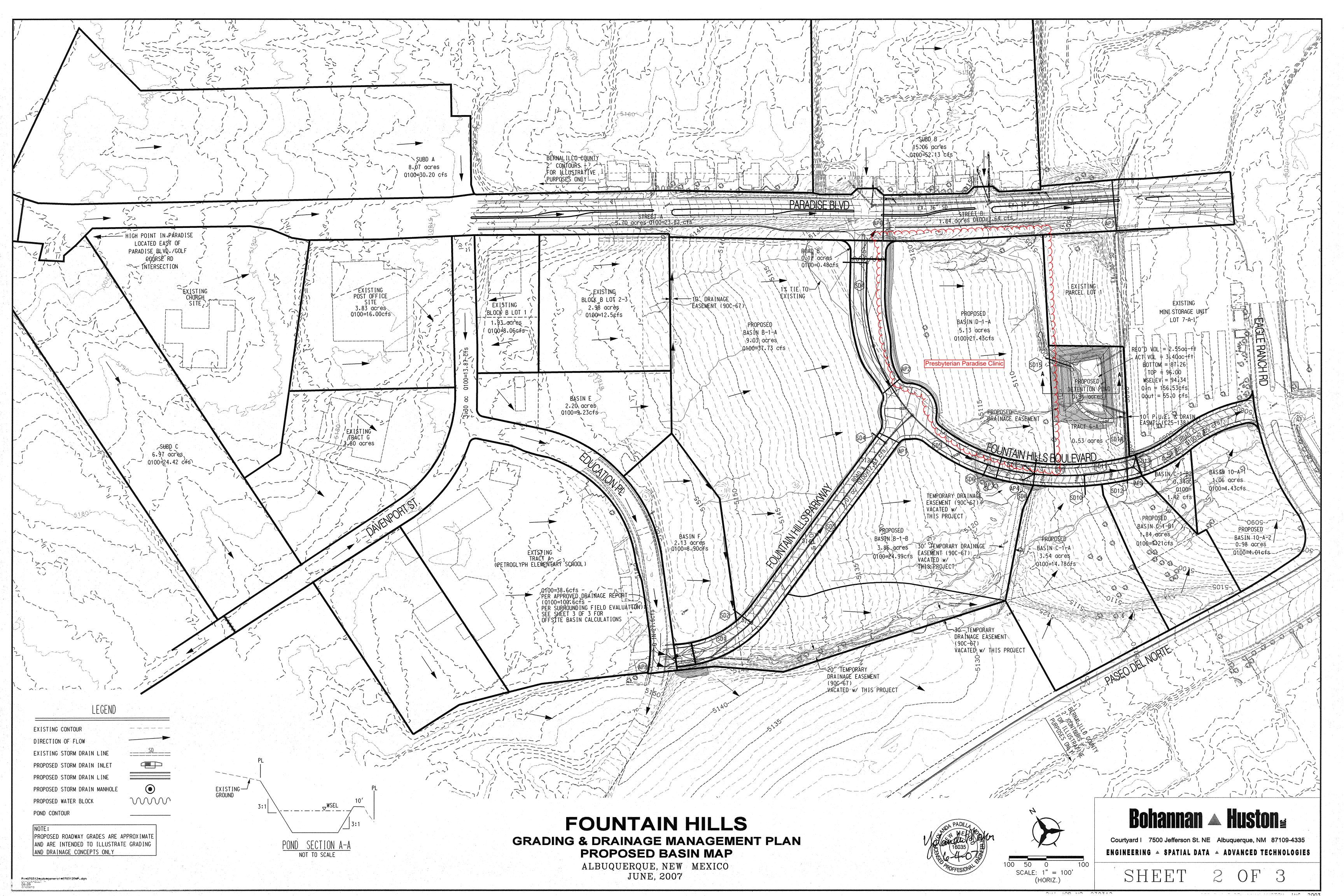
Building Permit #: City Drainage #: _

DRB#: EPC	#: Work Orde	er#:
Legal Description:		
City Address:		
Engineering Firm:	Contact:	
Address:		
Phone#: Fax#	: E-mail:	
Owner:	Contact:	
Address:		
Phone#: Fax#	: E-mail:	
Architect:	Contact:	
Address:		
Phone#: Fax#	E-mail:	
Surveyor:	Contact:	
Address:		
Phone#: Fax#	E-mail:	
Contractor:	Contact:	
Address:		
Phone#: Fax#	E-mail:	
TYPE OF SUBMITTAL:	CHECK TYPE OF APPROVAL/ACCEP	TANCE SOUGHT:
DRAINAGE REPORT	SIA/FINANCIAL GUARANTEE RELEA	SE
DRAINAGE PLAN 1st SUBMITTAL	PRELIMINARY PLAT APPROVAL	
DRAINAGE PLAN RESUBMITTAL	S. DEV. PLAN FOR SUB'D APPROVAL	_
CONCEPTUAL G & D PLAN	S. DEV. FOR BLDG. PERMIT APPROV	AL
GRADING PLAN	SECTOR PLAN APPROVAL	
EROSION & SEDIMENT CONTROL PLAN (E	SC)FINAL PLAT APPROVAL	
ENGINEER'S CERT (HYDROLOGY)	CERTIFICATE OF OCCUPANCY (PERI	M)
CLOMR/LOMR	CERTIFICATE OF OCCUPANCY (TCL	TEMP)
TRAFFIC CIRCULATION LAYOUT (TCL)	FOUNDATION PERMIT APPROVAL	
ENGINEER'S CERT (TCL)	BUILDING PERMIT APPROVAL	
ENGINEER'S CERT (DRB SITE PLAN)	GRADING PERMIT APPROVAL	SO-19 APPROVAL
ENGINEER'S CERT (ESC)	PAVING PERMIT APPROVAL	ESC PERMIT APPROVAL
SO-19	WORK ORDER APPROVAL	ESC CERT. ACCEPTANCE
OTHER (SPECIFY)	GRADING CERTIFICATION	OTHER (SPECIFY)
WAS A PRE-DESIGN CONFERENCE ATTENDED:	Yes No Copy Provided	
DATE SUBMITTED:	By:	

Requests for approvals of Site Development Plans and/or Subdivision Plats shall be accompanied by a drainage submittal. The particular nature, location, and scope to the proposed development defines the degree of drainage detail. One or more of the following levels of submittal may be required based on the followin

- 1. Conceptual Grading and Drainage Plan: Required for approval of Site Development Plans greater than five (5) acres and Sector Plans
- Drainage Plans: Required for building permits, grading permits, paving permits and site plans less than five (5) acres Drainage Report: Required for subdivision containing more than ten (10) lots or constituting five (5) acres or more
- Erosion and Sediment Control Plan: Required for any new development and redevelopment site with 1-acre or more of land disturbing area, including project less than 1-acre than are part of a larger common plan of development





OTTEVER					OR FOUNTAIN					
Fountain l	lills Parky	wav								
	Roadway Grade (%)	Roadway Cross-slope (%)	Q (100 YR) in roadway (cfs)	Curb Type	Depth of water in roadway (ft.)	Velocity of storm water in roadway (ft/s)	V^2/2*g	EGL	ROW Elevation (ft.)	Comments
AP1	3.74	2.00	6.38	STD	0.27	3.59	0.20	0.47	0.85	OK
Fountain l	Hills Blvd.									
Location		Roadway Cross-slope (%)	Q(100 YR) in roadway (cfs)	Curb Type	Depth of water in roadway (ft.)	Velocity of storm water in roadway (ft/s)	V^2/2*g	EGL	ROW Elevation (ft.)	Comments
AP2	0.60	2.00	6.74	STD	0.35	1.78	0.05	0.4	0.85	OK
AP3	2.63	2.00	19.99	STD	0.40	4.15	0.27	0.65	0.85	OK
AP4	5.35	2.00	25.53	STD	0.38	5.67	0.50	0.85	0.85	OK
AP5	5.35	2.00	15.39	STD	0.33	5.06	0.40	0.73	0.85	OK
Paradise 1	Blvd.									
Location	Grade (%)	Roadway Cross-slope (%)	Q (100 YR) in roadway (cfs)	Curb Type	Depth of water in roadway (ft.)	Velocity of storm water in roadway (ft/s)	V^2/2*g	EGL	ROW Elevation (ft.)	Comments
AP6	*north ha	lf of road - Q10	Oyr=7.8cfs pro	vides a	depth=0.31' < 0	.45 for one free lane	of traffic			
AP6**	5.00		11.99	STD	0.35	4.93	0.38	0.73	0.85	OK
**south h						one free lane of traf				
AP7	*north ha	lf of road - Q10	0yr=19.0cfs pi	rovides	a depth=0.38' <	0.45 for one free lan	e of traffic			
AP7**	6.66	2.00	15.79	STD	0.36	5.80	0.52	0.88	0.85	OK
**south h	alf of road	- Q10yr=10.23	cfs provides	a depth=	=0.32' < 0.45 for	one free lane of traf	fic			
Note:										

By observation, the product of the depth times the velocity is below 6.5 for the 100-year design storm, therefore for the 10-year design storm the product of the depth times the velocity will be well below the recommended value of 6.5

JET FALOI		DITIONS FOR INLE						
	INLET	CONDITION	TYPE	CONTRIBUTING BASIN AND RESIDUAL INLET	FLOW TO INLET 100-YR	STREET DEPTH (FT.)	GRATE CAP. (cfs)	RESIDUAL FLOW (cfs)
		ON 5.35% GRADE	"A" Double Grate	FOUNTAIN HILLS BLVD	12.76	0.38	7.90	4.86
AP4	2	ON 5.35% GRADE	"A" Double Grate	FOUNTAIN HILLS BLVD	12.76	0.38	7.90	4.86
	3	ON 5.35% GRADE	"A" Double Grate	FOUNTAIN HILLS BLVD	7.70	0.33	5.90	1.80
AP5	4	ON 5.35% GRADE	"A" Double Grate	FOUNTAIN HILLS BLVD	7.70	0.33	5.90	1.80

PIPE	DRAIN ANALYSIS I		LENGTH (ft.)	SLOPE(%)	PROPOSED Q	PIPE CAPACITY
NO.					(100-YR, cfs)	(100-YR, CFS)
SD1	EXISTING	30" RCP	228.00	1.10	27.60	43.02
SD2	TRACT F	18" RCP	58.00	1.00	7.12	10.50
SD3	TRUNKLINE 1	30" RCP	688.00	3.74	34.72	79.32
SD4	TRACT B-1-A	30" RCP	478.00	1.00	29.91	41.02
SD5	TRUNKLINE 2	30" RCP	386.00	2.63	64.63	66.51
SD6	TRACT B-1-B	30" RCP	57.00	1.00	19.99	41.02
SD7	TRUNKLINE 3	30" RCP	128.00	5.35	84.62	94.87
SD8	INLETS	18" RCP	32.00	1.00	7.90	10.50
SD9	TRUNKLINE 4	36" RCP	134.00	5.35	100.42	154.27
SD10	TRACT C-1-A	18" RCP	59.00	1.00	11.82	10.50
SD11	TRUNKLINE 5	36" RCP	101.00	5.35	112.25	154.27
SD12	TRACT C-1-B1	18" RCP	55.00	1.00	5.77	10.50
SD13	INLETS	18" RCP	84.00	1.00	5.90	10.50
SD14	TRUNKLINE 6	42" RCP	294.00	2.20	129.82	149.23
SD15	TRACT D-1-A	24" RCP	27.74	1.50	21.73	27.72

BASIN	AREA	UNITS		% LAND TI	REATMENT	_		DISCHAR	GE (CFS
	(AC)	#	Α	B	C	D		10 YR*	100YR
НУГ	RDOLOG	SICAL VOLU	METRIC & D	SCHARGE	DATA (EX	ISTING CA	ALCUL	ATED)	
BASIN A	15.60		100.0%	0.0%	0.0%	0.0%		3.9	19.79
BASIN B	8.22		100.0%	0.0%	0.0%	0.0%		2.1	10.43
BASIN C	7.35		100.0%	0.0%	0.0%	0.0%		1.8	9.32
BASIN D	2.62		100.0%	0.0%	0.0%	0.0%		0.7	3.32
BLOCK B LOT 2-3	2.98		100.0%	0.0%	0.0%	0.0%		0.7	3.78
TRACT E	2.20		60.0%	5.0%	5.0%	30.0%		2.5	5.10
TRACT F	2.13		100.0%	0.0%	0.0%	0.0%		0.5	2.70
TOTAL	41.10							12.3	54.46
	LIVO	TOLOGICAL	VOLUMETRI	C & DISCH	APCE DAT	CA (DEVEL	OPED		
	HYKL	JULUGICAL		ONSITE	ARGE DA	IM (DEVEL	.UFED		
B-1-A	9.03		0.0%	5.0%	5.0%	90.0%		24.5	37.73
B-1-B	5.98		0.0%	5.0%	5.0%	90.0%		16.2	24.99
C-1-A	3.54		0.0%	5.0%	5.0%	90.0%		9.6	14.78
C-1-B1	1.73		0.0%	5.0%	5.0%	90.0%		4.7	7.21
C-1-B2	0.34		100%	5.0%	√5.8%	90/0%		√0.8 ✓	1.42
D-1-A	5.13		0.0%	5.0%	5.0%	90.0%		13.9	21.43
	4.44	ىرىرىر	10:0%×	15.0%X	~5.0%~	90.0%	444	<u>vaav</u>	~6.02
	2.20		0.0%	5.0%	5.0%	90.0%		6.0	9.19
	2.13		0.0%	5.0%	5.0%	90.0%		5.8	8.90
10-A-1	1.06		0.0%	5.0%	5.0%	90.0%		2.9	4.43
10-A-2	0.98		0.0%	5.0%	5.0%	90.0%		2.6	4.08
ROAD A	3.07		0.0%	5.0%	5.0%	90.0%		8.3	12.83
ROAD B	0.12		0.0%	5.0%	5.0%	90.0%		0.3	0.48
ROAD C	0.49		0.0%	5.0%	5.0%	90.0%		1.3	2.05
SUBTOTAL	37.23	0.00						100.94	155.8
				OFFSITE					
TRACT A	6.91	0	0.0%	25.0%	25.0%	50.0%		13.8	23.5
BLOCK B LOT 1	1.93	0	0.0%	5.0%	5.0%	90.0%		5.2	8.06
TRACT G	3.80	11	0.0%	12.5%	12.5%	75.0%		9.3	14.78
POST OFFICE	3.83	0	0.0%	5.0%	5.0%	90.0%		10.4	16.00
SUBD C	6.97	40	0.0%	22.5%	22.5%	55.0%		14.6	24.42
EDUCATION PL	3.30	0	0.0%	5.0%	5.0%	90.0%		9.0	13.8
SUBTOTAL	26.74	51						62.3	100.6
				ADICE DIV					
OTDEET A	5.70	0	0.0%	ADISE BLV 5.0%	5.0%	90.0%		15.5	23.82
STREET A		0	0.0%	5.0%	5.0%	90.0%		5.0	7.69
STREET B	1.84	48	0.0%	23.4%	23.4%	53.3%		17.9	30.20
SUBD A	8.70 45.06	82	0.0%	23.4%	23.6%	52.8%		30.9	52.13
SUBD B	15.06	02	0.076	20.070	20.070	JZ.070		69.3	113.8

AHYMO PROGRAM INPUT FILE = FHILL	SUMMARY TABLE (AI .HYM	HYMO_97) -			- VERSIO	ON: 1997.0		ATE (MON/ = AHYMO-S		
PROJECT NAME: DATE: INPUT FILE NAME: OUTUPUT FILE NAM PROJECT NUMBER: COMMENTS:		R STORM								
COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	PAGE = 1 NOTATION
START RAINFALL TYPE:	=1									TIME= .00 RAIN6= 2.200
COMPUTE NM HY	/D BASIN.A		10	.05950	156.53	5.878	1.85238	1.500	4.111	PER IMP= 90.00
ROUTE RESERVO	OIR POND	10	20	.05950	51.77	5.878	1.85238	1.850	1.360	AC-FT= 2.552

DRAINAGE MANAGEMENT PLAN

I. PURPOSE

The purpose of this submittal is to present the grading and drainage management plan for Fountain Hills Development. The project consists of approximately 38 acres of land to be developed for commercial use.

II. SITE LOCATION AND CHARACTERISTICS

The project is south of Paradise Blvd and north of Paseo Del Norte and bordered to the west by Education Place and to the east by Eagle Ranch Road. The site is currently undeveloped tracts, with native shrubs and grasses.

III. EXISTING HYDRAULIC AND HYDROLOGIC CONDITIONS

In the existing conditions, the site drains across relatively steep terrain towards the east. Currently offsite flow from the west flows through Education PI and is captured by an existing storm drain system (Q100=27.6cfs) and conveyed to the drainage easement bordering the southern edge of the property, which directs the flow to a detention pond through an earthen channel with a series of check dams. Existing Tracts E and F drain to the east, into ponds on the west side of Tract B-1-A, contrary to the Drainage Report for Albuquerque West Unit 1 Improvements, Oct. 1990; however these tracts are not part of this submittal. The existing northern basin (Basin A - 15.6 ac, Q100=19.8cfs), along with offsite basin flow (Block B Lot 2-3 – 2.98 acres, Q100=3.80cfs), currently drains across the site to a detention pond in existing Tract 6-A-1. The southwestern basin (Basin B - 8.22 ac, Q100=10.4cfs) presently drains to a detention pond at the northeastern corner of the basin. The remaining portion of the site (Basin C - 7.35ac, Q100=9.3cfs and Basin D – 2.62 ac, Q100=3.32cfs), located in the southeast corner of the site, drains to the east and directly into Eagle Ranch Road.

IV. PROPOSED HYDRAULIC AND HYDROLOGIC CONDITIONS

In the proposed improvements, the west portion of the site (Basins B-1-A, B-1-B, C-1-A, C-1-B1, D-1-A, E and F) will drain through the proposed roadways or storm drains and ultimately to a permanent detention pond located in existing Tract 6-A-1. The existing earthen channel with check dams and the detention pond in Tract B-1-B will be removed and replaced with a storm drain in Fountain Hills Parkway and Fountain Hills Blvd. The developed tracts west of the proposed detention pond (Basins B-1-A, B-1-B, C-1-A, C-1-B1, D-1-A, E and F) will be assumed to discharge 20% of developed flows directly into the adjacent roadways, where it will be collected by proposed inlets in Fountain Hills Boulevard. The remaining 80% will be captured by proposed storm drain stubs. Runoff from Fountain Hills Boulevard and Fountain Hills Parkway will flow through the streets to inlets, which will tie to the proposed storm drain; see Summary of Roadway Capacities for Fountain Hills and Developed Conditions for Inlets in Fountain Hills. The proposed storm drain will outlet to the proposed detention pond in existing Tract 6-A-1; see Storm Drain Analysis for Fountain Hills. The proposed detention pond will be sized to accept a developed flow of 156.53cfs, based on AHYMO methodology, with a controlled outlet of 55cfs, in accordance with the approved grading and drainage plan, COA Hydrology File C12-D3B mentioned above. The minimum required volume is 2.50 ac-ft with the actual volume being 3.14 ac-ft; see AHYMO Program Summary Table. The east side of the site (Basins 10-A-1 and 10-A-2 and C-1-B2), along with the remaining portion of Fountain Hills Boulevard (0.49ac, Q100=2.05cfs) and the residual from the inlets (Q=3.59cfs), will flow through the street and directly into Eagle Ranch Rd (Q100=15.57cfs).

The south half of Paradise Blvd., fronting the Fountain Hills site, has been analyzed for the 100-yr and 10-yr storm capacities, assuming proposed street width of 30' F-F minimum (south side of road will be widened to include new curb and gutter, as well as new median curb and gutter, as part of this development); see Summary of Roadway Capacities for Fountain Hills. The north half of the street has been analyzed for the 10-yr storm capacities, one lane free. The proposed roadway is adequate to handle these conditions.

V. CONCLUSION

This plan provides hydrologic and hydraulic considerations of Fountain Hills Development. These flows can be safely conveyed by the improvements proposed in this plan to the existing storm systems, which have adequate capacity to accept such runoff. With the exception of existing Tracts E and F which drain to the east, to ponds on the west side of Tract B-1-A, contrary to the approved Drainage Report for Albuquerque West Unit 1 Improvements, Oct. 1990; this submittal is in compliance with existing approved grading and drainage plans. Tracts E and F are not part of this submittal. This information provides adequate supporting documentation and guidance for approval of this plan.

FOUNTAIN HILLS

GRADING & DRAINAGE MANAGEMENT PLAN

ALBUQUERQUE, NEW MEXICO JUNE, 2007



Bohannan A Huston

Courtyard | 7500 Jefferson St. NE Albuquerque, NM 87109-4335

ENGINEERING A SPATIAL DATA A ADVANCED TECHNOLOGIES

2. THE COST FOR REQUIRED CONSTRUCTION DUST AND EROSION CONTROL MEASURES SHALL BE INCIDENTAL TO THE PROJECT COST.

3. ALL WORK RELATIVE TO FOUNDATION CONSTRUCTION, SITE PREPARATION, AND PAVEMENT INSTALLATION, AS SHOWN ON THIS PLAN, SHALL BE CONSTRUCTED IN ACCORDANCE WITH THE "GEOTECHNICAL INVESTIGATION". ALL OTHER WORK SHALL, UNLESS OTHERWISE STATED OR PROVIDED FOR HEREON, BE CONSTRUCTED IN ACCORDANCE WITH THE PROJECT, (FIRST PRIORITY) SPECIFICATIONS, AND/OR THE CITY OF

4. EARTH SLOPES SHALL NOT EXCEED 3 HORIZONTAL TO 1 VERTICAL UNLESS SHOWN OTHERWISE.

ALBUQUERQUE (COA) STANDARD SPECIFICATIONS FOR PUBLIC WORKS (SECOND PRIORITY).

5. IT IS THE INTENT OF THESE PLANS THAT THIS CONTRACTOR SHALL NOT PERFORM ANY WORK OUTSIDE OF THE PROPERTY BOUNDARIES EXCEPT AS REQUIRED BY THIS PLAN.

6. THE CONTRACTOR IS TO ENSURE THAT NO SOIL ERODES FROM THE SITE ONTO ADJACENT PROPERTY OR PUBLIC RIGHT-OF-WAY. 7. A DISPOSAL SITE FOR ANY & ALL EXCESS EXCAVATION MATERIAL, AND UNSUITABLE MATERIAL AND/OR

A BORROW SITE CONTAINING ACCEPTABLE FILL MATERIAL SHALL BE OBTAINED BY THE CONTRACTOR IN COMPLIANCE WITH APPLICABLE ENVIRONMENTAL REGULATIONS AND APPROVED BY THE OBSERVER. ALL COSTS INCURRED IN OBTAINING A DISPOSAL OR BORROW SITE AND HAUL TO OR FROM SHALL BE CONSIDERED INCIDENTAL TO THE PROJECT AND NO SEPARATE MEASUREMENT OR PAYMENT SHALL BE MADE.

8. PAVING AND ROADWAY GRADES SHALL BE +/- 0.1' FROM PLAN ELEVATIONS. PAD ELEVATION SHALL BE +/- 0.05' FROM BUILDING PLAN ELEVATION.

9. VERIFY ALL ELEVATIONS SHOWN ON PLAN FROM BASIS OF ELEVATION CONTROL STATION PRIOR TO BEGINNING CONSTRUCTION.

INV=6.51

-FL10.82

KEYED NOTES

- 1. SEGMENTAL RETAINING WALL, SEE DETAIL 1/C-100.
- 2. INSTALL STORM DRAIN PIPE. SEE PLAN FOR SIZE & SLOPE.
- 3. INSTALL 18" NYLOPLAST DRAIN BASIN WITH DOME GRATE. TOP OF GRATE
- 4. CONSTRUCT 4' DIA TYPE "E" STORM DRAIN MANHOLE PER COA STD DWG
- 5. CONSTRUCT RIP-RAP BOWL DISSIPATER, SEE 1/C-101.

ELEVATION SHOWN ON PLAN IS BASE OF DOME.

- 6. CONSTRUCT CONCRETE RUNDOWN, SEE DETAIL 2/C-100.
- 7. INSTALL STORM DRAIN TO WITHIN 5' OF BUILDING. SEE PLUMBING PLANS FOR CONTINUATION.
- 8. INSTALL 12" CURB OPENING FOR DRAINAGE.
- 9. NOT USED.
- 10. INSTALL SHORING WALL, SEE STRUCTURAL PLANS FOR DETAILS.
- 11. CONSTRUCT 2' WIDE CONCRETE RIBBON CHANNEL, SEE DETAIL 2/C-100.
- 12. CONSTRUCT 2' HIGH LANDSCAPE BERM, SEE LANDSCAPE PLAN FOR DETAILS. MAINTAIN POSITIVE DRAINAGE AROUND BERM ADJACENT TO BUILDING.
- 13. INSTALL NYLOPLAST INLINE DRAIN WITH 12" ROUND PEDESTRIAN GRATE. SEE ARCHITECTURAL DETAILS FOR 'RAIN CHAIN' CONNECTION DETAIL.
- 14. CONSTRUCT 1' HIGH LANDSCAPE BERM, SEE LANDSCAPE PLAN FOR DETAILS.
- 15. REMOVE AND SALVAGE EXISTING WIRE FENCING AND TEE POST. CONTACT KURT WAGENER AT (505) 884-2115 TO COORDINATE DELIVERY OF SALVAGED MATERIALS.
- 16. INSTALL 4" PIPE SLEEVE THRU WALL FOR DRAINAGE.
- 17. CONSTRUCT TYPE "C" SINGLE GRATE STORM DRAIN INLET PER COA STD DWG 2205.
- 18. CONSTRUCT TYPE "C" DOUBLE GRATE STORM DRAIN INLET PER COA STD DWG 2205.
- 19. SIDEWALK AND ROADWAY IMPROVEMENTS WITHIN RIGHT OF WAY TO BE CONSTRUCTED UNDER CITY WORK ORDER, CITY PROJECT NUMBER 674481.
- 20. INSTALL END SECTION.
- 21. CONSTRUCT FLARED OPENING IN CONCRETE RUNDOWN PER 2/C101.
- 22. INSTALL 11.25° VERTICAL BEND.
- 23. REMOVE AND SALVAGE EXISTING WIRE FENCE CORNER PANELS. CONTACT KURT WAGNER AT (505) 884-2115 TO COORDINATE DELIVERY OF SALVAGED MATERIALS.
- 24. CONSTRUCT WIRE FENCE PULL PANEL AT END OF EXISTING WIRE FENCE PER AMAFCA STD DWG 203. NEW PULL PANEL TO ABUT NEW RETAINING
- 25. EXISTING WIRE FENCE TO REMAIN.

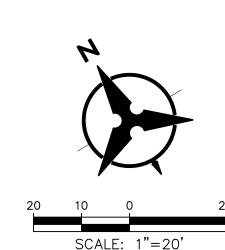
LEGEND

PROPERTY LINE
EXISTING CONTOURS
EXISTING GROUND SPOT ELEVATION
PROPOSED SPOT ELEVATION TC=TOP OF CURB, FL=FLOW LINE TS=TOP OF SIDEWALK, TA=TOP OF ASPHAL EX=EXISTING, FG=FINISHED GRADE TG=TOP OF GRATE, INV=INVERT FGH=FINISHED GRADE HIGH FGL=FINISHED GRADE LOW
PROPOSED DIRECTION OF FLOW
WATER BLOCK / RIDGE OR HIGH POINT
PROPOSED RETAINING WALL
PROPOSED INDEX CONTOURS
PROPOSED INTER CONTOURS
PROPOSED CURB & GUTTER
EASEMENT
PROPOSED STORM DRAIN LINE
PROPOSED STORM DRAIN MANHOLE
PROPOSED STORM DRAIN INLET

WORK ORDER NOTES

SIDEWALK & ROADWAY IMPROVEMENTS WITHIN RIGHT OF WAY WILL B A PART OF THE CONTRACT, HOWEVER CITY WORK ORDER (INCLUDING APPROVED DRAWINGS) IS REQUIRED FOR CONSTRUCTION.

ALBUQUERQUE STANDARD SPECIFICATIONS FOR PUBLIC WORKS CONSTRUCTION 1986 EDITION AS REVISED THROUGH UPDATE #8.



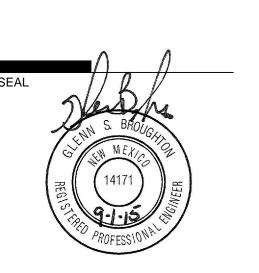


DEKKER PERICH

ARCHITECTURE / DESIGN / INSPIRATION

7/11 JEFFERSON NE, SUITE 100 ALBUQUERQUE, NM 87109

505.761.9700 / DPSDESIGN.ORG



PROJECT

CONSTRUCTION

DOCUMENTS

DRAWN BY **REVIEWED BY** DATE 09-03-2015

PROJECT NO: DRAWING NAME **GRADING &** DRAINAGE PLAN

SHEET NO. C-100

P:\20160014\CDP\Plans\General\CDs\20160014gp01.dwg Tue, 1—Sep—2015 — 4:59:pm, Plotted by: BORTEGA

SEGMENTAL RETAINING WALL CONSTRUCTION NOTES:

RETAINING WALL SYSTEM REQUIREMENTS (PROVIDED BY MANUFACTURER OR CONTRACTOR).

CONTRACTOR TO PROVIDE OWNER WITH COLOR SAMPLES FOR FINAL COLOR SELECTION.

APPROVED SHOP DRAWINGS MUST BE AVAILABLE ON-SITE AT ALL TIMES.

BLOCK WALL - TO BE TREATED -

WITH WAX TYPE ANTI-GRAFFITI

APPROVED BY OWNER)

TO GOVERNING ENTITY.

IN RETAINING WALL BID.

TOP OF WALL

BOTTOM OF WALL

RETAINING WALL CONTRACTOR TO MEET CONTRACTOR'S QUALIFICATIONS LISTED BELOW (UNLESS OTHERWISE

CONTRACTOR TO PROVIDE SHOP DRAWINGS PREPARED BY A PROFESSIONAL. ENGINEER REGISTERED IN NEW MEXICO

& QUALIFIED TO DESIGN SEGMENTAL RETAINING WALLS TO BOHANNAN HUSTON FOR REVIEW PRIOR TO SUBMITTING

SOIL AND COMPACTION TESTING SHALL BE PERFORMED DURING CONSTRUCTION (BASE, 1/2 HEIGHT, AND FINAL).

ELEVATIONS SHOWN ARE TO TOP OF WALL AND BOTTOM OF WALL AT FINAL GRADE (DOES NOT INCLUDE FOOTER).

CONTRACTOR TO INCLUDE ANY OVER EXCAVATION AND CONTROLLED BACKFIELD ASSOCIATED WITH RETAINING WALL

4" CAP UNIT —

∠ UNDISTURBED

SOIL

- GEOGRID(TYP)

- REINFORCED -

BACKFILL

1' ZONE OF

CRUSHED ROCK

BEHIND WALL

─ BASE LEVELING PAD

TYPICAL SEGMENTAL RETAINING WALL SECTION

(NOTE: RETAINING WALL CONTRACTOR TO PERMIT WALL DIRECTLY THROUGH THE

PERMITTING AUTHORITY. CONTRACTOR TO PROVIDE BOHANNAN HUSTON INC. WITH

SHOP DRAWINGS (FOR APPROVAL) FOR EACH WALL PRIOR TO CONSTRUCTION.)

MATCHLINE - SEE SHEET C-101 50-

FF=5117.00

GENERAL NOTES 1. ALL WORK DETAILED ON THESE PLANS AND PERFORMED UNDER THIS CONTRACT SHALL BE CONSTRUCTED IN ACCORDANCE WITH THE PROJECT SPECIFICATIONS AND THE PROJECT GEOTECHNICAL REPORT. WHERE APPLICABLE, CITY OF ALBUQUERQUE PUBLIC WORKS STANDARDS SHALL APPLY. 2. THE CONTRACTOR SHALL ABIDE BY ALL LOCAL, STATE, AND FEDERAL LAWS, RULES AND REGULATIONS WHICH

APPLY TO THE CONSTRUCTION OF THESE IMPROVEMENTS, INCLUDING EPA REQUIREMENTS WITH RESPECT TO STORM WATER DISCHARGE. 3. PRIOR TO CONSTRUCTION, THE CONTRACTOR SHALL FIELD VERIFY THE HORIZONTAL AND VERTICAL LOCATIONS OF ALL POTENTIAL OBSTRUCTIONS INCLUDING ALL UNDERGROUND UTILITIES. SHOULD A CONFLICT EXIST, THE

CONTRACTOR SHALL NOTIFY THE CONSTRUCTION OBSERVER OR ENGINEER SO THAT THE CONFLICT CAN BE RESOLVED WITH A MINIMUM AMOUNT OF DELAY. 4. TWO (2) WORKING DAYS PRIOR TO ANY EXCAVATION, THE CONTRACTOR SHALL CONTACT LINE LOCATING SERVICE

FOR LOCATION OF EXISTING UTILITIES.

5. ALL ELECTRICAL, TELEPHONE, CABLE TV, GAS AND OTHER UTILITY LINES, CABLES, AND APPURTENANCES ENCOUNTERED DURING CONSTRUCTION THAT REQUIRE RELOCATION, SHALL BE COORDINATED WITH THAT UTILITY. THE CONTRACTOR SHALL BE RESPONSIBLE FOR COORDINATION OF ALL NECESSARY UTILITY ADJUSTMENTS. NO ADDITIONAL COMPENSATION WILL BE ALLOWED FOR DELAYS OR INCONVENIENCES CAUSED BY UTILITY COMPANY WORK CREWS. THE CONTRACTOR MAY BE REQUIRED TO RESCHEDULE HIS ACTIVITIES TO ALLOW UTILITY CREWS TO PERFORM THEIR

6. THE CONTRACTOR IS RESPONSIBLE FOR PROTECTING ALL EXISTING UTILITY LINES WITHIN THE CONSTRUCTION AREA. ANY DAMAGE TO EXISTING FACILITIES CAUSED BY CONSTRUCTION ACTIVITY SHALL BE REPAIRED OR REPLACED

AT THE CONTRACTOR'S EXPENSE AND APPROVED BY THE CONSTRUCTION OBSERVER. 7. CONSTRUCTION ACTIVITY SHALL BE LIMITED TO THE PROPERTY AND/OR PROJECT LIMITS. ANY DAMAGE TO ADJACENT PROPERTIES RESULTING FROM THE CONSTRUCTION PROCESS SHALL BE REPAIRED OR REPLACED AT THE

CONTRACTOR'S EXPENSE. 8. OVERNIGHT PARKING OF CONSTRUCTION EQUIPMENT SHALL NOT OBSTRUCT DRIVEWAYS OR DESIGNATED TRAFFIC

9. THE CONTRACTOR SHALL OBTAIN ALL THE NECESSARY PERMITS FOR THE PROJECT PRIOR TO COMMENCING CONSTRUCTION (I.E., BARRICADING, TOPSOIL DISTURBANCE, EXCAVATION PERMITS, EPA STORM WATER PERMITS, ETC.).

LANES. THE CONTRACTOR SHALL NOT STORE ANY EQUIPMENT OR MATERIAL WITHIN THE PUBLIC RIGHT-OF-WAY.

10. ALL PROPERTY CORNERS DESTROYED DURING CONSTRUCTION SHALL BE REPLACED AT THE CONTRACTOR'S EXPENSE. ALL PROPERTY CORNERS MUST BE RESET BY A REGISTERED LAND SURVEYOR. 11. THE CONTRACTOR SHALL PREPARE A CONSTRUCTION TRAFFIC CONTROL AND SIGNING PLAN AND OBTAIN

APPROVAL OF SUCH PLAN FROM THE CITY OF ALBUQUERQUE, TRAFFIC ENGINEERING DEPARTMENT, PRIOR TO BEGINNING ANY CONSTRUCTION WORK ON OR ADJACENT TO EXISTING STREETS. 12. ALL BARRICADES AND CONSTRUCTION SIGNING SHALL CONFORM TO APPLICABLE SECTIONS OF THE "MANUAL ON

UNIFORM TRAFFIC CONTROL DEVICES" (MUTCD), US DEPARTMENT OF TRANSPORTATION, LATEST EDITION. 13. THE CONTRACTOR SHALL MAINTAIN ALL CONSTRUCTION BARRICADES AND SIGNING AT ALL TIMES. THE CONTRACTOR SHALL VERIFY THE PROPER LOCATION OF ALL BARRICADING AT THE END AND BEGINNING OF EACH DAY.

14. THE CONTRACTOR SHALL TAKE ALL STEPS NECESSARY TO CONFORM WITH EPA REQUIREMENTS, INCLUDING COMPLIANCE WITH NPDES PHASE 2 REQUIREMENTS.

PARADISE BOULEVARD

\INV 12"=10.00 ₩ 6"=10.13 🔏 🦥 FL12.02-

TS16.88-FG16.75-

- COMPACT SUBGRADE TO A MINIMUM

OF 95% MAXIMUM DENSITY

CONCRETE RUNDOWN / RIBBON CHANNEL

TS16.98-TS16.86-

TC15.50_ FL15.50

P:\20160014\CDP\Plans\General\CDs\20160014gp02.dwg Tue, 1—Sep—2015 — 5:01:pm, Plotted by: BORTEGA

ARCHITECTURE / DESIGN / INSPIRATION

DEKKER

PERICH

SABATINI

7601 JEFFERSON NE, SUITE 100 ALBUQUERQUE, NM 87109

505.761.9700 / DPSDESIGN.ORG

Clinic

aradise

4588 PARADISE BLVD ALBUQUERQUE, NM 871

100%

CONSTRUCTION

DOCUMENTS

REVISIONS

DRAWN BY

DATE

REVIEWED BY

PROJECT NO:

4. CONSTRUCT 4' DIA TYPE "E" STORM DRAIN MANHOLE PER COA STD DWG

7. INSTALL STORM DRAIN TO WITHIN 5' OF BUILDING. SEE PLUMBING PLANS

11. CONSTRUCT 2' WIDE CONCRETE RIBBON CHANNEL, SEE DETAIL 2/C-100.

DETAILS. MAINTAIN POSITIVE DRAINAGE AROUND BERM ADJACENT TO

SEE ARCHITECTURAL DETAILS FOR 'RAIN CHAIN' CONNECTION DETAIL.

KURT WAGENER AT (505) 884-2115 TO COORDINATE DELIVERY OF

17. CONSTRUCT TYPE "C" SINGLE GRATE STORM DRAIN INLET PER COA STD

18. CONSTRUCT TYPE "C" DOUBLE GRATE STORM DRAIN INLET PER COA STD

CONSTRUCTED UNDER CITY WORK ORDER, CITY PROJECT NUMBER 674481.

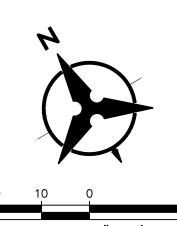
21. CONSTRUCT FLARED OPENING IN CONCRETE RUNDOWN PER 2/C101.

23. REMOVE AND SALVAGE EXISTING WIRE FENCE CORNER PANELS. CONTACT KURT WAGNER AT (505) 884-2115 TO COORDINATE DELIVERY OF

PER AMAFCA STD DWG 203. NEW PULL PANEL TO ABUT NEW RETAINING

I. SIDEWALK & ROADWAY IMPROVEMENTS WITHIN RIGHT OF WAY WILL BE A PART OF THE CONTRACT, HOWEVER CITY WORK ORDER (INCLUDING

2. THE CONTRACTOR SHALL BE RESPONSIBLE FOR CONTRACTING WITH A REGISTERED PROFESSIONAL ENGINEER FOR CONSTRUCTION ENGINEERING CONSTRUCTION). THE ENGINEER SHALL BE LICENSED IN THE STATE OF NEW MEXICO. THE ENGINEER SHALL BE RESPONSIBLE FOR CONSTRUCTION OVERSIGHT, CONSTRUCTION OBSERVATION REPORTS, CERTIFICATION CLOSE OUT AND ACCEPTANCE BY THE CITY IN ACCORDANCE WITH THE CITY OF



Bohannan A Huston

DRAWING NAME **GRADING &** DRAINAGE PLAN

SHEET NO.

C-101

09-03-2015

15-0028.001

CONSTRUCTION

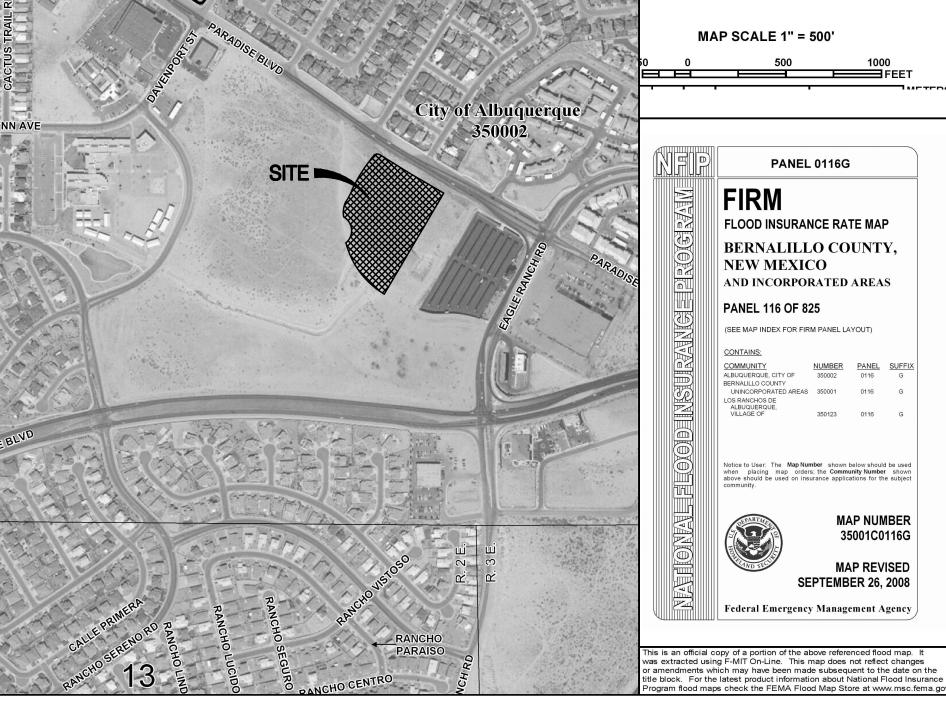
PROJECT NO: 15-0028.001 DRAWING NAME DRAINAGE MANAGEMENT

PLAN

SHEET NO.



VICINITY MAP: C-12



FEMA FIRM MAP # 35001C0116G

						PRE	BYTERIA	AN PARA	ADISE C	LINIC					
				1	Proposed	l Ultimat	e Develo	pment Co	ondition	s Basin D	Data Table				
This ta	able is based	on the DPM	Section 22.2,	Zone:	1								"First Flus	sh" Vol. Cal	с.
Basin	Area	Area	Lan	d Treatme	nt Percenta	iges	Q (100yr)	Q (100yr-6hr)	WTE	V (100yr-6hr)	V (100yr-10day)	*Precip.	Impervious	Vol. Rqd.	Vol. Provided
ID	(SQ. FT)	(AC.)	Α	В	С	D	(cfs/ac.)	(CFS)	(inches)	(CF)	CF	Depth (in.)	Area (SF)	(CF)	(CF)
Prop	osed			•									, ,	•	
1	36392	0.84	0.0%	0.0%	34.0%	66.0%	3.86	3.22	1.64	4964	7906	0.30	24019	600	1807
2	41613	0.96	0.0%	0.0%	15.0%	85.0%	4.15	3.96	1.82	6322	10655	0.30	35371	884	450
3	13109	0.30	0.0%	0.0%	6.0%	94.0%	4.28	1.29	1.91	2088	3597	0.30	12322	308	125
4	62956	1.45	0.0%	0.0%	17.0%	83.0%	4.12	5.95	1.80	9461	15862	0.30	52253	1306	1348
5	24362	0.56	0.0%	0.0%	63.0%	37.0%	3.43	1.92	1.35	2746	3850	0.30	9014	225	2607
6	28527	0.65	0.0%	0.0%	27.0%	73.0%	3.97	2.60	1.71	4054	6605	0.30	20825	521	190
7	5114	0.12	0.0%	0.0%	73.0%	27.0%	3.28	0.38	1.25	535	704	0.30	1381	35	0
8	10309	0.24	0.0%	0.0%	85.0%	15.0%	3.10	0.73	1.14	977	1166	0.30	1546	39	0
9	948	0.02	0.0%	0.0%	59.0%	41.0%	3.49	0.08	1.39	110	158	0.30	389	10	0
TOTAL	212073	5.13		•				20.1		-	50503		157120	3928	6527

* Precipitation depth includes a deduction for initial abstraction of 0.1" and 0.04" for infiltration over a one hour period for a total abstraction of 0.14" for Land Treatment D.

P8

PREBYTERIAN PARADISE CLINIC STORM DRAIN PIPE TABLE PIPE INVERT INVERT Size | Slope | Capacity* | FLOW | Velocity | LENGTH | Basins & Pipes cfs STORM DRAIN PIPE 12 | 2.01% | 5.0 | 3.2 | 6.8 | 174.0 | 5110.00 | 5106.51 18 0.50% 7.4 7.2 4.8 275.0 5106.51 12 1.00% 3.6 2.6 4.9 135.0 12 2.00% 5.0 2.6 6.4 208.0 12 2.22% 5.3 4.5 7.6 67.5 5106.00 5104.50 18 1.17% 11.4 10.5 8.6 72.5 5104.50 5103.65 B4+B5+B6 B3+B4+B5+B6 | 24 | 0.50% | 16.0 | 11.7 | 5.5 | 74.0 | 5103.65 | 5103.28

 24
 8.47%
 65.9
 18.9
 18.1
 70.8
 5096.00
 5090.00

CAPACITY IS BASED ON GRAVITY FLOW, USING MANNING'S EQUATION WITH n=0.013

		PREBYTE			CLINIC			
			INLET T	ABLE				
INLET	CONTRIBUTING	INLET	TOP OF	ACTUAL	AVAIL	CAPACITY	CAPACITY	Grate
#	BASIN	TYPE	GRATE	FLOW	HEAD		WITH 50%	Calculation
					FT		CLOGGING	
IN-1	B1	18" DOME	5112.50	3.2	1.0	5.6	2.8	Sump
IN-2	B2	TYPE C DBL	5108.99	4.0	0.5	7.6	3.8	Sump
IN-3	B3	TYPE C SGL	5109.46	1.3	0.5	5.1	2.5	Sump
IN-4	B4	24" DOME	5107.00	5.9	1.0	9.0	4.5	Sump
IN-5	B5	18" DOME	5108.50	1.9	1.0	5.6	2.8	Sump
IN-6	B6	18" DOME	5115.00	2.6	1.1	6.0	3.0	Sump

LEGEND

	PROPERTY LINE
	EXISTING CONTOURS
● ^{65.23}	PROPOSED SPOT ELEVATION TC=TOP OF CURB, FL=FLOW LINE FGL=FINISHED GRADE LOW SIDE OF WAI FGH=FINISHED GRADE HIGHT SIDE OF W EX=EXISTING, TG=TOP OF GRATE
S=2.0%	PROPOSED DIRECTION OF FLOW
\\\\\	WATER BLOCK
	PROPOSED RETAINING WALL
5305	PROPOSED INDEX CONTOURS
	PROPOSED INTER CONTOURS
	EASEMENT
OD	

	PROPERTY LINE
5301	EXISTING CONTOURS
● ^{65.23}	PROPOSED SPOT ELEVATION TC=TOP OF CURB, FL=FLOW LINE FGL=FINISHED GRADE LOW SIDE OF WALL FGH=FINISHED GRADE HIGHT SIDE OF WALL EX=EXISTING, TG=TOP OF GRATE
S=2.0%	PROPOSED DIRECTION OF FLOW
\\\\\	WATER BLOCK
	PROPOSED RETAINING WALL
5305	PROPOSED INDEX CONTOURS
	PROPOSED INTER CONTOURS
	EASEMENT
SD	PROPOSED STORM DRAIN LINE

INTRODUCTION:

THE PROJECT IS LOCATED ON THE SOUTHEAST CORNER OF THE INTERSECTION OF PARADISE PROVIDE A DRAINAGE MANAGEMENT PLAN FOR THE DEVELOPMENT OF THE PRESBYTERIAN PARADISE CLINIC AND REQUEST FOR BUILDING PERMIT APPROVAL.

EXISTING CONDITIONS:

THE 5.13 ACRE SITE IS CURRENTLY UNDEVELOPED. THE SITE SLOPES TO THE EAST / SOUTHEAST WHERE THE RUNOFF DISCHARGES TO AN EXISTING AMAFCA DETENTION FACILITY ADJACENT TO THE SUBJECT PROPERTY.

BASED ON THE DRAINAGE STUDY FOR FOUNTAIN HILLS (CITY OF ALBUQUERQUE DRAINAGE FILE #C12/D3B), THE ALLOWABLE PEAK DISCHARGE FROM THE SITE IS 21.43 CFS (BASIN D-1-A). THE DRAINAGE MANAGEMENT PLAN ASSUMES THAT UP TO 20% OF THE DEVELOPED FLOW WILL DISCHARGE TO ADJACENT ROADWAYS AND THE REMAINING 80% WILL BE CAPTURED BY PROPOSED STORM DRAIN STUBS.

METHODOLOGY:

THE HYDROLOGIC ANALYSIS PROVIDED WITH THIS DRAINAGE MANAGEMENT PLAN HAS BEEN PREPARED IN ACCORDANCE WITH SECTION 22.2 OF THE DPM. THE SITE IS LOCATED WEST OF THE RIO GRANDE WITHIN PRECIPITATION ZONE 1. LAND TREATMENT PERCENTAGES WERE CALCULATED BASED ON THE ACTUAL CONDITIONS IN EACH ONSITE BASIN AND ARE SUMMARIZED IN THE "PROPOSED BASIN DATA TABLE" (THIS SHEET).

PROPOSED CONDITIONS:

THIS DRAINAGE MANAGEMENT PLAN WAS DEVELOPED IN ACCORDANCE WITH A FULLY DEVELOPED SITE. IT WAS DETERMINED THAT THE MAXIMUM ALLOWABLE PEAK DISCHARGE FROM THE SITE IS 21.43 CFS (AS MENTIONED ABOVE).

THE SITE IS DIVIDED INTO NINE SMALL DRAINAGE BASINS. BASINS B7 AND B9 FREE DISCHARGE TO NUNZIO DRIVE. BASIN B8 FREE DISCHARGES TO PARADISE BOULEVARD AND THE ADJOINING PROPERTY TO THE EAST. THE REMAINDER OF THE DRAINAGE BASINS ARE CONVEYED TO THE AMAFCA DETENTION FACILITY VIA AN ONSITE STORM DRAIN SYSTEM.

THE PRIVATE ONSITE STORM DRAIN SYSTEM CAPACITY WAS CALCULATED BASED ON MANNING'S EQUATION WITH AN N VALUE OF 0.013. ALL STORM DRAIN PIPES SIZED TO CONVEY THE 100 YEAR, 6 HOUR PEAK FLOW. ONSITE INLETS ARE DESIGNED IN A SUMP CONDITION WITH A CLOGGING FACTOR OF APPROXIMATELY 50%.

FIRST FLUSH CALCULATIONS:

IMPERVIOUS AREA: 154,538 SF (PAVED AREA 119,754 & ROOF AREA 34,784 SF) *RAINFALL DEPTH: 0.30" TOTAL VOLUME REQUIRED: 3862 CF

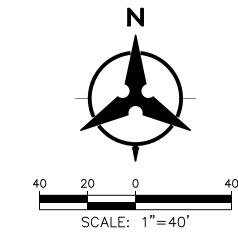
TOTAL VOLUME PROVIDED: 6527 CF *INITIAL ABSTRACTION OF 0.1" WAS USED FOR PAVED AREAS AND AN INFILTRATION RATE OF 0.04" PER OVER A PERIOD OF ONE HOUR PER DPM CHAPTER 22, TABLES A-6 AND A-7.

THE REQUIRED VOLUME AND PROVIDED VOLUME ARE SHOWN ON THE DEVELOPED CONDITIONS BASIN TABLE. LANDSCAPE ISLANDS WILL BE DEPRESSED WITH CURB OPENINGS TO INTERCEPT DRAINAGE. BASINS B2 AND B3 PROVIDE ROUGHLY HALF OF THE REQUIRED VOLUME. RETENTION VOLUME IN BASIN B6 IS UNDER SIZED; HOWEVER THIS BASIN OVERFLOWS INTO THE RETENTION AREA IN BASIN B5 WHICH HAS EXCESS CAPACITY. DUE TO THE EXISTING TOPOGRAPHY IT WAS NOT POSSIBLE TO PROVIDE ONSITE RETENTION FOR BASINS B7, B8 AND B9. THESE BASINS ARE SMALL AND ARE MOSTLY PERVIOUS LAND TREATMENT. THESE BASINS REQUIRE VERY SMALL RETENTION VOLUMES. OVERALL THE TOTAL "FIRST FLUSH" VOLUME THAT IS NOT INTERCEPTED WAS CALCULATED TO BE 84 CUBIC FEET. THE TOTAL REQUIRED "FIRST FLUSH" VOLUME WAS CALCULATED TO BE 3,928 CUBIC FEET. "FIRST FLUSH" RUNOFF THAT IS NOT INTERCEPTED ONSITE REPRESENTS APPROXIMATELY 2% OF THE TOTAL FIRST FLUSH VOLUME..

THERE ARE SEVERAL CONTRIBUTING FACTORS THAT LIMITED THE VOLUME THAT COULD BE INCORPORATED INTO THE SITE DESIGN. THE SITE HAS AN EXISTING SLOPE OF APPROXIMATELY 10.00%. STEEP SLOPE SITES MAKE IT CHALLENGING TO INCORPORATE SHALLOW RETENTION AREAS IN THE LANDSCAPE DESIGN AND REDUCE THE ACTUAL VOLUME INTERCEPTED. THIS DESIGN MAXIMIZED THE "FIRST FLUSH" RETAINED WITHIN THE SITE.

CONCLUSION:

THE PEAK DISCHARGE FROM THE SITE IS 20.0 CFS WHICH IS LESS THAN THE ALLOWABLE PEAK DISCHARGE RATE, THEREFORE WE ARE IN CONFORMANCE WITH CITY OF ALBUQUERQUE HYDROLOGY REQUIREMENTS AND REQUEST FOR BUILDING PERMIT APPROVAL.



SD MHHT178

RNE 5123.39

CONDITION: DID NOT OPEN

FF=5117.00

B6

CONDITION: DID NOT OPEN
FOUND 37 ALLIMINAL CAP
STAMPED "COA CL. MON LS 7119"