

May 22, 1996

Martin J. Chávez, Mayor

Scott McGee Isaacson & Arfman 128 Monroe NE Albuquerque, NM

AG SPANOS APARTMENTS (C13-D1D) ENGINEER'S CERTIFICATION FOR CERTIFICATE OF OCCUPANCY. ENGINEER'S CERTIFICATION DATED 5-3-96.

Dear Mr. McGee:

Based on the information provided on your May 7, 1996 submittal, the above referenced project is approved for a Certificate of Occupancy (Buildings 1-17).

If I can be of further assistance, please feel free to contact me at 768-3622.

Sincerely

Engineering Assoc./Hyd.

Andrew Garcia c:

File





## City of Albuquerque

P.O. BOX 1293 ALBUQUERQUE, NEW MEXICO 87103

December 13, 1994

Scott M. McGee, P.E. Isaacson & Arfman, P.A. 128 Monroe Street NE Albuquerque, N.M. 87108

RE: DRAINAGE REPORT FOR AG SPANOS APTS PHASE 2 (C-13/D10)
RECEIVED NOVEMBER 21, 1994 FOR BUILDING PERMIT APPROVAL

ENGINEER'S STAMP DATED 11-17-94

Dear Mr. McGee:

Based on the information included in the submittal referenced above, City Hydrology approves this project for Building Permit.

Include a copy of the approved Grading Plan, dated 11-17-94, in the set of construction documents that will be submitted to the "one stop" for the Building Permit.

Engineer's Certification of grading & drainage per DPM checklist must be approved before any Certificate of Occupancy is released.

If you have any questions about this project, You may contact me at 768-2727.

Sincerely,

John P. Curtin, P.E.

Civil Engineer/Hydrology

mit austin

c: Andrew Garcia

### DRAINAGE REPORT

### **FOR**

## AG SPANOS APARTMENT PHASE 2

### NORTHEAST CORNER OF

## PARADISE BLVD. AND EAGLE RANCH ROAD NW

ALBUQUERQUE NEW MEXICO NOVEMBER 1994

Prepared by:

ISAACSON & ARFMAN, P.A. 128 Monroe Street, NE Albuquerque, NM 87108

Scott M. McGee, P.E. Date

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### I. INTRODUCTION

The AG Spanos Apartment complex is a two-phase, 432 unit multi-family apartment project consisting of 33 apartment buildings and two community buildings. Phase One construction began this summer and is now underway. The land area for this second phase of the development is 434,655 SF (9.9783 acres). See Vicinity Map in Appendix.

A pre-design meeting was held with Mr. Fred Aguirre of the Hydrology Section of the Public Works Department the City of Albuquerque. The following findings were established as design criteria for this project:

- Discharge from site is to be determined by analysis of capacity for downstream improvements constructed as part of the Market Center West development.
- An erosion control plan for the period of construction will be required to ensure that sediment remains on-site. All sedimentation measures shall be approved by AMAFCA.



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Lisa Ann Manwil

Engineering Assoc./Hyd.

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- Discharge from site is to be determined by analysis of capacity for downstream improvements constructed as part of the Market Center West development.
- 2. An erosion control plan for the period of construction will be required to ensure that sediment remains on-site. All sedimentation measures shall be approved by AMAFCA.

### II. EXISTING SITE CONDITIONS

- A. Flood Hazard--This site is not within an established flood hazard area as shown on Panel 8 of the FEMA flood maps.
- B. Soils--Site soils are bluepoint loamy fine sand and kokan gravelly sand. This soil is classified in the SCS Soil Survey of Bernalillo County as a Hydrologic Group 'A' soil. (Refer to the September 30, 1993 Geotechnical Report by Western Technologies, Inc. a copy of the drainage recommendations is included in the Appendix).
- C. Topography--Present conditions are such that the site slopes to the south and southeast at four to five percent. No drainage courses are present, all runoff is overland sheet flow due to rough grading done as part of the first phase of this development.

### III. OFF-SITE CONDITIONS

The site is bounded on the west by Eagle Ranch Road; on the north and east by phase one of this apartment development; and on the south by Paradise Blvd. NW. Minor off-site flows presently cross Eagle Ranch Road and are carried

east as surface flow within the right-of-way of Paradise Blvd. A new storm drain will be built in Paradise Blvd. as part of City Project No. 3575.92 which includes paving improvements to the north half of this street east of Eagle Ranch Road. The north-bound lanes of Eagle Ranch Road NW will also be constructed by this project where it abuts the site. The two site entrances on Eagle Ranch Road and

Paradise Blvd. have been designed to match the private driveways shown in the

City Project No. 3575.92.

PROPOSED CONDITIONS IV.

Phase Two includes 208 apartments in 15 buildings, a community building

and pool, sports courts, and associated parking. All runoff will flow to the south

and southeast where it will be intercepted by catch basins in paved parking areas.

This runoff will be carried to the existing detention pond located within Phase

One. Flows are then routed out of this pond and into a second existing pond

located south of the existing movie theater building. This drainage concept is

unchanged from that previously approved.

The following breakdown gives peak runoff data for both undeveloped and

developed conditions for Phase Two:

Area: 9.9783 Ac.

Precipitation Zone: 1

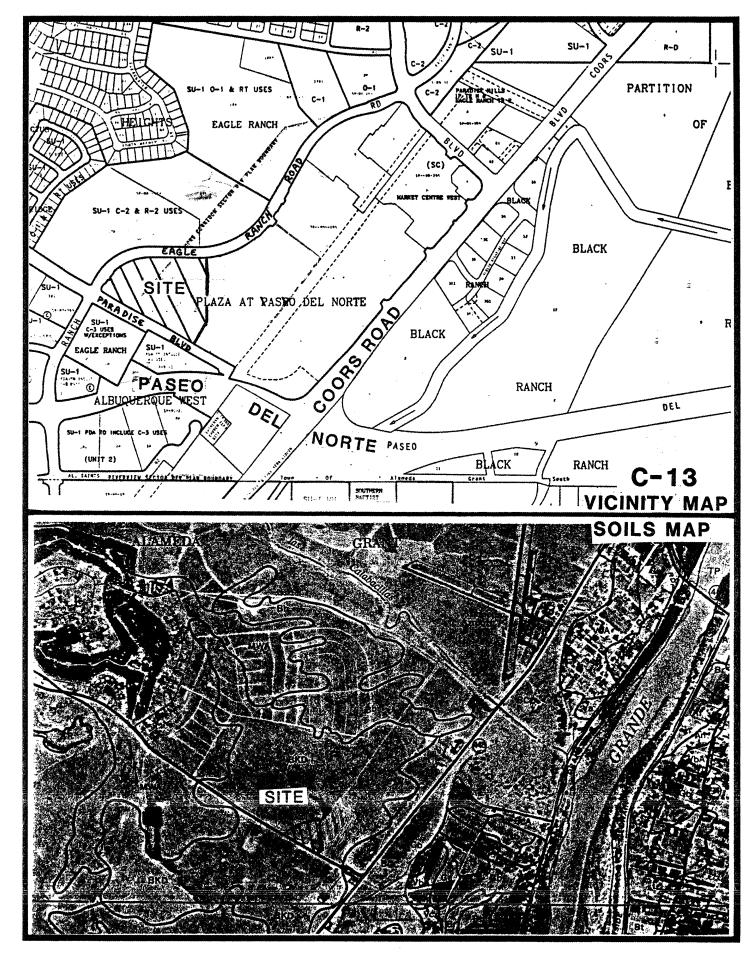
3

Development	Land	Trea	tment	(%)			
Condition	Α	В	С	D	$Q_{100}$	Q <sub>10</sub>	
Undeveloped	0	0	100	0	28.6	14.9	
Developed	0	32	7	61	35.1	21.1	

The developed runoff rate of 35.1 cfs was accounted for during phase one development and a 24-inch storm drain was extended into phase two to convey this flow to the existing detention pond. Installed at a slope of 2.46%, this storm drain has a capacity of 35.6 cfs using Manning's equation. An overland flow path is available in the event of complete inlet clogging.

## **APPENDIX**

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RUNOFF CALCULATIONS		
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GRADING & DRAINAGE PLAN (IN POC	KI	ET)



ISAACSON & ARFMAN, P.A.

## **RUNOFF CALCULATIONS**

PROJECT: AG SPANOS APTS PHASE Z Analysis Point # Tract A-4
DRAINAGE AREA:  Planimeter Rdg x .015
x Map Scale2 ÷ 43,560 A = $\frac{9.9783}{}$ acres
TIME OF CONCENTRATION: Drainage Basin Data:
L = 900 ft., fall = 35 ft., slope = .039 ft/ft
$v = \frac{2.0}{\text{Tc}} = \frac{\text{ft/sec} (\frac{P1-22.2 \text{ B-1}}{13})}{\text{sec.}} = \frac{7.5}{\text{min.}} \text{ Tc} = \frac{(\text{USE})}{2} = \frac{1}{2}$ min.
☐ Street Flow:  v =ft/sec (P1 22.2 B-2)  Tc =min. Tc =min.
$\square$ Arroyo Flow: Tc = 0.0078 $\underline{LO.77}$ = min. Tc = min. SO.385
LAND TREATMENT:  Land Use Type Percentage Acres Precipitation Zone:
A
B
100% <u>9,9783</u> PEAK DISCHARGE (Table 9):
<u>Treatment</u> <u>Area(acres)</u> <u>Discharge (cfs/ac)</u> $Q_{100}$ (cfs)
A B
C 9.9783 2.87 28.6
TOTAL 28.6
VOLUME OF RUNOFF:  Soil Group A B C D  CN (Previous) = (P1 22.2 C-2)  Percent Impervious = %  CN (Composite) = (P1 22.2 C-3)  Direct Runoff, $q_{100}$ = " (P1 22.2 C-4) $V_{100}$ = $q$ A = x ac x 43,560 = cu. ft.

## **RUNOFF CALCULATIONS**

PROJECT: AG SPANOS APTS PHASE 2 Analysis Point # Tract A	1-4
DRAINAGE AREA:  Planimeter Rdg x .015	
x Map Scale $2 \div 43,560$ $A = 9.9783$ acres	
TIME OF CONCENTRATION: Drainage Basin Data:	
L = 1200 ft., fall = 32.5 ft., slope = .027	ft/ft
☐ Overland Flow:  v = ft/sec (P1 22.2 B-1)  Tc = min. Tc =	min.
Street Flow: (13) $v = \frac{4.9}{\text{Tc}} = \frac{\text{ft/sec}}{245} = \frac{(13)}{\text{sec.}} = \frac{(13)}{4.1} = \frac{(13)}{\text{min.}} = \frac{(13)}{12} = \frac$	min.
□ Arroyo Flow: $Tc = 0.0078 \frac{LO.77}{SO.385} = min. Tc =$	min.
LAND TREATMENT:	
Land Use Type         Percentage         Acres         Precipitation Zone:           A         0           B         32         3.193           C         7         0,699           D         6.086           100%         9.978   PEAK DISCHARGE (Table 9):	
<u>Treatment</u> <u>Area(acres)</u> <u>Discharge (cfs/ac)</u> $Q_{100}$ (cfs)	
A B C O.699 D TOTAL  A 3, 193 C 0.699 C 4.37 C 26.60 A 35.08	
VOLUME OF RUNOFF:  Soil Group A B C D  CN (Previous) = (P1 22.2 C-2)  Percent Impervious = %  CN (Composite) = (P1 22.2 C-3)  Direct Runoff, q <sub>100</sub> = " (P1 22.2 C-4)	

Analysis pt <	$\langle A \rangle$
---------------	---------------------

ONSITE
Street Hydraulics
Street: PARALLEL TO PARADISE  100-yr. Q: 21.0 1/2 Street Flow: NA
Street slope:
Max. flow depth: $0.35$ FT $0.2/$ FT $(Manning Eq)$
(41.2 - 40.85 = 0.35  FT)
(TIL - TOISS - 0.33 FT)
Catch Basin Hydraulics
Outer Dubit Try Grantes
Upstream inlet(s): DBL'D'
DPM grating capacity plate:
*Intercepted Q: 8.3 CFS
· · · · · · · · · · · · · · · · · · ·
Remaining surface flow: 12.7 CFS  Max. flow depth: 5EE ANALYSIS PT C
Max. flow depth: SEE ANALYSIS PT CD
DPM grating capacity plate:
Intercepted Q:
Total interception capacity:
As % of design Q:
INTERCEPTED Q IS LESSER OF:
ORIFICE EQ: Q = CA NZgH
AREA: DBL 'D' = 8.32 SF USE C= 0.6
Accuse Crate AREA 500 Reserve
Assume GRATE AREA 50% BLOCKED
$Q = (0.6) (8.32) (0.50) \{(2)(32.2)(0.35)\}^{1/2}$
Q = 11.8 cfs
* WEIR EQ: Q = CLH" C=2.7
$L = [(35.5)(4) + (18.5)(2)] \div 12 = 14.9FT$
$Q = (2.7)(14.9)(0.35)^{1.5}$
Q = 8,3 cfs
W = 0,3 CF5 €

A NALYSIS	PT	$\langle B \rangle$
-----------	----	---------------------

Street Hydraulics Street: Deposits 1911 18 To PARAMES	
Street: PERPENDICULAR TO PARADISE  100-yr O: 14.0 1/2 Street Flow:	NA
Street slope: /.5%	
Max. flow depth: 0.35FT 0.16 FT (Manning 6	<del>y)</del>
100-yr. Q: 14.0 1/2 Street Flow:	
Catch Basin Hydraulics	~
Unstream inlet(s): DRI DI	
DPM grating capacity plate: NA	
Upstream inlet(s): DBL 'D' DPM grating capacity plate: NA Intercepted Q: 8.3 CFS	=
Remaining surface flow: 5.7 C FS  Max. flow depth: SEE ANALYSIS PT	<b>∕</b>
Max. flow depth: SEE ANALYSIS PT	<u>C</u> /
Downstream inlet(s):	
DPM grating capacity plate:	
Intercepted Q:	
Total interception capacity:	
As % of design Q:	
INTERCEPTED Q IS LESSER OF:	
ORIFICE EQ: Q = CANZgH	
Assume GRATE AREA 50% BLOCKED	
$Q = (0.6)(8.32)(0.50) \{(2)(32.2)(.35)\}^{1/2}$	
Q= 11.8 cfs	
1	
WER EQ: Q = CLH!5 C=2.7	
$L = [(35.5)(4) + (18.5)(2)] \div 12 = 14.9 \text{ FT}$	
$Q = (2.7)(14.9)(0.35)^{1.5}$	
Q = 8.3 cfs	

ANALYSIS	PT (C)
	· · ·

Street: SE CORNER OF SITE  100-yr. Q: 18.4 (12.7+5.7) 1/2 Street Flow: NA  Street slope: SUMP  Max. flow depth: 0.7 FT (41.2-40.5)
Catch Basin Hydraulics
Upstream inlet(s): TRIPLE ID' DPM grating capacity plate: NA Intercepted Q: 18.4 c.fs
Remaining surface flow:  Max. flow depth:  Downstream inlet(s):  DPM grating capacity plate:  Intercepted Q:  Total interception capacity:  As % of design Q:
INTERCEPTED Q IS LESSER OF:
# ORIFICE Ea: $Q = CA \sqrt{2gH}$ (GRATE 50% BLOCKED) AREA = (4.16 SF)(3)=12.48SF () = (0.6)(12.48)(0.5)(32.2) <sup>1/2</sup>
Q = 21.2  cfs (> 18.4 + 0K)
WEIR EQ: $Q = CLH^{1.5}$ $C = 2.7$ $L = [(35.5)(6) + (18.5)(2)] \div 12 = 20.8 \text{ FT}$ $Q = (2.7)(20.8)(0.7)^{1.5} = 32.8 \text{ CFS}$

compaction, and approximate shrinkage of excavated on-site soils placed in compacted fills:

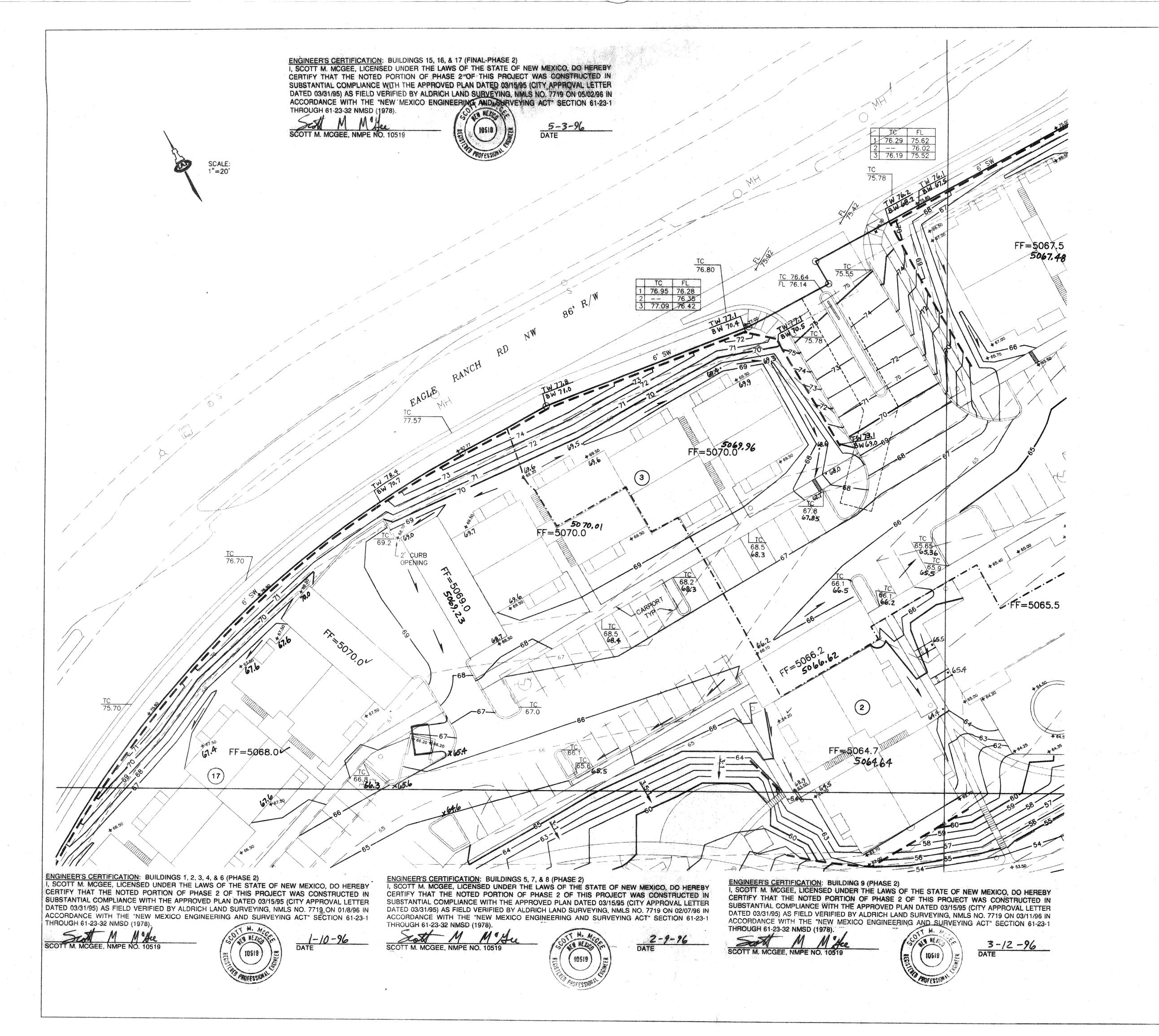
Excavated Depth Below Existing Grade (ft)	Percent Compaction in Fill (ASTM D1557)	Estimated Shrinkage (%)
0 to 3	90	5-10
0 to 3	95	10-15
3 to 6	90	2-7
3 to 6	95	7-12

The estimated shrinkages consider only material densification and do not consider other effects such as transport, wind, overcompaction, or compaction of subsoils to greater depths.

#### Drainage:

### Surface Drainage:

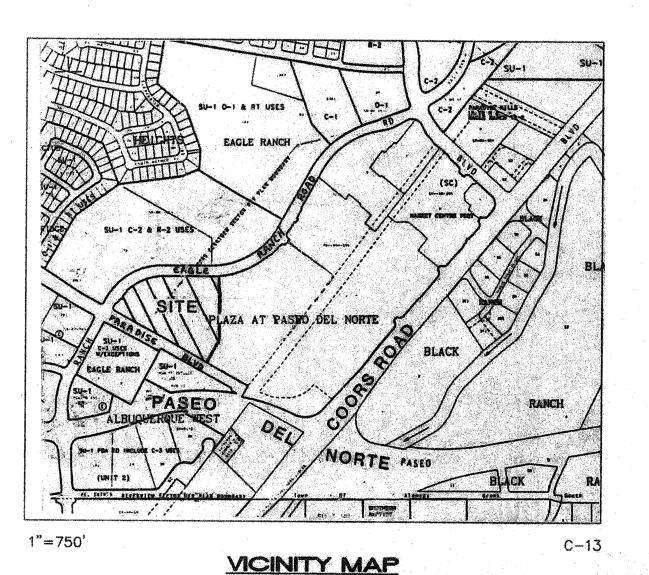
- Positive drainage should be provided during construction and maintained throughout
  the life of the proposed development. Infiltration of water into utility or foundation
  excavations must be prevented during construction. Planters and other surface
  features which could retain water in areas adjacent to all structures should be
  eliminated.
- In areas where sidewalks or paving do not immediately adjoin the structure, we recommend that protective slopes be provided with a minimum grade of approximately 5 percent for at least 10 feet from perimeter walls. Backfill against footings, exterior walls, and in utility and sprinkler line trenches should be well compacted and free of all construction debris to minimize the possibility of moisture infiltration.
- <u>Drainage Behind Walls</u>: Free-draining, granular soils containing less than five percent fines (by weight) passing a No. 200 sieve should be placed adjacent to all retaining or below grade walls. A drainage system consisting of either weep holes or perforated drain lines (placed near the base of the wall) should be used to intercept and discharge water which would tend to saturate the backfill. Where used, drain lines should be embedded in a

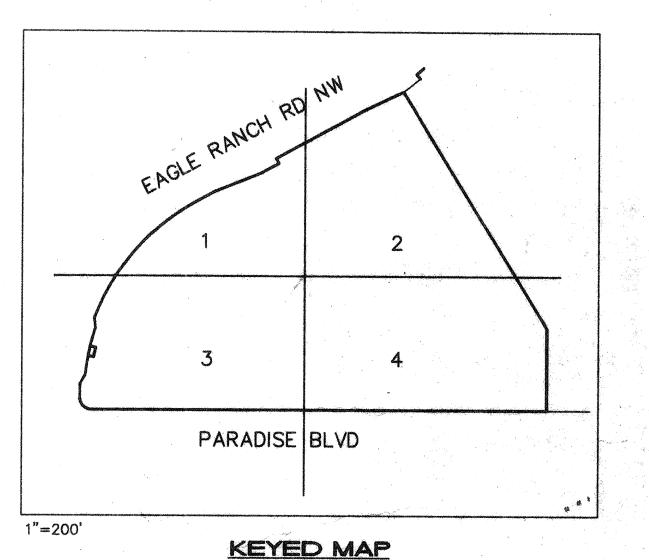


# AG SPANOS APARTMENTS PHASE II GRADING PLAN

TRACT A-4, THE PLAZA AT PASEO DEL NORTE

MARCH 1995





## <u>LEGEND</u>

PROPOSED CONTOUR

PROPOSED CONTOUR

RETAINING WALL

FF=5042.5 FINISH FLOOR ELEVATION

SW CULVERT

4" DRAIN PIPE

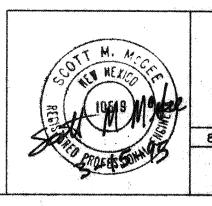
DRAINAGE SUB-BASIN BOUNDARY

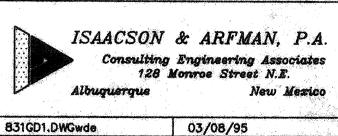
+ 65.40 PROPOSED SPOT ELEVATION

(8.3 AS-BUILT ELEVATION

NOTE:
SEE SHEET 3 FOR EROSION CONTROL PLAN.







SHEET 1 OF 6

