CITY OF ALBUQUERQUE



October 31, 2017

Fred Arfman, P.E. Isaacson & Arfman, P.A. 128 Monroe St. N.E Albuquerque, NM 87108

RE: Industrial Water Engineering Conceptual Grading and Drainage Plan Engineer's Stamp Date 10/26/17 Hydrology File: C16D006LL

Dear Mr. Arfman:

Based on the information provided in the submittal received on 10/26/17 the above-referenced submittal cannot be approved for Building Permit until the following are addressed.

PO Box 1293

Prior to Building Permit:

Albuquerque

1. Correct text size on callouts to be 0.10" or larger; this applies mostly to the survey info for the water valves, drop inlets, and storm drain.

NIM 07102

2. Call-out top of pond, bottom of pond, and volume on sheet CG-101.

NM 87103

3. This project will require an ESC plan, submitted to the Storm Water Quality Engineer (Curtis Cherne, PE ccherne@cabq.gov, 924-3420).

www.cabq.gov

- 4. A Private Facility Drainage Covenant is required for the first flush ponds. The original notarized form, pond exhibit, and recording fee (\$25 payable to City of Albuquerque) must be turned into DRC (4th, Plaza del Sol) for routing. Please contact Charlotte LaBadie (clabadie@cabq.gov, 924-3996) or Madeline Carruthers (mtafoya@cabq.gov, 924-3997) regarding the routing and recording process for covenants.
- 5. Provide hydraulic capacity calculations for the new sidewalk culvert and for the drainage channel downstream of the overflow from Pond P3.

Prior to Hydrology approval for Certificate of Occupancy:

1. The Private Facility Drainage Covenant must be recorded with Bernalillo County and a copy included with the drainage certification.

Orig: Drainage file

CITY OF ALBUQUERQUE



- 2. Payment of Fee-in-Lieu will be required for any ponding areas not constructed and certified.
- 3. Close-out and City acceptance of the Public Work Order is required.

If you have any questions, please contact me at 924-3695 or dpeterson@cabq.gov.

Sincerely,

Dana Peterson, P.E.

Senior Engineer, Planning Dept. Development Review Services

PO Box 1293

Albuquerque

NM 87103

www.cabq.gov



COA STAFF:

ELECTRONIC SUBMITTAL RECEIVED: ____

City of Albuquerque

Planning Department

Development & Building Services Division

DRAINAGE AND TRANSPORTATION INFORMATION SHEET (REV 09/2015)

Project Title: Industrial Water Engineering	Building Permit #:	City Drainage #: C16D006LL
DRB#: EPC#:	- Building 1 clinit "	Work Order#:
Legal Description: Lots 2 & 3, Alameda Business Park		Work Ordern.
City Address: 8701 Alameda Park Drive NE - Albuquerque, NM 87	113	
		Contact: Fred C. Arfman or Bryan J. Bobrick
Address: 128 Monroe Street NE - Albuquerque, NM 87108		Contact. 1 red C. Arman of Bryan 3. Boblick
		E-mail: freda@iacivil.com
Phone#: (505) 268-8828 Fax#:		bryanb@iacivil.com
Owner:		Contact:
Address:		
Phone#: Fax#:		E-mail:
Architect: Dekker Perich Sabatini		Contact:
Address: 7601 Jefferson Street NE, Suite 100 - Albuquerque, NM	87109	
Phone#: (505) 761-9700 Fax#:		E-mail:
Other Contact: Cartesian Surveys, Inc.		Contact: Will W. Plotner, Jr.
Address: P.O. Box 44414 - Rio Rancho, NM 87174		
Phone#: (505) 896-3050 Fax#:		E-mail:
Check all that Apply:		
DEPARTMENT:	CHECK TYPE OF	APPROVAL/ACCEPTANCE SOUGHT:
X HYDROLOGY/ DRAINAGE TRAFFIC/ TRANSPORTATION	X BUILDING PE	ERMIT APPROVAL
MS4/ EROSION & SEDIMENT CONTROL CERTIFICAT		E OF OCCUPANCY
TYPE OF SUBMITTAL:	PRELIMINAR	Y PLAT APPROVAL
ENGINEER ARCHITECT CERTIFICATION		OR SUB'D APPROVAL
CONCEDIUAL C & D DI ANI		OR BLDG. PERMIT APPROVAL
CONCEPTUAL G & D PLAN X GRADING PLAN	FINAL PLAT	
DRAINAGE MASTER PLAN	SE OF FINANCIAL GUARANTEE	
DRAINAGE MASTER FLAN DRAINAGE REPORT	· · · · · · · · · · · · · · · · · · ·	N PERMIT APPROVAL
DRAINAGE REFORT		ERMIT APPROVAL
CLOWN LOWN	SO-19 APPRO	
TRAFFIC CIRCULATION LAYOUT (TCL)		MIT APPROVAL
TRAFFIC IMPACT STUDY (TIS)	AD CERTIFICATION	
EROSION & SEDIMENT CONTROL PLAN (ESC)	WORK ORDER CLOMR/LOM	
	eboimeboin	
OTHER (SPECIFY)		MEETING
V.	OTHER (SPE	CIFY)
IS THIS A RESUBMITTAL?: X Yes No		
DATE SUBMITTED: October 26, 2017 By: Fred 0		

GENERAL NOTES

- A. ALL WORK DETAILED ON THESE PLANS, PERFORMED UNDER THIS CONTRACT SHALL BE CONSTRUCTED IN ACCORDANCE WITH THE PROJECT GEOTECHNICAL REPORT. WHERE APPLICABLE, CITY OF ALBUQUERQUE AND NMDOT STANDARDS APPLY.
- CONTRACTOR SHALL ABIDE BY ALL STATE, LOCAL, AND FEDERAL LAWS, CODES, RULES AND REGULATIONS WHICH APPLY TO THE CONSTRUCTION OF THESE IMPROVEMENTS, INCLUDING EPA AND ADA.
- ALL SUBGRADE, OVEREXCAVATION, BACKFILL, AND FILL SHALL BE PLACED AND / OR COMPACTED PER THE GEOTECHNICAL REPORT AND CITY OF ALBUQUERQUE SPECIFICATIONS.
- CONTRACTOR SHALL OBTAIN ALL REQUIRED PERMITS FOR THE PROJECT PRIOR TO COMMENCING CONSTRUCTION, OR PRIOR TO OCCUPANCY, AS APPROPRIATE.
- IT IS THE CONTRACTOR'S RESPONSIBILITY TO FIELD VERIFY HORIZONTAL AND VERTICAL LOCATIONS OF ALL EXISTING OBSTRUCTIONS, AND CONDITION OF ALL EXISTING INFRASTRUCTURE PRIOR TO CONSTRUCTION. REPORT ALL DISCREPANCIES TO THE ARCHITECT AND VERIFY THE ARCHITECT'S INTENT BEFORE PROCEEDING.
- F. CONTRACTOR SHALL BE SOLELY RESPONSIBLE FOR SITE SAFETY.
- G. CONTRACTOR SHALL MAINTAIN RECORD DRAWINGS ON SITE. . CONTRACTOR SHALL OBTAIN ALL REQUIRED INSPECTIONS OF THE WORK.
- CONSTRUCTION ACTIVITY SHALL BE LIMITED TO THE PROPERTY AND/OR PROJECT LIMITS UNLESS NOTED. ANY DAMAGE TO ADJACENT STRUCTURES RESULTING FROM THE CONSTRUCTION PROCESS SHALL BE REPAIRED OR REPLACED AT THE CONTRACTOR'S EXPENSE. CONTRACTOR SHALL BE RESPONSIBLE FOR DOCUMENTING EXISTING CONDITIONS PRIOR TO CONSTRUCTION.
- CONSTRUCTION EQUIPMENT SHALL NOT OBSTRUCT DRIVEWAYS. EQUIPMENT SHALL ONLY OBSTRUCT DESIGNATED TRAFFIC LANES IF APPROPRIATE BARRICADING PERMITS HAVE BEEN OBTAINED.
- CONTRACTOR SHALL PROVIDE A CONSTRUCTION TRAFFIC CONTROL AND SIGNING PLAN THAT CONFORMS TO THE LATEST EDITION OF THE "MANUAL OF UNIFORM TRAFFIC CONTROL DEVICES" (MUTCD) AND LOCAL REQUIREMENTS. THE CONTRACTOR SHALL OBTAIN BARRICADING PERMITS FROM THE APPROPRIATE AUTHORITIES PRIOR TO ANY CONSTRUCTION WORK ON OR ADJACENT TO EXISTING STREETS.
- CONTRACTOR SHALL MAINTAIN ALL BARRICADING AND CONSTRUCTION SIGNING AT ALL TIMES. THE CONTRACTOR SHALL VERIFY THE PROPER LOCATION OF ALL BARRICADING EACH DAY.
- M. PAVEMENT GRADES IN MARKED HANDICAPPED PARKING AREAS SHALL NOT EXCEED 2.0% IN ANY DIRECTION. FOR ALL ACCESSIBLE ROUTES, MAXIMUM ALLOWABLE CROSS SLOPE IS 2.0% AND MAXIMUM LONGITUDINAL SLOPE WITHOUT RAMP IS
- N. ALL TRASH, DEBRIS, & SURFACE VEGETATION SHALL BE CLEARED AND LEGALLY DISPOSED OF OFFSITE.
- DESCRIPTION OF THE PROPOSED SPOT AND CONTOUR ELEVATIONS SHOWN REPRESENT TOP OF FINISH MATERIAL (I.E. TOP OF CONCRETE, FINISH FLOOR OF BUILDING, TOP OF LANDSCAPE MATERIAL, ETC.). CONTRACTOR SHALL GRADE, COMPACT SUBGRADE AND DETERMINE EARTHWORK ESTIMATES BASED ON ELEVATIONS SHOWN MINUS FINISH MATERIAL THICKNESSES.
- NOTE: TO ENSURE READABILITY, NOT ALL PAVEMENT SPOT ELEVATIONS SHOW ADJACENT TOP OF CURB / TOP OF WALK. TEXT SHOWN WITHIN FLOWLINE REPRESENTS FLOWLINE ELEVATION. ADD 0.5' TYPICAL FOR TOP OF ADJACENT CURB OR WALK ELEVATIONS.
- Q. IF FIELD GRADE ADJUSTMENTS ARE REQUIRED, THE CONTRACTOR SHALL NOTIFY THE ARCHITECT.
- R. EXISTING UTILITY LINES ARE SHOWN IN AN APPROXIMATE MANNER ONLY AND MAY BE INCOMPLETE OR OBSOLETE. SUCH LINES MAY OR MAY NOT EXIST WHERE SHOWN OR NOT SHOWN. CONTRACTOR SHALL CONTACT NM-811 FOR UTILITY LINE SPOTS TWO WORKING DAYS PRIOR TO CONDUCTING SITE FIELD WORK. CONTRACTOR SHALL FIELD VERIFY AND LOCATE ALL UTILITIES PRIOR TO COMMENCEMENT OF ANY CONSTRUCTION.
- CONTRACTOR IS FULLY RESPONSIBLE FOR ANY AND ALL DAMAGE CAUSED BY ITS FAILURE TO LOCATE, IDENTIFY AND PRESERVE ANY AND ALL EXISTING UTILITIES, PIPELINES, AND UNDERGROUND UTILITY LINES. THE CONTRACTOR SHALL BE RESPONSIBLE FOR COORDINATION OF NECESSARY DRY UTILITY ADJUSTMENTS.
- CONTRACTOR SHALL PROVIDE ALL CONSTRUCTION STAKING. CONTRACTOR SHALL LOCATE AND PRESERVE ALL BOUNDARY CORNERS AND REPLACE ANY LOST OR DISTURBED CORNERS AT CONTRACTOR'S SOLE EXPENSE. PROPERTY CORNERS SHALL ONLY BE RESET BY A REGISTERED LAND SURVEYOR.
- A CURRENT STORMWATER CONTROL PERMIT, INCLUDING AN EROSION SEDIMENT CONTROL PLAN (E.S.C.) IS REQUIRED FOR ALL CONSTRUCTION, DEMOLITION CLEARING, AND GRADING OPERATIONS THAT DISTURB THE SOIL ON ONE ACRE OR MORE OF LAND. OWNER WILL COORDINATE.
- POST-CONSTRUCTION MAINTENANCE FOR PRIVATE STORMWATER FACILITIES WILL BE THE RESPONSIBILITY OF THE FACILITIES
- . ADJUST ANY RIMS OF EXISTING UTILITY FEATURES AS NECESSARY TO MATCH NEW GRADES. UTILITIES IN PAVED AREAS SHALL BE HS-25 TRAFFIC RATED.
- N. PAVING AND ROADWAY GRADES SHALL BE ±0.1' FROM PLAN ELEVATIONS. BUILDING FINISH FLOOR ELEVATION SHALL BE ±0.05' FROM PLAN ELEVATION.
- WHERE GRADES BETWEEN NEW AND EXISTING ARE SHOWN AS 'MATCH' OR '±', TRANSITIONS SHALL BE SMOOTH.
- ALL EROSION CONTROL TO BE ANGULAR ROCK (F.F. ROCK) DEFINED AS 6" DEEP X 3" AVG. DIA. ANGULAR FACED ROCK PLACED OVER GEOTEX 501 NON-WOVEN GEOTEXTILE (O.E.).
- SIDESLOPES STEEPER THAN 3:1 BUT LESS THAN 2:1 MUST HAVE PERMANENT EROSION CONTROL (F.F. ROCK O.E.) INSTALLED, TYPICAL. NO SLOPE SHALL BE STEEPER THAN 2:1.
- AA. ENGINEER RECOMMENDS THAT OWNER INSPECT SITE YEARLY AND AFTER EACH RAINFALL TO IDENTIFY NEW AREAS OF EROSION AND INSTALL ADDITIONAL EROSION PROTECTION AS NEEDED BASED ON ACTUAL OCCURRENCES.
- AB. DEPRESS LANDSCAPING FOR WATER HARVESTING. NOTE: NO WATER HARVESTING SHALL OCCUR WITHIN 10' OF BUILDING.

CONSTRUCTION STAKING

UPON WRITTEN REQUEST COORDINATED THROUGH THE PROJECT ARCHITECT, THE ELECTRONIC FILE OF THE GRADING AND DRAINAGE WILL BE PROVIDED TO THE CONTRACTOR FOR VERTICAL CONTROL. DO NOT USE THIS PLAN FOR PROJECT STAKING.

SITE CONSTRUCTION LAYOUT / STAKING SHALL BE COORDINATED WITH THE ARCHITECT USING THE ARCHITECT PROVIDED SITE PLAN.

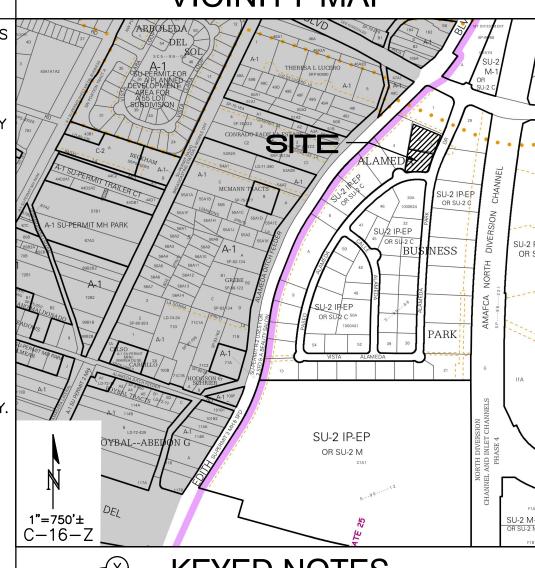
LEGEND

EXISTING SPOT ELEVATION EXISTING CONTOUR PROPOSED CONTOUR PROPOSED SPOT ELEVATION FLOW ARROW FINISH FLOOR ELEVATION

PROPOSED WATER BAR

FIRST FLUSH PONDING LIMITS LIMITS OF EROSION CONTROL

VICINITY MAP



KEYED NOTES

CONSTRUCT PROPOSED PAVING / WALKS / CROSSWALKS / CURB AND GUTTER TO ELEVATIONS SHOWN.

EXISTING PAVING TO REMAIN. CONSTRUCT HC PARKING AREA TO ADA STANDARDS, MAX. 2%

SLOPE IN ANY DIRECTION.

CONSTRUCT ADA COMPLIANT HANDICAP ACCESS RAMP AT ELEVATIONS SHOWN (1" PER FOOT - LENGTH VARIES). MAX. 2% CROSS-SLOPE. SEE ARCHITECTURAL FOR ADDITIONAL

TOP OF ASPHALT TO BE FLUSH WITH TOP OF CONCRETE WALK THIS AREA. SEE ARCHITECTURAL FOR CONCRETE PARKING BUMPER LOCATIONS (TYP).

PROVIDE 24" WIDE CURB OPENING AT FLOWLINE SHOWN. SEE DETAIL SHEET CG-501.

INSTALL ROCK EROSION PROTECTION AT CURB OPENING AND WITHIN FLOWLINES CARRYING CONCENTRATED FLOW (VARY FROM 2'-3' WIDE). LIMITS HATCHED PER LEGEND. SEE DETAIL SHEET CG-501.

DOT HATCHED AREA REPRESENTS EXTENTS OF 'FIRST FLUSH' RETENTION PONDING. CONSTRUCT TO ELEVATIONS SHOWN. 9. CONSTRUCT CONCRETE RUNDOWN.

10. CONSTRUCT NEW CONCRETE DUMPSTER PAD AND ENCLOSURE AT ELEVATIONS SHOWN.

11. EXTENDED BUILDING STEMWALL REQUIRED THIS AREA. SEE ARCHITECTURAL FOR ADDITIONAL INFORMATION.

2. ROOF DRAIN TO DISCHARGE TO LANDSCAPE AREA. PROVIDE EROSION CONTROL AT OUTLET (CONCRETE SPLASHPAD O.E.). INSTALL DRAINAGE SWALE WITH ANGULAR ROCK EROSION CONTROL TO EXTENTS SHOWN. SEE PLUMBING PLAN FOR SPECIFIC ROOF DRAIN LOCATIONS.

PROJECT DATA

PROPERTY: THE SITE IS AN UNDEVELOPED COMMERCIAL PROPERTY WITHIN C.O.A. VICINITY MAP C-16. THE SITE IS BOUND TO THE WEST AND NORTH BY DEVELOPED COMMERCIAL TO THE EAST BY ALAMEDA PARK DR. NE AND TO THE SOUTH BY PASEO ALAMEDA NE.

PROPOSED IMPROVEMENTS: THE PROPOSED IMPROVEMENTS INCLUDE AN APPROXIMATELY 14,000 SF BUILDING (FOOTPRINT) WITH DOCK, PAVED PARKING, PEDESTRIAN WALKS, DRAINAGE IMPROVEMENTS, AND LANDSCAPING.

LEGAL DESCRIPTION: LOT 2 AND 3 ALAMEDA BUSINESS PARK CITY OF ALBUQUERQUE. BERNALILLO COUNTY, NEW MEXICO.

<u>SITE AREA:</u> 1.7676 AC.

BENCHMARK: ALBUQUERQUE CONTROL SURVEY BENCHMARK "NM 47-2", ELEVATION = 4997.592 (NAVD 1988)

OFF-SITE: LOT 1, ALAMEDA BUSINESS PARK DISCHARGES 6.72 CFS (FULLY DEVELOPED DISCHARGE) TO THE EXISTING DRAINAGE CHANNEL RUNNING NORTH TO SOUTH ALONG THE WEST PROPERTY BOUNDARY.

STORMWATER CONTROL MEASURES ARE REQUIRED TO PROVIDE MANAGEMENT OF 'FIRST FLUSH' DEFINED AS THE 90TH PERCENTILE STORM EVENT OR 0.34" [0.44" LESS 0.1" FOR INITIAL ABSTRACTION OF STORMWATER WHICH DISCHARGES DIRECTLY TO A PUBLIC STORM DRAINAGE SYSTEM.

FIRST FLUSH RETENTION PONDS WILL BE CONSTRUCTED WITHIN THE LANDSCAPE AREAS AS DESIGNATED BY DOT HATCH. STORM WATER FROM THE IMPERVIOUS AREAS SHALL BE DIRECTED TO THESE PONDS. STORMWATER WILL THEN FREE DISCHARGE TO MCMAHON BLVD. TO FOLLOW THE HISTORIC FLOWPATHS.

FLOOD ZONE: THE SUBJECT PROPERTY APPEARS TO LIE WITHIN ZONE "X" (AREAS DETERMINED TO BE OUTSIDE 0.2% ANNUAL CHANCE FLOODPLAIN) IN ACCORDANCE WITH THE NATIONAL FLOOD INSURANCE PROGRAM RATE MAP NO. 3500136 G,

EFFECTIVE DATE 9-26-2008.

ENGINEER: FRED C. ARFMAN, PE ISAACSON & ARFMAN, P.A. 128 MONROE ST NE, ABQ. NM 87108

(505) 896-3050

PHONE: (505) 268-8828 WILL W. PLOTNER, JR, NMPS NO. 14271 CARTESIAN SURVEYS INC. P.O. BOX 44414 RIO RANCHO, NM 87174 PHONE:

DRAINAGE PLAN

ISAACSON & ARFMAN, P.A. **Consulting Engineering Associates** 128 Monroe Street N.E. Albuquerque, New Mexico 87108 Ph. 505-268-8828 www.iacivil.com

SHEET NO. CG-101

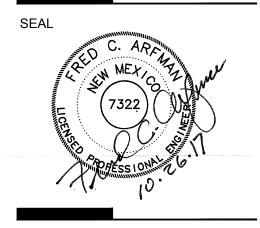
7601 JEFFERSON NE, SUITE 100 ALBUQUERQUE, NM 87109 505.761.9700 / DPSDESIGN.ORG

ARCHITECTURE / DESIGN / INSPIRATION

DEKKER

PERICH

SABATINI



PROJECT

0

CONSTRUCTION

DOCUMENTS **REVISIONS**

DRAWN BY

REVIEWED BY 10/17/2017 16-0113 PROJECT NO.

DRAWING NAME **GRADING &**

DRAINAGE SUB-BASINS FOR FIRST FLUSH RETENTION
Basin 2 5,489 sf Basin 3 30,954 sf Basin 4 20,279 4st Basin 7 237 sf

BASIN NO. 1		DESCRIPTION	Draining to Pond P1	PO	POND P1	
Area of basin flows =	13508		0.3 Ac.	Contour Area	Volume	
The following calculation		Treatment areas as shown in table to the right	LAND TREATMENT	5044.00 440		
	Sub-basin Weigl	hted Excess Precipitation (see formula above)	A = 0%	5042.50	339 CF	
	Weighted E	= 1.86 in.	B= 8%		339 CF	
	Sub-basin Volum	ne of Runoff (see formula above)	C = 15%	TOTAL MOI	220 GE	
	V360	= 2099 CF	D = 77%	TOTAL VOL.	339 CF	
	Sub-basin Peak I	Discharge Rate: (see formula above)	FIRST FLUSH VOL.			
	QP	= 1.3 cfs	295 CF			
BASIN NO. 2		DESCRIPTION	Draining to Pond P2	POI	ND P2	
Area of basin flows =	5489	SF =	0.1 Ac.	Contour Area	Volume	
The following calculation	ons are based on	Treatment areas as shown in table to the right	LAND TREATMENT	5044.00 440		
		hted Excess Precipitation (see formula above)	A = 0%	5042.50	339 CF	
	Weighted E		B = 10%		339 CI	
	Sub-basin Volun	ne of Runoff (see formula above)	C = 15%	TOTAL YEAR	220 67	
	V360		D = 75%	TOTAL VOL.	339 CF	
	Sub-basin Peak I	Discharge Rate: (see formula above)	FIRST FLUSH VOL.			
	Qp	= 0.5 cfs	117 CF			
BASIN NO. 3		DESCRIPTION	Draining to Pond P3	PO	POND P3	
Area of basin flows =	30954		0.7 Ac.	Contour Area	Volume	
The following calculation		Treatment areas as shown in table to the right	LAND TREATMENT	5038.50 748		
		hted Excess Precipitation (see formula above)	A = 0%	5038.00	326 CF	
	Weighted E		B = 8%	5037.00 255	405 CF	
	V ₃₆₀	ne of Runoff (see formula above) = 4962 CF	C = 9%		403 CF	
		Discharge Rate: (see formula above)	D = 83% FIRST FLUSH VOL.	TOTALLIO		
	Q _P	= 3.1 cfs	728 CF	TOTAL VOL.	730 CF	
BASIN NO. 4		DESCRIPTION	Draining to Pond P4	POND P4		
Area of basin flows =	20279		0.5 Ac.	Contour Area	Volume	
		Treatment areas as shown in table to the right	LAND TREATMENT	5039.00 916	Volume	
Č		hted Excess Precipitation (see formula above)	A = 0%		524 CE	
	Weighted E	= 1.72 in.	B= 17%	5038.00 132	524 CF	
		ne of Runoff (see formula above)	C = 17%			
	V360		D= 66%	TOTAL VOL.	524 CF	
		Discharge Rate: (see formula above)	FIRST FLUSH VOL.	· · · · · · · · · · · · · · · · · · ·		

ON-SITE DRAINAGE SUB-BASIN CALCULATIONS Existing Entry Drive - Free Discharge Area of basin flows = 4303 SF The following calculations are based on Treatment areas as shown in table to the right LAND TREATMENT Sub-basin Weighted Excess Precipitation (see formula above) Weighted E = B = 15%1.67 in. Sub-basin Volume of Runoff (see formula above) C = 25%599 CF D = 60% $V_{360} =$ Sub-basin Peak Discharge Rate: (see formula above) FIRST FLUSH VOL. $Q_P =$ 0.4 cfs **DESCRIPTION** Existing Asphalt Rundown - Free Discharge BASIN NO. 6 Area of bas in flows = 2200 SF LAND TREATMENT The following calculations are based on Treatment areas as shown in table to the right Sub-basin Weighted Excess Precipitation (see formula above) Weighted E = 2.12 in. B = 0%Sub-basin Volume of Runoff (see formula above) C = 0%D = 100% $V_{360} =$ 389 CI Sub-basin Peak Discharge Rate: (see formula above) FIRST FLUSH VOL. 62 CF $O_P =$ 0.2 cfs BASIN NO. 7 DESCRIPTION New Entry Drive - Free Discharge Area of basin flows = LAND TREATMENT The following calculations are based on Treatment areas as shown in table to the right Sub-basin Weighted Excess Precipitation (see formula above) A = 0%B = 0%Weighted E = 2.12 in. C = 0% Sub-basin Volume of Runoff (see formula above) D = 100% $V_{360} =$ Sub-basin Peak Discharge Rate: (see formula above) FIRST FLUSH VOL. $Q_P =$ 7 CF

> THE FIRST FLUSH VOLUME FOR BASINS 1 THRU 4 ARE CAPTURED WITHIN THE ASSOCIATED PONDS (P1 THRU P4).

> BASIN 5 IS A PORTION OF AN EXISTING ASPHALT DRIVE THAT WILL CONTINUE TO FREE DISCHARGE TO ALAMEDA PARK DRIVE NE (APPROX. 1200 SF OF THIS ENTRY DRIVE IS BEING DEMOLISHED AS PART OF THE PROPSED CONSTRUCTION). BASIN 6 IS AN EXISTING ASPHALT RUNDOWN THAT WILL CONTINUE TO FREE DISCHARGE TO PASEO ALAMEDA NE. BASIN 7, CONSISTING OF THE NEW SOUTH ENTRY DRIVE (237 SF OF IMPERVIOUS AREA) IS OFF-SET BY THE 1200 SF OF EXISTING ASPHALT PAVEMENT THAT IS BEING REMOVED AT THE NORTHEAST DRIVE.

> BASED ON THIS ANALYSIS, THERE IS NO FIRST FLUSH VOLUME THAT WILL BYPASS OTHER THAN THE EXISTING PAVEMENT AND THE SOUTH ENTRY DRIVE THAT WILL BE OFF-SET BY THE EXISTING PAVEMENT BEING REMOVED.

OVERALL CALCULATIONS - 100-YEAR 6-HOUR STORM

CALCULATIONS: 2214 - Industrial Water Engineering (Alameda Office/Warehouse): 09-11-17

Based on Drainage Design Criteria for City of Albuquerque Section 22.2, DPM, Vol 2, dated Jan., 1993 ON-SITE AREA OF SITE: 76978 SF = 1.8 100-year, 6-hour DEVELOPED FLOW FREE DISCHARGE MAX. **EXCESS PRECIP:** Treatment SF % Precip. Zone Treatment SF % Area A Area A 0 0% $E_A = 0.53$ 9237.36 | 12% 8199 $E_{\rm B} = 0.78$ Area B Area B 11% 8467.58 11% 10159 | 13% | $E_{\rm C} = 1.13$ Area C Area C $E_D = 2.12$ 59273.06 77% 58620 | 76% |

On-Site Weighted Excess Precipitation (100-Year, 6-Hour Storm) Weighted E = $E_A A_A + E_B A_B + E_C A_C + E_D A_D$ $A_A + A_B + A_C + A_D$ 1.85 in. Developed E =1.85 in. Historic E =

On-Site Volume of Runoff: V360 = E*A / 12 11869 CF Developed $V_{360} =$ On-Site Peak Discharge Rate: $Qp = Q_{pA}A_A + Q_{pB}A_B + Q_{pC}A_C + Q_{pD}A_D / 43,560$

For Precipitation Zone 2 $Q_{pA} = 1.56$ $Q_{pC} = 3.14$ $Q_{pB} = 2.28$ $Q_{pD} = 4.70$ 7.5 CFS Developed $Q_p =$ Historic Q_p

THE FULLY DEVELOPED PROPERTY WILL DISCHARGE 7.5 CFS (MAXIMUM) DURING THE 100-YEAR 6-HOUR STORM.

THIS SITE IS LOCATED WITHIN THE ALAMEDA BUSINESS PARK (MASTER DRAINAGE PLAN 'MDP'

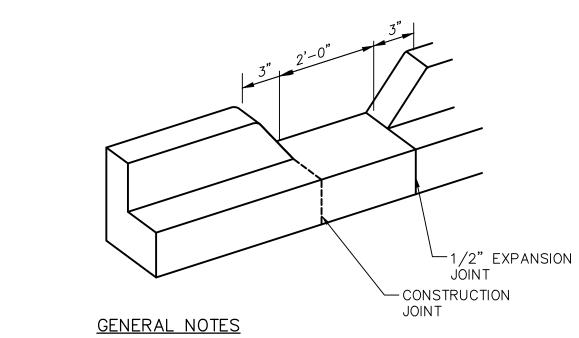
EXISTING CONDITION: THE SITE IS AN UNDEVELOPED TRACT THAT WAS MASS GRADED AS PART OF THE ALAMEDA BUSINESS PARK DEVELOPMENT. THE SITE SLOPES FROM THE NORTHEAST TO THE SOUTHWEST. PER THE MDP, THIS PROPERTY IS PERMITTED FREE DISCHARGE. RETENTION / DETENTION IS PROVIDED WITHIN A POND LOCATED AT THE EASTERN BOUNDARY OF THE OVERALL BUSINESS PARK DEVELOPMENT.

WEST EXISTING DRAINAGE CHANNEL: PER THE DRAINAGE REPORT FOR LOT 1 - ALAMEDA BUSINESS PARK, PREPARED BY RIO GRANDE ENGINEERING, DATED APRIL 2008, (C16/D6U1), LOT 1 FREE DISCHARGES 6.72 CFS TO THE EXISTING DRAINAGE CHANNEL LOCATED ALONG THE WEST PROPERTY LINE. THE EXISTING CHANNEL AT 4' WIDE X 6" HIGH, HAS A CAPACITY

PROPOSED CONDITION: THE PROPOSED IMPROVEMENTS INCLUDE A 14,000± SF BUILDING WITH DOCK, PAVED PARKING, PEDESTRIAN ACCESS AND ASSOCIATED LANDSCAPING. THE SITE WILL BE GRADED TO DISCHARGE DEVELOPED FLOW FROM IMPERVIOUS AREAS TO FIRST FLUSH RETENTION BASINS LOCATED THROUGHOUT THE SITE. ONCE THESE FILL, EXCESS WILL FREE

DISCHARGE. THE DEVELOPED SITE CONSISTS OF A SINGLE DRAINAGE BASIN WITH ALL FLOW DISCHARGE SOUTH TO PASEO ALAMEDA NE. THE INTERIOR OF THE PROPERTY HAS BEEN DIVIDED INTO 10 DRAINAGE SUB-BASINS FOR FIRST FLUSH CALCULATIONS (SEE DRAINAGE SUB-BASINS AND

ASSOCIATED CALCULATIONS THIS SHEET.)

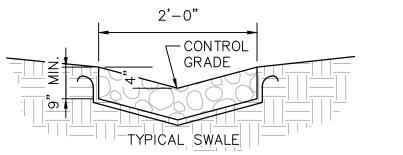


1. EDGES NOT SPECIFICALLY DIMENSIONED SHALL BE SHAPED WITH A 3/8" EDGING TOOL.

CURB OPENING

SCALE: N.T.S.

- VARY ANGULAR FACE ROCK SIZE BETWEEN 2" AND 4" DIA. (AVG.=3")
- PLACE GEOTEX 501 NON-WOVEN GEOTEXTILE (O.E.) BENEATH ALL EROSION PROTECTION
- CONSTRUCT ALL EROSION PROTECTION INSET INTO (NOT ON TOP OF) GRADE TO ENSURE RUNOFF CAN BE CAPTURED AND CONVEYED PROPERLY



ROCK EROSION PROTECTION

SCALE: N.T.S.

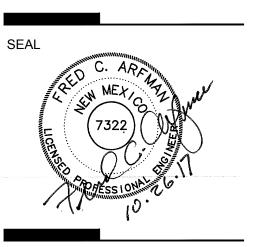
ISAACSON & ARFMAN, P.A. Consulting Engineering Associates 128 Monroe Street N.E. Albuquerque, New Mexico 87108 Ph. 505-268-8828 www.iacivil.com

ARCHITECTURE / DESIGN / INSPIRATION

DEKKER PERICH

7601 JEFFERSON NE, SUITE 100 ALBUQUERQUE, NM 87109

505.761.9700 / DPSDESIGN.ORG



PROJECT

gineering Water 8701 Alameda F Albuquerque,

100% CONSTRUCTION DOCUMENTS

lustrial

pu

REVISIONS
\triangle
DRAWN BY
REVIEWED BY

DRAWING NAME

DATE

PROJECT NO.

GRADING AND DRAINAGE CALCS AND DETAILS

10/17/2017

16-0113

SHEET NO. CG-501