

City of Albuquerque

P.O. BOX 1293 ALBUQUERQUE, NEW MEXICO 87103

MAYOR

KEN SCHULTZ

CHIEF ADMINISTRATIVE OFFICER

DEPUTY CAO PUBLIC SERVICES

DEPUTY CAO PLANNING/DEVELOPMENT

GENE ROMO

FRANK MARTINEZ

BILL MUELLER

January 21, 1988

Jefff Mortensen, P.E. Tom Mann & Associates, Inc. 811 Dallas, NE Albuquerque, New Mexico 87110

> REVISED DRAINAGE PLAN FOR LOT 33-A OF CLIFFORD INDUSTRIAL PARK (C-17/D1A5) REVISION DATE OF JANUARY 18, 1988

Dear Mr. Mortensen:

Based on the information provided on your resubmittal of January 18, 1988, revisions as indicated are acceptable.

Please attach a copy of this plan to the construction sets prior to sign-off by Hydrology.

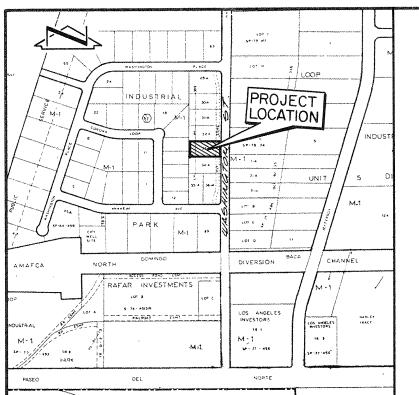
If I can be of further assistance, please feel free to call me at 768-2650.

Cordially,

Bernie J. Montoya, C.E.

For Engineering Assistant

BJM/bsj



PROJECT BENCHMARK

A.C.S. STATION 8-CIT, BEING A STD. BRAGO TABLET, STAMPED'8-CIT, 1915' CEMENTED IN TOP OF CURB. STATION 19 LOCATED ON LUEST CURB OF WASHINGTON ST. N.E. D.3 MI. NORTH OF INTERSECTION OF LOS ANGECES ST. N.E. & WASHINGTON ST. N.E., 45 SHOWN HEREON.

STATION ELEVATION = 5111.01 FEET (M.S.L.D.)

LEGAL DESCRIPTION

LOT 33-A, CLIFFORD INDUSTRIAL PARK ALBUQUERQUE NEW MEXICO



VICINITY MAP SCALE : 1" = 800'

LEGEND PROPERTY LINE --08- EXISTING CONTOUR

C - 17

OB PROPOSED CONTOUR \$ 4.08 EXIST, SPOT ELEVATION \$3.98 PROPOSED SPOT ELEV. T.C. / F.L. TOP OF CURB/FLOW CINE TOP OF ASPHACT EXISTING SWALE

PROPOSED SWALE PROPOSED CONCRETE PROPOSED ASPHACT WWW PROPOSED WATERBLOCK

LOT 32-A - EXISTING ASPHACT BLOCKWALL (SHALL BE WELL GRADGD, I"MAXIMYN CURB & GUTTER GRADE GWALE, TYP. FOUND REBUR T.C.11.10 S 89°33'39" £ SOB GRAVEL M . 09 ROOF DRAINAGE --10' PUBLIC UTILITY EGMT. PROPOSED 09.40 2-STORY PROPOSED OFFICE 09.40 HIGH BAY F.F. = 5/09.50 645 MH RIM=5111.08 WAREHOUSE F.F. = 5109.50 LOT 16 - MATCH \EXIST. CHADE CONSTRUCT ~ GRAVEI (SHALL BE WELL GRADED SWALE, TYP. Ø√09 - NEW DRIVERAD EXIGTING 1 ELEVATION = 5/1/0/FT FOUND REBAR =11.00 ^{25.5}♦♦< LOT 15 CONSTRUCT CONSTRUCT 1'-O" CURB OPENING / 1933 N 89° 33′ 39″ W 250.00 \$ \$ 14 DIRTÉ ASPHACT LOT 34-A

DRAINAGE PLAN

The following items concerning the Lot 33-A Clifford Industrial Park Drainage Plan are contained hereon: 1. Vicinity Map 2. Grading Plan

3. Calculations

As shown by the Vicinity Map, this site is located on the west side of Washington Street N.E. between Washington Place N.E. and Anaheim Avenue N.E. At present, the site is partially developed with some minor paving and an asphalt drainage swale located on the west side of the site. Much of the surrounding area is currently undergoing similar development. As shown by Panel 9 of the Federal Emergency Management Agency Flood Boundary and Floodway Map dated October 14, 1983, the site does not lie within a designated Flood Hazard Zone. Furthermore, downstream flooding is not evident and therefore does not appear to be a problem. In addition, an approved master drainage report for Clifford Industrial Park prepared by Bohannan-Huston, Inc. dated August, 1982, allows for the free discharge of runoff onto the existing asphalt swale. No offsite flows enter the west, north and south property lines since the existing lots are graded in a manner which will route runoff away from the project site. No offsite flows enter the site along the east property line because the existing road street routes runoff away from the project site.

The Grading Plan shows 1) existing grades indicated by spot elevations and contours at 1'0" intervals, 2) continuity between existing and proposed grades, and 3) the limit and character of the proposed improvements. As shown by this plan, the proposed improvements consist of the construction of a new office warehouse building along with adjacent paving and landscaping. Flows generated by the proposed improvements will be routed from east to west onto the aforementioned asphalt swale. From that point, runoff will flow in a southerly direction onto Anaheim Avenue N.E. This is in compliance with the previously approved master drainage report.

The Calculations which appear hereon analyze both the existing and developed conditions for the 100-year, 6-hour rainfall event. The Rational Method has been used to quantify the peak rate of discharge and the SCS Method has been used to quantify the volume of runoff. Both Methods have been used in accordance with the City of Albuquerque Development Process Manual, Volume II, and the Mayor's Emergency Rule adopted January 14, 1986. As shown by these calculations, the proposed improvements will result in a net increase in peak runoff by approximately 1.3 cfs.

CALCULATIONS

Ground Cover Information

From SCS Bernalillo County Soil Survey, Plate 11: EmB - Embudo Complex Hydrologic Soil Group: B Existing Pervious CN = 70 (DPM Plate 22.2 C-2 Pasture or Range Land: fair condition) Developed Pervious CN = 61 (DPM Plate 22.2 C-2)

Time of Concentration/Time to Peak

 $T_C = 0.0078 L^{0.77}/S^{0.385}$ (Kirpich Equation)

 $T_D = T_C = 10 \text{ min.}$

Point Rainfall

 $P_6 = 2.2$ in. (DPM Plate 22.2 D-1)

Rational Method

Discharge: Q = CiA

where C varies $i = P_6 (6.84) T_C^{-0.51} = 4.65 in/hr$ $P_6 = 2.2$ in (DPM Plate 22.2D-1) $T_{\rm C} = 10 \text{ min} \text{ (minimum)}$ A = area, acres

SCS Method

Volume: V = 3630(DRO) A

Where DRO = Direct runoff in inches

A = area, acres

Existing Condition

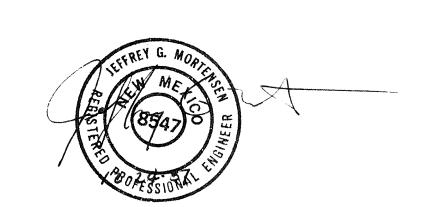
 $A_{total} = 29,885 \text{ sf} = 0.69 Ac$ Paved area = 4.780 sf (0.16) Dirt area = 25,105 sf (0.84)C = 0.49 (Weighted average per Emergency Rule, 1/14/86) $Q_{100} = CiA = (0.49)(4.65)(0.69) = 1.6 cfs$ $A_{imp} = 4,780 \text{ sf}; % impervious = 16 %$ Composite CN = 74 (DPM Plate 22.2 C-3) DRO = 0.48 in (DPM Plate 22.2 C-4) $V_{100} = 3630 (DRO)A = 1,200 cf$

Developed Condition

 $A_{total} = 29,885 \text{ sf} = 0.69 \text{ Ac}$ Roof area = 5,200 sf (0.17) Paved area = 22,435 sf (0.75) Landscaped area = 2,250 sf (0.08) C = 0.89 (Weighted average per Emergency Rule, 1/14/86) $Q_{100} = CiA = (0.89)(4.65)(0.69) = 2.9 cfs$ Aimp = 27,635 sf; % impervious = 92 % Composite CN = 95.5 (DPM Plate 22.2 C-3) DRO = 1.7 in (DPM Plate 22.2 C-4) $V_{100} = 3630$ (DRO) A = 4,260 cf

Comparison

 $\Delta Q_{100} = 2.9 - 1.6 = 1.3 \text{ cfs (increase)}$ $\Delta V_{100} = 4,260 - 1,200 = 3,060 \text{ cf (increase)}$

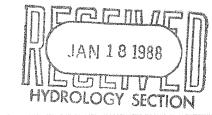


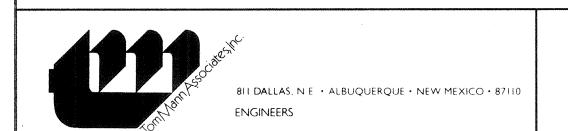
CONSTRUCTION NOTES:

- 1. TWO (2) WORKING DAYS PRIOR TO ANY EXCAVATION, CONTRACTOR MUST CONTACT LINE LOCATING SERVICE 765-1234, FOR LOCATION OF EXISTING UTILITIES.
- 2. PRIOR TO CONSTRUCTION, THE CONTRACTOR SHALL EXCAVATE AND VERIFY THE HORIZONTAL AND VERTICAL LOCATION OF ALL POTENTIAL OBSTRUCTIONS. SHOULD A CONFLICT EXIST, THE CONTRACTOR SHALL NOTIFY THE ENGINEER SO THAT THE CONFLICT CAN BE RESOLVED WITH A MINIMUM AMOUNT OF DELAY.
- 3. ALL WORK ON THIS PROJECT SHALL BE PERFORMED IN ACCORDANCE WITH APPLICABLE FEDERAL, STATE AND LOCAL LAWS, RULES AND REGULATIONS CONCERNING CONSTRUCTION SAFETY AND HEALTH.
- 4. ALL CONSTRUCTION WITHIN PUBLIC RIGHT-OF-WAY SHALL BE PERFORMED IN ACCORDANCE WITH APPLICABLE CITY OF ALBUQUERQUE STANDARDS AND PROCEDURES.
- 5. IF ANY UTILITY LINES, PIPELINES, OR UNDERGROUND UTILITY LINES ARE SHOWN ON THESE DRAWINGS, THEY ARE SHOWN IN AN APPROXIMATE MANNER ONLY, AND SUCH LINES MAY EXIST WHERE NONE ARE SHOWN. IF ANY SUCH EXISTING LINES ARE SHOWN, THE LOCATION IS BASED UPON INFORMATION PROVIDED BY THE OWNER OF SAID UTILITY, AND THE INFORMATION MAY BE INCOMPLETE, OR MAY BE OBSOLETE BY THE TIME CONSTRUCTION COMMENCES. THE ENGINEER HAS UNDERTAKEN NO FIELD VERIFICATION OF THE LOCATION, DEPTH, SIZE, OR TYPE OF EXISTING UTILITY LINES, PIPELINES, OR UNDERGROUND UTILITY LINES, MAKES NO REPRESENTATION PERTAINING THERETO, AND ASSUMES NO RESPONSIBILITY OR LIABILITY THEREFOR. THE CONTRACTOR SHALL INFORM ITSELF OF THE LOCATION OF ANY UTILITY LINE, PIPELINE, OR UNDERGROUND UTILITY LINE IN OR NEAR THE AREA OF THE WORK IN ADVANCE OF AND DURING EXCAVATION WORK. THE CONTRACTOR IS FULLY RESPONSIBLE FOR ANY AND ALL DAMAGE CAUSED BY ITS FAILURE TO LOCATE, IDENTIFY AND PRESERVE ANY AND ALL EXISTING UTILITIES, PIPELINES, -AND UNDERGROUND UTILITY LINES. IN PLANNING AND CONDUCTING EXCAVATION, THE CONTRACTOR SHALL COMPLY WITH STATE STATUTES, MUNICIPAL AND LOCAL ORDINANCES, RULES AND REGULATIONS, IF ANY, PERTAINING TO THE LOCATION OF THESE LINES AND FACILITIES.

EROSION CONTROL MEASURES

- 1. THE CONTRACTOR SHALL ENSURE THAT NO SOIL ERODES FROM THE SITE INTO PUBLIC RIGHT-OF-WAY OR ONTO PRIVATE PROPERTY. THIS CAN BE ACHIEVED BY CONSTRUCTING TEMPORARY BERMS AT THE PROPERTY LINES AND WETTING THE SOIL TO KEEP IT FROM BLOWING.
- 2. THE CONTRACTOR SHALL PROMPTLY CLEAN UP ANY MATERIAL EXCAVATED WITHIN THE PUBLIC RIGHT-OF-WAY SO THAT THE EXCAVATED MATERIAL IS NOT SUSCEPTIBLE TO BEING WASHED DOWN THE STREET.
- 3. THE CONTRACTOR SHALL SECURE "TOPSOIL DISTURBANCE PERMIT" PRIOR TO BEGINNING CONSTRUCTION.





GRADING AND DRAINAGE PLAN LOT 33-A, CLIFFORD INDUSTRIAL PARK

JOB NO. 871381 IAU. 88 L.P.U. SPECIFY TYPE OF GRAVEL & PROVIDE DATE 12-1987 WATER BLOCK @ ENTRANCE J.G.M.