

FLOOD INSURANCE RATE MAP

MAP NO. 3501C0136 D

GENERAL NOTES:

- 1: CONTOUR INTERVAL IS ONE (1) FOOT.
- 2: ELEVATIONS ARE BASED ON CITY OF ALBUQUERQUE STATION No. "12-C17", HAVING AN ELEVATION OF 5107.95.
- 3: UTILITIES SHOWN HEREON ARE IN THEIR APPROXIMATE LOCATION BASED ONLY ON ABOVE GROUND EVIDENCE FOUND IN THE FIELD AND AS-BUILT INFORMATION PROVIDED BY THE CLIENT. UTILITIES SHOWN HEREON, WHETHER INDICATED AS ABANDONED OR NOT, SHALL BE VERIFIED BY OTHERS FOR EXACT LOCATION AND/OR DEPTH PRIOR TO EXCAVATION OR DESIGN CONSIDERATIONS.
- 5: THIS IS NOT A BOUNDARY SURVEY. BEARINGS AND DISTANCES SHOWN HEREON ARE FOR REFERENCE ONLY.

LEGEND:

EXISTING	NEW	DESCRIPTION
5080	80	CONTOUR
80.00	80.00	SPOT ELEVATION
	AS-CONST. SPOT ELEV. (NEW LOCATION)	
	AS-CONST. SPOT ELEV. (EXIST. LOCATION)	
		PROPERTY LINE
		SWALE
		SHEET FLOW
		ROOF FLOW
		ROOF GUTTER
		DOWNSPOUT
		LEGEND OF ABBREVIATIONS:
		TC = TOP OF CONCRETE
		FL = FLOW LINE
		TG = TOP OF GRADE
		EOW = END OF WALL

LEGAL DESCRIPTION:

LOT 49, CLIFFORD INDUSTRIAL PARK, ALBUQUERQUE, NEW MEXICO.

CERTIFICATION WITH VERIFICATION BY ENGINEER OF RECORD

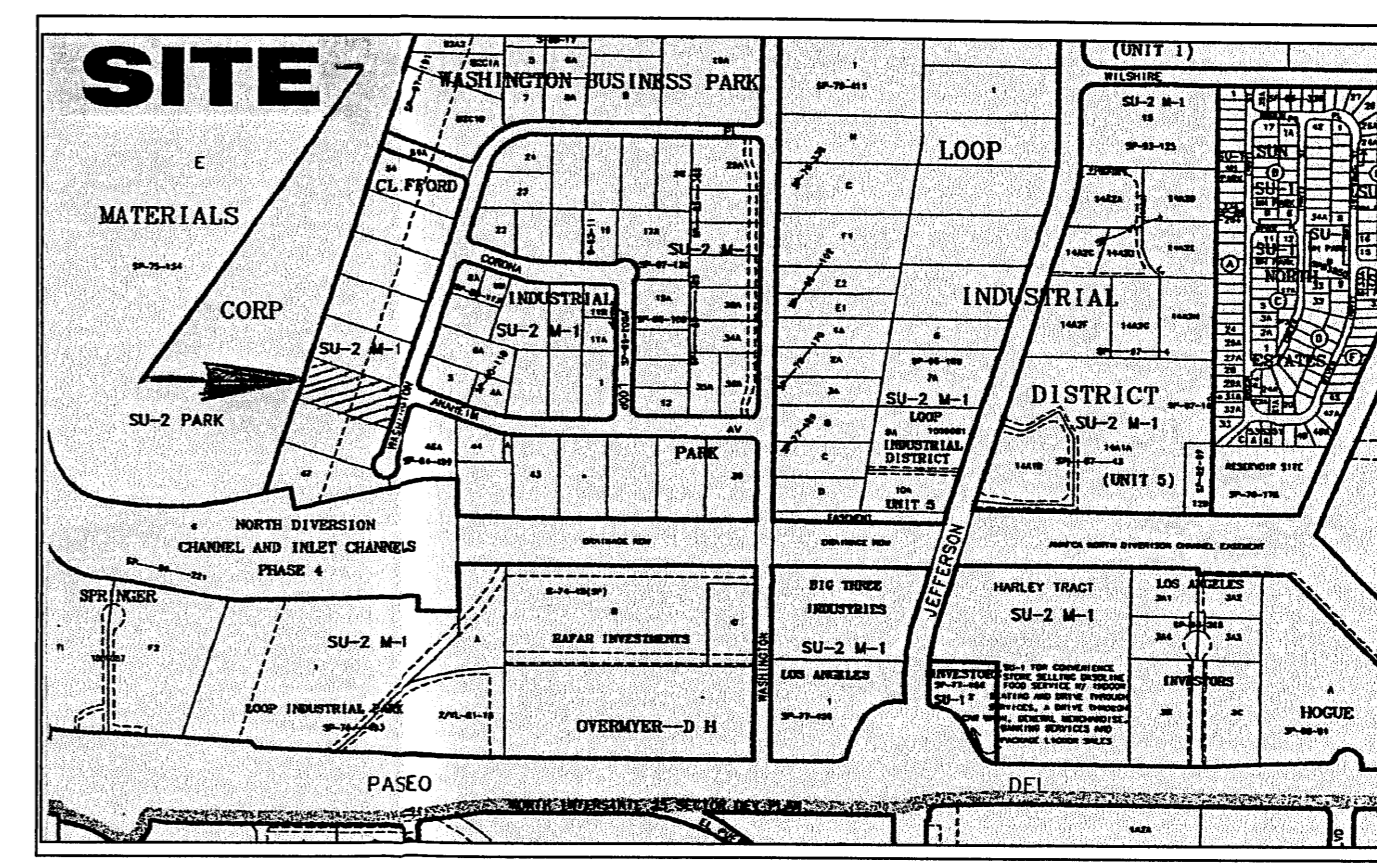
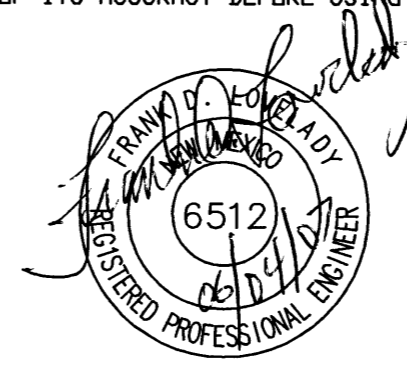
I, FRANK D. LOVELADY, NMPE 6512 OF THE FIRM FRANK D. LOVELADY P.C. HEREBY CERTIFY THAT THIS PROJECT HAS BEEN GRADED AND WILL DRAIN IN SUBSTANTIAL COMPLIANCE WITH AND IN ACCORDANCE WITH THE DESIGN INTENT OF THE APPROVED PLAN DATED 01-15-2004. THE RECORD INFORMATION EDITED ONTO THE ORIGINAL DESIGN DOCUMENT HAS BEEN OBTAINED BY HARRIS SURVEYING COMPANY WITH THE EXCEPTION OF THE SHOTS ALONG THE CHANNEL WALL WHICH WERE OBTAINED BY ME.

I FURTHER CERTIFY THAT I HAVE PERSONALLY VISITED THE PROJECT SITE ON 06-01-07 AND HAVE DETERMINED BY VISUAL INSPECTION THAT THE SURVEY DATA PROVIDED IS REPRESENTATIVE OF ACTUAL SITE CONDITIONS AND IS TRUE AND CORRECT TO THE BEST OF MY KNOWLEDGE AND BELIEF. THIS CERTIFICATION IS SUBMITTED IN SUPPORT OF A REQUEST FOR PERMANENT CERTIFICATE OF OCCUPANCY.

CORRECTIONS: THE CONCRETE CHANNEL, RECENTLY CONSTRUCTED, WAS NOT IN PLACE WHEN THE DRAINAGE PLAN WAS SUBMITTED AND APPROVED WITH UNRESTRICTED DISCHARGE. THE C.D.A. PROVIDED ONLY ONE 6" DIA. PVC PIPE THROUGH THE WALL TO DRAIN THE SITE. CALCULATIONS ARE SHOWN ON SHEET TWO THAT AT LEAST THREE MORE 6" DIA. PIPES ARE REQUIRED TO ADEQUATELY DRAIN THE SITE WITH DETENTION POND DEPTH NOT EXCEEDING ONE FOOT.

THE RECORD INFORMATION PRESENTED HEREON IS NOT NECESSARILY COMPLETE AND INTENDED ONLY TO VERIFY SUBSTANTIAL COMPLIANCE OF THE GRADING AND DRAINAGE ASPECTS OF THIS PROJECT. THOSE RELYING ON THIS RECORD DOCUMENT ARE ADVISED TO OBTAIN INDEPENDENT VERIFICATION OF ITS ACCURACY BEFORE USING IT FOR ANY OTHER PURPOSE.

FRANK D. LOVELADY
NMPE 6512
DATE: 06-01-2007



VICINITY MAP

ZONE ATLAS NO. C-17-Z

EROSION CONTROL NOTES:

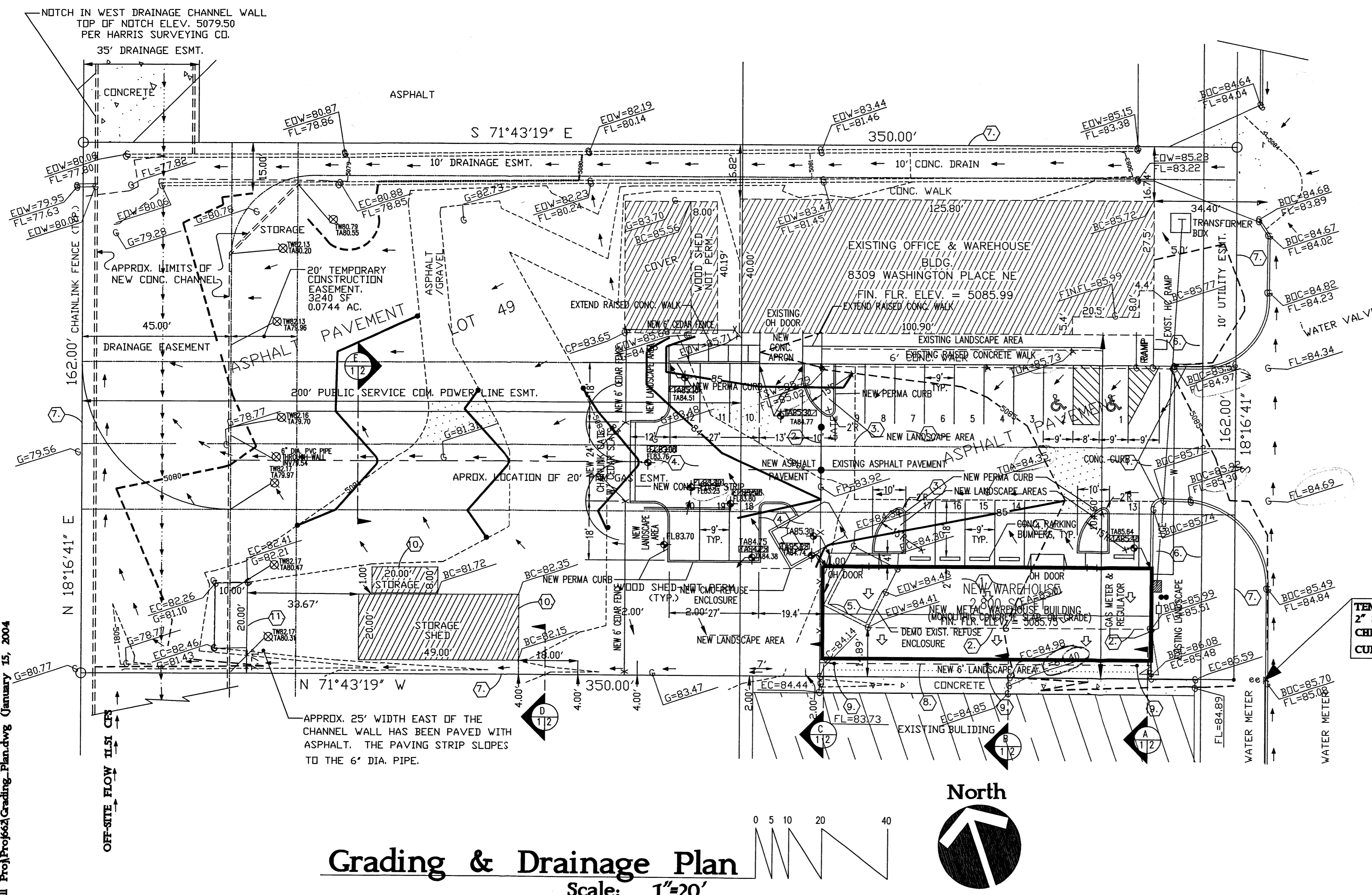
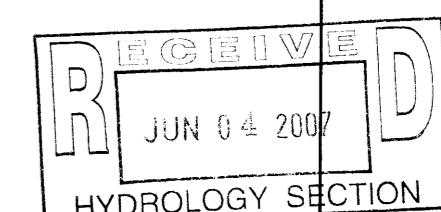
1. THE CONTRACTOR SHALL BE RESPONSIBLE FOR COMPLIANCE WITH THE FOLLOWING:
 1. NO SEDIMENT-BEARING WATER SHALL BE ALLOWED TO DISCHARGE FROM THE SITE DURING CONSTRUCTION.
 2. DURING GRADING OPERATIONS AND UNTIL THE PROJECT HAS BEEN COMPLETED, ALL ADJACENT PROPERTY, RIGHTS-OF-WAY, AND EASEMENTS SHALL BE PROTECTED FROM FLOODING BY RUNOFF FROM THE SITE.
 3. SHOULD THE CONTRACTOR FAIL TO PREVENT SEDIMENT-BEARING WATER FROM ENTERING PUBLIC RIGHT-OF-WAY, HE SHALL PROMPTLY REMOVE FROM THE PUBLIC RIGHT-OF-WAY ANY AND ALL SEDIMENT ORIGINATING FROM THE SITE.
 4. CONTROL OF SEDIMENT-LADEN WATERS WILL BE ACCOMPLISHED BY USE OF A COMPACTED EARTH BERM OF ADEQUATE HEIGHT. THE BERM SHALL BE LOCATED ALONG THE DOWNSTREAM PERIMETER OF THE PROPERTY.

GENERAL NOTES

1. PRIOR TO CONSTRUCTION, THE CONTRACTOR SHALL EXCAVATE AND VERIFY THE LOCATION OF ALL POTENTIAL OBSTRUCTIONS. SHOULD A CONFLICT EXIST, THE CONTRACTOR SHALL NOTIFY THE ENGINEER AS SOON AS POSSIBLE TO RESOLVE THE CONFLICT WITH A MINIMUM AMOUNT OF DELAY.
2. ALL WORK ON THIS PLAN SHALL BE PERFORMED IN ACCORDANCE WITH APPLICABLE FEDERAL, STATE AND LOCAL LAWS, RULES AND REGULATIONS CONCERNING CONSTRUCTION SAFETY AND HEALTH.
3. IF ANY UTILITY LINES, PIPELINES, OR UNDERGROUND UTILITY LINES ARE SHOWN ON THESE DRAWINGS, THEY ARE SHOWN IN AN APPROXIMATE LOCATION ONLY, AND LINES MAY EXIST WHERE NONE ARE SHOWN. THE LOCATION IS BASED UPON INFORMATION PROVIDED BY THE UTILITY OWNER OR FROM EXISTING PLANS, AND THIS INFORMATION MAY BE INCOMPLETE, OR OBSOLETE AT THE TIME OF CONSTRUCTION. THE ENGINEER HAS NOT UNDERTAKEN ANY FIELD VERIFICATION OF THESE LOCATIONS. LINE SIZES OR MATERIAL TYPE, MAKES NO REPRESENTATION THEREOF, AND ASSUMES NO RESPONSIBILITY OR LIABILITY THEREFOR. THE CONTRACTOR SHALL INFORM ITSELF OF THE LOCATIONS OF ANY UTILITY LINE, PIPELINE OR UNDERGROUND INSTALLATION IN OR NEAR THE AREA IN ADVANCE OF OR DURING ANY EXCAVATION WORK. THE CONTRACTOR IS FULLY RESPONSIBLE FOR ANY AND ALL DAMAGES CAUSED BY ITS FAILURE TO LOCATE, IDENTIFY AND PRESERVE ANY AND ALL EXISTING UTILITIES, PIPELINES AND UNDERGROUND FACILITIES. IN PLANNING AND CONDUCTING EXCAVATIONS, THE CONTRACTOR SHALL COMPLY WITH ALL STATE STATUTES, MUNICIPAL AND LOCAL ORDINANCES, RULES AND REGULATIONS, IF ANY, PERTAINING TO THE LOCATION OF THESE LINES AND FACILITIES.
4. THE CONTRACTOR SHALL ENSURE THAT NO SOIL ERODES FROM THE SITE INTO PUBLIC RIGHTS-OF-WAY OR ONTO PRIVATE PROPERTY. THIS CAN BE ACHIEVED BY CONSTRUCTING TEMPORARY BERMS AND BY WETTING THE SOIL TO KEEP IT FROM BLOWING.
5. THE CONTRACTOR SHALL OBTAIN ANY AND ALL PERMITS REQUIRED BY THE CITY OF ALBUQUERQUE FOR THE COMPLETION OF THE WORK PRIOR TO BEGINNING CONSTRUCTION.

KEYED NOTES

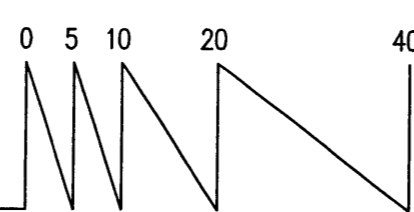
1. EXISTING ASPHALT PAVEMENT.
2. EDGE OF EXISTING ASPHALT PAVEMENT.
3. EDGE OF ASPHALT REMOVAL AND REPLACEMENT.
4. NEW ASPHALT PAVEMENT.
5. EXISTING REFUSE ENCLOSURE - DEMOLISH.
6. EXISTING LANDSCAPING.
7. PROPERTY LINE.
8. ROOF GUTTER.
9. DOWNSPOUT.
10. EXISTING TEMPORARY BUILDINGS TO BE REMOVED, RELOCATED OR, LEFT IN PLACE, AS DETERMINED BY THE OWNER. NOTE: ALL TEMPORARY BUILDINGS WITHIN THE CHANNEL CONST. AREA HAVE BEEN REMOVED.
11. EXISTING CONCRETE PAD. CITY PLANS FOR CLIFFORD CHANNEL, REALIGNMENT CALL FOR REMOVAL AND DISPOSAL OF THIS PAD.



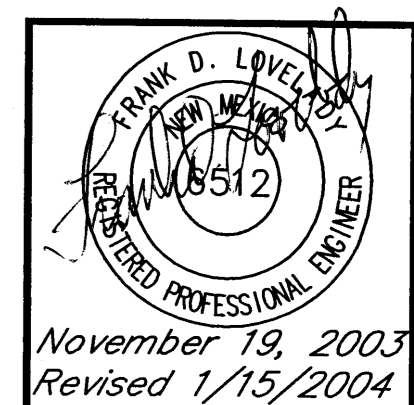
Washington Place NE



Grading & Drainage Plan
Scale: 1"=20'



TEMPORARY BENCH MARK (TBM)
2" SQUARE AND LETTERS "TBM"
CHISELED ON TOP OF CONCRETE
CURB. ELEVATION = 5065.70



November 19, 2003
Revised 1/15/2004

CERTIFIED
GRADING AND DRAINAGE PLAN
Warehouse for Commercial Enterprises
8309 WASHINGTON PLACE NE ALBUQUERQUE, NEW MEXICO

KEN HOVEY, ARCHITECT
505) 259-8458 • 3808 SIMMS AVENUE SE • ALBUQUERQUE, NM • 87108

JOB NO:	0309
DATE:	15 JANUARY 2004
REVISIONS	
1	TBM RELOCATED, TEMP. CONST. EASEMENT ADDED.

SHEET NO.
C.I.1

DRAINAGE CALCULATIONS:

EXISTING CONDITIONS:

The site is located on the west side of Washington Place NE, the third lot north of the Domingo Baca Channel. There is a drainage concrete channel along the north edge of the site. The existing building roof has an east-west ridge and the north side of the building drains directly into the drainage channel via splash blocks that extend over the channel wall. The parking lot is paved and there are several non-permanent buildings on the site. drainage is from east to west. At the NW corner of the site the drainage channel continues west into the old baloon Fiesta area. The lots north and south of the site are also developed. (See aerial photo, Sheet C.1.2)

DEVELOPED CONDITIONS:

It is proposed to construct a new metal storage building on the site as shown. The building will be constructed in an area which is now partially paved. The pavement will be extended to the west as shown. All runoff will continue to flow to the west and then north into the existing easement as it presently does.

FUTURE IMPROVEMENTS:

The City of Albuquerque proposes to construct 'Clifford Channel Realignment at Former Los Angeles Landfill'. Construction plans have been prepared and a reduced set will be submitted with this grading and drainage submittal. No attempt has been made to show the channel grades on this plan.

DRAINAGE CRITERIA:

The calculations shown on this plan were prepared in accordance with Section 22.2, Hydrology, of the Development Process Manual, Volume 2, Design Criteria, for the City of Albuquerque in cooperation with Bernalillo County, New Mexico and the Metropolitan Arroyo Flood Control Authority, January, 1993.

PRECIPITATION ZONE:

The site is between the Rio Grande River and San Mateo Blvd. and is, therefore, in Precipitation Zone 2.

LAND TREATMENT "D":

SURFACE	EXISTING	DEVELOPED
CONCRETE	4,095	5,236
ASPHALT PAVING	8,623	9,698
BUILDING	8,369	11,169
TOTAL	21,087	26,103

LAND TREATMENT AREAS:

The peak discharge per acre and excess precipitation are shown for the four land treatments in Zone 2 in the table below, and the values shown are from the City of Albuquerque D.P.M. Also shown are the existing and proposed land treatment areas.

LAND TREAT.	q (cfs/ac)		E (in)		Existing Site Areas		Developed Site Areas	
	100-yr.	10-yr.	100-yr.	10-yr.	% Sq. Ft.	Acres	% Sq. Ft.	Acres
A	1.56	0.38	0.53	0.13	0.0	0.0000	0.0	0.0000
B	2.28	0.95	0.78	0.28	4.8	2,720	0.0624	4.8
C	3.14	1.71	1.13	0.52	58.0	32,893	0.7551	49.2
D	4.70	3.14	2.12	1.34	32.2	21,087	0.4841	46.0
Totals					100.0	56,700	1.3016	100.0

PEAK DISCHARGE:

EXISTING CONDITIONS:

$$Q_{100} = 0.0624 \times 2.28 + 0.7551 \times 3.14 + 0.4841 \times 4.70 = 4.79 \text{ cfs}$$
$$Q_{10} = 0.0624 \times 0.95 + 0.7551 \times 1.71 + 0.4841 \times 3.14 = 2.87 \text{ cfs}$$

DEVELOPED CONDITIONS:

$$Q_{100} = 0.0624 \times 2.28 + 0.6400 \times 3.14 + 0.5992 \times 4.70 = 4.97 \text{ cfs}$$
$$Q_{10} = 0.0624 \times 0.95 + 0.6400 \times 1.71 + 0.5992 \times 3.14 = 3.04 \text{ cfs}$$

VOLUME, 100-YEAR AND 10-YEAR, 6-HOUR:

EXISTING CONDITIONS:

$$V_{100} = (2,720 \times 0.78 + 32,893 \times 1.13 + 21,087 \times 2.12) / 12 = 7,000 \text{ cf}$$
$$V_{10} = (2,720 \times 0.28 + 32,893 \times 0.52 + 21,087 \times 1.34) / 12 = 3,844 \text{ cf}$$

DEVELOPED CONDITIONS:

$$V_{100} = (2,720 \times 0.78 + 27,877 \times 1.13 + 26,103 \times 2.12) / 12 = 7,413 \text{ cf}$$
$$V_{10} = (2,720 \times 0.28 + 27,877 \times 0.52 + 26,103 \times 1.34) / 12 = 4,186 \text{ cf}$$

SUMMARY OF ON-SITE VOLUMES AND PEAK DISCHARGE RATES:

	V100(CF)	V10(CF)	Q100(CFS)	Q10(CFS)
DEVELOPED	7,413	4,186	4.97	3.04
EXISTING	7,000	3,844	4.79	2.87
INCREASE	413	342	0.18	0.17

NEW BUILDING ROOF RUNOFF:

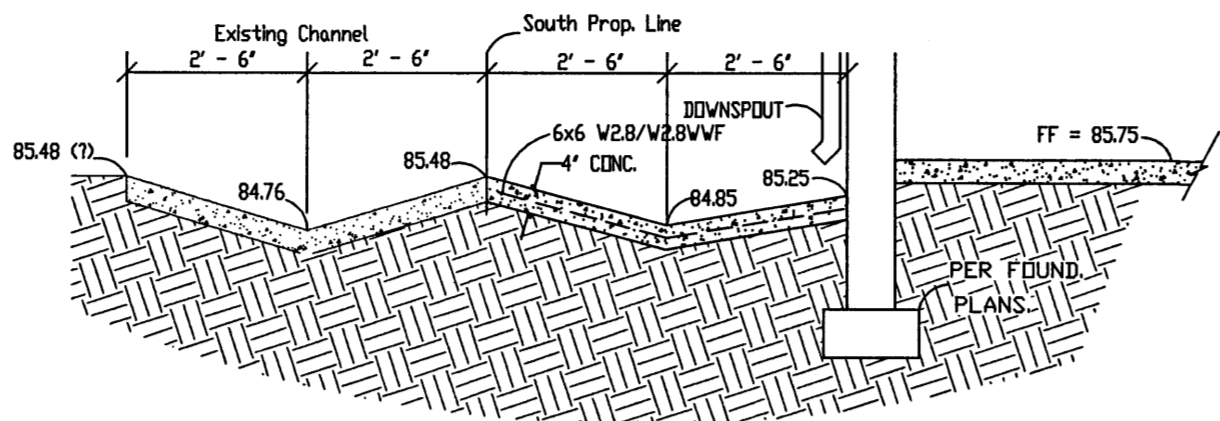
AREA = 3300 SF

$$Q_{100} = (3300 / 43,560) \times 4.7 = 0.36 \text{ CFS}$$

CAPACITY OF SWALE (SECTION D):

SWALE 5' WIDE X 0.5' DEEP

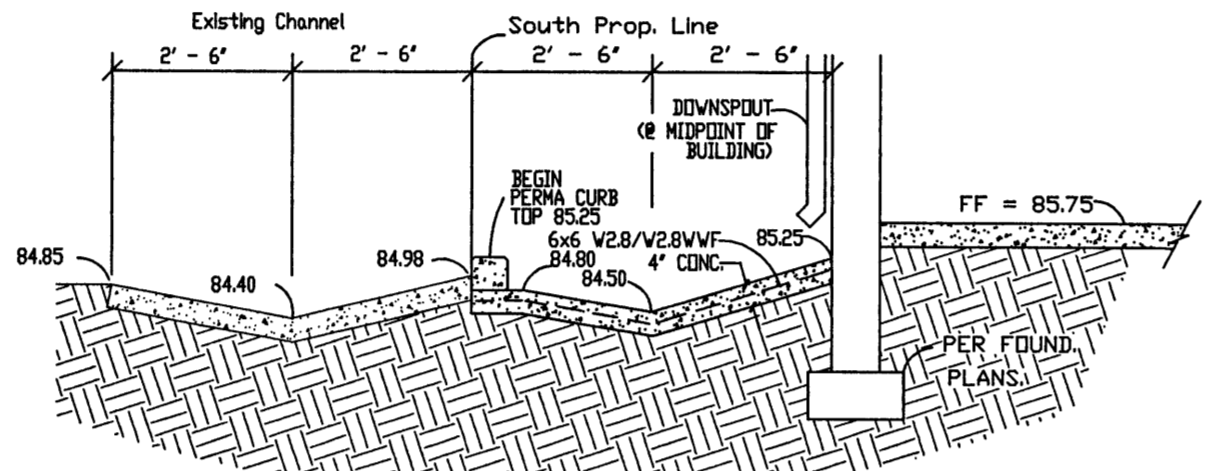
$$\text{AREA} = 1.25 \text{ SF}$$
$$P = 5.1 \quad \text{SLOPE} = 4.14 = 200 = 0.0207$$
$$R = A/P = 1.25 / 5.1 = 0.25 \quad N = 0.035 \text{ (GRAVEL)}$$
$$V = (1.486 / 0.035) \times (0.25)^{2/3} \times (0.0207)^{1/2} = 2.4 \text{ FPS}$$
$$Q = AV = 1.25 \times 2.4 = 3.00 \text{ CFS}$$
$$3.00 \text{ CFS} > 0.36 \text{ CFS} \quad \text{ADEQUATE}$$



A
1/2
3/8" = 1' - 0"

NOTE:

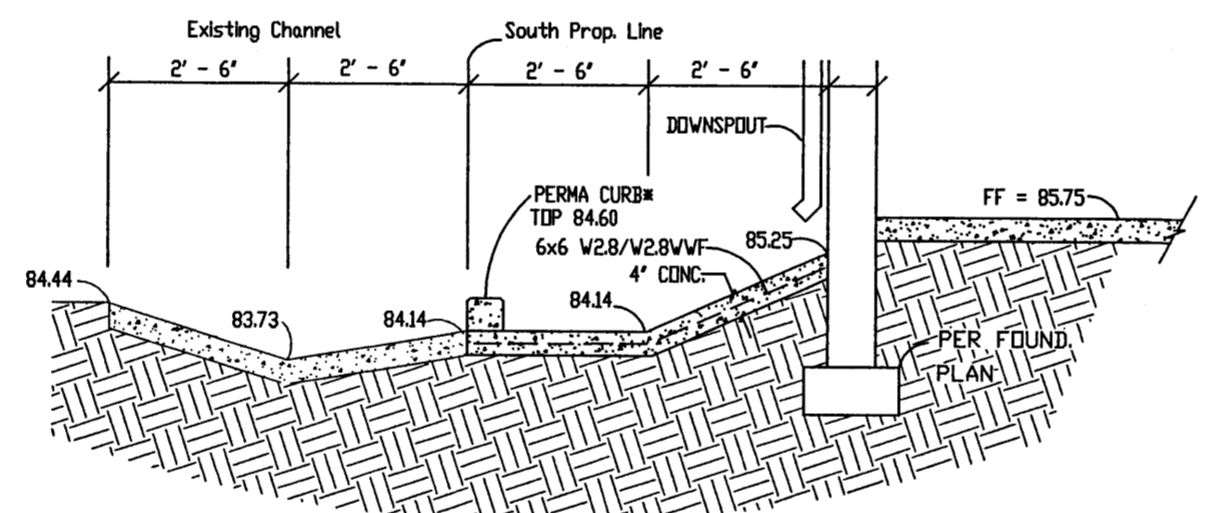
TOTAL DISTANCE BETWEEN SECTIONS 'A' & 'B' IS 43.6'
SLOPE = (84.85 - 84.50) / 43.6 = 0.0080 FT./FT.



B
1/2
3/8" = 1' - 0"

NOTE:

TRANSITION FROM 'V' SHAPED CHANNEL TO FLAT BOTTOMED
CHANNEL BETWEEN SECTIONS 'B' AND 'C'. TOTAL DISTANCE
IS 56.0'. SLOPE = (84.5 - 84.14) / 56 = 0.0064 FT./FT.



C
1/2
3/8" = 1' - 0"

PROPERTY LINE

EXISTING GRADE

EXISTING GRADE

4" THICK, CRUSHED
3/4" TO 1" DIA. CRUSHED GRAVEL

FINISH GRADE

FINISH GRADE

4" SUBGRADE
COMPACT TO 90%
PER ASTM D1557

GRAVEL BASE COURSE CLASS II
PER C.O.A. STD. SPECS - COMPACT TO
90% PER ASTM D1557.

FINISH GRADE

FINISH GRADE

4" SUBGRADE
COMPACT TO 90%
PER ASTM D1557

GRAVEL BASE COURSE CLASS II
PER C.O.A. STD. SPECS - COMPACT TO
90% PER ASTM D1557.

FINISH GRADE

FINISH GRADE

4" SUBGRADE
COMPACT TO 90%
PER ASTM D1557

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FINISH GRADE

FINISH GRADE

4" SUBGRADE
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FINISH GRADE

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4" SUBGRADE
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FINISH GRADE

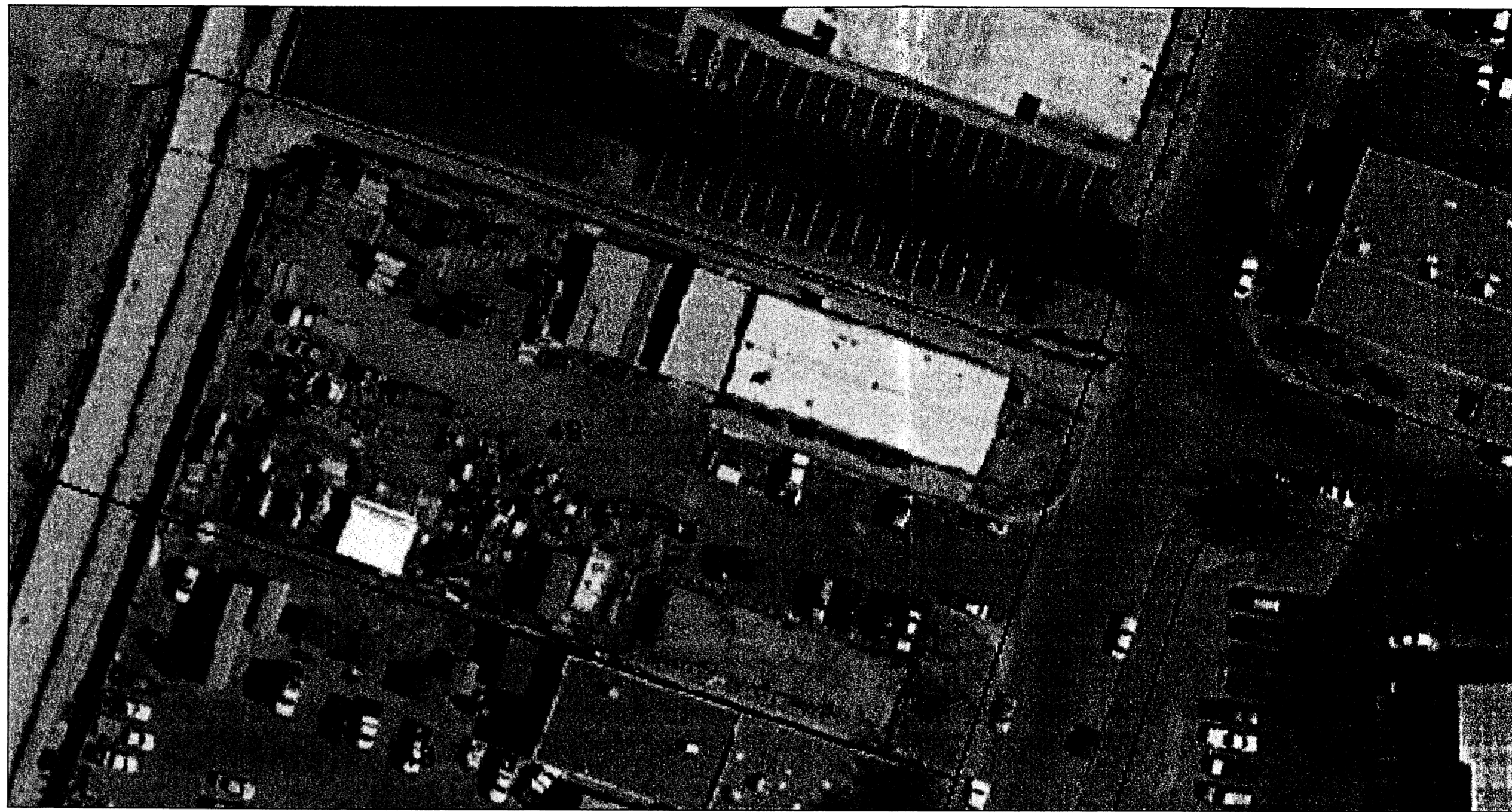
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4" SUBGRADE
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GRAVEL BASE COURSE CLASS II
PER C.O.A. STD. SPECS - COMPACT TO
90% PER ASTM D1557.

FINISH GRADE

FINISH GRADE



2006 AERIAL PHOTO OF SITE AREA

APPROX. SCALE 1" = 100'

OFF-SITE FLOW CALCULATIONS:

AREA = 2.72 ACRES - SEE ABOVE MAP.

PER TABLE A-5 OF THE DPM, LIGHT INDUSTRIAL IS 70% TREATMENT D.

ASSUME 30 PERCENT TREATMENT C.

WEIGHTED 100-YEAR PEAK DISCHARGE PER ACRE:

$$q_w = 0.3 \times 3.14 + 0.7 \times 4.70 = 4.23 \text{ CFS PER ACRE.}$$

$$Q_{100} = 4.23 \text{ CFS PER ACRE} \times 2.72 \text{ ACRES} = 11.51 \text{ CFS}$$

PREVIOUSLY, THERE WAS A PORTION OF THE PROPERTY SOUTH OF LOT 49 THAT APPEARED TO DRAIN TO THE NW CORNER OF LOT 49 WHERE DRAINAGE WAS PICKED UP BY THE THEN EXISTING CHANNEL WHICH RAN DUE WEST ACROSS THE OLD BALLOON FIELD. SINCE THE CONSTRUCTION OF THE NEW CITY DRAINAGE CHANNEL, WATER NO LONGER LEAVES LOT 49 AT THIS POINT. APPARENTLY, EACH LOT SOUTH OF LOT 49 WAS PROVIDED WITH IT'S OWN DRAIN PIPE INTO THE CHANNEL. THEREFORE, LOT 49 NO LONGER ACCEPTS OFF-SITE FLOW FROM PROPERTY TO THE SOUTH SINCE THERE IS NO OUTLET.

REVISED FLOW CALCULATIONS DUE TO LOSING 45-FOOT-WIDE STRIP:

AREA = 45 X 162 = 7,290 SF (0.1674 ACRE) ALL TREATMENT C. 3.14 CFS PER ACRE. E = 1.13 INCH.

$$Q_{100} = 0.1674 \text{ AC} \times 3.14 \text{ CFS/ACRE} = 0.53 \text{ CFS} \quad 4.97 - 0.53 = 4.44 \text{ CFS}$$

$$V_{100} = 7,290 \text{ SF} \times 1.13 \text{ INCH} / 12' / \text{FT} = 686 \text{ CF} \quad 7,413 - 686 = 6,727 \text{ CF}$$

REVISED SUMMARY OF ON-SITE VOLUMES AND PEAK DISCHARGE RATES:

	V100(CF)	Q100(CFS)
EXISTING	7,000	4.79
DEVELOPED	6,727	4.44
DECREASE	273	0.35

CALCULATIONS FOR 6" DIA. PIPE THROUGH CONCRETE WALL INTO CHANNEL:

USE DRIFICE EQUATION: $Q = CA(2GH)^{1/2}$ C = 0.6 A = 0.1963 G = 32.16 Q = 5 CFS

ASSUME A DEPTH OF POND OF 1.0' ABOVE THE INVERT OF PIPE. H = 1.0 - 0.25 = 0.75'

$$Q = 0.6 \times 0.1963 \times (2 \times 32.16 \times 0.75)^{1/2} = 0.82 \text{ CFS.}$$

ASSUME A DEPTH OF POND OF 1.5' ABOVE THE INVERT OF PIPE. H = 1.5 - 0.25 = 1.25'

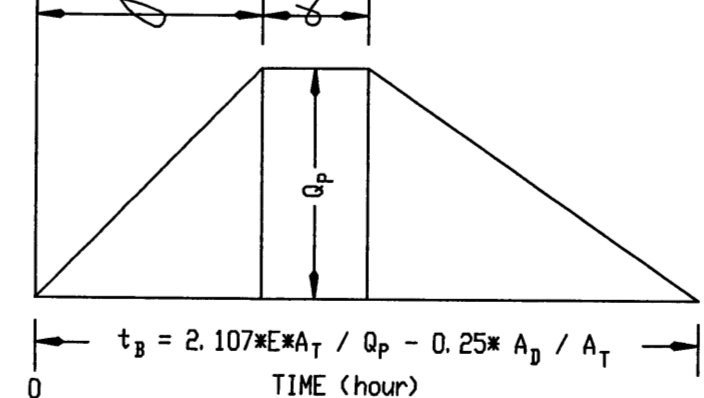
$$Q = 0.6 \times 0.1963 \times (2 \times 32.16 \times 1.25)^{1/2} = 1.56 \text{ CFS.}$$

USE ONE FOOT DEPTH RATHER THAN 1.5 FOOT DEPTH.

DETENTION POND ANALYSIS:

The hydrograph method shown below is used to determine the required volume of the detention pond. (See City of Albuquerque D.P.M., Figure A-3).

$$t_p = 0.7 t_{tc} + (1.6 - (A_b / A_t)) / 12$$
$$0.25 \times A_b / A_t$$



TOTAL SITE
E = (2,720 * 0.78 + (20,587) * 1.13 + 26,103 * 2.12) / (49,410) = 1.63 IN.

$$A_t = 49,410 \text{ SF} = 1.1343 \text{ AC}$$

$$A_b = 26,103 \text{ SF} (0.5992 \text{ ac})$$

$$Q_p = 4.44 \text{ CFS}$$

$$t_p = 2.107 \times 1.63 \times 1.1343 / 4.44 - 0.25 \times (0.5992 / 1.1343) = 0.7453 \text{ hour.}$$

$$t_p = 0.7 t_{tc} + (1.6 - (A_b / A_t)) / 12$$

$$t_p = (0.7 \times 0.2) + (1.6 - (0.5992 / 1.1343)) / 12$$

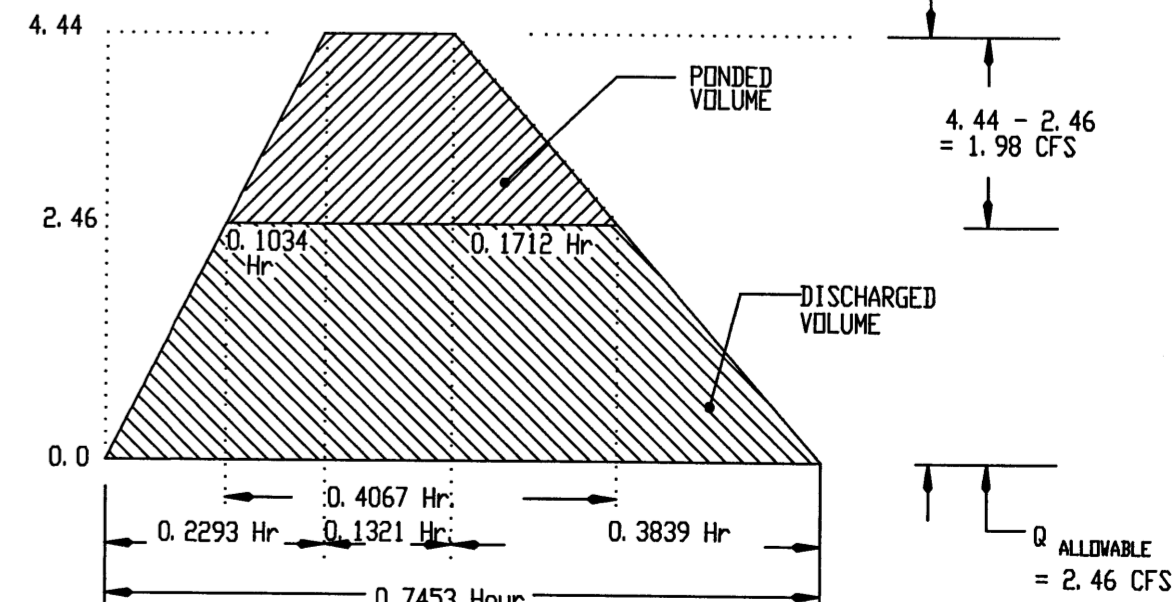
$$t_p = 0.14 + (1.6 - (0.5283)) / 12$$

$$t_p = 0.14 + (1.0717 / 12) = 0.14 + 0.0893 = 0.2293 \text{ hours.}$$

Continue the peak for 0.25 A_b / A_t hours.

$$0.25 \times 0.5992 / 1.1343 = 0.1321 \text{ hour}$$

ALLOWABLE DISCHARGE:
THREE 6" DIA. PIPES AT 0.82 CFS PER PIPE, OR 2.46 CFS.



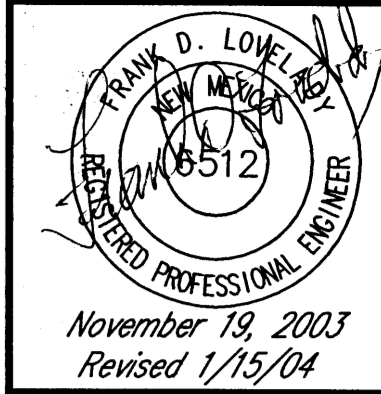
TOTAL VOLUME:
V = 1/2 (0.1321 + 0.7453) (4.44) 3600 = 7,012 CF

PONDED VOLUME:
V = 1/2 (0.1321 + 0.4067) (1.98) (3600) = 1,920 CF

DISCHARGED VOLUME:
V = 1/2 (0.4067 + 0.7453) (2.46) (3600) = 5,101 CF

ACTUALLY, THREE ADDITIONAL 6" PVC PIPES WOULD REDUCE THE PONDED VOLUME TO A QUANTITY THAT COULD PROBABLY BE CONTAINED WITHIN THE LIMITS OF THE 25' ASPHALT PAVING STRIP PROVIDED BY C.D.A.

NOTE: SEE CERTIFICATION BLOCK ON SHEET C.1.1



CERTIFIED GRADING AND DRAINAGE CALCULATIONS
Warehouse for Commercial Enterprises
8309 WASHINGTON PLACE NE ALBUQUERQUE, NEW MEXICO

KEN HOWEY, ARCHITECT
505 259-8458 * 3808 SIMMS AVENUE SE * ALBUQUERQUE, NM * 87108

JOB NO:	0309
DATE:	15 JANUARY 2004
REVISIONS	
1. OFF-SITE FLOW AREA & CALCOS	

SHEET NO.

C.1.2

[illegible]

STATE OF NEW MEXICO
KENNETH R. HOVEY
NO. 1966
REGISTERED ARCHITECT

JUN 13 2007

JOB NO: 0309	
DATE: 16 AUGUST 2005	
REVISIONS	

SD.1