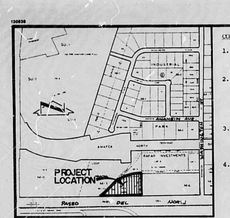
PUBLIC WORKS/HYDROLOGY

C17/D8



VICINITY MAP C-17

SCALE : 1" = 800'

LEGAL DESCRIPTION

TRACT 2, LOOP INDUSTRIAL PARK

PROJECT BENCHMARK

ACS STATION "NAA-9": AN AMARCA ROW BRASS CAP LOCATED IN THE SE QUADRANT OF THE INTERSECTION OF PASEO DEL NORTE BLVD & THE AMARCA NORTH DIVERSION CHANNEL. ELEV.: 5069.27 FEET (MSLD)

T. B. M.

EXISTING PANEL POINT LOCATED APPROXIMATELY 185 FEET WEST OF THE BUILDING AS SHOWN HEREON ELEVATION: 5085.11 FEET (MSLD)

LEGEND

5080	EXISTING CONTOUR
+	EXISTING SPOT ELEVATION
	EXISTING CONC BARRIER WALL
TC	TOP OF CURB
FL	FLOWLINE
	EXISTING RAILROAD TRACK
	EXISTING SIDE SLOPE
	NEW SIDE SLOPE
+	PROPOSED SPOT ELEVATION
87	PROPOSED CONTOUR
	NEW CONCRETE
	NEW ASPHALT

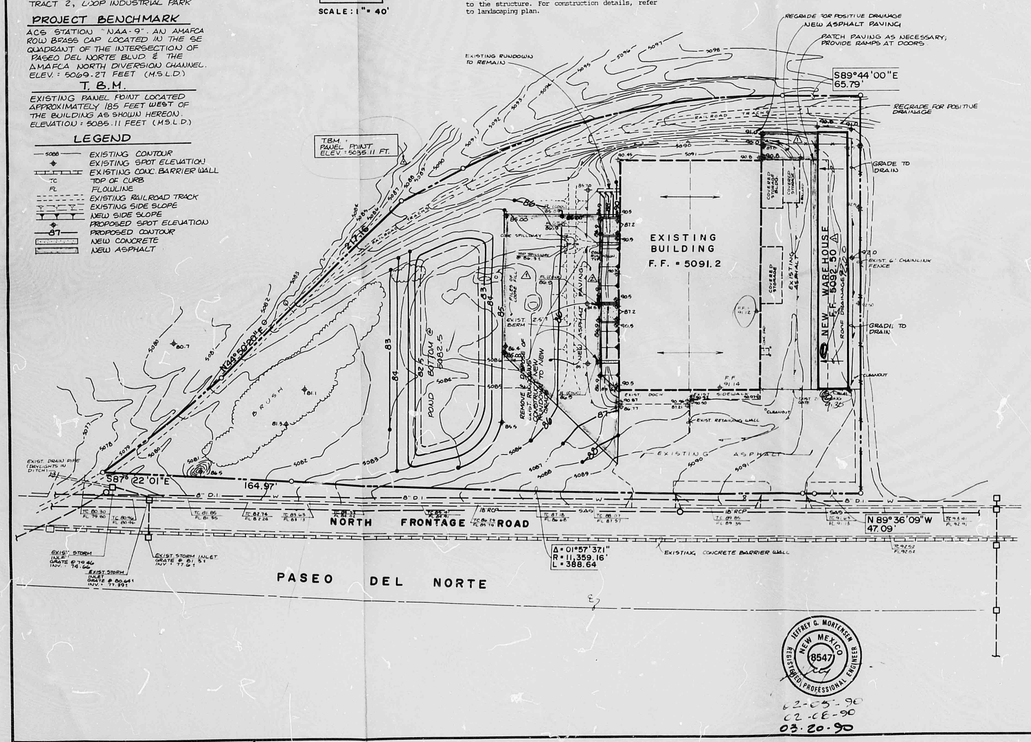
CONSTRUCTION NOTES:

1. Two (2) working days prior to any excavation, contractor must contact line locating Service 765-1234, for location of existing utilities.
2. Prior to construction, the contractor shall excavate and verify the horizontal and vertical location of all potential obstructions. Should a conflict exist, the contractor shall notify the engineer in writing so that the conflict can be resolved with a minimum amount of delay.
3. All work on this project shall be performed in accordance with applicable federal, state and local laws, rules and regulations concerning construction safety and health.
4. All construction within public right-of-way shall be performed in accordance with applicable City of Albuquerque Standards and Procedures.
5. If any utility lines, pipelines, or underground utility lines are shown on these drawings, they are shown in an approximate manner only, and such lines may exist where none are shown. If any such existing lines are shown, the location is based upon information provided by the owner of said utility, and the information may be incomplete, or may be obsolete by the time construction commences. The engineer has conducted only preliminary investigation of the location, depth, size, or type of existing utility lines, pipelines, or underground utility lines. This investigation is not conclusive, and may not be complete, therefore, makes no representation pertaining thereto, and assumes no responsibility or liability therefor. The contractor shall inform itself of the location of any utility line, pipeline, or underground utility line in or near the area of the work in advance of and during excavation work. The contractor is fully responsible for any and all damage caused by its failure to locate, identify and preserve any and all existing utilities, pipelines, and underground utility lines. In planning and conducting excavation, the contractor shall comply with state statutes, municipal and local ordinances, rules and regulations, if any, pertaining to the location of these lines and facilities.
6. The design of planters and landscaped areas is not part of this plan. All planters and landscaped areas adjacent to the building(s) shall be provided with positive drainage to avoid any ponding adjacent to the structure. For construction details, refer to landscaping plan.

SCALE : 1" = 40'

Erosion Control Measures

1. The contractor shall ensure that no soil erodes from the site into public right-of-way or onto private property. This can be achieved by constructing temporary berms at the property lines and wetting the soil to keep it from blowing.
2. The contractor shall promptly clean up any material excavated within the public right-of-way so that the excavated material is not susceptible to being washed down the street.
3. The contractor shall secure "Topsoil Disturbance Permit" prior to beginning construction.



DRAINAGE PLAN

The following items concerning the Western Building Supply Drainage Plan are contained hereon:

1. Vicinity Map
2. Grading Plan
3. Calculations

As shown by the Vicinity Map, the site is located on the north side of Paseo Del Norte Boulevard N.E. between Edith Boulevard N.E. and Washington Street N.E. At present, the site is developed as a commercial business with a building and asphalt paving. The building and paving occupy much of the easterly portion of the site while the westerly portion remains undeveloped. Adjacent sites to the east are also commercially developed with warehouse-like facilities.

Panel 9 of 50 of the National Flood Insurance Program Flood Boundary and Floodway Maps for the City of Albuquerque, New Mexico dated October 14, 1981, indicate that this site does not lie within a 100-year floodplain. Further review does not indicate that this site contributes runoff to an existing flood hazard area. The review of this panel, coupled with site observation, reveals that offsite flows do not impact this site. The warehouses that are constructed to the east drain in a southerly and northerly direction toward the railroad spur to the north and Paseo Del Norte Boulevard N.E. to the south. At present, the site drains from east to west to the southwesterly tip of the site. Runoff not retained in ponds previously constructed and depressions on the site will enter an existing storm inlet which has been constructed as part of the Paseo Del Norte Boulevard N.E. construction project. This was a New Mexico State Highway and Transportation Department project. This inlet discharges runoff through a culvert onto the property which lies to the west of the site. It is because of this existing condition that the discharge of developed runoff from this site to that storm drain facility is not permissible and that 100% retention of the developed runoff will be implemented.

The Grading Plan shows 1) existing and proposed grades indicated by spot elevations and contours at 1'0" intervals, 2) the limit and character of the existing improvements, 3) the limit and character of the proposed improvements, and 4) continuity between existing and proposed grades. As shown by this plan, the proposed improvements consist of the construction of a new warehouse building also the east portion of the site and the construction of new asphalt paving and two truck docks along the west side of the building. The construction of the new paving and truck docks will interfere with the existing drainage improvements previously constructed on this site. Consequently, a new retention ponding facility is proposed to hold a maximum depth of 1.5 feet. This new pond is located to the west of the proposed paving improvements so that the developed runoff generated east of the facility can be collected and held within the pond.

The Calculations which appear hereon analyze both the existing and developed conditions for the 100-year, 24-hour rainfall event. Normally the 6-hour rainfall event is used for the hydrologic computations for a project of this size, however, due to the fact that the retention of runoff is proposed, the 24-hour runoff volume is more appropriate. The National Method has been used to quantify the peak rate of discharge generated by the site while the SCS Method has been used to quantify the volume of runoff generated. The Average End Area Method has been used to determine the pond volume. As shown by these calculations, the proposed improvements will result in an increase in developed runoff from the site. The calculations further demonstrate that ample pond volume is being provided to retain 100% of the developed runoff anticipated from this site. As stated above, the 24-hour rainfall event has been used for these calculations so that ample pond volume can be provided. Both Methods have been used in accordance with the City of Albuquerque Development Process Manual, coupled with the Mayor's Emergency Rule dated January 14, 1986.

CALCULATIONS

Ground Cover Information

From SCS Bernalillo County Soil Survey,
Plate 11: EmB (Embudo gravelly fine sandy loam)
Hydrologic Soil Group: B
Existing Pervious CN = 70 (DPM Plate 22.2 C-2
Pasture or Range Land: fair condition)
Developed Pervious CN = 70 (DPM Plate 22.2 C-2
Pasture or Range Land: fair condition)

Time of Concentration/Time to Peak

$T_C = 0.0078\ L^{0.77}/S^{0.385}$ (Kirpich Equation)

$T_P = T_C = 10$ min.

Point Rainfall

$P_{24} = 2.6$ in. (100-year, 24-hr)

National Method

Discharge: Q = CiA

where C varies
$i = P_{24}\ (6.84)\ T_C^{-0.51} = 5.50$ in/hr
$P_{24} = 2.6$ in
$T_C = 10$ min (minimum)
A = area, acres

SCS Method

Volume: V = 3630(DRO) A

Where DRO = Direct runoff in inches
A = area, acres

Existing Condition

$A_{total} = 158,520$ sf = 3.64 Ac
Roof area = 26,610 sf (0.17)
Paved area = 26,720 sf (0.17)
Landscaped area = 105,190 sf (0.66)
C = 0.48 (Weighted average per Emergency Rule, 1/14/86)
$Q_{100} = CiA = 0.48(5.50)(3.64) = 9.6$ cfs
$A_{imp} = 53,330$ sf; % impervious = 34 %
Composite CN = 79 (DPM Plate 22.2 C-2)
DRO = 0.90 in (DPM Plate 22.2 C-4)
$V_{100} = 3630\ (DRO)A = 11,890$ cf

Developed Condition

$A_{total} = 158,520$ sf = 3.64 Ac
Roof area = 32,390 sf (0.21)
Paved area = 43,220 sf (0.27)
Landscaped area = 82,910 sf (0.52)
C = 0.57 (Weighted average per Emergency Rule, 1/14/86)
$Q_{100} = CiA = 0.57(5.50)(3.64) = 11.4$ cfs
$A_{imp} = 75,610$ sf; % impervious = 48 %
Composite CN = 83 (DPM Plate 22.2 C-2)
DRO = 1.15 in (DPM Plate 22.2 C-4)
$V_{100} = 3630\ (DRO)A = 15,200$ cf

$V_{pond} = (A_{s4} + A_{s3}) \times .5 \times 1 + (A_{s3} + A_{s2.5}) \times .5 \times .5$
$A_{s3} = 12,600$ sf
$A_{s3} = 9,900$ sf
$A_{s2.5} = 7,500$ sf
$V_{pond} = 15,600$ cf > V_{100}

Comparison

$\Delta Q_{100} = 11.4 - 9.6 = 1.8$ cfs (increase)
$\Delta V_{100} = 15,200 - 11,890 = 3,310$ cf (increase)

HYDROLOGY APPROVAL & INSPECTION

APPROVED FOR BUILDING PERMIT
ENGINEER Lindsay Mayer DATE 3/2/90
INSPECTION REQUESTED DATE 4-4-90
APPROVAL DATE _____ DISAPPROVED _____
SDP APPROVAL DATE N/A
SURVEY DATE 4-3-90
HYDROLOGY BOOK NO/PAGE NO. 89-1 pg 75 & 37
COMMENTS FF elev 9100

STATE OF NEW MEXICO
LINDSAY MAYER
TRAUERNICHT
1285
REGISTERED ARCHITECT

akt architects
p.o. box 3366
albuquerque, nm
87190-(505) 281-9560

project
WESTERN BUILDING SUPPLY
WAREHOUSE

project no. 90504
date 2-2-90
sheet title
RECEIVED MAR 2 1990 HYDROLOGY DIVISION
1

STATE OF NEW MEXICO
JEFFREY G. MORTENSEN
8547
REGISTERED PROFESSIONAL ENGINEER

02-26-90
02-26-90
03-20-90

jma
JEFF MORTENSEN & ASSOCIATES, INC.
8110 DALLAS, N.E. ALBUQUERQUE, NM 87109
ENGINEERS TELEPHONE (505) 292-5611

GRADING AND DRAINAGE PLAN
WESTERN BUILDING SUPPLY

DESIGNED BY J.G.M
DRAWN BY S.G.H
APPROVED BY J.G.M

NO.	DATE	BY	REVISIONS
1	02/90	JGH	ADD PAVING & RAISE W.F. ELEVATION, REVISE CALCS.
2	03/90	GCF	REVISE PAVING LIMITS

JOB NO. 900021
DATE 01-90
SHEET 1 OF 1

R# 24 DATE 3/14/90 DOP

CITY OF ALBUQUERQUE

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DIRECTOR

NOTARY PUBLIC

OFFICIAL SEAL
Roberta Duran
NOTARY PUBLIC
STATE OF NEW MEXICO

PLAN SHOT TWICE
ONCE ON LIGHT
ONCE ON DARK

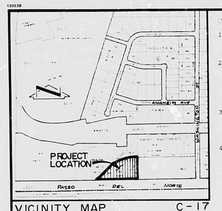
CITY OF ALBUQUERQUE
THIS MICROIMAGE IS THE BEST POSSIBLE REPRODUCTION DUE TO THE POOR QUALITY OF THE ORIGINAL DOCUMENT.

PLANS/PLATS ON FILE

FILE DESC: C17/D8

PLANS/PLATS 1

E 30 D 24 C 20 B 16 A 12 CITY OF ALBUQUERQUE C D F



VICINITY MAP C-17

SCALE : 1" = 800'

LEGAL DESCRIPTION

TRACT 2, LOOP INDUSTRIAL PARK

PROJECT BENCHMARK

ACS STATION NAA-9 AD AMAFCA ROW BRASS CAP LOCATED AT THE SE QUADRANT OF THE INTERSECTION OF PASEO DEL NORTE BLVD & THE AMAFCA NORTH DIVERSION CHANNEL ELEV = 5089.27 FEET (MSLD)

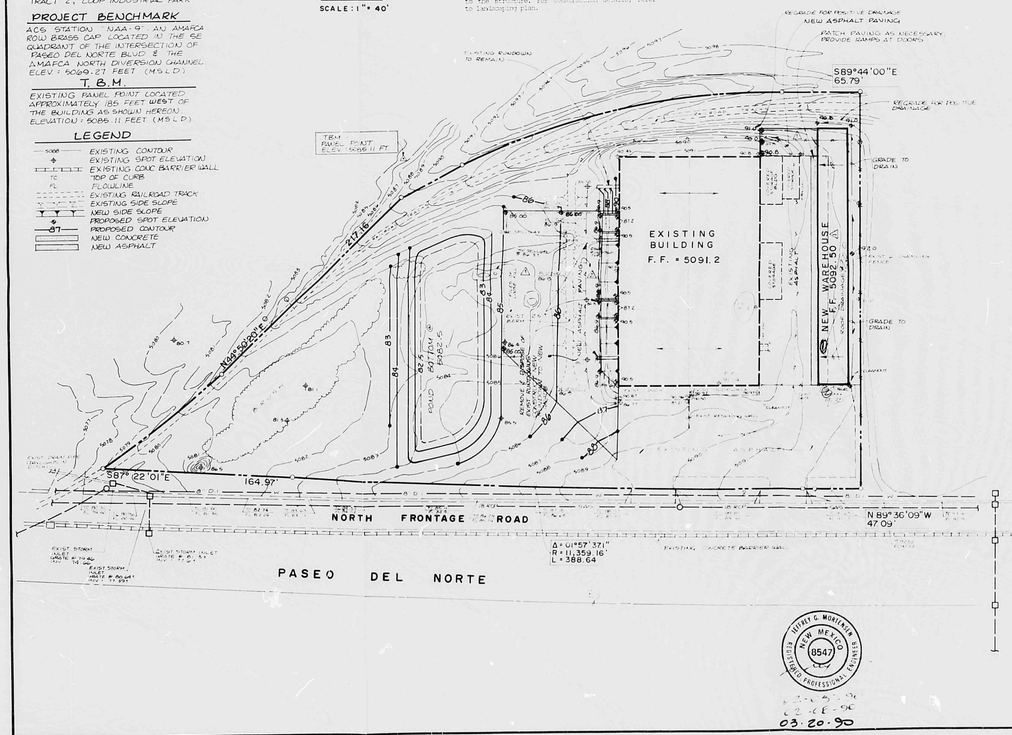
T.B.M.

EXISTING PAVEL POIAT LOCATED APPROXIMATELY 185 FEET WEST OF THE BUILDING AS SHOWN HEREON ELEVATION = 5088.11 FEET (MSLD)

LEGEND

- 5088 — EXISTING CONTOUR
- EXISTING SPOT ELEVATION
- EXISTING CONC BARRIER WALL
- TC — TOP OF CURB
- FL — FLOWLINE
- EXISTING RAILROAD TRACK
- EXISTING SIDE SLOPE
- NEW SIDE SLOPE
- PROPOSED SPOT ELEVATION
- 5091 — PROPOSED CONTOUR
- NEW CONCRETE
- NEW ASPHALT

SCALE : 1" = 40'



CONSTRUCTION NOTES

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2. Prior to construction, the contractor shall excavate and verify the horizontal and vertical location of all potential obstructions. Should a conflict exist, the contractor shall notify the engineer in writing so that the conflict can be resolved with a minimum amount of delay.
3. All work on this project shall be performed in accordance with applicable federal, state and local laws, rules and regulations concerning construction safety and health.
4. All construction within public right-of-way shall be performed in accordance with applicable City of Albuquerque Standards and Procedures.
5. If any utility lines, pipelines, or underground utility lines are shown on these drawings, they are shown in an approximate manner only, and such lines may exist where none are shown. If any such existing lines are shown, the location is based upon information provided by the owner of said utility, and the information may be incomplete, or may be obsolete by the time construction commences. The engineer has considered only preliminary investigation of the location, depth, size, or type of existing utility lines, pipelines, or underground utility lines. This investigation is not comprehensive, and may not be complete, therefore, makes no representation pertaining thereto, and assumes no responsibility or liability therefor. The contractor shall inform itself of the location of any utility line, pipeline, or underground utility line in or near the area of the work in advance of and during excavation work. The contractor is fully responsible for any and all damage caused by its failure to locate, be fully and preserve any and all existing utilities, pipelines, and underground utility lines. In planning and conducting excavation, the contractor shall comply with state statutes, municipal and local ordinances, rules and regulations, if any, pertaining to the location of these lines and facilities.
6. The design of planters and landscaped areas is not part of this plan. All planters and landscaped areas adjacent to the buildings shall be provided with positive drainage to avoid any ponding adjacent to the structure. For construction details, refer to landscape plan.

Erosion Control Measures

1. The contractor shall ensure that no soil erodes from the site into public right-of-way or onto private property. This can be achieved by constructing temporary berms at the property lines and cutting the soil to keep it from flowing.
2. The contractor shall promptly clean up any material excavated within the public right-of-way so that the excavated material is not susceptible to being washed down the street.
3. The contractor shall secure "Grading Operations Permit" prior to beginning construction.

DRAINAGE PLAN

The following items concerning the Western Building Supply Drainage Plan are contained hereon:

1. Vicinity Map 2. Grading Plan 3. Calculations

As shown by the Vicinity Map, the site is located on the north side of Paseo Del Norte Boulevard N.E. between Edith Boulevard N.E. and Washington Street N.E. At present, the site is developed as a commercial business with a building and asphalt paving. The building and paving occupy much of the easterly portion of the site while the westerly portion remains undeveloped. Adjacent sites to the east are also commercially developed with warehouse-like facilities.

Panel 9 of 50 of the National Flood Insurance Program Flood Boundary and Floodway Maps for the City of Albuquerque, New Mexico dated October 16, 1980, indicate that this site does not lie within a 100-year flood plain. Further review does not indicate that this site contributes runoff to an existing flood hazard area. The review of this panel, coupled with site observation reveals that offsite flows do not impact this site. The warehouses that are constructed to the east drain in a southerly and northerly direction toward the railroad spur to the north and Paseo Del Norte Boulevard N.E. to the south. At present, the site drains from east to west to the southwesterly tip of the site. Runoff not retained in ponds previously constructed as depressions on the site will enter an existing storm inlet which has been constructed as part of the Paseo Del Norte Boulevard N.E. construction project. This was a New Mexico State Highway and Transportation Department project. This inlet discharges runoff through a culvert onto the property which lies to the west of the site. It is because of this existing condition that the discharge of developed runoff from this site to that storm drain facility is not permissible and that 100% retention of the developed runoff will be implemented.

The Grading Plan shows 1) existing and proposed grades indicated by spot elevations and contours at 1'0" intervals, 2) the limit and character of the existing improvements, 3) the limit and character of the proposed improvements, and 4) continuity between existing and proposed grades. As shown by this plan, the proposed improvements consist of the construction of a new warehouse building along the east portion of the site and the construction of new asphalt paving and two truck docks along the west side of the building. The construction of the new paving and truck docks will interfere with the existing drainage improvements previously constructed on this site. Consequently, a new retention ponding facility is proposed to hold a maximum depth of 2.0 feet. The retention pond is located to the west of the proposed paving improvements so that the developed runoff generated east of the facility can be collected and held within the pond.

The Calculations which appear hereon analyze both the existing and developed conditions for the 100-year, 24-hour rainfall event. Normally the 6-hour rainfall event is used for the hydrologic computations for a project of this size. However, due to the fact that the retention of runoff is proposed, the 24-hour runoff volume is more appropriate. The Rational Method has been used to quantify the peak rate of discharge generated by the site while the SCS Method has been used to quantify the volume of runoff generated. The Average End Area Method has been used to determine the pond volume. As shown by these calculations, the proposed improvements will result in an increase in developed runoff anticipated from this facility. It is also apparent that ample pond volume is provided to retain 100% of the developed runoff anticipated from this site. As stated above, the 24-hour rainfall event has been used for these calculations so that ample pond volume can be provided. Both Methods have been used in accordance with the City of Albuquerque Development Process Manual, coupled with the Mayor's Emergency Rule dated January 14, 1980.

CALCULATIONS

Ground Cover Information

From SCS Bernalillo County Soil Survey,
Plate 111 ESR (Embedo gravelly fine sandy loam)
Hydrologic Soil Group: B
Existing Pervious CN = 79 (DPM Plate 22.2 C-2
Pasture or Range Lands fair condition)
Developed Pervious CN = 79 (DPM Plate 22.2 C-2
Pasture or Range Lands fair condition)

Time of Concentration, Time to Peak

$T_c = 0.0078\ L^{0.77}/S^{0.385}$ (Kirpich Equation)

$T_p = T_c + 10$ min.

Point Rainfall

$P_{24} = 2.6$ in. (100-year, 24-hr)

Rational Method

Discharge: Q = CIA

where C varies
$I = P_{24}/(T_p/24)^{0.51} = 5.50$ in/hr
$P_{24} = 2.6$ in
$T_p = 15$ min (minimum)
A = area, acres

SCS Method

Volume: V = 3630(DRO) A

Where DRO = Direct runoff in inches
A = area, acres

Existing Condition

$A_{total} = 158,620$ sf = 3.64 Ac
Roof area = 26,610 sf (0.17)
Paved area = 26,720 sf
Landscaped area = 105,190 sf (0.66)
C = 0.48 (Weighted average per Emergency Rule, 1/14/86)
$Q_{100} = CIA = 0.48*5.50(3.64) = 9.4$ cfs
$A_{imp} = 53,330$ sf % impervious = 34 %
Composite CN = 84 (DPM Plate 22.2 C-2)
DRO = 0.90 in (DPM Plate 22.2 C-4)
$V_{100} = 3630$ (DRO) A = 11,800 cf

Developed Condition

$A_{total} = 158,620$ sf = 3.64 Ac
Roof area = 32,990 sf (0.21)
Paved area = 43,220 sf (0.27)
Landscaped area = 82,410 sf (0.52)
C = 0.57 (Weighted average per Emergency Rule, 1/14/86)
$Q_{100} = CIA = 0.57(5.50)(3.64) = 11.4$ cfs
$A_{imp} = 75,610$ sf % impervious = 48 %
Composite CN = 86 (DPM Plate 22.2 C-2)
DRO = 1.15 in (DPM Plate 22.2 C-4)
$V_{100} = 3630$ (DRO)A = 15,200 cf

$V_{pond} = (A_{bg} + A_{ss}) \times .5 \times 1 + (a_{ss} + A_{ss}) \times .5 \times .5$
$A_{bg} = 12,600$ sf
$A_{ss} = 9,900$ sf
$A_{ss} = 7,500$ sf
$V_{pond} = 15,600$ cf $> V_{100}$

Comparison

$Q_{100} = 11.4 - 9.4 = 1.8$ cfs (increase)
$V_{100} = 15,200 - 11,800 = 3,400$ cf (increase)

HYDROLOGY APPROVAL & INSPECTION
APPROVED FOR BUILDING PERMIT
ENGINEER Robert Wolfe DATE 3/27/90
INSPECTION REQUESTED DATE 4-4-90
APPROVAL DATE ____ DISAPPROVED ____
SDP APPROVAL DATE N/A
SURVEY DATE 4-3-90
HYDROLOGY FILE NO/PAGE NO 89-1 pg 75 #37
CONDITIONS
CONTACT EE Lee 9600

STATE OF NEW MEXICO
ROBERT J. TRAUERNICHT
12B3
REGISTERED ARCHITECT

akt architects
po box 1366
albuqu nm 87103 (505)281-1560

project
WESTERN BUILDING SUPPLY
WAREHOUSE

project no. 900021
date 01-90
sheet title

RECEIVED
MAR 21 1990
HYDROLOGY DIVISION

1

JEFFREY G. MORTENSEN
NEW MEXICO
8547
REGISTERED PROFESSIONAL ENGINEER

03-20-90

jma JEFF MORTENSEN & ASSOCIATES, INC.
810 DALLAS, N.E. ALBUQUERQUE, NM 87110
ENGINEERS TELEPHONE (505) 265-5611

GRADING AND DRAINAGE PLAN
WESTERN BUILDING SUPPLY

DESIGNED BY J.G.M.
DRAWN BY S.G.H.
APPROVED BY J.G.M.

NO.	DATE	BY	REVISIONS
1	02/90	JGM	ADD PAVING & RAMPS, ADD F.F. ELEVATION, REVISE GRADES
2	03/20/90	GCF	REVISED PAVING LIMITS

JOB NO. 900021
DATE 01-90
SHEET 1 OF 1

PLAN SHOT TWICE
ONCE ON LIGHT
ONCE ON DARK

CITY OF ALBUQUERQUE
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R# 24 DATE 3/4/92 OP 27

CITY OF ALBUQUERQUE
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The photographic process used meets the Basic Microfilm Standards of the National Micrographics Association (MS115-1977)
DIRECTOR 11/5/92

NOTARY PUBLIC
OFFICIAL SEAL
Robert Duran
NOTARY PUBLIC
STATE OF NEW MEXICO
8/10/92 Bond

F30 D24 C20 B16 A12

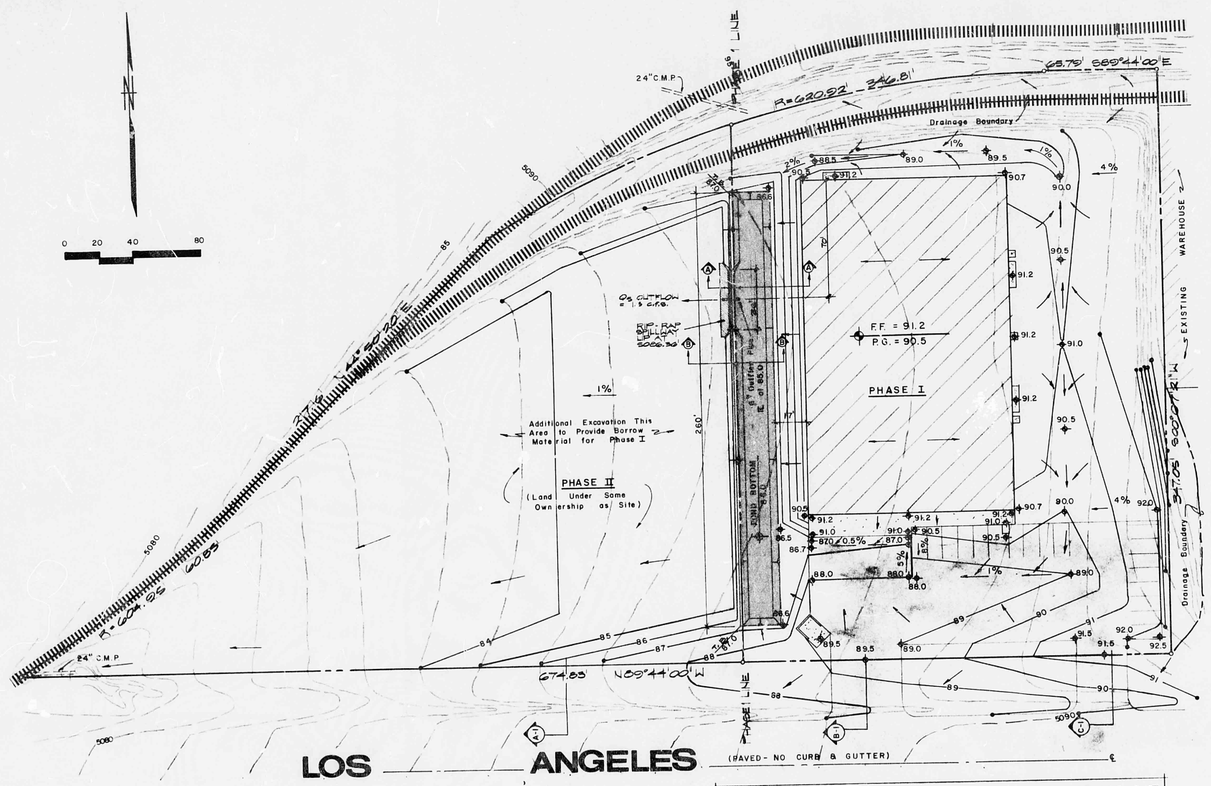
CITY OF ALBUQERQUE C D F

HYDROLOGY

C17/D8
ONE PLAN

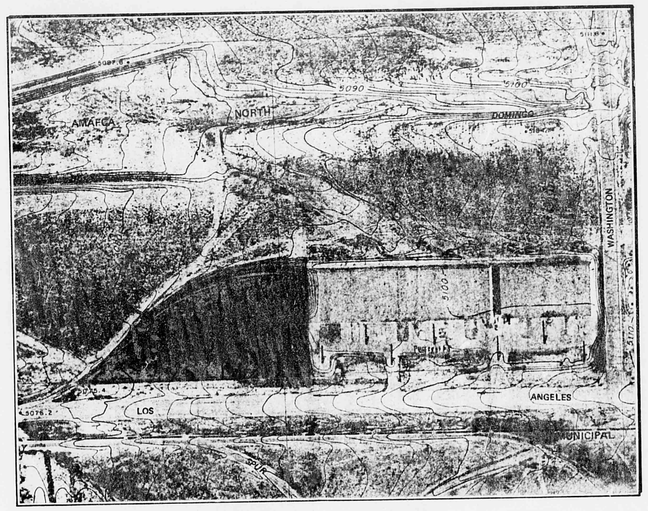
C17- D8

1 PLAN



SITE PLAN 1"=40'-0"

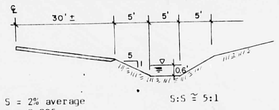
NOTE: CONTRACTOR'S OPTION: CONTRACTOR MAY USE ASPHALT PAVING OR R.P.RAP FOR SPILLWAY CONSTRUCTION



OFFSITE FLOWS

Offsite Flows along North side of Los Angeles Blvd.

Offsite Area = 8.6 Acres
Q100 = (5.4)(8.6)(0.55) = 25 cfs
Capacity of Existing Side Ditch - side slopes and bottom width varies; values shown are averages.



S = 2% average
n = 0.025
Dn = 0.6'
Vel = 5'/sec.

SECTION A-1

LEGEND

	FINISHED CONTOUR
	EXISTING CONTOUR
89.5	FINISHED SPOT ELEVATION
	SURFACE FLOW
	RAILROAD TRACKS

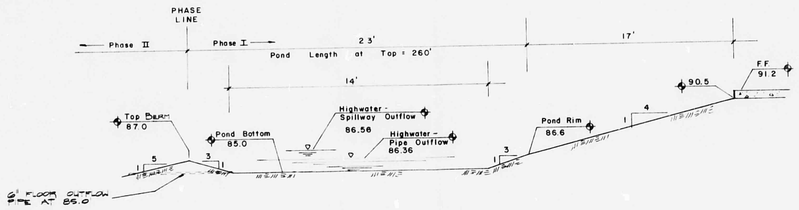
APPROVED FOR DRAINAGE 9-28-81 Benny Montoya SIGNATURE: CITY ENGINEER ASSISTANT CITY ENGINEER DRAINAGE DIVISION WITH CONDITIONS NOTED Revised

GRADING/DRAINAGE

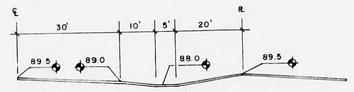
NOTE: DRAINAGE REPORT & GRADING PLAN WESTERN BUILDING SUPPLY ON LOS ANGELES BLVD. NE MARCH 1981 has been submitted to the City Engineer

Notes

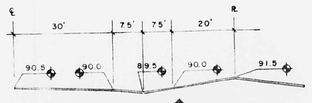
- Offsite flows originating along north side of Los Angeles Blvd. are carried within existing side ditch without affecting proposed site.
- Driveway connections to site will be paved dip sections to convey surface flow and expected trash from City dump across the intersections.
- Existing grades of Los Angeles Blvd. adjacent to site will not be subject to change due to improvements of Washington at Los Angeles to the east, and the existing railroad spur located to the west of the site. These improvements, plus the existence of a master water line in the R/W, will dictate that future construction of Los Angeles follow existing grades.



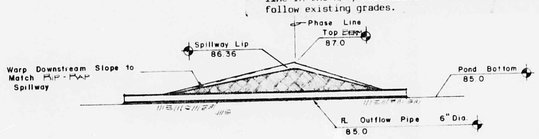
SECTION A



SECTION B-1



SECTION C-1



SECTION B

PREPARED BY Consultants terra-tol inc.

4201 Los Angeles N.E.

REVISED 4-23-81 TO ADD SECTIONS A-1, B-1, & C-1

C17/D8

BILL J. SHELTON & ASSOCIATES ARCHITECTS, A.I.A.
1113 RHODE ISLAND NE ALBUQUERQUE, NEW MEXICO 87110 TELEPHONE (505) 262-2363

REGISTERED ARCHITECT BILL J. SHELTON No. 1099 STATE OF NEW MEXICO

NAME-LOCATION OF BUILDING
WESTERN BUILDING SUPPLY

REVISIONS
4-23-81 RT-MAP

DATE 4-6-81

SHEET NO 2 of 7