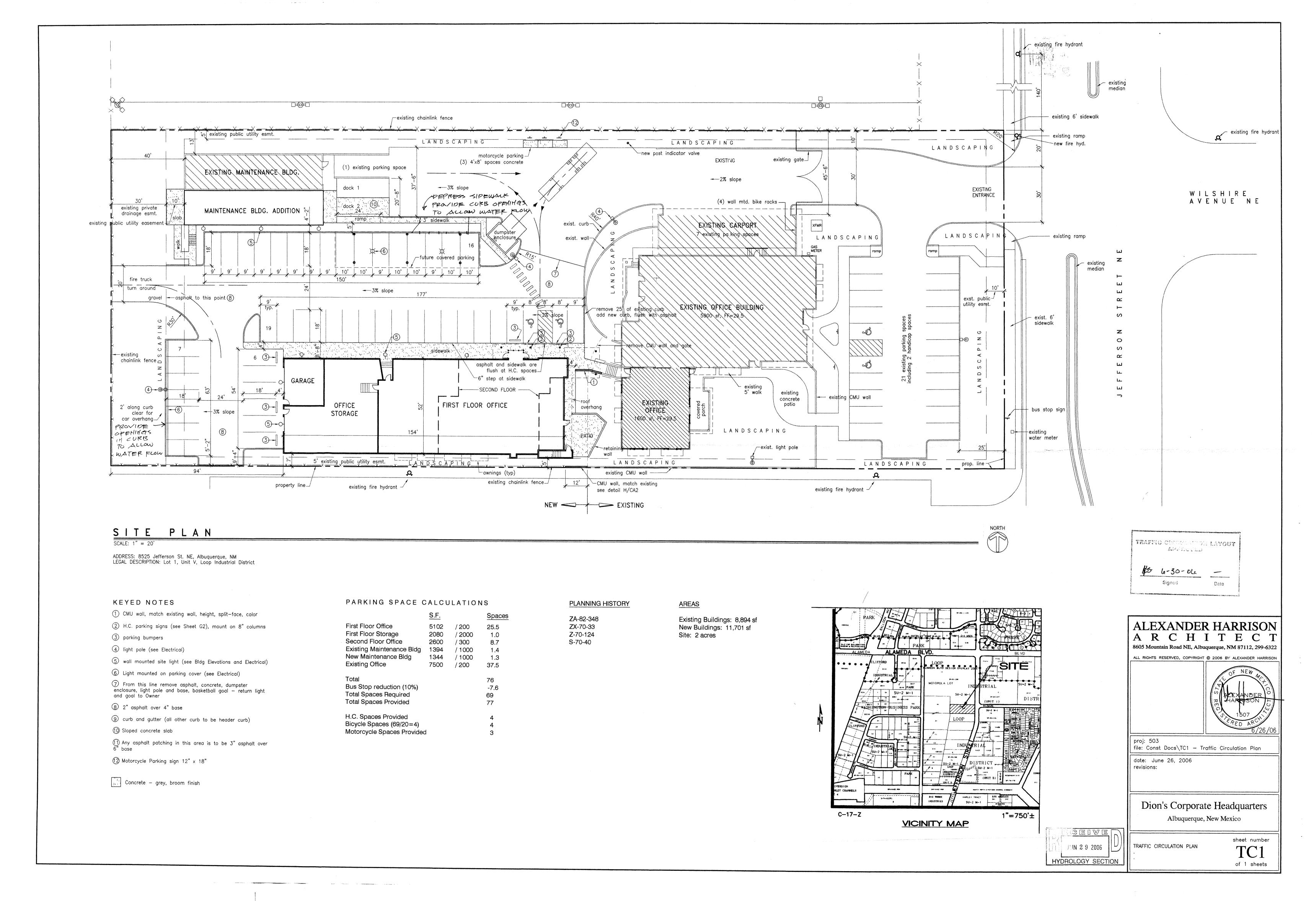
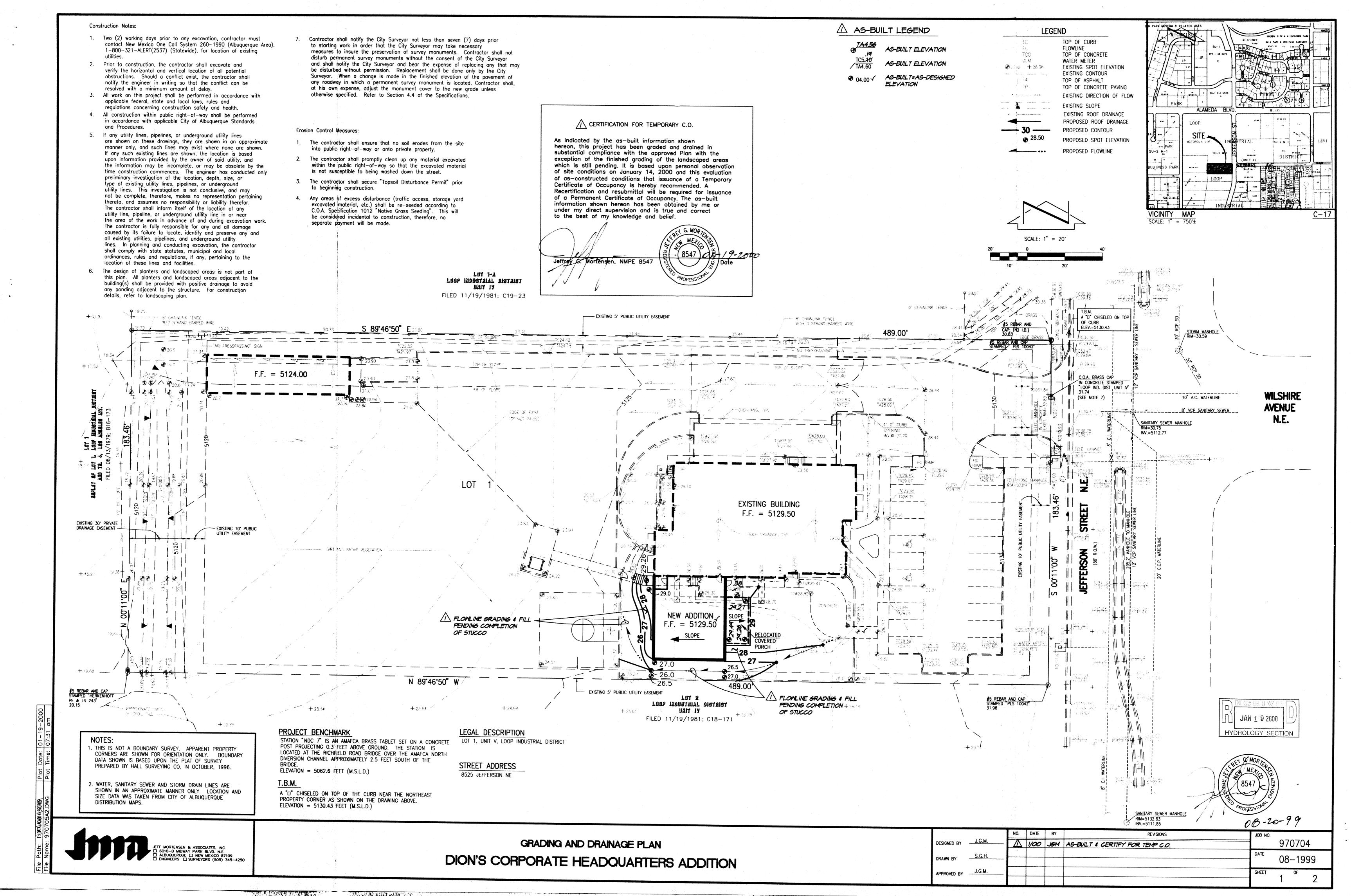


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THIS SUBMITTAL IS MADE IN SUPPORT OF A BUILDING PERMIT FOR THE PROPOSED ADDITION.

II. PROJECT DESCRIPTION

AS SHOWN BY THE VICINITY MAP, THE SITE IS LOCATED ON THE WEST SIDE OF JEFFERSON NE JUST SOUTH OF ALAMEDA BLVD. NE. THE CURRENT LEGAL DESCRIPTION IS LOT 1, LOOP INDUSTRIAL DISTRICT, UNIT V. AS SHOWN BY PANEL 137 OF 825 OF THE NATIONAL FLOOD INSURANCE PROGRAM FLOOD INSURANCE RATE MAPS PUBLISHED BY FEMA FOR BERNALILLO COUNTY, NEW MEXICO, SEPTEMBER 20, 1996, THIS SITE DOES NOT LIE WITHIN NOR ADVERSELY IMPACT A DESIGNATED FLOOD HAZARD ZONE (ZONE A).

III. BACKGROUND DOCUMENTS

THE FOLLOWING IS A BRIEF LIST OF DRAINAGE PLANS RELEVANT TO THE DEVELOPMENT OF THIS SITE:

1. MASTER DRAINAGE PLAN PREPARED BY LEEDSHILL HERKENHOFF (C17/D12). THIS PLAN STATES THAT "LOTS 1-11 SHALL BE DEVELOPED SUCH THAT ONSITE STORM WATERS FLOW WEST INTO THE PRIVATE DRAINAGE EASEMENT AND FROM THERE INTO THE DOMINGO, BACA APPROVO, THROUGH A SOFT LINED CHANNEL.

THE PRIVATE DRAINAGE EASEMENT AND FROM THERE INTO THE DOMINGO BACA ARROYO THROUGH A SOFT LINED CHANNEL. THERE IS NO PONDING REQUIREMENT FOR ANY OF THESE LOTS PROVIDED THEY CONFORM TO THE EXTENT OF DEVELOPMENT AS MENTIONED UNDER 'GENERAL' AND 'RECOMMENDATIONS'." THESE RECOMMENDATIONS STATE THAT THE TOTAL IMPERVIOUS LAND TREATMENT CANNOT EXCEED 73% OR ELSE ONSITE DETENTION IS REQUIRED.

2. GRADING AND DRAINAGE PLAN (C17/D13G) FOR "DION'S CORPORATE HEADQUARTERS" DATED 03-10-99 PREPARED BY THIS OFFICE AND SUBSEQUENTLY CERTIFIED BY THE UNDERSIGNED ON 08-13-98. THIS PLAN VALIDATES THE DRAINAGE CONCEPTS SET FORTH BY THE MASTER DRAINAGE PLAN AND ESTABLISHES THE EXISTING CONDITIONS OF THE SITE BASED UPON THE PREVIOUS DEVELOPMENT.

IV. EXISTING CONDITIONS

THE EXISTING CONDITIONS OF THE SITE ARE ILLUSTRATED BY THE ABOVE REFERENCED DRAINAGE CERTIFICATION DATED 08-13-98. AT PRESENT, THE SITE DISCHARGES ITS DEVELOPED RUNOFF TO AN EXISTING PRIVATE DRAINAGE EASEMENT LOCATED ALONG THE WEST PROPERTY LINE OF THE SITE. FROM THIS POINT, THE RUNOFF FLOWS SOUTH WITHIN THE SOFT LINED EASEMENT TO ENTER THE DOMINGO BACA ARROYO, THE OUTFALL.

V. DEVELOPED CONDITIONS

THE PROPOSED DEVELOPMENT CONSISTS OF AN ATTACHED BUILDING ADDITION TO THE LARGE OFFICE BUILDING ON SITE. THE ADDITION IS LOCATED AT THE SOUTHWEST CORNER OF THE EXISTING BUILDING WITHIN A LANDSCAPED COURTYARD AREA. ROOF RUNOFF WILL BE DIRECTED TO THE WEST. COURTYARD RUNOFF WILL BE DIRECTED AROUND THE ADDITION TO DISCHARGE AT THE EXISTING GATE WEST OF THE ADDITION. FROM THIS POINT, RUNOFF WILL FLOW ACROSS EXISTING PAVING AND WILL EVENTUALLY REACH THE ABOVE REFERENCED PRIVATE DRAINAGE EASEMENT. FROM THIS POINT, THE RUNOFF WILL FOLLOW THE PATTERN DESCRIBED ABOVE UNDER "EXISTING CONDITIONS".

DRAINAGE PLAN (CONTINUED)

VI. GRADING PLAN

THE GRADING PLAN SHOWS 1.) EXISTING GRADES INDICATED BY SPOT ELEVATIONS AND CONTOURS AT 1'-0" INTERVALS COMPILED FROM THE PREVIOUSLY APPROVED GRADING AND DRAINAGE PLAN AND DRAINAGE CERTIFICATION FOR THE SITE PREPARED BY THIS OFFICE, 2.) PROPOSED GRADES INDICATED BY SPOT ELEVATIONS AND CONTOURS AT 1'-0" INTERVALS, 3.) THE LIMIT AND CHARACTER OF THE EXISTING IMPROVEMENTS COMPILED FROM THE PREVIOUSLY APPROVED GRADING AND DRAINAGE PLAN AND DRAINAGE CERTIFICATION PREPARED BY THIS OFFICE, 4.) THE LIMIT AND CHARACTER OF THE PROPOSED IMPROVEMENTS, AND 5.) CONTINUITY BETWEEN EXISTING AND PROPOSED GRADES. THIS PLAN ILLUSTRATES THE DRAINAGE PATTERNS DESCRIBED IN THE SECTION ABOVE.

V11. CALCULATIONS

THE CALCULATIONS WHICH APPEAR HEREON ANALYZE BOTH THE EXISTING AND DEVELOPED CONDITIONS FOR THE 100-YEAR, 6-HOUR RAINFALL EVENT. THE PROCEDURE FOR 40 ACRE AND SMALLER BASINS, AS SET FORTH IN THE REVISION OF SECTION 22.2, HYDROLOGY OF THE DEVELOPMENT PROCESS MANUAL, VOLUME 2, DESIGN CRITERIA, DATED JANUARY, 1993, HAS BEEN USED TO QUANTIFY THE PEAK RATE OF DISCHARGE AND VOLUME OF RUNOFF GENERATED. AS SHOWN BY THESE CALCULATIONS, THERE WILL BE A SLIGHT INCREASE IN RUNOFF ASSOCIATED WITH THE PROPOSED CONSTRUCTION. FURTHERMORE, THE PERCENTAGE OF IMPERVIOUS LAND TREATMENT FALLS WELL BELOW THE THRESHOLD VALUE SET FORTH IN THE MASTER DRAINAGE PLAN.

VII. CONCLUSION

MAINTAINED.

THE CONTINUED FREE DISCHARGE OF RUNOFF FROM THIS SITE TO THE DOMINGO BACA ARROYO IS APPROPRIATE DUE TO FOLLOWING FACTORS:

1. MODIFICATION TO AN EXISTING SITE WITHIN AN INFILL AREA

2. NEGLIGIBLE INCREASE IN DEVELOPED RUNOFF

3. PROXIMITY TO DOWNSTREAM FACILITIES AND APPARENT DOWNSTREAM CAPACITY

4. CONFORMANCE WITH PREVIOUSLY APPROVED PLANS
5. NO IMPACT ON DOWNSTREAM FLOOD ZONES
THE PRIVATE DRAINAGE EASEMENT REFERENCED ABOVE IS OWNED,
OPERATED AND MAINTAINED BY EACH UNDERLYING PROPERTY OWNER.

THE DOMINGO BACA ARROYO IS PUBLICLY OWNED, OPERATED AND

CALCULATIONS

Site Characteristics

1. Precipitation Zone =

2. $P_{6,100} = P_{360} = 2.35$ in.

3. Total Area $(A_T) = 89,710 \text{ sf}/2.06 \text{ ac}$

4. Existing Land Treatment

Treatment	Area (sf/ac)	%
В	16,310/0.37	18
C	33,390/0.77	3
D	40,010/0.92	4

5. Developed Land Treatment

Treatment	Area (sf/ac)		
В	16,310/0.37	18	
С	31,887/0.73	35	
D	41,513/0.95	47	

Existing Condition

1. Volume

 $E_{\mathbf{W}} = (E_{\mathbf{A}}A_{\mathbf{A}} + E_{\mathbf{B}}A_{\mathbf{B}} + E_{\mathbf{C}}A_{\mathbf{C}} + E_{\mathbf{D}}A_{\mathbf{D}})/A_{\mathbf{T}}$

 $E_{W} = [(0.78)(0.37)+(1.13)(0.77)+(2.12)(0.92)]/2.06 = 1.51 in.$

 $V_{100} = (E_{w}/12)$

 $V_{100} = (1.51/12)2.06 = 0.2591$ ac.ft.; 11,290 cf

2. Peak Discharge

 $Q_p = Q_{PA}A_A + Q_{PB}A_B + Q_{PC}A_C + Q_{PD}A_D$

 $Q_p = Q_{100} = (2.28)(0.37) + (3.14)(0.77) + (4.70)(0.92) = 7.6 \text{ cfs}$

Developed Condition

1. Volume

 $E^{\mathbf{M}} = (E^{\mathbf{A}} \mathbf{A}^{\mathbf{A}} + E^{\mathbf{B}} \mathbf{A}^{\mathbf{B}} + E^{\mathbf{C}} \mathbf{A}^{\mathbf{C}} + E^{\mathbf{D}} \mathbf{A}^{\mathbf{D}}) / \mathbf{A}^{\mathbf{L}}$

 $E_{w} = [(0.78)(0.37) + (1.13)(0.73) + (2.12)(0.95)]/2.06 = 1.52 in.$

 $V_{100} = (E_W/12)A_T$

 $V_{100} = (1.52/12)2.06 = 0.2609$ ac.ft.; 11,370 cf

2. Peak Discharge

 $d^{b} = d^{b} d^{b} + d^{b} d^{b} + d^{b} d^{c} + d^{b} d^{b}$

 $Q_{\rm p} = Q_{100} = (2.28)(0.37) + (3.14)(0.73) + (4.70)(0.95) = 7.6 \text{ cfs}$

Comparison

1. $\Delta V_{100} = 11,370 - 11,290 = 80 \text{ cf} = 0.0018 \text{ ac.ft.}$ (increase)

2. $\Delta Q_{100} = 7.6 - 7.6 = 0$ (NO CHANGE)

CERTIFICATION FOR TEMPORARY C.O.

As indicated by the as-built information shown hereon, this project has been graded and drained in substantial compliance with the approved Plan with the exception of the finished grading of the landscaped areas which is still pending. It is based upon personal observation of site conditions on January 14, 2000 and this evaluation of as-constructed conditions that issuance of a Temporary Certificate of Occupancy is hereby recommended. A Recertification and resubmittal will be required for issuance of a Permanent Certificate of Occupancy. The as-built information shown hereon has been obtained by me or under my direct supervision and is true and correct to the best of my knowledge and belief.

ey-G. Mortenser, NMPE 8547

AN 1 9 2000 DLOGY SECTION

JAN 1 9 2000

HYDROLOGY SECTION

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2 - 9 9

JEFF MORTENSEN & ASSOCIATES, INC.

| 6010-B MIDWAY PARK BLVD. N.E.

| ALBUQUERQUE | NEW MEXICO 87109

| ENGINEERS | SURVEYORS (505) 345-4250

DRAINAGE PLAN AND CALCULATIONS
DION'S CORPORATE HEADQUARTERS ADDITION

DESIGNED BY J.G.M.

DRAWN BY D.L.M.

APPROVED BY J.G.M.

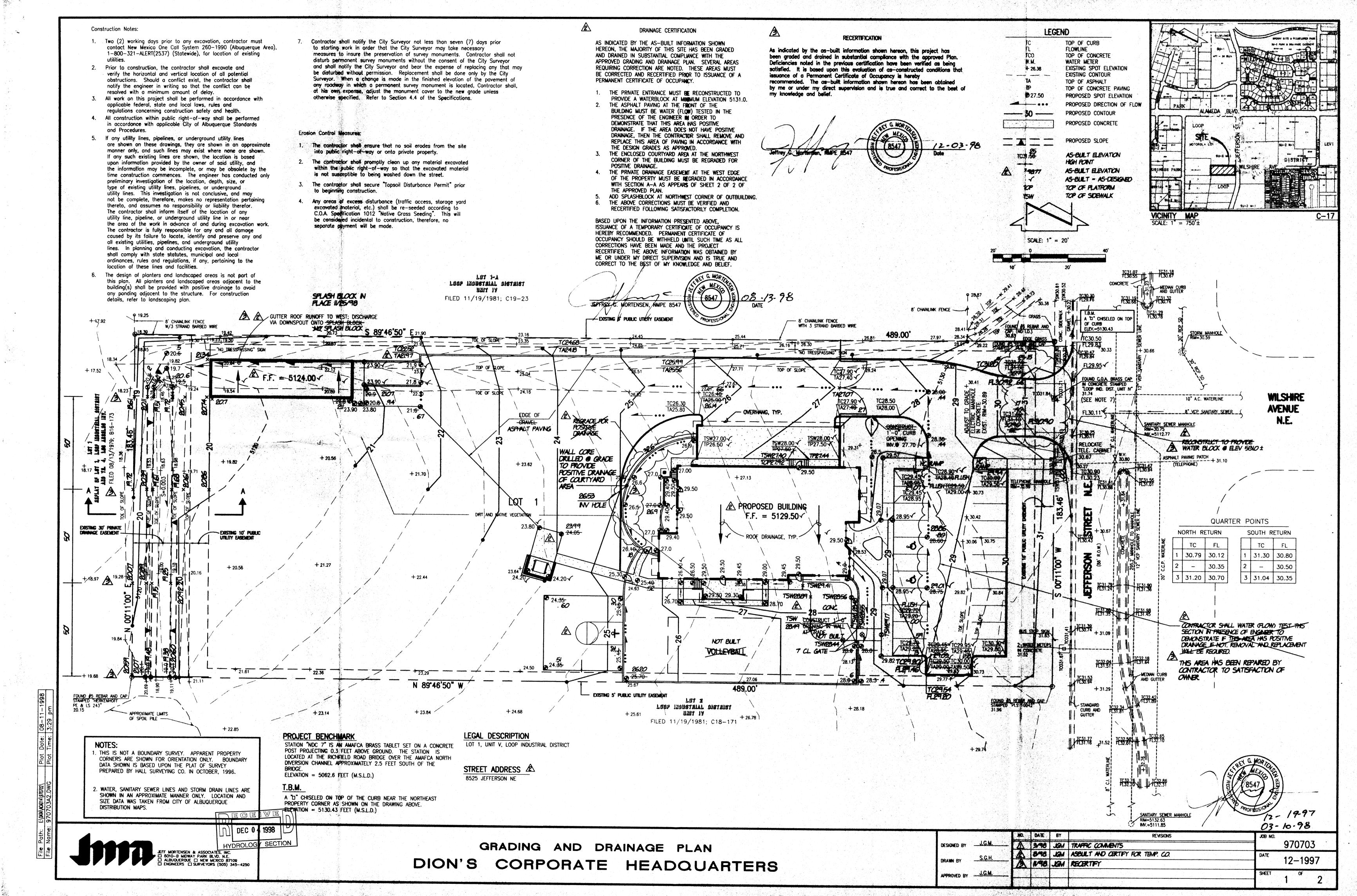
DATE BY REVISIONS

JOB NO.

970704

DATE 08-1999

SHEET 2 OF 2



Grading Plan

As shown by the Vicinity Map, the site is located on the west side of Jefferson Street N.E., between Paseo del Norte and Alameda. The present legal description is "Lot 1, Loop Industrial District, Unit V".

As shown by Panel 137 of 825 of the National Flood Insurance Program Flood Insurance Rate Maps published by F.E.M.A. for Bernalillo County, New Mexico dated September 20, 1996, this site does not lie within a designated flood hazard zone. As stated by the Master Drainage Plan prepared by Leedshill Herkenhoff, C17/D12, "Lots 1-11 shall be developed such that onsite storm waters flow west into the private drainage easement and from there into the Domingo Baca Arroyo through a soft lined channel. There is no panding requirement for any of these lots provided they conform to the extent of the development as mentioned under 'General' and 'Recommendations'." These recommendations state that the total impervious land treatment cannot exceed 73%, or else onsite detention is required. In the proposed condition, the site will have a 45% total impervious area, therefore, no ansite detention facilities are proposed. In addition, a bottom width of ten feet will be graded for the onsite drainage easement as required per the above referenced Master Drainage Plan.

The Grading Plan shows: 1) existing and proposed grades indicated by spat elevations and contours at 1'0" intervals, 2) the limit and character of the existing improvements, 3) the limit and character of the proposed improvements, and 4) continuity between existing and proposed grades. As shown by this plan, the proposed construction consists of an office building with associated parking and landscaping. Developed flows for the site are directed to the west toward an existing private drainage easement. This easement outfalls into the North Diversion Channel located south of the site

the site is bordered by Jefferson Street, a fully developed read. The site and does not allow flows to enter the site. The property to the south (Lot 2) was under construction at the time of the survey (Hydrology file. C17/D13E) and should not contribute offsite flows in its developed condition At the time of the survey, there was a spoils pile on the adjacent property which blocks flows from Lot 1 within the drainage easement. This pile must be removed as part of the grading of Lot 2. The land to the west is topographically lower, hence does not contribute offsite flows.

The Calculations which appear hereon analyze both the existing and developed conditions for the 100-year, 6-hour rainfall event. The Procedure for 40acre and Smaller Basins, as set forth in the Revision of Section 22.2, Hydrology of the Development Process Manual, Volume 2, Design Criteria, dated January, 1993, has been used to quantify the peak rate of discharge and volume of runoff generated. As shown by these calculations, there will be an increase in the peak rate and total volume of runoff generated by the site.

CALCULATIONS

Site Characteristics

1. Precipitation Zone =

2. $P_{6,100} = P_{360} = 2.35$ in.

3. Total Area $(A_T) = 89,710 \text{ sf}/2.06 \text{ ac}$

4. Existing Land Treatment Treatment

> Developed Land Treatment Area (sf/ac) 16,310/0.37 33,390/0.77 40,010/0.92

Area (sf/ac)

37

Existing Condition

 $E^{\mathbf{M}} = (E^{\mathbf{A}} \mathbf{A}^{\mathbf{A}} + E^{\mathbf{B}} \mathbf{A}^{\mathbf{B}} + E^{\mathbf{C}} \mathbf{A}^{\mathbf{C}} + E^{\mathbf{D}} \mathbf{A}^{\mathbf{D}}) / \mathbf{A}^{\mathbf{D}}$ $E_{W} = (1.13)(2.06)/2.06 = 1.13$ in.

 $V_{100} = (E_W/12)A_T$

 $V_{100} = (1.13/12)2.06 = 0.1940 \text{ ac.ft.}; 8,450 \text{ cf}$

2. Peak Discharge

 $o^{b} = o^{b} v^{A} + o^{b} v^{B} + o^{b} v^{C} + o^{b} v^{D}$

 $Q_p = Q_{100} = (3.14)(2.06) = 6.5 \text{ cf}$

 $E^{\mathbf{M}} = (E^{\mathbf{A}} \mathbf{A}^{\mathbf{A}} + E^{\mathbf{B}} \mathbf{A}^{\mathbf{B}} + E^{\mathbf{C}} \mathbf{A}^{\mathbf{C}} + E^{\mathbf{D}} \mathbf{A}^{\mathbf{D}}) / \mathbf{A}^{\mathbf{L}}$

 $E_W = [(0.78)(0.37) + (1.13)(0.77) + (2.12)(0.92)]/2.66 = 1.51 in.$

 $V_{100} = (E_W/12)A_T$

 $V_{100} = (1.51/12)2.06 = 0.2591$ ac.ft.; 11,290 cf

2. Peak Discharge

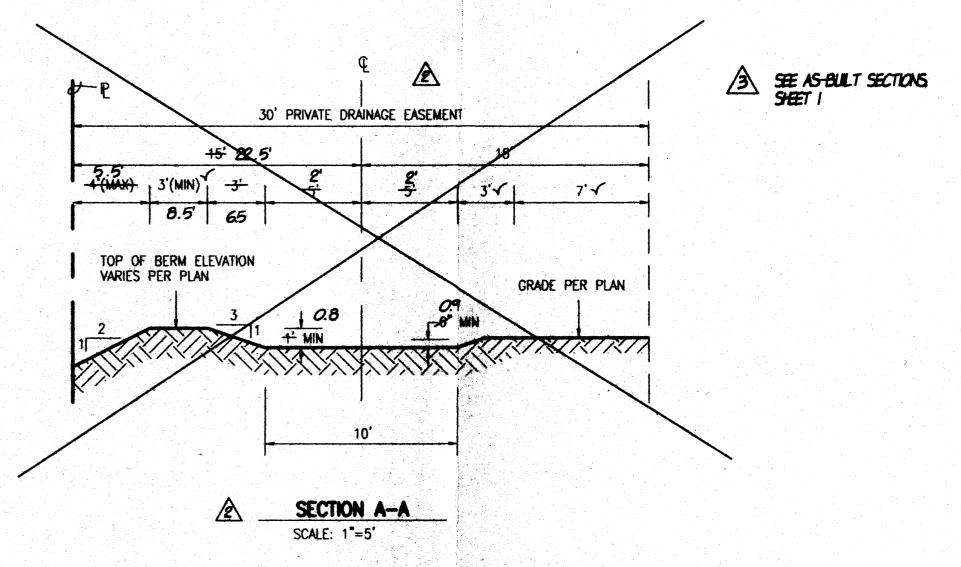
 $o^b = o^{b} v^a + o^{b} v^B + o^{b} v^C + o^{b} v^D$

 $Q_{\rm p} = Q_{100} = (2.28)(0.37) + (3.14)(0.77) + (4.70)(0.92) = 7.6 \text{ cfs}$

Comparison

1. $\Delta V_{100} = 11,290 - 8,450 = 2,840 \text{ cf} = 0.0651 \text{ ac.ft. (increase)}$

2. $\Delta Q_{100} = 7.6 - 6.5 = 1.1 \text{ cfs (increase)}$



CHANNEL CAPACITY AT DEPTH = 0,5':

d = 0.5n = 0.030

S = 0.0030

A = 5.75 sf

 $Q = (1.49/n)AR^{2/3}S^{1/2} = 9.00 \text{ cfs} > 0.00$

DRAINAGE CERTIFICATION

AS INDICATED BY THE AS-BUILT INFORMATION SHOWN HEREON, THE MAJORITY OF THIS SITE HAS BEEN GRADED AND DRAINED IN SUBSTANTIAL COMPLIANCE WITH THE APPROVED GRADING AND DRAINAGE PLAN. SEVERAL AREAS REQUIRING CORRECTION ARE NOTED. THESE AREAS MUST BE CORRECTED AND RECERTIFIED PRIOR TO ISSUANCE OF A PERMANENT CERTIFICATE OF OCCUPANCY.

- 1. THE PRIVATE ENTRANCE MUST BE RECONSTRUCTED TO PROVIDE A WATERBLOCK AT MINIMUM ELEVATION
- THE ASPHALT PAVING AT THE FRONT OF THE BUILDING MUST BE WATER (FLOW) TESTED IN THE PRESENCE OF THE ENGINEER IN ORDER TO DEMONSTRATE THAT THIS AREA HAS POSITIVE DRAINAGE. IF THE AREA DOES NOT HAVE POSITIVE DRAINAGE, THEN THE CONTRACTOR SHALL REMOVE AND REPLACE THIS AREA OF PAVING IN ACCORDANCE WITH THE DESIGN GRADES AS APPROVED.
- 3. THE ENCLOSED COURTYARD AREA AT THE NORTHWEST CORNER OF THE BUILDING MUST BE REGRADED FOR POSITIVE DRAINAGE.
- 4. THE PRIVATE DRAINAGE EASEMENT AT THE WEST EDGE OF THE PROPERTY MUST BE REGRADED IN ACCORDANCE WITH SECTION A-A AS APPEARS OF SHEET 2 OF 2 OF THE APPROVED PLAN.
 5. ADD SPLASHBLOCK AT NORTHWEST CORNER OF
- THE ABOVE CORRECTIONS MUST BE VERIFIED AND RECERTIFIED FOLLOWING SATISFACTORILY COMPLETION.

BASED UPON THE INFORMATION PRESENTED ABOVE, ISSUANCE OF A TEMPORARY CERTIFICATE OF OCCUPANCY IS HEREBY RECOMMENDED. PERMANENT CERTIFICATE OF OCCUPANCY SHOULD BE WITHHELD UNTIL SUCH TIME AS ALL CORRECTIONS HAVE BEEN MADE AND THE PROJECT RECERTIFIED. THE ABOVE INFORMATION WAS OBTAINED BY ME OR UNDER MY DIRECT SUPERVISION AND IS TRUE AND CORRECT TO THE BEST OF MY KNOWLEDGE AND BELIEF.

RECERTIFICATION

As indicated by the as-built information shown hereon, this project has been graded and drained in substantial compliance with the approved Plan. Deficiencies noted in the previous certification have been verified as being satisfied. It is based upon this evaluation of as-constructed conditions that issuance of a Permanent Certificate of Occupancy is hereby recommended. The as-built information shown hereon has been obtained by me or under my direct supervision and is true and correct to the best of my knowledge and belief.

12-03-98

03-10-98

DRAINAGE PLAN, CALCULATIONS AND SECTIONS DION'S CORPORATE HEADQUARTERS

970702 3 13/98 J.G.M. ADJUST CALCULATIONS FOR INCREASE IN ASPHALT PAVING. 848 JGM ASSULT AND CERTIFY FOR TEMP CO. 12-1997 A 146 JON RECERTEY