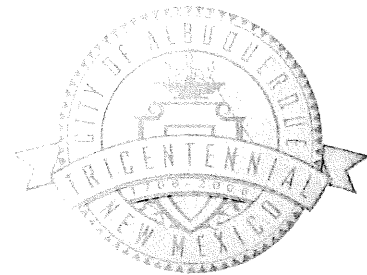


CITY OF ALBUQUERQUE



March 30, 2007

Arthur Blessen, PE
J. Arthur Blessen Engineering
11930 Menaul Blvd. NE Suite 104
Albuquerque, NM 87112

**Re: OGB Architectural Millwork TI and Improvement 3711 Paseo Del Norte
Grading and Drainage Plan
Engineer's Stamp dated 1/23/07 (C17/D019)**

Dear Mr. Blessen,

Based upon the information provided in your submittal dated 3-19-07, the above referenced plan is approved for Building Permit. Please attach a copy of this approved plan to the construction sets prior to sign-off by Hydrology. Also, prior to Certificate of Occupancy release, Engineer Certification of the grading plan per the DPM checklist will be required.

If you have any questions, you can contact me at 924-3977.

Sincerely,

Rudy E. Rael, Associate Engineer
Planning Department.
Building and Development Services

C: file

Tim Eichenberg - Chair
 Danny Hernandez - Vice Chair
 Daniel F. Lyon - Secretary - Treasurer
 Ronald D. Brown - Assistant Secretary - Treasurer
 Janet Salers - Director

John P. Kelly, P.E.
 Executive Engineer



**Albuquerque
 Metropolitan
 Arroyo
 Flood
 Control
 Authority**

2600 Prospect N.E., Albuquerque, NM 87107
 Phone: (505) 884-2215 Fax: (505) 884-0214

Post-it® Fax Note

7671

Date	8-15	# of pages	1
To	JEREMY HOOVER	From	LYNN MAZUR
Co./Dept.	HYDROLOGY	Co.	AMAFCA
Phone #		Phone #	
Fax #		Fax #	

August 15, 2006

Mr. Verlyn A. Miller, P.E.
 Miller Engineering Consultants
 P.O. Box 520
 Edgewood, NM 87015

Re: Bradbury Stamm Construction Headquarters, ZAP C-17
 Engineer's Stamp Dated July 28, 2006

C17/D19
 DRTB 1000057

Dear Mr. Miller:

I received the Grading & Drainage Plan for the referenced property and the comment letter from the City of Albuquerque dated August 9, 2006. I would like to clarify some points in comment number 7 in the City letter. AMAFCA received several calls from Western Assurance after the recent rains that their parking lot was flooded. AMAFCA engineers determined that the ditch adjacent to Paseo del Norte was causing the problem. This ditch is in New Mexico Department of Transportation (NMDOT) right-of-way and is their maintenance responsibility, unless there is an agreement or permit for private maintenance. AMAFCA staff has contacted the NMDOT regarding the drainage issues with the ditch. If this site discharges to it, it will require some improvements. Any work in the ditch will require a permit from NMDOT.

I have reviewed the plan with respect to drainage to the existing rundown on the North Diversion Channel (NDC) and offer the following comments:

1. Identify the 10-foot Drainage Easement onsite as "Private Drainage Easement". Is this easement existing, or will the property be replatted?
2. Provide a riprap detail for the NDC inlet with appropriate dimensions.

If you have any questions, please call me at 884-2215.

Sincerely,
 AMAFCA

Lynn M. Mazur

Lynn M. Mazur, P.E., C.F.M.
 Development Review Engineer

Cc: Jeremy Hoover, City Hydrology
 Kathy Trujillo, NMDOT, District 3

Parking Lot Flowline
Cross Section for Irregular Section - 1

Project Description

Flow Element: Irregular Section
Friction Method: Manning Formula
Solve For: Discharge

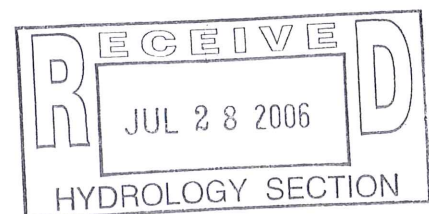
Section Data

Roughness Coefficient: 0.013
Channel Slope: 0.00500 ft/ft
Normal Depth: 0.67 ft
Elevation Range: 0.00 to 0.67 ft
Discharge: 13.59 ft³/s

ASPHALT



V:1
H:1



Worksheet for Irregular Section - 1

Project Description		
Flow Element:	Irregular Section	
Friction Method:	Manning Formula	
Solve For:	Discharge	

Input Data		
Channel Slope:	0.00500	ft/ft
Water Surface Elevation:	0.67	ft

Options		
Current Roughness Weighted Metho	ImprovedLotters	
Open Channel Weighted Roughnes	ImprovedLotters	
Closed Channel Weighted Roughne	Hortons	

Results		
Roughness Coefficient:	0.013	
Discharge:	13.59	ft ³ /s
Elevation Range:	0.00 to 0.67 ft	
Flow Area:	3.85	ft ²
Wetted Perimeter:	13.36	ft
Top Width:	11.50	ft
Normal Depth:	0.67	ft
Critical Depth:	0.70	ft
Critical Slope:	0.00374	ft/ft
Velocity:	3.53	ft/s
Velocity Head:	0.19	ft
Specific Energy:	0.86	ft
Froude Number:	1.07	
Flow Type:	Supercritical	

Segment Roughness		
Start Station	End Station	Roughness Coefficient
(0+00, 0.67)	(0+13, 0.67)	0.013

Section Geometry	
Station	Elevation
0+00	0.67
0+11	0.00

Worksheet for Irregular Section - 1

Station	Elevation
0+12	0.67
0+13	0.67

V-Ditch

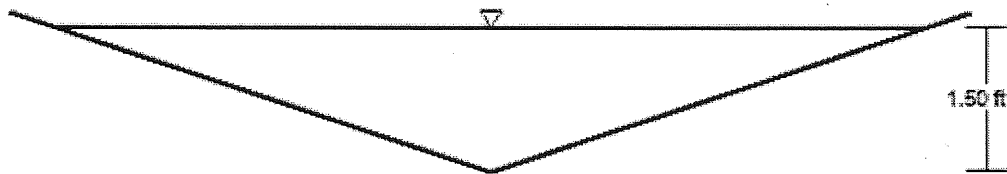
Cross Section for Triangular Channel - 1


Project Description

Flow Element:	Triangular Channel
Friction Method:	Manning Formula
Solve For:	Discharge

Section Data

Roughness Coefficient:	0.020	
Channel Slope:	0.00700	ft/ft
Normal Depth:	1.50	ft
Left Side Slope:	3.00	ft/ft (H:V)
Right Side Slope:	3.00	ft/ft (H:V)
Discharge:	33.44	ft ³ /s



V:1  H:1

Worksheet for Triangular Channel - 1

ASSUME .26 FOR
DIRTY/SOME
VEGETATION

Project Description

Flow Element: Triangular Channel
Friction Method: Manning Formula
Solve For: Discharge

Input Data

Roughness Coefficient: 0.020 - GRAVEL LINED
Channel Slope: 0.00700 ft/ft
Normal Depth: 1.50 ft
Left Side Slope: 3.00 ft/ft (H:V)
Right Side Slope: 3.00 ft/ft (H:V)

Results

Discharge: 33.44 ft³/s
Flow Area: 6.75 ft²
Wetted Perimeter: 9.49 ft
Top Width: 9.00 ft
Critical Depth: 1.51 ft
Critical Slope: 0.00687 ft/ft
Velocity: 4.95 ft/s
Velocity Head: 0.38 ft
Specific Energy: 1.88 ft
Froude Number: 1.01
Flow Type: Supercritical

GVF Input Data

Downstream Depth: 0.00 ft
Length: 0.00 ft
Number Of Steps: 0

GVF Output Data

Upstream Depth: 0.00 ft
Profile Description: N/A
Profile Headloss: 0.00 ft
Downstream Velocity: 0.00 ft/s
Upstream Velocity: 0.00 ft/s
Normal Depth: 1.50 ft
Critical Depth: 1.51 ft
Channel Slope: 0.00700 ft/ft
Critical Slope: 0.00687 ft/ft

$$\frac{1.49}{.026} (6.75) \left(\sqrt{\frac{6.75}{9.49}} \right) = 25.8 \text{ cfs}$$

PEAK DISCHARGE (cfs/acre)

CURRENT CONDITIONS

BASIN	AREA (ACRES)	% LAND TREATMENT*				PEAK DISCHARGE - (CFS/ACRE)**				Q(100-YR) UNDEVELOPED (CFS)
		A	B	C	D	A	B	C	D	
1	9.60	85.00	15.00	0.00	0.00	1.56	2.28	3.14	4.7	16.01
2	4.10	85.00	15.00	0.00	0.00	1.56	2.28	3.14	4.7	6.84
3	1.68	85.00	15.00	0.00	0.00	1.56	2.28	3.14	4.7	2.80
4	2.94	85.00	15.00	0.00	0.00	1.56	2.28	3.14	4.7	4.90
5	2.96	85.00	15.00	0.00	0.00	1.56	2.28	3.14	4.7	4.94
										35.50

PHASE I CONDITIONS

Construction of Office/Warehouse Only

BASIN	AREA (ACRES)	% LAND TREATMENT*				PEAK DISCHARGE - (CFS/ACRE)**				Q(100-YR) DEVELOPED (CFS)
		A	B	C	D	A	B	C	D	
1	9.60	85.00	15.00	0.00	0.00	1.56	2.28	3.14	4.7	16.01
2	4.10	85.00	15.00	0.00	0.00	1.56	2.28	3.14	4.7	6.84
3	1.68	0.00	15.00	15.00	70.00	1.56	2.28	3.14	4.7	6.89
4	2.94	0.00	10.00	30.00	60.00	1.56	2.28	3.14	4.7	11.73
5	2.96	0.00	20.00	30.00	50.00	1.56	2.28	3.14	4.7	11.09
										52.57

FULLY DEVELOPED CONDITIONS

Construction of Office Building and East Side Development

BASIN	AREA (ACRES)	% LAND TREATMENT*				PEAK DISCHARGE - (CFS/ACRE)**				Q(100-YR) DEVELOPED (CFS)
		A	B	C	D	A	B	C	D	
1	9.60	85.00	15.00	0.00	0.00	1.56	2.28	3.14	4.7	16.01
2	4.10	0.00	10.00	15.00	75.00	1.56	2.28	3.14	4.7	17.32
3	1.68	0.00	15.00	15.00	70.00	1.56	2.28	3.14	4.7	6.89
4	2.94	0.00	10.00	30.00	60.00	1.56	2.28	3.14	4.7	11.73
5	2.96	0.00	15.00	15.00	70.00	1.56	2.28	3.14	4.7	12.14
										64.10

NOTES:

Obtained from Section 22.2, Hydrology of the Development Process Manual, Volume 2, Design Criteria for the City of Albuquerque, Jan. 1993

* Table A-4

** Table A-9

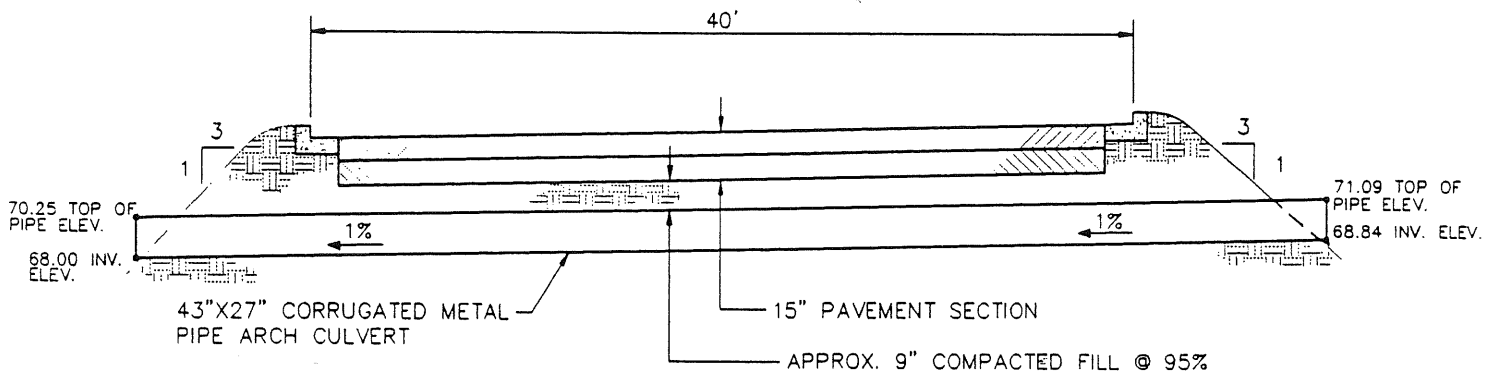
CULVERT CALCULATIONS

FLOW CAPACITY

Manning's Coefficient	0.022	(corrugated steel)
Slope	0.01	
Section	43" x 27" pipe-arch	
Area	6.4 sf	
Perimeter	Equivalent to a 36" circular pipe, or 9.42 ft	
Hydraulic Radius (R)	Area/Perimeter = 6.4/9.42 = 0.68	
Manning's Equation	$Q_{cfs} = (1.49/n) A \times R^{2/3} \times S^{1/2}$ $= (1.49/.022) 6.4 \times 0.68^{2/3} \times 0.01^{1/2}$ $= (67.73) 6.4 \times 0.7723 \times 0.1$ $= (67.73) 6.4 \times 0.7723 \times 0.1$ $= 33.48 \text{ cfs} \checkmark$	
Velocity Equation	$V = Q/A$ $= 33.48/6.4$ $= 5.23 \text{ fps}$	

100-YEAR STORM FLOW

BASINS	B-1 (Phase III)	16.01	CFS
	B-2 (Phase III)	17.32	
TOTAL FLOW		33.33	CFS



SECTION G

N.T.S.

RUNDOWN STREAM CALCULATIONS

FLOW CAPACITY

Manning's Coefficient	0.013	(concrete)
Slope	0.0122	

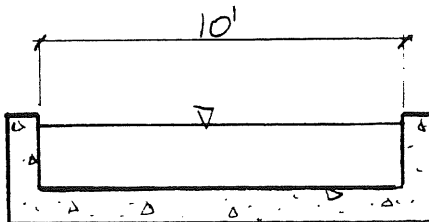
Section Coordinates (ft.)	1.5,0
	0,0
	0,10
	1.5,10

DEPTH	FLOW AREA	FLOW RATE	WETTED PERIMETER	FLOW VELOCITY	TOP WIDTH
INC	(SF)	(CFS)	(FT)	(FPS)	(FT)
0.10	1.00	2.70	10.20	2.70	10.00
0.20	2.00	8.40	10.40	4.20	10.00
0.30	3.00	16.30	10.60	5.40	10.00
0.40	4.00	26.00	10.80	6.50	10.00
0.50	5.00	37.30	11.00	7.50	10.00
0.60	6.00	50.00	11.20	8.30	10.00
0.90	9.00	94.90	11.80	10.50	10.00
1.00	10.00	111.80	12.00	11.20	10.00
1.10	11.00	129.60	12.20	11.80	10.00
1.20	12.00	148.20	12.40	12.40	10.00
1.30	13.00	167.60	12.60	12.90	10.00
1.40	14.00	187.60	12.80	13.40	10.00
1.50	15.00	208.30	13.00	13.90	10.00

100-YEAR STORM FLOW

BASINS	B-1 (Phase III)	16.01	CFS
	B-2 (Phase III)	17.32	
	B-3 (Phase III)	6.89	
	B-4 (Phase III)	11.73	
	B-5 (Phase III)	12.14	

TOTAL FLOW	64.09	CFS	DEPTH =	0.702 FT
			VELOCITY =	9.12 FPS



SECTION F

N.T.S.

RUNDOWN NECK CAPACITY

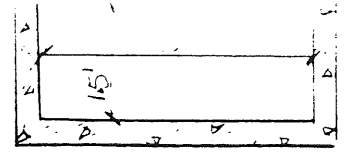
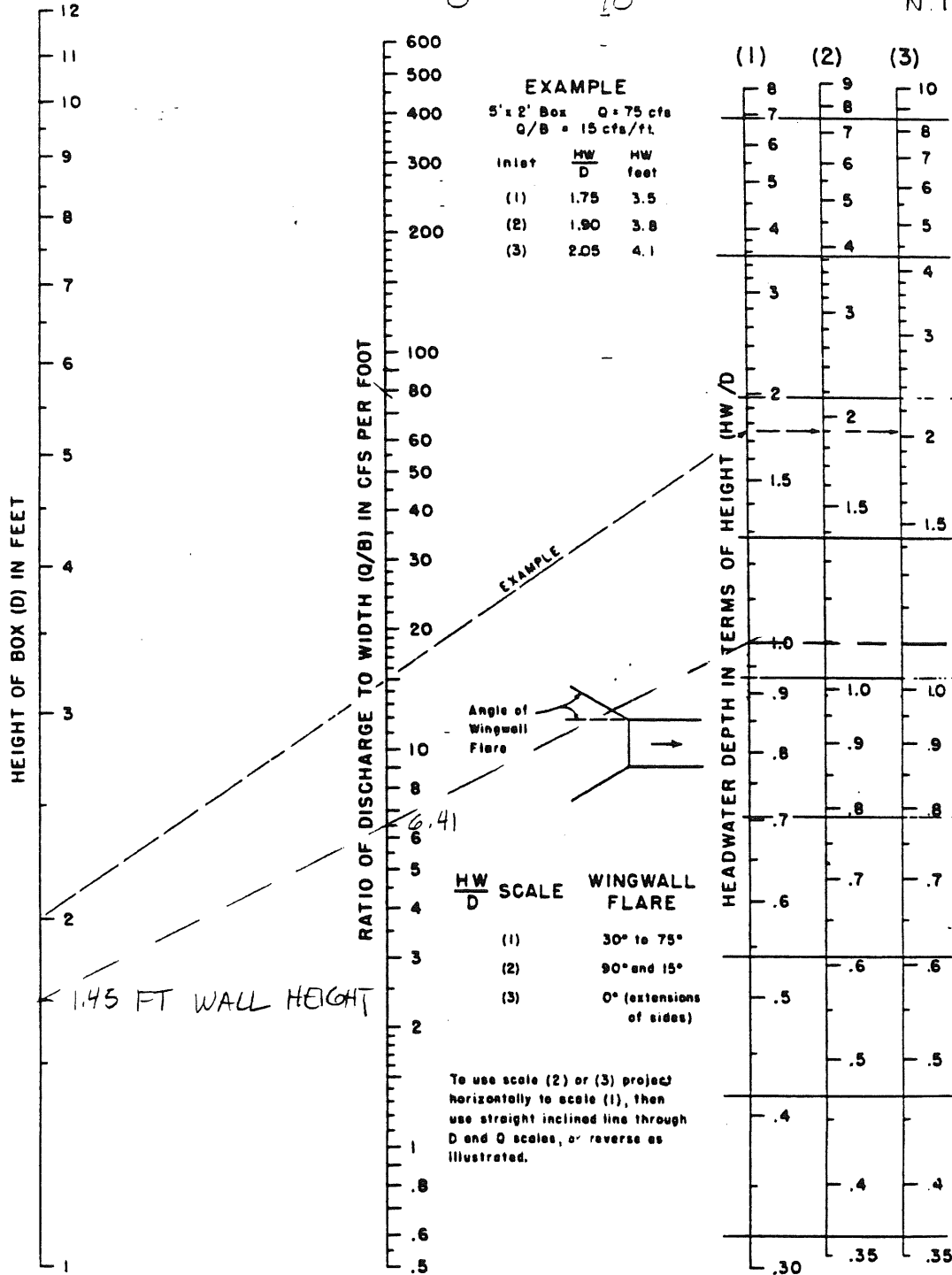


CHART 8

10' WIDE RUNDOWN

$$\frac{Q}{B} = \frac{64.1 \text{ CFS}}{10'} = 6.41$$

SECTION E
N.T.S.



1.5 FT WALL HEIGHT ACCOMMODATES FLOW OF 64.10 CFS IN THE 10' WIDE NECK.

ACCESS STREET HYDRAULICS

FLOW CAPACITY

Manning's Coefficient 0.017
Slope 0.010

Section Coordinates (ft.) .5,0
 0,0
 .3,15
 0,30
 .5,30

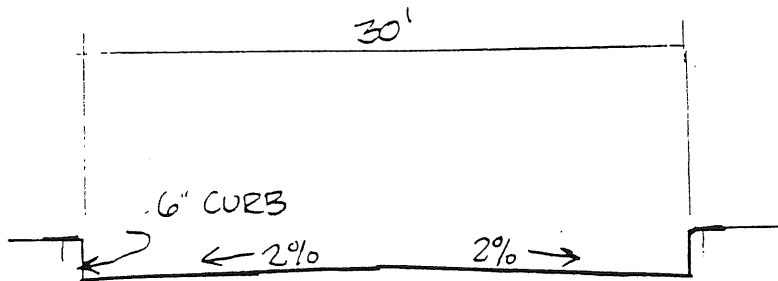
DEPTH	FLOW AREA	FLOW RATE	WETTED PERIMETER	FLOW VELOCITY	TOP WIDTH
INC	(SF)	(CFS)	(FT)	(FPS)	(FT)
0.10	0.50	0.60	10.20	1.20	10.00
0.20	2.00	2.40	20.40	2.40	20.00
0.30	4.50	5.40	30.60	3.60	30.00
0.40	7.50	25.60	30.80	3.40	30.00
0.50	10.50	44.60	31.00	4.20	30.00

100-YEAR STORM FLOW

BASINS B-3 (Phase III) 6.89 CFS

TOTAL FLOW 6.89 CFS

DEPTH = 0.25 FT
VELOCITY = 2.13 FPS



SECTION C
N.T.S.

CUL-DE-SAC STREET HYDRAULICS

FLOW CAPACITY

Manning's Coefficient 0.017 (street)
Slope 0.010 (minimum)

Section Coordinates (ft.), .5,0
 0,0
 .2,40
 .7,40

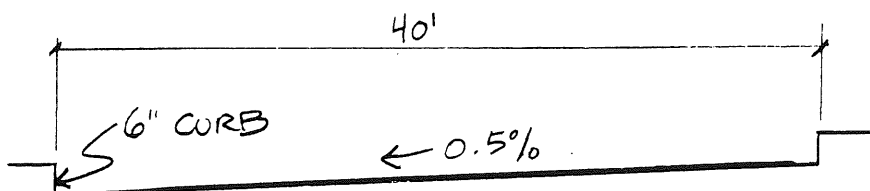
DEPTH	FLOW AREA	FLOW RATE	WETTED PERIMETER	FLOW VELOCITY	TOP WIDTH
INC	(SF)	(CFS)	(FT)	(FPS)	(FT)
0.10	1.00	1.20	20.10	1.20	20.00
0.20	4.00	7.50	40.20	1.90	40.00
0.50	16.00	74.90	40.80	4.70	40.00

100-YEAR STORM FLOW

BASINS	B-1 (Phase III)	16.01	CFS
	B-2 (Phase III)	17.32	
	B-3 (Phase III)	<u>6.89</u>	

TOTAL FLOW 40.22 CFS

DEPTH = 0.37 FT
VELOCITY = 3.65 FPS



SECTION B
N.T.S.

RUNDOWN WEIR OPENING

WEIR CAPACITY

Weir Coefficient 3.000

Section Coordinates (ft.) .8 ,0
 0,0
 0,30
 .8 ,30

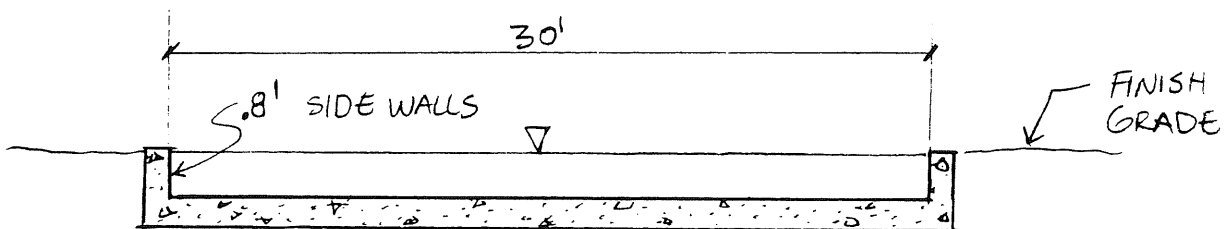
DEPTH INC	FLOW AREA (SF)	FLOW RATE (CFS)	FLOW VELOCITY (FPS)	TOP WIDTH (FT)
0.10	3.00	2.85	0.95	30.00
0.20	6.00	8.05	1.34	30.00
0.30	9.00	14.79	1.64	30.00
0.40	12.00	22.77	1.90	30.00
0.50	15.00	31.82	2.12	30.00
0.60	18.00	41.83	2.32	30.00

100-YEAR STORM FLOW

BASINS	B-1 (Phase III)	16.01	CFS
	B-2 (Phase III)	17.32	
	B-3 (Phase III)	6.89	
	B-4 (Phase III)	11.73	
	B-5 (Phase III)	12.14	

TOTAL FLOW 64.09 CFS

DEPTH = 0.797 FT
 VELOCITY = 2.68 FPS



SECTION D

N.T.S.

FACILITY ACCESSIBILITY
ALL SURFACES ALONG THE ACCESSIBLE ROUTE SHALL COMPLY WITH ANSI A117-1998.

WALKING SURFACES SHALL BE STABLE, FIRM, AND SLIP RESISTANT. THE RUNNING SLOPE OF WALKING SURFACES SHALL NOT BE STEEPER THAN 1:20 WITH A CROSS SLOPE NOT STEEPER THAN 1:48.

CURB RAMP AND RAMP RUNS SHALL HAVE A RUNNING SLOPE NOT STEEPER THAN 1:12 WITH A CROSS SLOPE NOT STEEPER THAN 1:48. COUNTER SLOPES OF ADJOINING GUTTERS AND ROAD SURFACES IMMEDIATELY ADJACENT TO THE CURB RAMP OR ACCESSIBLE ROUTE SHALL NOT BE STEEPER THAN 1:20. TRANSITIONS FROM RAMP TO WALKS, GUTTERS OR STREETS SHALL BE AT THE SAME LEVEL. WHERE PEDESTRIANS MUST WALK ACROSS A CURB RAMP, THE RAMP SHALL HAVE FLARED SIDES WITH SLOPES NOT STEEPER THAN 1:10. WHERE THE TOP OF THE RAMP PARALLEL TO THE RUN OF THE RAMP IS LESS THAN 48 INCHES WIDE, THE FLARED SIDES SHALL HAVE A SLOPE NOT STEEPER THAN 1:12.

HANDICAP PARKING SPACES AND ACCESS AISLES SHALL HAVE SURFACE SLOPES NOT STEEPER THAN 1:48. ACCESS AISLES SHALL BE AT THE SAME LEVEL AS THE PARKING SPACES THEY SERVE.

TRAFFIC CONTROL
THE CONTRACTOR SHALL PROVIDE ALL REQUIRED TRAFFIC CONTROL PLANS AND DEVICES. ALL SIGNS, BARRICADES, CHANNELIZATION DEVICES, SIGN FRAMES AND ERECTION OF SUCH DEVICES SHALL CONFORM TO THE REQUIREMENTS OF THE "MANUAL OF UNIFORM TRAFFIC CONTROL DEVICES FOR STREETS AND HIGHWAYS" LATEST EDITION. PRIOR TO CONSTRUCTION PRIOR TO CONSTRUCTION, THE TRAFFIC CONTROL PLAN SHALL BE SUBMITTED AND APPROVED BY THE GOVERNING AUTHORITY.

- CONSTRUCTION NOTES
1. TWO WORKING DAYS PRIOR TO ANY EXCAVATION, THE CONTRACTOR SHALL CALL FOR LOCATION OF EXISTING UTILITIES.
 2. ALL WORK WITHIN THE CITY RIGHT-OF-WAY SHALL BE PERFORMED IN ACCORDANCE WITH THE APPLICABLE CITY OF ALBUQUERQUE STANDARDS AND PROCEDURES.
 3. ALL WORK SHALL BE PERFORMED IN ACCORDANCE WITH THE APPLICABLE FEDERAL, STATE, AND LOCAL REGULATIONS, LAWS, AND RULES CONCERNING SAFETY AND HEALTH.
 4. PRIOR TO CONSTRUCTION THE CONTRACTOR SHALL VERIFY THE EXISTING SITE CONDITIONS AND INFORM THE ARCHITECT/ENGINEER OF ANY DISCREPANCY BETWEEN THE INFORMATION SHOWN ON THE PLANS AND THOSE OF THE EXISTING SITE.
 5. THE CONTRACTOR SHALL PROVIDE THE ARCHITECT / ENGINEER WITH AN AS BUILT SURVEY FOR ENGINEER'S CERTIFICATION.
 6. THE CONTRACTOR SHALL MAINTAIN A RECORD DRAWING SET OF PLANS AND PROMPTLY LOCATE EXISTING AND NEW ELEVATIONS (FINISH FLOORS, TOPS OF CURBS AND ASPHALT, FLOW LINE, PIPE INVERTS, ETC.), ON THE RECORD SET. THE RECORD SET SHALL BE MAINTAINED ON THE PROJECT SITE AND SHALL BE AVAILABLE TO THE OWNER AND ARCHITECT AT ANY TIME DURING CONSTRUCTION. UPON COMPLETION OF THE PROJECT, THE RECORD SET SHALL BE TURNED OVER TO THE OWNER.
 7. THE OWNER / CONTRACTOR SHALL SUBMIT A NOTICE OF INTENT (NOI) TO THE EPA PRIOR TO BEGINNING OF CONSTRUCTION.
 8. CONTRACTOR SHALL COMPLY WITH STORM WATER POLLUTION PREVENTION PLAN (SWPPP) SPECIFIC TO THIS PROJECT.

DRAINAGE PLAN

THE PROPOSED IMPROVEMENTS AS SHOWN BY THE VICINITY MAP, ARE LOCATED ON THE NORTH SIDE OF PASEO DEL NORTE WEST OF JEFFERSON. THE SITE IS DEVELOPED AND SLOPES TO THE EAST TO THE EXISTING DRAINAGE INLET TO THE NORTH DIVERSION CHANNEL. ACCORDING TO FIRM PANEL 136F THE SITE DOES NOT LIE WITHIN A FLOOD HAZARD ZONE.

THE SITE IS PART OF THE BRADBURY STAM HEIGHTS MASTER DRAINAGE PLAN PREPARED BY BOHANNAN HOUSTON. THE PROPOSED ADDITION IS LOCATED WITHIN BASIN B4 OF THE MASTER PLAN, AND IS LOCATED OVER THE EXISTING PAVED AREA, THEREFORE NO SIGNIFICANT CHANGE IN RUNOFF ARE ANTICIPATED.

THE NORTH DIVERSION CHANNEL IS LOCATED ALONG THE NORTH AND WEST PROPERTY LINES, AND THE EXISTING CURB AND GUTTER ALONG PASEO DEL NORTE TO THE SOUTH BLOCK FLOWS FROM THOSE DIRECTIONS. THE SITE IS HIGHER THAN THE LAND TO THE EAST, THEREFORE OFFSITE FLOWS ARE CONSIDERED INSIGNIFICANT.

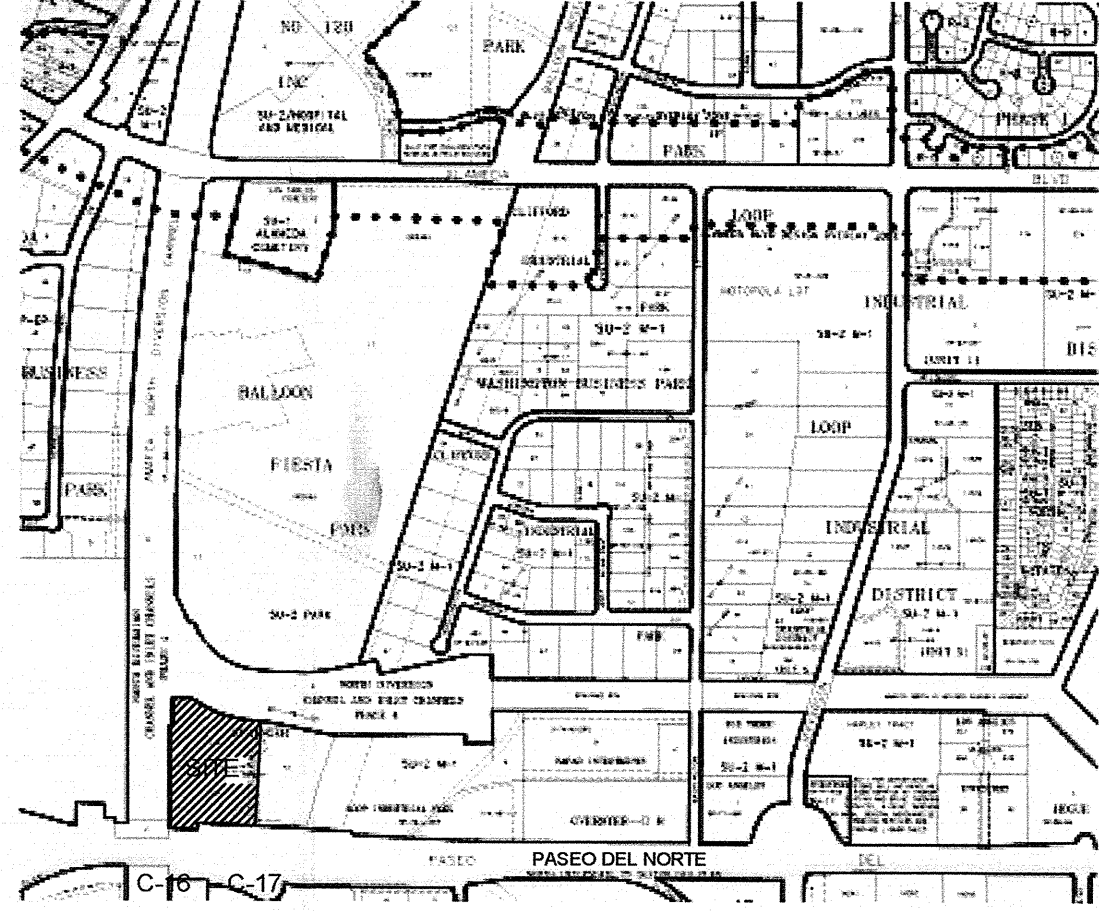
THE PROPOSED IMPROVEMENTS CONSIST OF A 1553 SF ADDITION TO THE EXISTING SHOP FACILITY LOCATED ALONG THE NORTH PROPERTY LINE. NO ADDITIONAL PARKING NOR LANDSCAPING ARE ANTICIPATED. AS SHOWN BY THE CALCULATION THE PROPOSED DEVELOPMENT WILL INCREASE THE RATE AND VOLUME OF RUNOFF. THE EXISTING RATE OF DISCHARGE IS 25.31 CFS (4.03 CFS/ACRE). NO PONDING IS ANTICIPATED FOR THE SITE.

THE CALCULATIONS ANALYZE THE EXISTING AND PROPOSED CONDITIONS FOR 6-HOUR, 100 YEAR RAINFALL EVENT. THE ANALYSIS IS IN ACCORDANCE WITH THE CITY OF ALBUQUERQUE DEVELOPMENT PROCESS MANUAL VOLUME II.

CALCULATIONS
PRECIPITATION ZONE = 2
TOTAL SITE AREA = 6.279 ACRES

EXISTING CONDITIONS
LAND TREATMENT B=15% C=20% D=65%
E = $0.78(0.14)+1.13(0.20)+2.12(0.65) = 1.72$ INCHES
V = $1.72(6.279) / 12 = 0.902$ ACRE FEET
Q = $(2.28(0.14)+3.14(0.20)+4.70(0.65))(6.279) = 25.3$ CFS

INCREASE IN VOLUME OF RUNOFF = 0.0 ACRE FT
INCREASE IN RATE OF RUNOFF = 0.0 CFS



D1 VICINITY MAP
NOT TO SCALE
C16 & C17
FILE C17/D019

LEGAL DESCRIPTION:
TRACK F1A & F1B, LAND OF SPRINGER CORPORATION
ADDRESS:
3711 PASEO DEL NORTE, ALBUQUERQUE, NM

- LEGEND:
- | | | | |
|--------------------|-------------------------|-----|----------------------|
| 93.1 | EXISTING SPOT ELEVATION | FL | FLOW LINE |
| 51.00 | NEW SPOT ELEVATION | GND | GROUND |
| 51 | EXISTING CONTOUR | INV | INVERT |
| 51 | NEW CONTOUR | TA | TOP OF ASPHALT |
| SWALE | | TC | TOP OF CURB |
| VERIFIED ELEVATION | | TG | TOP OF GRATE |
| AS BUILT ELEVATION | | TS | TOP OF CONCRETE SLAB |
| BASIN BOUNDARY | | TW | TOP OF WALL |
| PROPERTY LINE | | TBM | TEMPORARY BENCH MARK |
| | | | GRAVEL |
| | | | ASPHALT PAVING |
| | | | CONCRETE |

REMODEL AND NEW ADDITION
OGB-ARCHITECTURAL MILLWORK
PASEO DEL NORTE, N.E.
ALBUQUERQUE, NEW MEXICO

REVISIONS

DATE JANUARY 23, 2007

NORTH SCALE

DRAWING NAME

GRADING PLAN

SHEET NUMBER

C-101

RECEIVED
MAR 19 2007
HYDROLOGY SECTION

j arthur blessen engineering
architect engineer

11930 Menaul Suite 109
Albuquerque, NM 87112
(505) 293-1477



A1 SITE GRADING PLAN
1"=50'-0"