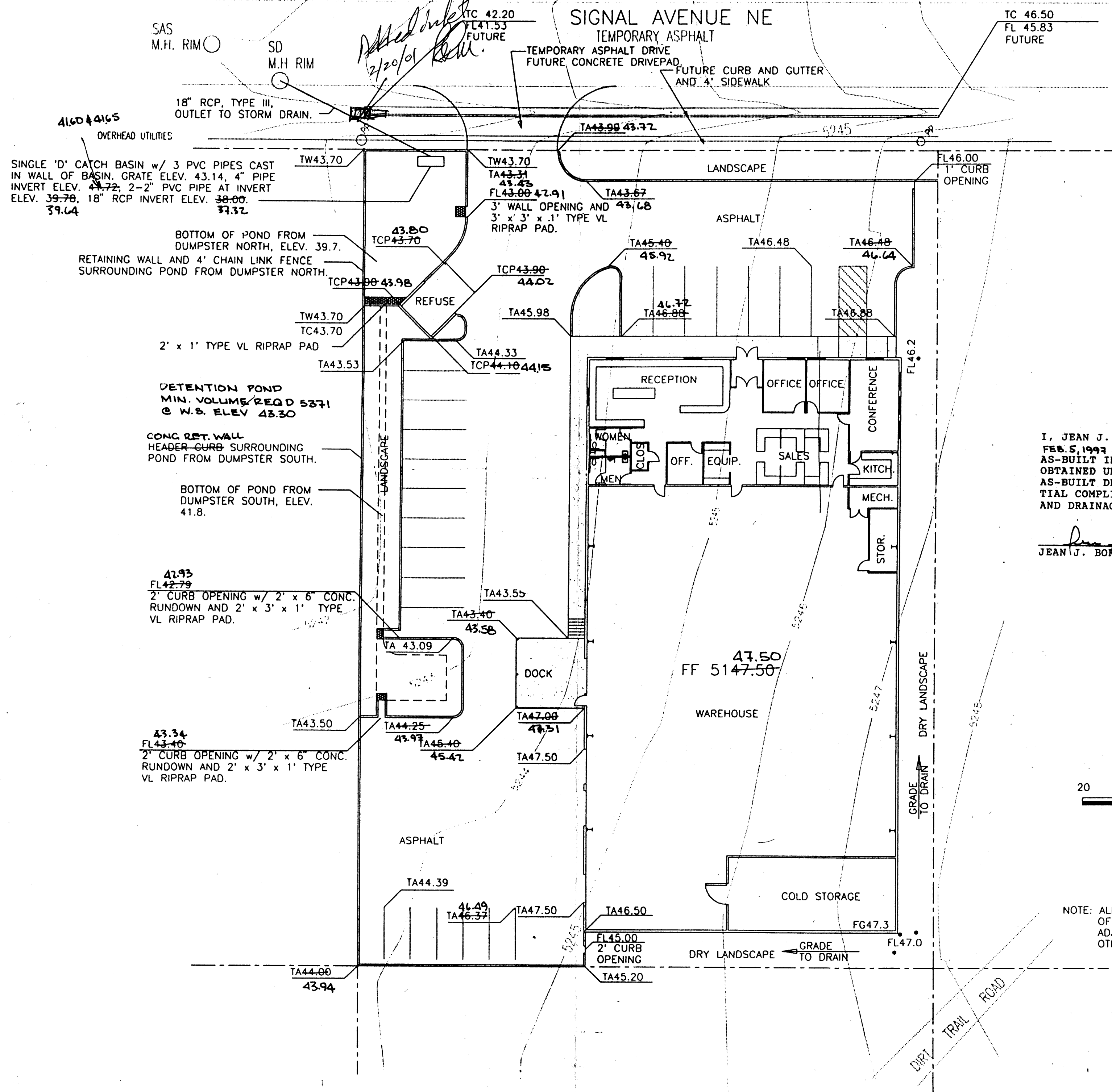
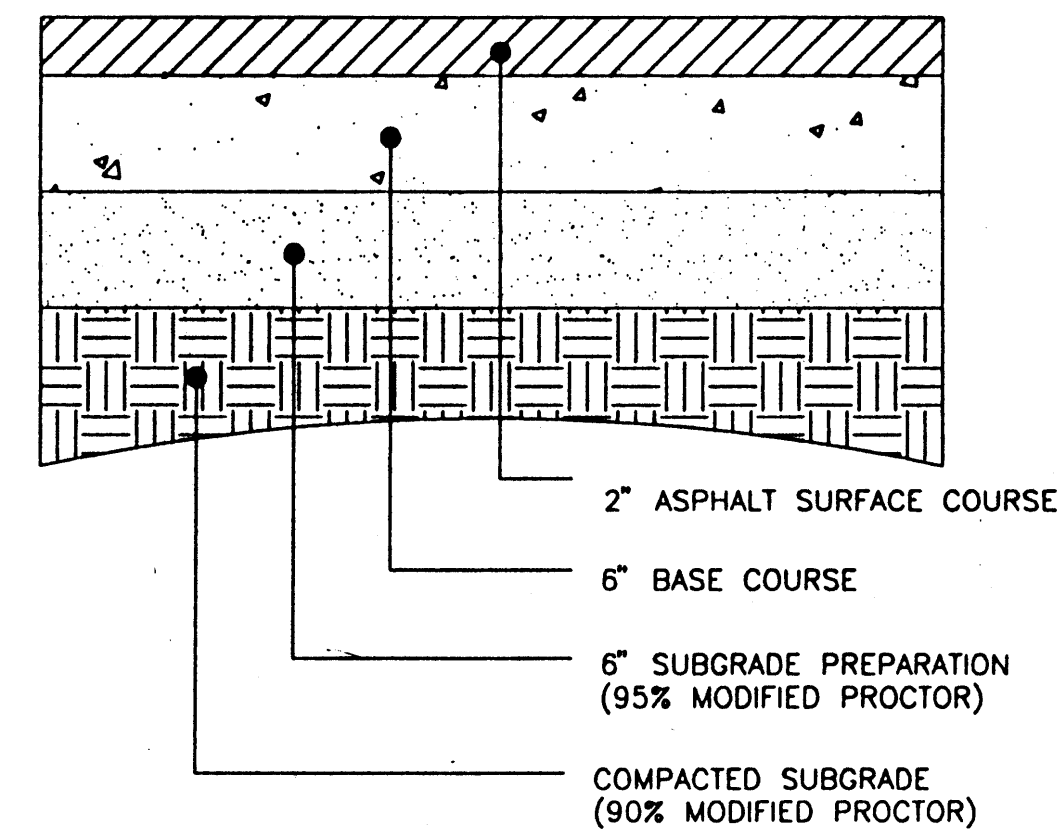




ASPHALT PAVING SECTION



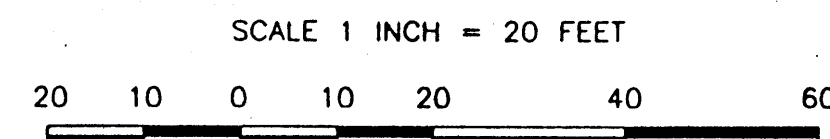
- NOTES: 1. ASPHALT MIX DESIGN TO USE CITY OF ALBUQUERQUE AGGREGATE CLASS 'B' AND HAVE A MARSHALL STABILITY GREATER THAN 1800 LBS.
2. ASPHALT CONCRETE COURSES GREATER THAN 3" IN THICKNESS SHALL BE PLACED WITH MULTIPLE LIFTS. MINIMUM LIFT THICKNESS 1 1/2".

I, JEAN J. BORDENAVE, HEREBY CERTIFY THAT ON FEB. 5, 1999 & JULY 23, 1999 THE AS-BUILT INFORMATION SHOWN ON THIS SHEET WAS OBTAINED UNDER MY SUPERVISION AND THAT THE AS-BUILT DRAINAGE CONDITIONS ARE IN SUBSTANTIAL COMPLIANCE WITH THE APPROVED GRADING AND DRAINAGE PLAN.

JEAN J. BORDENAVE, PE & PS # 5110

EROSION CONTROL NOTES:

1. THE CONTRACTOR SHALL ENSURE THAT NO SOIL ERODES FROM THE SITE INTO PUBLIC RIGHT-OF-WAY OR PRIVATE PROPERTY. THIS CAN BE ACHIEVED BY CONSTRUCTING TEMPORARY BERMS OR SILT FENCES AT THE PROPERTY LINES (OR CONSTRUCTION LIMITS) AND WETTING THE SOIL TO KEEP IT FROM BLOWING.
2. THE CONTRACTOR SHALL PROMPTLY CLEAN UP ANY MATERIAL EXCAVATED WITHIN THE RIGHT-OF-WAY SO THAT THE EXCAVATED MATERIAL IS NOT SUSCEPTIBLE TO BEING WASHED DOWN THE STREET OR INTO THE STORM DRAIN SYSTEM.
3. THE CONTRACTOR SHALL SECURE A TOPSOIL DISTURBANCE PERMIT PRIOR TO BEGINNING CONSTRUCTION.



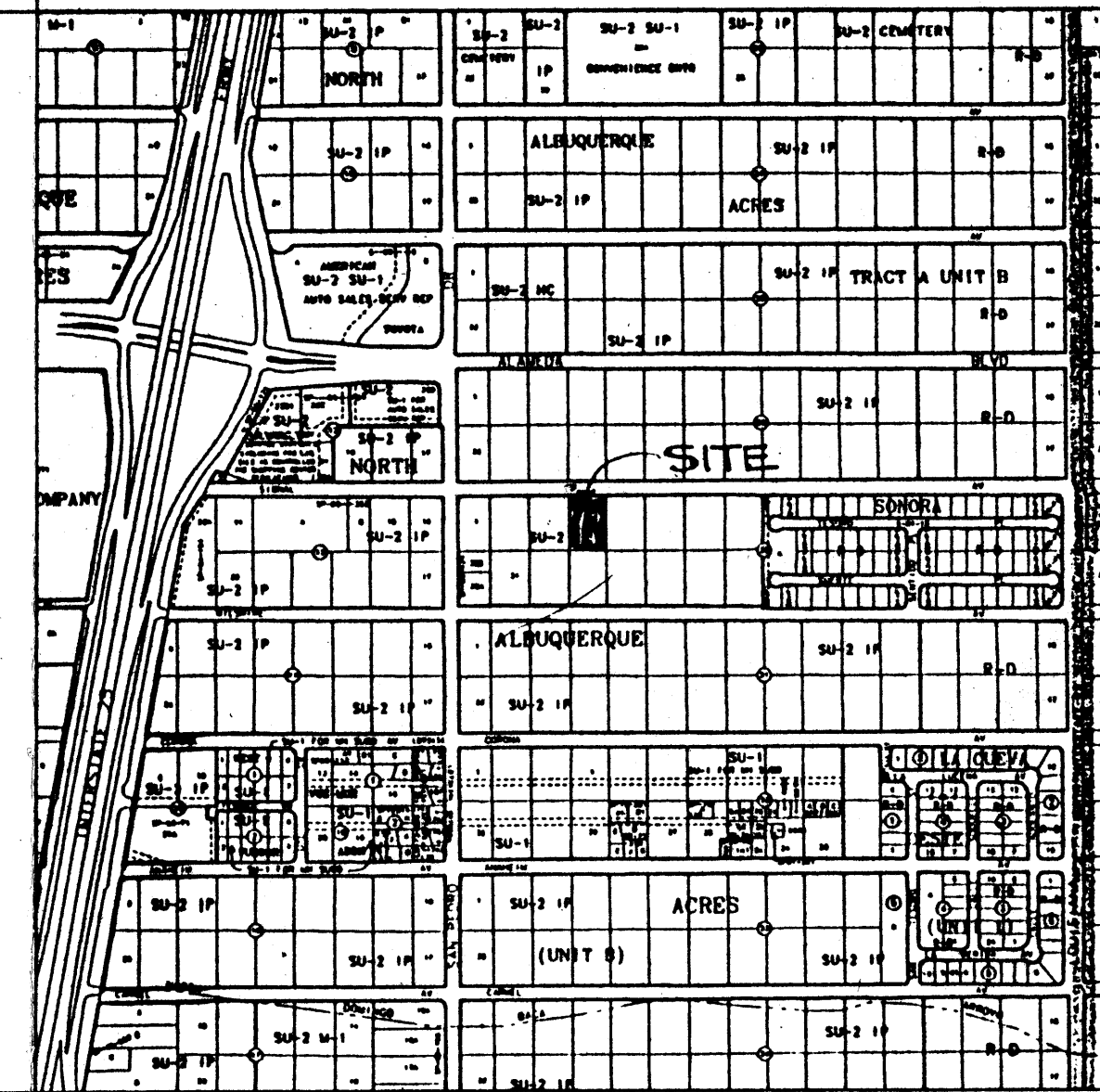
NOTE: ALL TOP OF SIDEWALK ELEVATIONS AND TOP OF CURB ELEVATIONS ARE 0.5 FEET ABOVE THE ADJACENT TOP OF ASPHALT ELEVATION UNLESS OTHERWISE SHOWN.

LEGEND

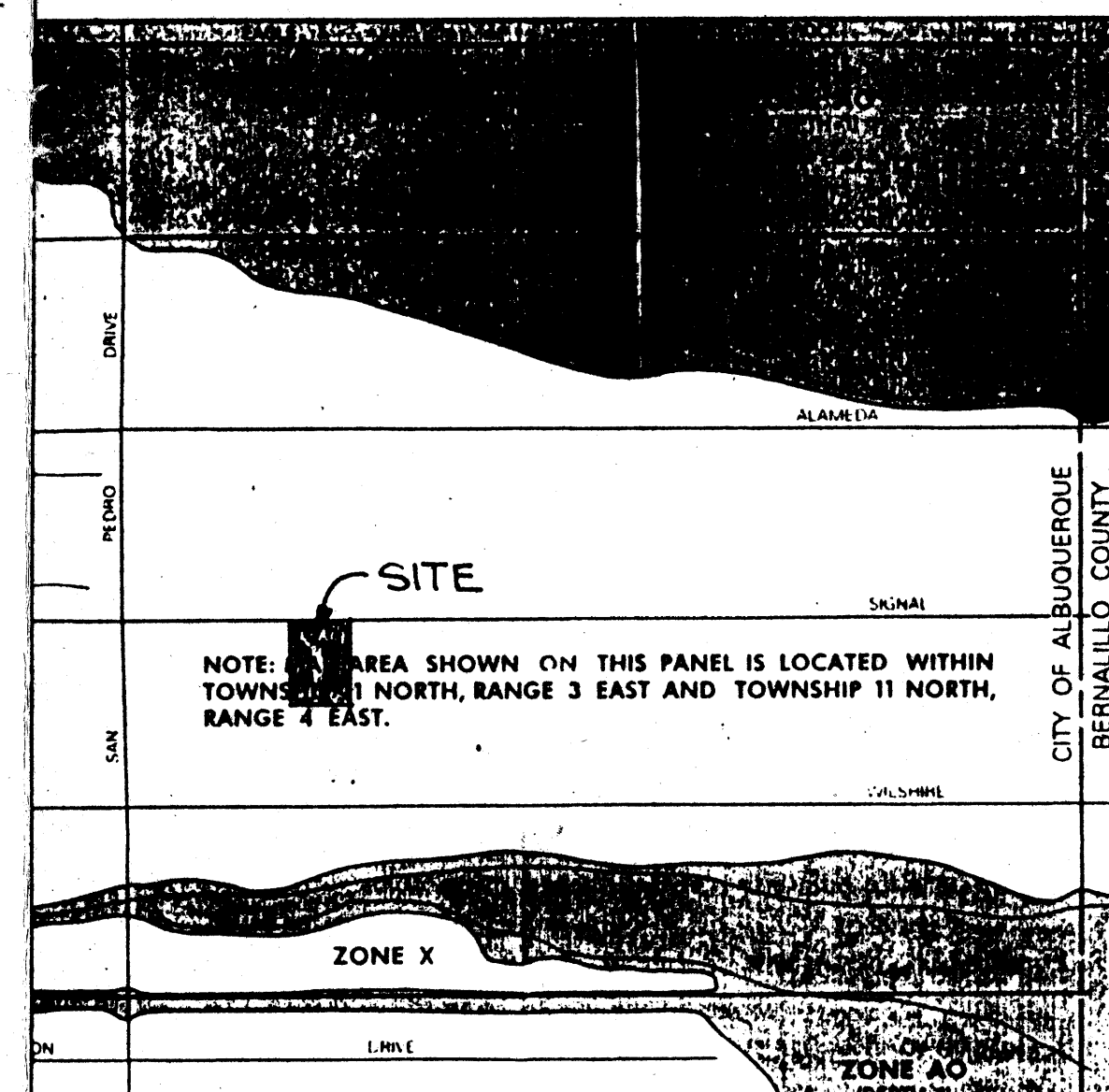
TBM	TEMPORARY BENCHMARK
FF	FINISH FLOOR
FG	FINISH GRADE
FL	FLOWLINE
TA	TOP OF ASPHALT
TCP	TOP OF CONCRETE
TC	TOP OF CURB
TP	TOP OF EARTH PAD
TS	TOP OF SIDEWALK
TW	TOP OF WALL
FH	FIRE HYDRANT
WM	WATER METER
WV	WATER VALVE
MH	MANHOLE
CB	CATCH BASIN GRATE
GM	GAS METER
GV	GAS VALVE
LP	LIGHT POLE
PP	POWER POLE
GW	GUY WIRE
PED	ELEC. OR TEL. PEDESTAL
RD	ROOF DRAINAGE POINT

	FEMA FLOODPLAIN BOUNDARY
	DRAINAGE BASIN BOUNDARY
	EROSION SETBACK LINE
	EXISTING CONTOUR
	PROPOSED CONTOUR
	EXISTING SPOT ELEVATION
	PROPOSED SPOT ELEVATION
	RECORD SPOT ELEVATION

VICINITY MAP C-18



FEMA FIRM PANEL NO. 137



DRAINAGE NOTES

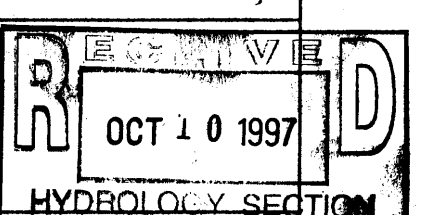
1. The site is located in a developing area where most of the street surfaces are native soil and underground infrastructure is scattered. The lot is located in the flow path of a local arroyo which has been significantly reduced in size due to the construction of Sonora Residential Subdivision on lots 11 thru 22 of block 30. Construction has also made water and sewer available to the area. In addition the roadway (Signal Avenue) is paved with a temporary asphalt section.
2. Offsite flows from the east will be accepted by the site, routed around the building and discharged at the right of way of Signal Avenue. The offsite basin will be eliminated in the near future due to construction of a residential subdivision and a commercial building on the vacant land between the subject site and Sonora Subdivision.
3. Runoff generated onsite will be detained such that the 100 year developed flow will not exceed the capacity of the existing storm drain located in San Pedro. The temporary allowable discharge rate for the subject site has been established as 1.41 cfs/acre in the report prepared by Avid Engineering for the Signal Hill Subdivision dated 09/27/96. The site discharge has been reduced to allow direct discharge of Signal Ave.
4. The site is not located in a designated flood hazard area per FEMA FIRM No. 137, dated September 20, 1996.

LEGAL DESCRIPTION

LOT 4, BLOCK 30, TRACT A, UNIT B, NORTH ALBUQUERQUE ACRES

PERMANENT BENCHMARK

ACS 9-C18 ELEVATION 5229.79



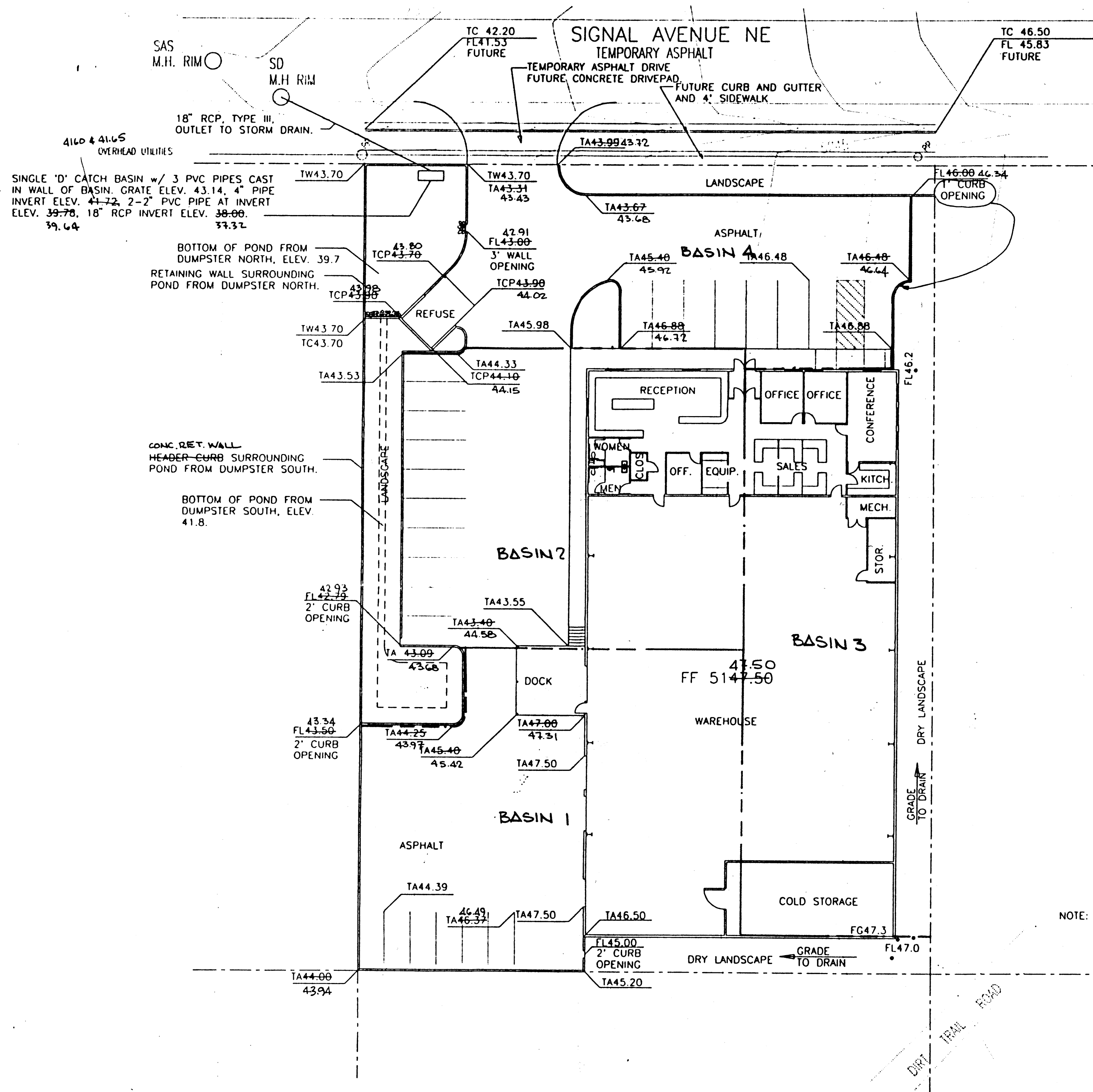
JOHN FRIEDMAN, AIA
ARCHITECT
120 MAIN STREET
LOS ALAMOS, N.M. 87531

DATE: 10/31/98
REVISED:

GRADING AND DRAINAGE PLAN

CIA-D29
176
NEW MEXICO WINE PATROL
CORONA STREET
ALBUQUERQUE, NEW MEXICO

C-2



DRAINAGE DATA

CURB OPENINGS (RECT. WIER- $Q = CLH^{1.5}$)

BASIN 1
where $Q = 1.26$ cfs, $C = 2.8$,
 $L = 2$ ft
therefore $H = 0.37$ ft < 0.5

BASIN 2
where $Q = 1.67$ cfs, $C = 2.8$,
 $L = 2$ ft
therefore $H = 0.45$ ft < 0.5

BASIN 3
where $Q = 0.99$ cfs, $C = 2.8$,
 $L = 3$ ft
therefore $H = 0.24$ ft < 0.5

EMERGENCY SPILLWAY (TRI. WIER- $Q = CAH^{0.5}$)

Basins 1 thru 4 + Offsite 'A' via driveway
where $Q = 4.17 + 3.75 = 7.92$ cfs,
 $A = 0.5$ LH, $L = H/0.0243$
therefore $H = 0.45$ ft < 0.5
 $L = 18.52$ ft

POND CHARACTERISTICS
INFLOW HYDROGRAPH (from DPM 22.2)
 $t_p = 0.7t_c + (1.6 - A_p/A_t)/12$
where $t_c = 0.2$ hr, $A_p = 31392$ sf
 $A_t = 38592$ sf
therefore $t_p = 0.20$ hr

$t_c = 0.25A_p/A_t$
where $A_p = 31392$ sf, $A_t = 38592$ sf
therefore $t_c = 0.20$ hr

$t_c = 2.107(E)(A_p/Q_p) - 0.25A_p/A_t$
where $E = ((10)(0.66) + (945)(0.92) + (6255)(1.29) + (31392)(2.36))/((0 + 945 + 6255 + 31392) - 2.15 \text{ in.}, A_t = 38592 \text{ sf})$
 $Q_p = 4.17$ cfs, $A_p = 31392$ sf
therefore $t_c = 0.759$ hr (note: This hydrograph yields a total volume in excess of the volume from the table. Use the value from the table to size the pond.)

ALLOWABLE DISCHARGE RATE

The allowable rate for the basin in which this site is located is 1.41 cfs per acre per the Signal Hill Subdivision report. The total discharge for the public street frontage (Signal Ave.) must be deducted from this value to determine the allowable site discharge rate.

Street Discharge
 $Q_s = ((5.5)(3.45) + (24.5)(5.02))(165/43560) = 0.54$ cfs

Allowable Site Discharge
 $Q = 1.41 - 0.54 = 0.87$ cfs

OUTFLOW HYDROGRAPH

Use average of triangular and rectangular hydrographs to establish pond storage requirement.

Triangular Hydrograph
Inflow and outflow rates are equal at time $t = 0.20 + 0.20 + ((4.17 - 0.87)/4.17)(0.359) = 0.684$ hr
 $V_r = ((0.87)(0.684)(0.5) + (0.759 - 0.684)(0.87))(3600) = 1306$ cf
 $V_s = (0.87)(0.759)(3600) = 1842$ cf
 $V_{\text{ave}} = (1306 + 1842)/2 = 1574$ cf

POND VOLUME REQUIRED

$V = 2130 + 2845 + 1607 + 336 - 1547 = 5371$ cf

POND VOLUME PROVIDED

South End (1.5 ft deep)
 $V = 1649$ cf

North End
Required volume = $5371 - 1649 = 3722$ cf
Surface area provided = 1067 sf (scaled)
Depth = $3722/1067 = 3.49$ ft Use 3.5 ft.

OUTLET ORIFICE

$Q = Ca(2gH)^{0.5}$
where $Q = 0.87$ cfs, $C = 0.6$, a = orifice area,
 H = orifice head, use multiple orifices to help avoid plugging.

set 2 - 2" orifices at bottom of pond
 $Q = 0.39$ cfs, balance = $0.87 - 0.39 = 0.48$ cfs
set 1 - 4" orifice at $H = 1.31$ ft

HYDROLOGY

CONDITION	STORM	TREATMENT	TREATMENT	EXCESS	PEAK	RUNOFF	RUNOFF
	RETURN	TYPE	AREA	PRECIPITATION	RUNOFF	VOLUME	RATE
	PERIOD			(in)	(in)	(cu ft)	(cfs)
DEVELOPED	10						
		(Table 4)	(sq ft)	(in)	(in)	(cu ft)	(cfs)
		A	0	0.19	0.58	0	0.00
		B	0	0.36	1.10	0	0.00
		C	1000	0.82	2	52	0.05
		D	10295	1.5	3.38	1288	0.08
		TOTAL	11295			1337	0.05
	100						
		A	0	0.06	1.87	0	0.00
		B	0	0.92	2.8	0	0.00
		C	1000	1.29	3.45	108	0.08
		D	10295	2.38	5.02	2023	1.18
		TOTAL	11295			2130	1.26
	2						
		A	0	0.19	0.58	0	0.00
		B	0	0.36	1.10	0	0.00
		C	0	0.82	2	0	0.00
		D	14468	1.5	3.38	1809	1.13
		TOTAL	14468			1809	1.13
	100						
		A	0	0.06	1.87	0	0.00
		B	0	0.92	2.8	0	0.00
		C	0	1.29	3.45	0	0.00
		D	14468	2.38	5.02	2645	1.67
		TOTAL	14468			2645	1.67
	3						
		A	0	0.19	0.58	0	0.00
		B	945	0.36	1.10	26	0.03
		C	2133	0.82	2	118	0.10
		D	8839	1.5	3.38	830	0.52
		TOTAL	8717			968	0.64
	100						
		A	0	0.06	1.87	0	0.00
		B	945	0.92	2.8	72	0.08
		C	2133	1.29	3.45	228	0.17
		D	8839	2.38	5.02	1308	0.77
		TOTAL	8717			1607	0.98
	4						
		A	0	0.19	0.58	0	0.00
		B	0	0.36	1.10	0	0.00
		C	3122	0.82	2	181	0.14
		D	0	1.5	3.38	0	0.00
		TOTAL	3122			181	0.14
	100						
		A	0	0.06	1.87	0	0.00
		B	0	0.92	2.8	0	0.00
		C	3122	1.29	3.45	338	0.25
		D	0	2.38	5.02	0	0.00
		TOTAL	3122			338	0.25
	100						
		A	87450	0.06	1.87	4810	3.75
		B	0	0.92	2.8	0	0.00
		C	0	1.29	3.45	0	0.00
		D	0	2.38	5.02	0	0.00
		TOTAL	87450			4810	3.75

PROFESSIONAL SEAL
NEW MEXICO
JULY 13, 1997
6110
NEW MEXICO

AIA
JOHN FRIEDMAN, ARCHITECT
100 MAIN STREET
LOS ALAMOS, NM 87501

DATE: 10/31/96
REVISED:

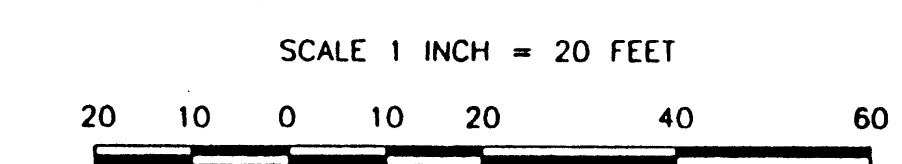
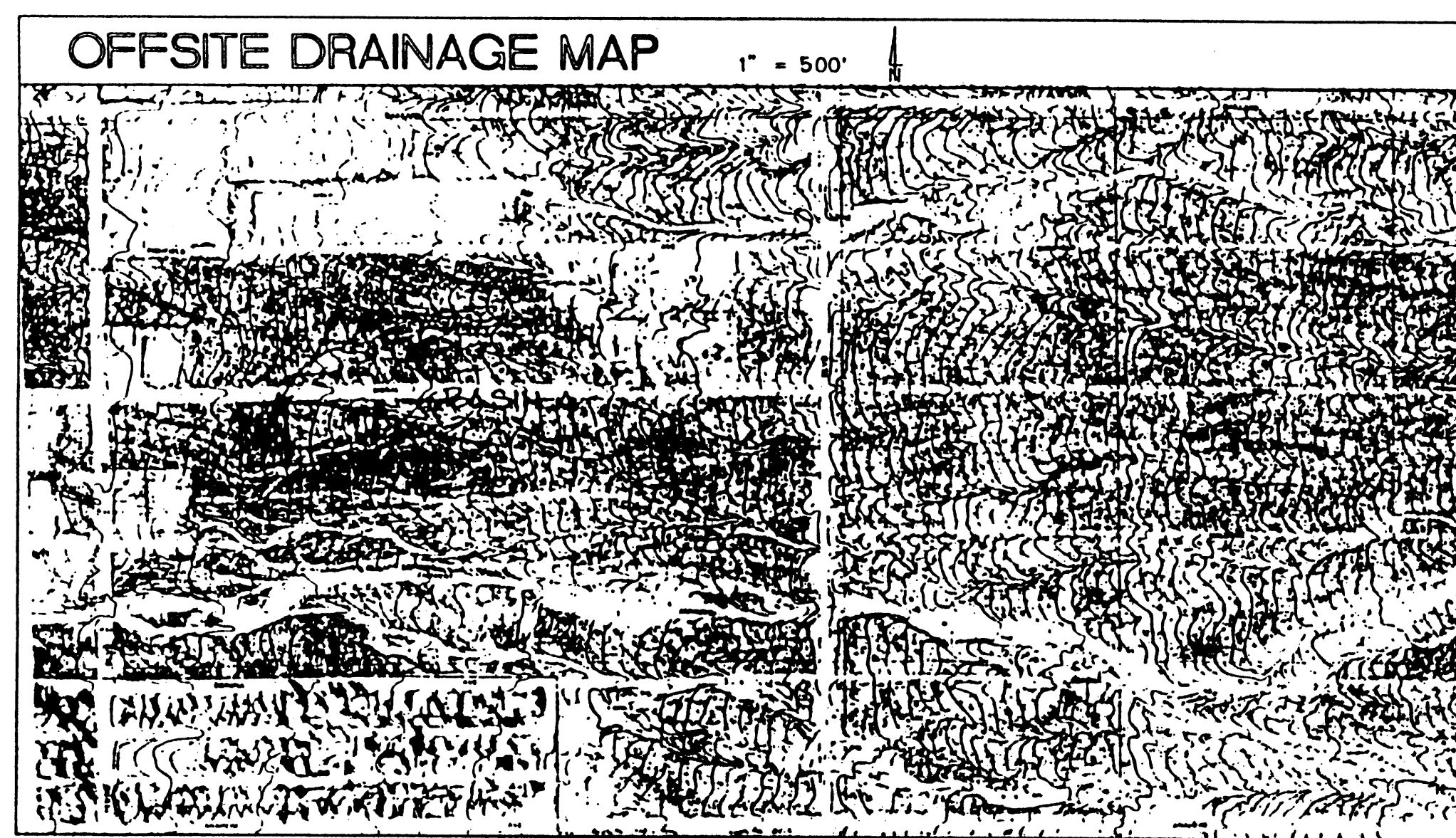
DRAINAGE NOTES

- The site is located in a developing area where most of the street surfaces are native soil and underground infrastructure is scattered. The lot is located in the flow path of a local arroyo which has been significantly reduced in size due to the construction of Sonora Residential Subdivision on lots 11 thru 22 of block 30. Construction has also made water and sewer available to the area. In addition the roadway (Signal Avenue) is paved with a temporary asphalt section.
- Offsite flows from the east will be accepted by the site, routed around the building and discharged at the right of way of Signal Avenue. The offsite basin will be eliminated in the near future due to construction of a residential subdivision and a commercial building on the vacant land between the subject site and Sonora Subdivision.
- Runoff generated onsite will be detained such that the 100 year developed flow will not exceed the capacity of the existing storm drain located in San Pedro. The temporary allowable discharge rate for the subject site has been established as 1.41 cfs/acre in the report prepared by Avid Engineering for the Signal Hill Subdivision dated 09/27/96. The site discharge has been reduced to allow direct discharge of Signal Ave.
- The site is not located in a designated flood hazard area per FEMA FIRM No. 137, dated September 20, 1996.

ENGINEER'S CERTIFICATION

I, JEAN J. BORDENAVE, HEREBY CERTIFY THAT ON FEB. 5, 1997, I WAS THE AS-BUILT INFORMATION SHOWN ON THIS SHEET WAS OBTAINED UNDER MY SUPERVISION AND THAT THE AS-BUILT DRAINAGE CONDITIONS ARE IN SUBSTANTIAL COMPLIANCE WITH THE APPROVED GRADING AND DRAINAGE PLAN.

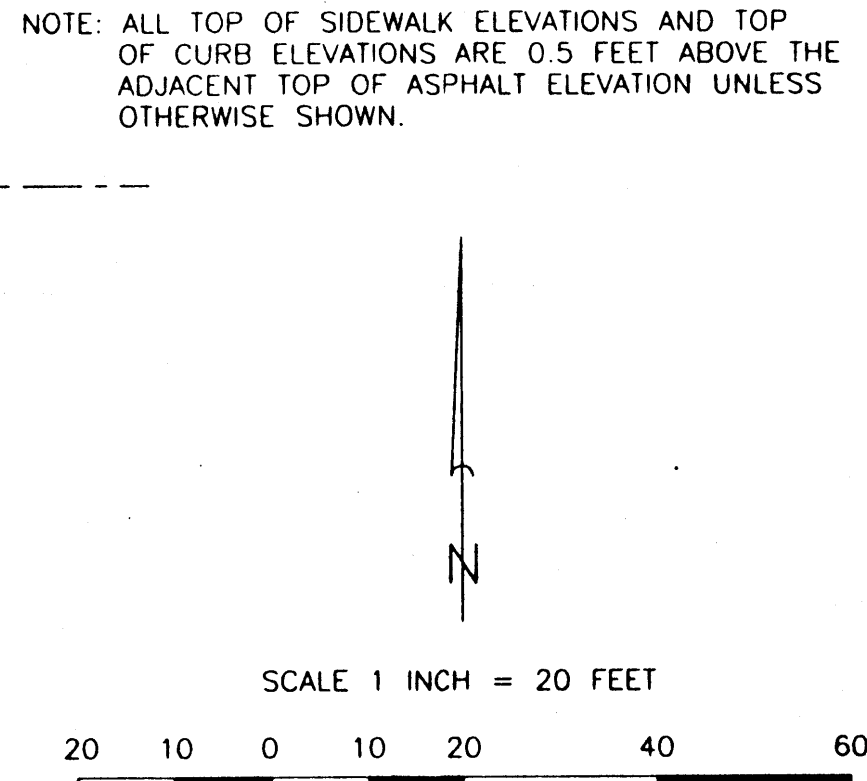
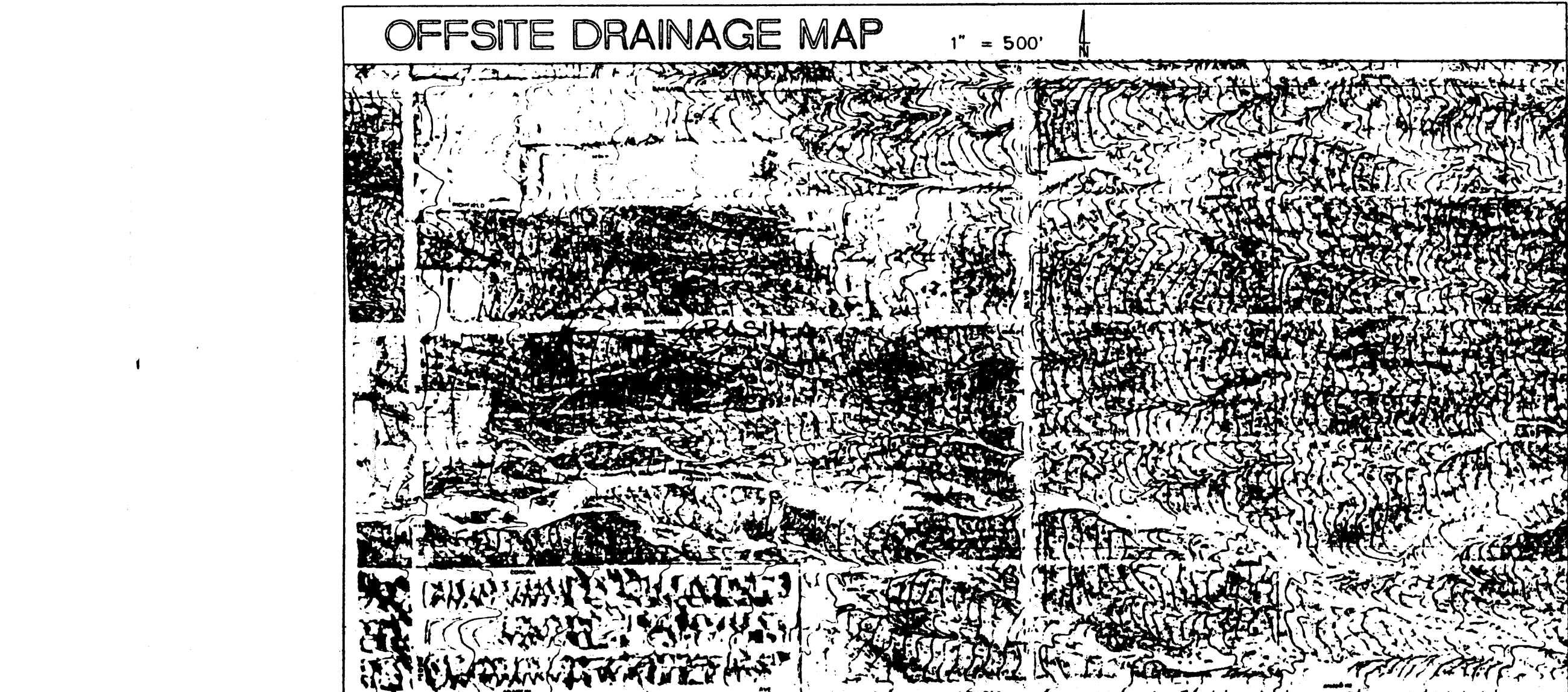
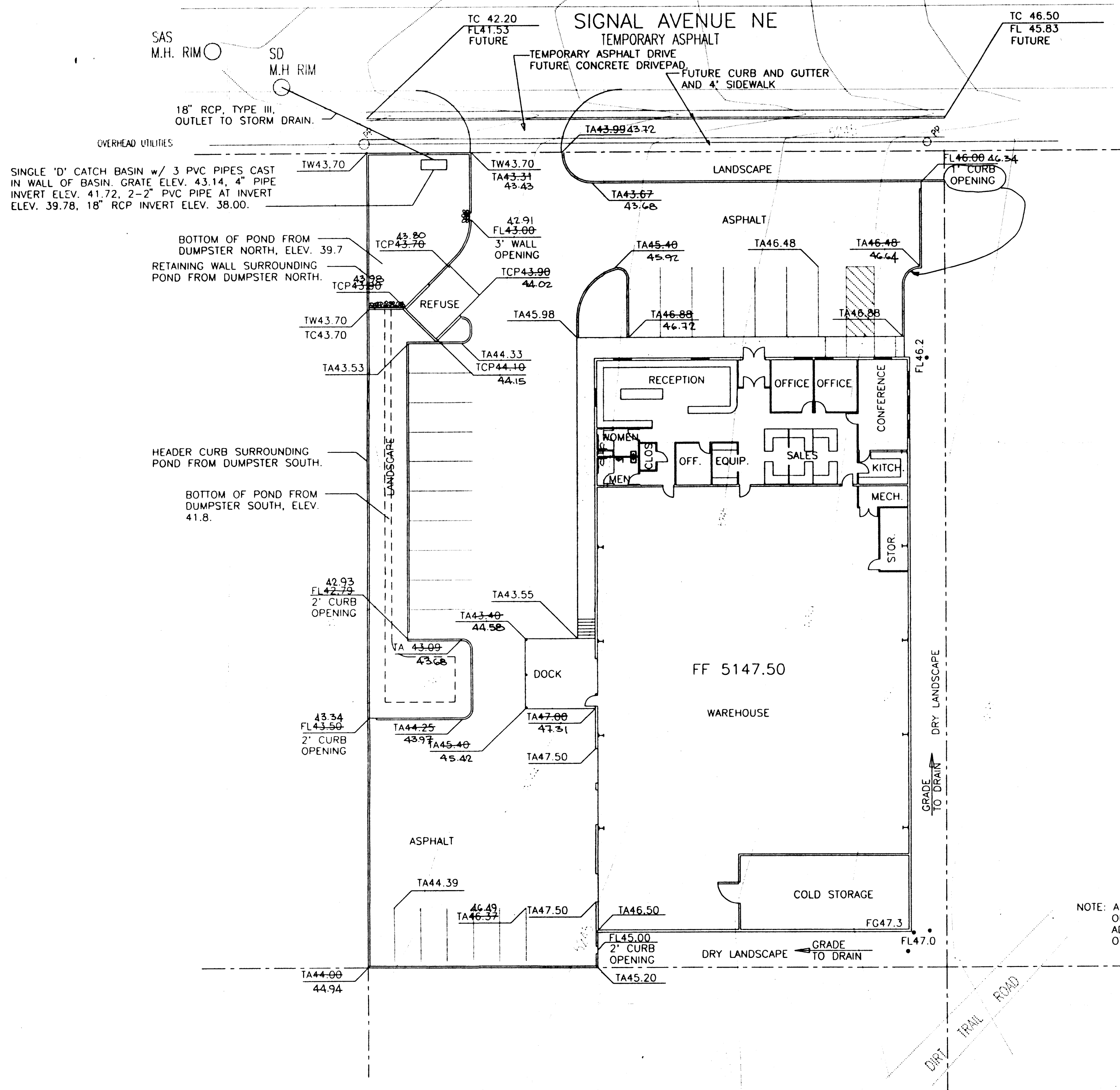
JEAN J. BORDENAVE 10/08/97
JEAN J. BORDENAVE, PE & PS # 5110



- LEGEND**
- TBM TEMPORARY BENCHMARK
 - FF FINISH FLOOR
 - FG FINISH GRADE
 - FL FLOWLINE
 - TA TOP OF ASPHALT
 - TCP TOP OF CONCRETE
 - TC TOP OF CURB
 - TP TOP OF EARTH PAD
 - TS TOP OF SIDEWALK
 - TW TOP OF WALL
 - FH FIRE HYDRANT
 - WM WATER METER
 - WV WATER VALVE
 - MH MANHOLE
 - CB CATCH BASIN GRATE
 - GM GAS METER
 - GV GAS VALVE
 - LP LIGHT POLE
 - PP POWER POLE
 - GW GUY WIRE
 - PEO ELEC. OR TEL. PEDESTAL
 - RD ROOF DRAINAGE POINT
 - FEMA FLOODPLAIN BOUNDARY
 - DRAINAGE BASIN BOUNDARY
 - EROSION SETBACK LINE
 - EXISTING CONTOUR
 - PROPOSED CONTOUR
 - EXISTING SPOT ELEVATION
 - PROPOSED SPOT ELEVATION
 - RECORD SPOT ELEVATION

NEW MEXICO WINE PATROL
612-810
CORONA STREET
ALBUQUERQUE, NEW MEXICO

C-2A



NOTE: ALL TOP OF SIDEWALK ELEVATIONS AND TOP OF CURB ELEVATIONS ARE 0.5 FEET ABOVE THE ADJACENT TOP OF ASPHALT ELEVATION UNLESS OTHERWISE SHOWN.

DRAINAGE DATA

CURB OPENINGS (RECT. WIER- $Q = CLH^{1.5}$)

BASIN 1
where $Q = 1.26$ cfs, $C = 2.8$,
 $L = 2$ ft
therefore $H = 0.37$ ft < 0.5

BASIN 2
where $Q = 1.67$ cfs, $C = 2.8$,
 $L = 2$ ft
therefore $H = 0.45$ ft < 0.5

BASIN 3
where $Q = 0.99$ cfs, $C = 2.8$,
 $L = 3$ ft
therefore $H = 0.24$ ft < 0.5

EMERGENCY SPILLWAY (TRI. WIER- $Q = CAH^{3/2}$)

Basins 1 thru 4 + Offsite 'A' via driveway
where $Q = 4.17 + 3.75 = 7.92$ cfs,
 $A = 0.5$ LH, $L = H/0.0243$
therefore $H = 0.45$ ft < 0.5
 $L = 18.52$ ft

POND CHARACTERISTICS
INFLOW HYDROGRAPH (from DPM 22.2)
 $t_p = 0.7t_c + (1.6 - A_0/A_T)/12$
where $t_c = 0.2$ hr, $A_0 = 31392$ sf
 $A_T = 38592$ sf
therefore $t_p = 0.20$ hr

$t_c = 0.25A_0/A_T$
where $A_0 = 31392$ sf, $A_T = 38592$ sf
therefore $t_c = 0.20$ hr

$t_c = 2.107(E)(A_0/Q_p) - 0.25A_0/A_T$
where $E = ((0)(0.66) + (945)(0.92) + (6255)(1.29) + (31392)(2.36))/ (0 + 945 + 6255 + 31392)$
 $= 2.15$ in, $A_T = 38592$ sf,
 $Q_p = 4.17$ cfs, $A_0 = 31392$ sf
therefore $t_c = 0.759$ hr (note: This hydrograph yields a total volume in excess of the volume from the table. Use the value from the table to size the pond.)

ALLOWABLE DISCHARGE RATE

The allowable rate for the basin in which this site is located is 1.41 cfs per acre per the Signal Hill Subdivision report. The total discharge for the public street frontage (Signal Ave.) must be deducted from this value to determine the allowable site discharge rate.

Street Discharge
 $Q_s = ((5.5)(3.45) + (24.5)(5.02))(165/43560)$
0.54 cfs

Allowable Site Discharge
 $Q = 1.41 - 0.54 = 0.87$ cfs

OUTFLOW HYDROGRAPH

Use average of triangular and rectangular hydrographs to establish pond storage requirement.

Triangular Hydrograph
inflow and outflow rates are equal at time $t = 0.20 + 0.20 + ((4.17 - 0.87)/4.17)(0.359)$
0.684 hr
 $V_t = ((0.87)(0.684)(0.5) + (0.759 - 0.684)(0.87))(3600) = 1306$ cf
 $V_R = (0.87)(0.759)(3600) = 1842$ cf
 $V_{AVE} = (1306 + 1842)/2 = 1574$ cf

POND VOLUME REQUIRED

$V = 2130 + 2845 + 1607 + 336 - 1547 = 5371$ cf

POND VOLUME PROVIDED

South End (1.5 ft deep)
 $V = 1649$ cf

North End
Required volume = $5371 - 1649 = 3722$ cf
Surface area provided = 1067 sf (scaled)
Depth = $3722/1067 = 3.49$ ft Use 3.5 ft.

OUTLET ORIFICE

$Q = Ca(2gh)^{0.5}$
where $Q = 0.87$ cfs, $C = 0.6$, a = orifice area,
 H = orifice head, use multiple orifices to help avoid plugging.

set 2 - 2" orifices at bottom of pond
 $Q = 0.39$ cfs, balance = $0.87 - 0.39 = 0.48$ cfs
set 1 - 4" orifice at $H = 1.31$ ft

LEGEND

- TBM TEMPORARY BENCHMARK
- FF FINISH FLOOR
- FG FINISH GRADE
- FL FLOWLINE
- TA TOP OF ASPHALT
- TCP TOP OF CONCRETE
- TC TOP OF CURB
- TP TOP OF EARTH PAD
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- EROSION SETBACK LINE
- EXISTING CONTOUR
- PROPOSED CONTOUR
- EXISTING SPOT ELEVATION
- PROPOSED SPOT ELEVATION
- RECORD SPOT ELEVATION

HYDROLOGY

CONDITION	STORM RETURN PERIOD	TREATMENT TYPE	TREATMENT AREA	EXCESS PRECIPITATION	PEAK RUNOFF	RUNOFF VOLUME	RUNOFF RATE
DEVELOPED	10	(Table 4)	(Table 5)	(Table 6)	(Table 7)	(Table 8)	(Table 9)
1	10	A	0.19	0.58	0.00	0.00	0.00
		B	0.36	1.19	0.00	0.00	0.00
		C	1000	0.82	2	52	0.05
		D	10285	2.36	5.02	2023	1.18
		TOTAL	11285	1.5	3.38	1337	0.85
100		A	0	0.00	1.87	0	0.00
		B	0	0.92	2.6	0	0.00
		C	1000	1.29	3.45	106	0.08
		D	10285	2.36	5.02	2023	1.18
		TOTAL	11285	1.5	3.38	1337	0.85
2	10	A	0	0.19	0.58	0	0.00
		B	0	0.36	1.19	0	0.00
		C	0	0.82	2	52	0.05
		D	14468	1.5	3.38	1808	1.13
		TOTAL	14468	1.5	3.38	1808	1.13
100		A	0	0.00	1.87	0	0.00
		B	0	0.92	2.6	0	0.00
		C	0	1.29	3.45	0	0.00
		D	14468	2.36	5.02	2845	1.87
		TOTAL	14468	2.36	5.02	2845	1.87
3	10	A	0	0.19	0.58	0	0.00
		B	945	0.36	1.19	26	0.03
		C	2133	0.82	2	110	0.10
		D	8839	1.5	3.38	830	0.52
		TOTAL	9717	1.5	3.38	906	0.64
100		A	0	0.00	1.87	0	0.00
		B	945	0.92	2.6	72	0.09
		C	2133	1.29	3.45	229	0.17
		D	8839	2.36	5.02	1308	0.77
		TOTAL	9717	2.36	5.02	1607	0.99
4	10	A	0	0.19	0.58	0	0.00
		B	0	0.36	1.19	0	0.00
		C	3122	0.82	2	181	0.14
		D	0	1.5	3.38	0	0.00
		TOTAL	3122	1.5	3.38	181	0.14
100		A	0	0.00	1.87	0	0.00
		B	0	0.92	2.6	0	0.00
		C	3122	1.29	3.45	336	0.25
		D	0	2.36	5.02	0	0.00
		TOTAL	3122	2.36	5.02	336	0.25
O 100		A	87450	0.00	1.87	4810	3.75
F		B	0	0.92	2.6	0	0.00
F		C	0	1.29	3.45	0	0.00
F		D	0	2.36	5.02	0	0.00
A		TOTAL	87450	2.36	5.02	4810	3.75

DRAINAGE NOTES

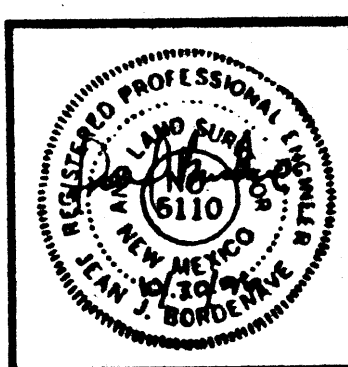
- The site is located in a developing area where most of the street surfaces are native soil and underground infrastructure is scattered. The lot is located in the flow path of a local arroyo which has been significantly reduced in size due to the construction of Sonora Residential Subdivision on lots 11 thru 22 of block 30. Construction has also made water and sewer available to the area. In addition the roadway (Signal Avenue) is paved with a temporary asphalt section.
- Offsite flows from the east will be accepted by the site, routed around the building and discharged at the right of way of Signal Avenue. The offsite basin will be eliminated in the near future due to construction of a residential subdivision and a commercial building on the vacant land between the subject site and Sonora Subdivision.
- Runoff generated onsite will be detained such that the 100 year developed flow will not exceed the capacity of the existing storm drain located in San Pedro. The temporary allowable discharge rate for the subject site has been established as 1.41 cfs/acre in the report prepared by Avid Engineering for the Signal Hill Subdivision dated 09/27/96. The site discharge has been reduced to allow direct discharge of Signal Ave.
- The site is not located in a designated flood hazard area per FEMA FIRM No. 137, dated September 20, 1996.

ENGINEERS CERTIFICATION

I, JEAN J. BORDENAVE, HEREBY CERTIFY THAT ON FEBRUARY 5, 1997 THE AS-BUILT INFORMATION SHOWN ON THIS SHEET WAS OBTAINED UNDER MY SUPERVISION AND THAT THE AS-BUILT DRAINAGE CONDITIONS ARE IN SUBSTANTIAL COMPLIANCE WITH THE APPROVED GRADING AND DRAINAGE PLAN EXCEPT AS NOTED BELOW

Jeann J. Bordenave 02/11/97
JEAN J. BORDENAVE, PE & PS # 5110

- HEADER CURB ON WEST SIDE OF POND, RETAINING WALL AT NORTH END OF POND AND CATCH BASIN AT POND OUTLET ARE UNDER CONSTRUCTION.
- PIPE OUTLET FROM POND TO PUBLIC STORM DRAIN IN CITY DRC REVIEW.



A.I.A.
ARCHITECT
JOHN FREDMAN, A.I.A.
130 MAIN STREET
LOS ALAMOS, N.M. 87531
P.O. BOX 338
87531

DATE: 10/31/96
REVISED:

DRAINAGE CALCULATIONS
NOTE: THIS SHEET IS FOR CITY HYDROLOGY
REVIEW AN IS NOT TO BE A PART OF THE PLANS.

NEW MEXICO WINE PATROL
CORONA STREET
ALBUQUERQUE, NEW MEXICO
6/6/97
CAB-2A

C-2A