



City of Albuquerque

P.O. BOX 1293 ALBUQUERQUE, NEW MEXICO 87103

3 May 1999

Diane Hoelzer, PE
Mark Goodwin & Associates, PA
PO Box 90606
Albuquerque, NM 87199

RE: EAGLE ROCK ESTATES, UNIT III (C18/D39B). ENGINEER'S CERTIFICATION
FOR FINAL PLAT APPROVAL. CERTIFICATION DATED 4-12-99

Dear Ms. Hoelzer:

Based upon the information provided in your 4-12-99 submittal, the referenced project is
approved for Final Plat.

If I can be of further assistance, feel free to contact me at 768-2766.

Sincerely,

Scott Davis
PWD, Hydrology Div.

c: Andrew Garcia
file

*Terri, this
is APPROVED &
FOR PROSEC
PLANT / SIA
ACCEPT
RELEASE
5-5-99*



D. Mark Goodwin & Associates, P.A.
Consulting Engineers

P.O. BOX 90606, ALBUQUERQUE, NM 87199
(505) 828-2200 FAX 797-9539
e-mail: dmgs@swcp.com

PROJECT Eagle Rock III
SUBJECT Hydrology Addendum
BY DLH DATE _____
CHECKED _____ DATE _____
SHEET _____ OF _____

Onsite Area = 6.3135 ac. = .009865 sqmi (ref. 12-28-97)

single family $n = 46 \text{ units} / 6.55 \text{ acres} = 7.0$

$$Tr. D = 7 (7^2 + 5(7))^{.5} = 64.15$$

$$Tr B = 17.925$$

$$Tr C = 17.925$$

$$Q_{100} (\text{revised}) = 26.0 \text{ cfs}$$

$$Vol = 0.940 \text{ ac-ft.}$$

$$Q_{100} (\text{previous}) = 24.6 \text{ cfs}$$

$$Vol = .871 \text{ ac-ft.}$$

$$D = 53.2$$

	Area (acres)	$Q_{previous}$	$Q_{revised}$
SB-1	1.554	6.1	6.5
SB-2	1.982	7.7	8.1
SB-3	2.762	10.8	11.4
		<u>24.6</u>	<u>26.0</u>

$$V_{10} = V_{360} + A_D \left(\frac{P_{10DAY} - P_{360}}{12} \right) \quad (\text{onsite, Eagle Rock, N. Louisiana})$$

$$V_{10} = (.94 + .029 + .071) + [6.31(.642) + .18(.755) + .429(.80)]$$

$$\left(\frac{4.9 - 2.6}{12} \right) = 1.04 + (4.53(.19166)) = 1.91 \text{ Ac-Ft.} = \text{Req'd. Vol.}$$

$$\begin{aligned} \text{Design Volume} &= 127' \times 86' = 10,922 = A_1 \\ (14' \text{ depth}) & \quad 71' \times 30' = 2130 = A_2 \end{aligned}$$

$$\begin{aligned} \text{Volume} &= \frac{1}{3} (14) (2130 + 10,922 + \sqrt{2130 \times 10,922}) = 83,418 \text{ CF} \\ &= 1.915 \text{ A.F.} \end{aligned}$$



March 17, 1998

Diane Hoelzer, P.E.
Mark Goodwin & Associates
P.O. Box 90606
Albuquerque, New Mexico 87199

RE: Revised Grading and Drainage Plan for Eagle Rock Estates Unit III (C18/D39B) Submitted for Preliminary Plat and Grading Permit Approval, Engineer's Stamp Dated 3/6/98.

Dear Ms. Hoelzer:

Based on the information provided in the submittal of March 9, 1998, the above referenced revised plan is acceptable for Preliminary Plat approval provided that it is approved at DRB.

Prior to Rough Grading permit release, the easement must be in place for the off-site grading just northwest of the site.

The submittal for the Letter of Map Revision to remove the existing floodplain is still in review at FEMA pending further analysis. The Final Plat, therefore, must indicate that the entire subdivision is encumbered with a FEMA floodplain.

If you have any questions, or if I may be of further assistance to you, please call me at 924-3982.

Sincerely,

Susan M. Calongne, P.E.
City/County Floodplain Administrator

c: DRB #97-487
Mark Pagels, Sr., Sunset West
☐ File

Good for You, Albuquerque!





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Consulting Engineers

P.O. BOX 90606, ALBUQUERQUE, NM 87199
(505) 828-2200 FAX 797-9539
e-mail: dmg@swcp.com

PROJECT Eagle Rock Unit 3
SUBJECT Oakland Ave. Pond.
BY DLH DATE 3-6-98
CHECKED _____ DATE _____
SHEET 1 OF 2

Oakland Ave. Temporary Retention Pond Calcs.

DEVELOPED CONDITIONS

	Oakland	Lot	Olivine	
Tr. D	29(620)	3993	36(85)	= 25033 SF (78.2%)
Tr. B	6(620)	2400	10(85)	= 6970 SF (21.8%)
				<u>32003 SF = .001148 S.M.</u>

USE IN AHYMO

$$TR. B = 10.9\%$$

$$TR. C = 10.9\%$$

$$TR. D = 78.2\%$$

$$Area = .001148$$

AHYMO

$$RESULT: Q = 3.25 cfs$$

$$V = .1197 AF$$

UNDEVELOPED CONDITIONS

	Oakland	
Tr. A	35(620)	= 21,700 SF (100%)
		= .000778 S.M.

USE IN AHYMO

$$TR. A = 100\%$$

$$Area = .000778 S.M.$$

AHYMO

$$RESULT: Q = .85 cfs$$

$$V = .0245 AF$$

Retention Volume Calc.

$$V_{10} = V_{360} + A_D \left(\frac{P_{10D} - P_{360}}{12} \right)$$

$$Tr. D = 25033 SF = .5747 acres$$

$$V_{10} = (.1197 + .0245) AF + .5747 A \left(\frac{4.9 - 2.6}{12} \right) =$$

$$= .0952 AF + .1101 = .2053 AF OF RETAINED VOLUME$$



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PROJECT Eagle Rock Unit 3
SUBJECT Oakland Ave Pond
BY DLH DATE 3-6-98
CHECKED _____ DATE _____
SHEET 2 OF 2

Pond Design

Top of Pond = 5281.0 Area_T = 3733.6 SF

Bottom of Pond = 5278.0 Area_B = 2292.1 SF

Depth = 3.0 Ft.

Design Volume = $\frac{1}{3}(\text{Depth})(A_T + A_B + \sqrt{A_T \cdot A_B})$

$= \frac{1}{3}(3)(3733.6 + 2292.1 + \sqrt{3733.6 \cdot 2292.1})$

$= 8951.07 \text{ Cu. Ft.}$

$= 2054 \text{ Ac. Ft.}$

Req'd Volume = 2053 Ac Ft.

TABLE 2: SUMMARY OF INLET CALCULATIONS

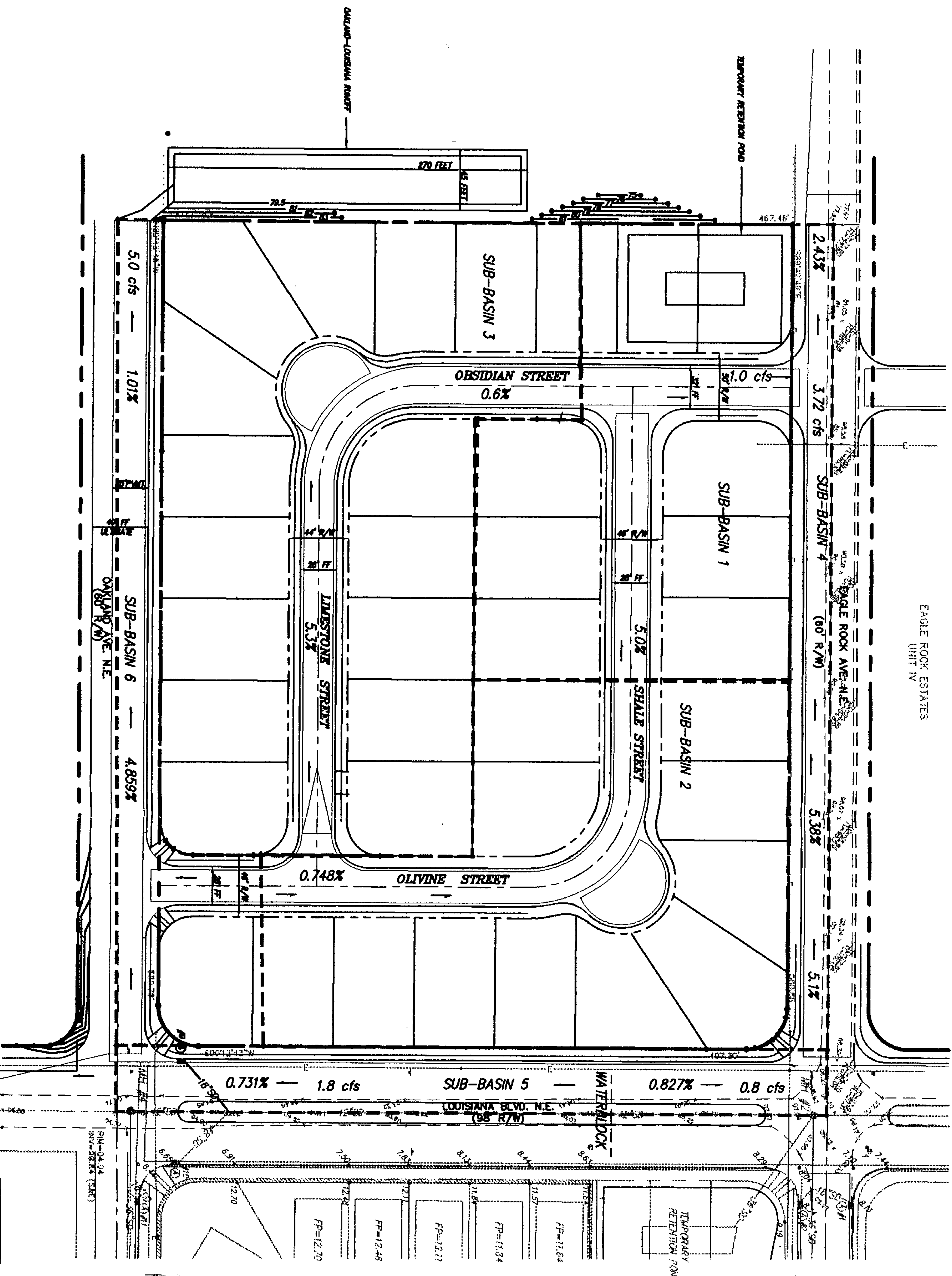
**EAGLE ROCK ESTATES
UNIT 3**

LOCATION	CURB	WIDTH ft.	SLOPE %	Q cfs	DEPTH ft	EG ft	Q INLET cfs	#/TYPE of INLETS	REMAIN Q (cfs)
Limestone Street	MTB	26' FF	5.3	5.4	0.17	0.39	N/A		
Shale Street	MTB	28' FF	5.0	7.7	0.23	0.37	N/A		
Obsidian Street	STD	32' FF	0.6	24.6	0.51	0.63	6.4	2 DBLA	11.80
Obsidian Street	STD	32' FF	0.6	11.8	0.42	0.48	3.6	2 SGL C	4.60
Obsidian Street	STD	32' FF	0.6	4.6	0.31	0.36	1.8	2 SGL C	1.00
Oakland Ave	STD	40' FF	4.86	5.0	0.29	.59	N/A		
Oakland Ave	STD	40' FF	1.01	5.0	.37	.46	N/A		
Louisiana Ave	STD	31'	0.731	1.8	.28	.33	1.8	1 SGL A	0
Eagle Rock Ave	STD	40' FF	5.38	2.72	.24	.48	N/A		
Eagle Rock Ave	STD	40' FF	2.43	3.72	.29	.45	3.2	1 SGL A	.52

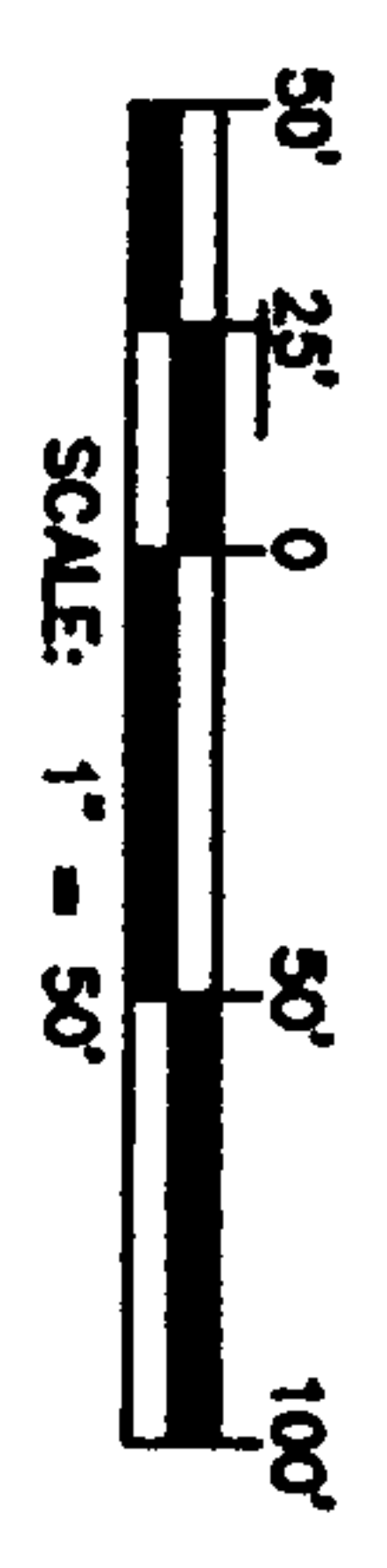
MTB = Mountable Curb

STD = Standard Curb

f:\eaglerck\inlet3.cal



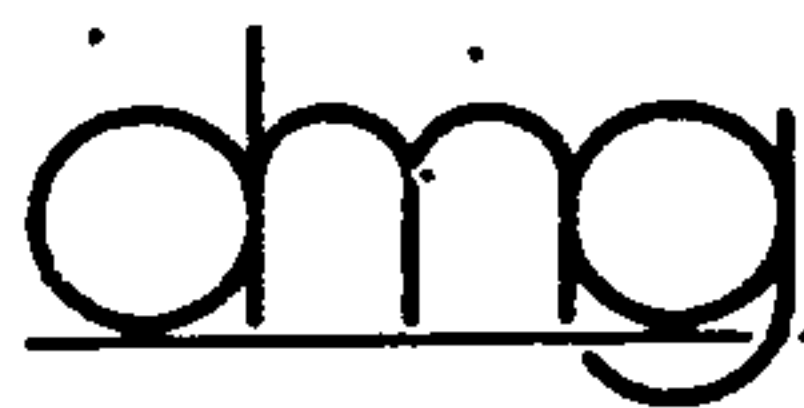
AREA 1 = 1.554 AC.	Q = 6.1 CFS
AREA 2 = 1.982 AC.	Q = 7.7 CFS
AREA 3 = 2.762 AC.	Q = 10.8 CFS
AREA 4 = 0.427 AC.	Q = 1.92 CFS
AREA 5 = 0.594 AC.	Q = 2.6 CFS
AREA 6 = 0.706 AC.	Q = 3.2 CFS
TOTAL = 8.025 AC.	Q = 32.3 CFS
TOTAL = 0.0125 SQ. MI.	



RECEIVED
DEC 09 1997
HYDROLOGY SECTION

EXHIBIT 3

EAGLE ROCK ESTATES UNIT 3
DRAINAGE BASIN BOUNDARIES



D. Mark Goodwin & Associates, P.A.
Consulting Engineers and Surveyors

PROJECT Eagle Rock Unit III
SUBJECT _____
BY _____ DATE _____
CHECKED _____ DATE _____
SHEET _____ OF _____

Onsite Retention Pond Volume (100-yr 10 day storm)

$$V_{10} = V_{360} + A_D \cdot \left(\frac{P_{100} - P_{360}}{12} \right) \quad \text{(Incl. Eagle Rock Area \& N. Louisiana Blvd.)}$$

$$V_{360}(\text{onsite}) = [.92(.234) + 1.29(.234) + 2.36(.532)] 6.313 \text{ ac.} +$$

$$V_{360}(\text{Eagle Rock}) = [.92(.10) + 1.29(.10) + 2.36(.80)] .427 \text{ ac.} +$$

$$V_{360}(\text{Louisiana}) = [.92(.1225) + 1.29(.1225) + 2.36(.755)] .179 \text{ ac.}$$

$$= (11.1908 \text{ in-ac.} + .9005 \text{ in-ac.} + .3674 \text{ in-ac.}) \div 12$$

$$= 1.038 \text{ ac-ft}$$

$$A_D = 6.313(.532) + .427(.80) + .179(.755) = 3.835 \text{ acres}$$

$$V_{10 \text{ DAY}} = 1.038 \text{ ac-ft} + 3.835 \text{ ac-ft} \left(\frac{4.9 - 2.6}{12} \right) = 1.773 \text{ ac-ft.}$$

$$V_{10 \text{ DAY}} = 1.773 \text{ AF}$$

Offsite Retention Pond Volume (100-yr 10 day storm) (Louisiana Blvd + S. Oakland Ave)

$$V_{360}(\text{S. Louisiana}) = [.92(.1225) + 1.29(.1225) + 2.36(.755)] .415$$

$$V_{360}(\text{Oakland}) = [.92(.109) + 1.29(.109) + 2.36(.782)] .734$$

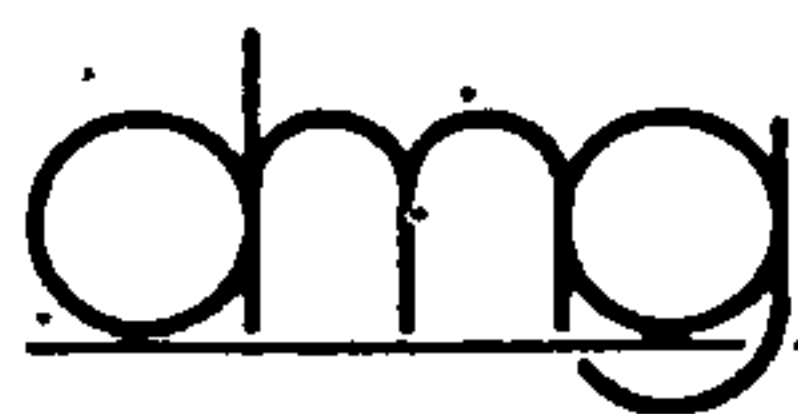
$$= (.8518 + 1.531) \div 12$$

$$= .1985 \text{ ac-ft.}$$

$$A_D = .755(.415) + .782(.734) = .8873 \text{ acres}$$

$$V_{10 \text{ DAY}} = .1985 \text{ ac-ft} + .8873 \left(\frac{4.9 - 2.6}{12} \right) =$$

$$V_{10 \text{ DAY}} = .3685 \text{ AF}$$



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Consulting Engineers and Surveyors

PROJECT Eagle Rock Unit III

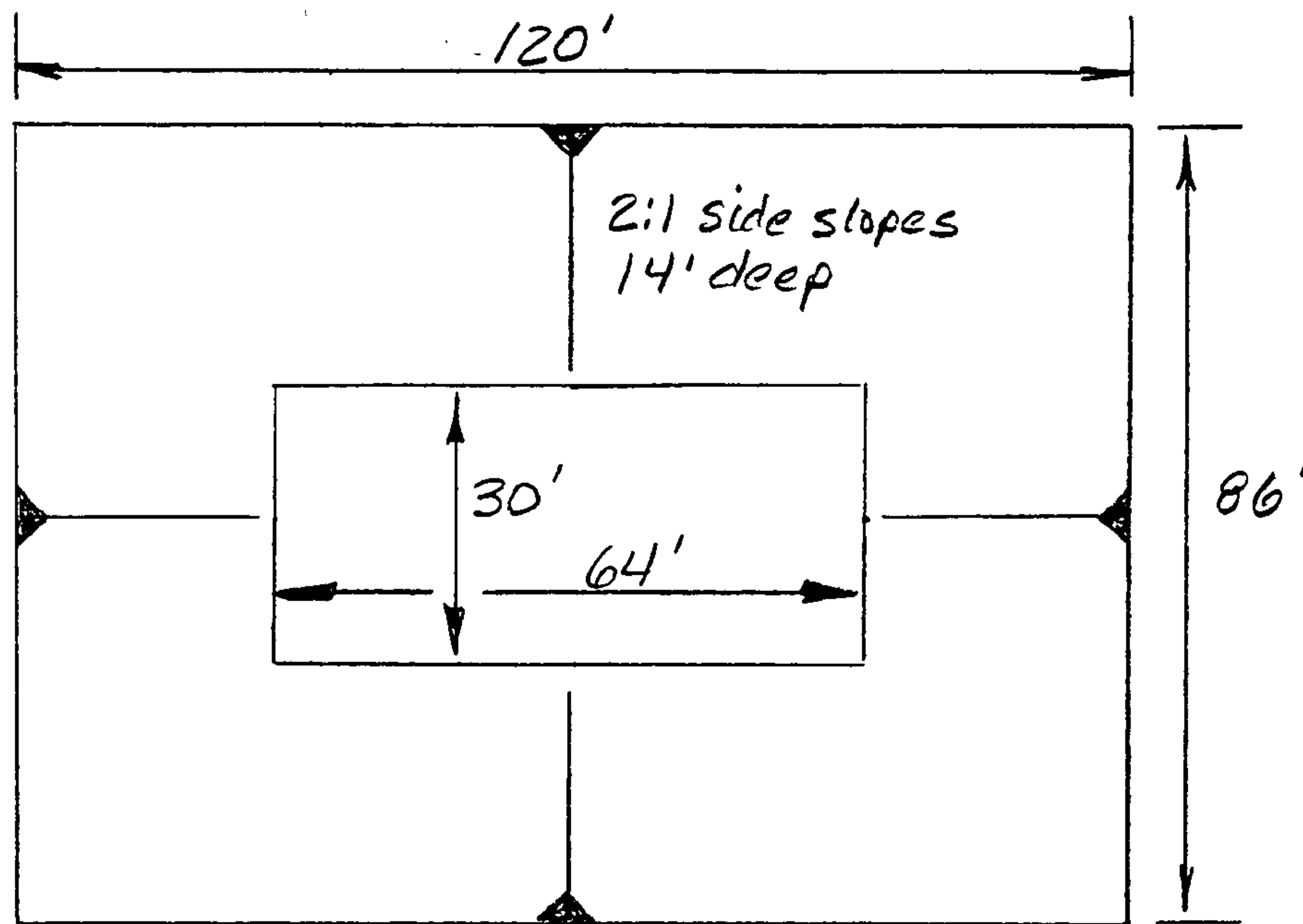
SUBJECT _____

BY _____ DATE _____

CHECKED _____ DATE _____

SHEET _____ OF _____

Onsite Retention Pond



$$\text{Design Volume} = \frac{1}{3} (\text{Depth}) (A_1 + A_2 + \sqrt{A_1 \cdot A_2})$$

$$A_1 = 64 \times 30 = 1920 \text{ SF}$$

$$A_2 = 120 \times 86 = 10320 \text{ SF}$$

$$\text{Vol. Design} = \frac{1}{3} (14) (1920 + 10320 + \sqrt{1920 \cdot 10320})$$

$$\text{Vol. Design} = 77892.9 \text{ CF} = 1.788 \text{ AF}$$

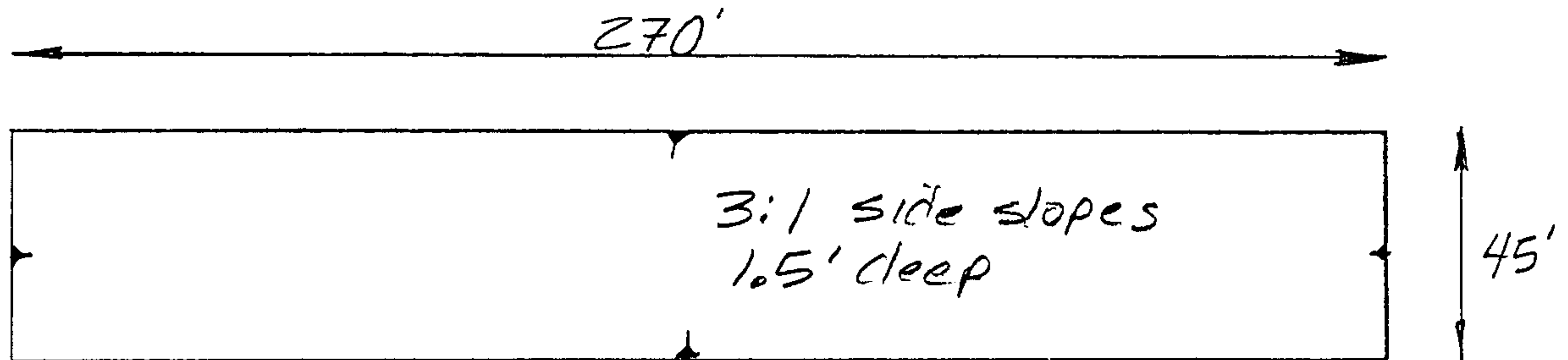
$$\text{Vol. Req'd} = 1.773 \text{ AF}$$



D. Mark Goodwin & Associates, P.A.
Consulting Engineers and Surveyors

PROJECT Eagle Rock Unit III
SUBJECT _____
BY _____ DATE _____
CHECKED _____ DATE _____
SHEET _____ OF _____

Offsite Retention Pond



$$\text{Design Volume} = \frac{1}{3} (\text{Depth}) (A_1 + A_2 + \sqrt{A_1 \times A_2})$$

$$A_1 (\text{bottom}) = 261 \times 36 = 9396 \text{ SF}$$

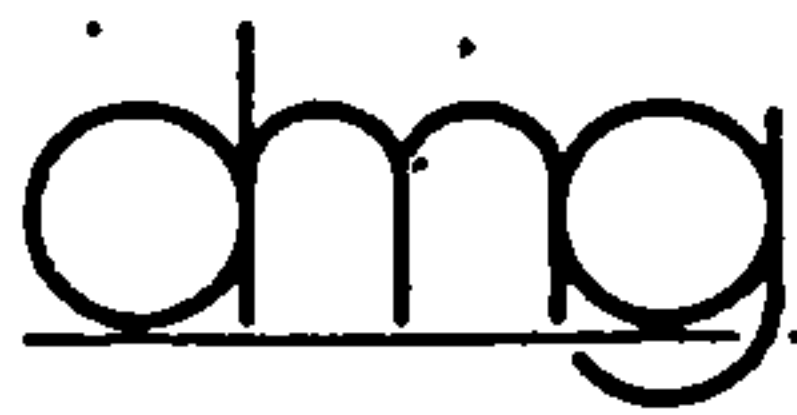
$$A_2 (\text{top}) = 270 \times 45 = 12150 \text{ SF}$$

$$\text{Vol. Design} = \frac{1}{3} (1.5) (9396 + 12150 + \sqrt{9396 \cdot 12150})$$

$$\text{Vol. Design} = 16115 \text{ SF}$$

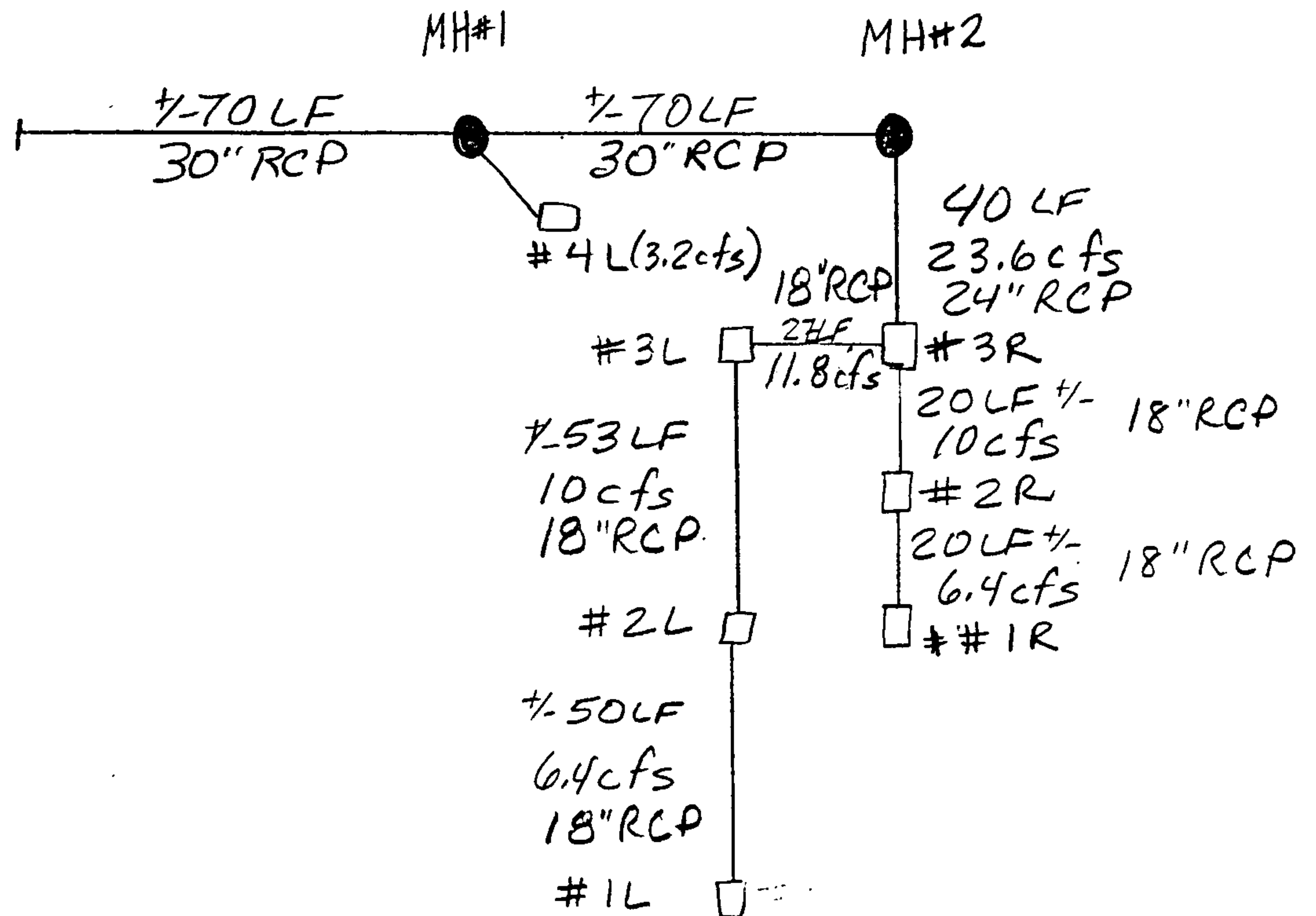
$$\text{Vol. Design} = 0.370 \text{ AF}$$

$$\text{Vol. Req'd} = 0.3685 \text{ AF}$$



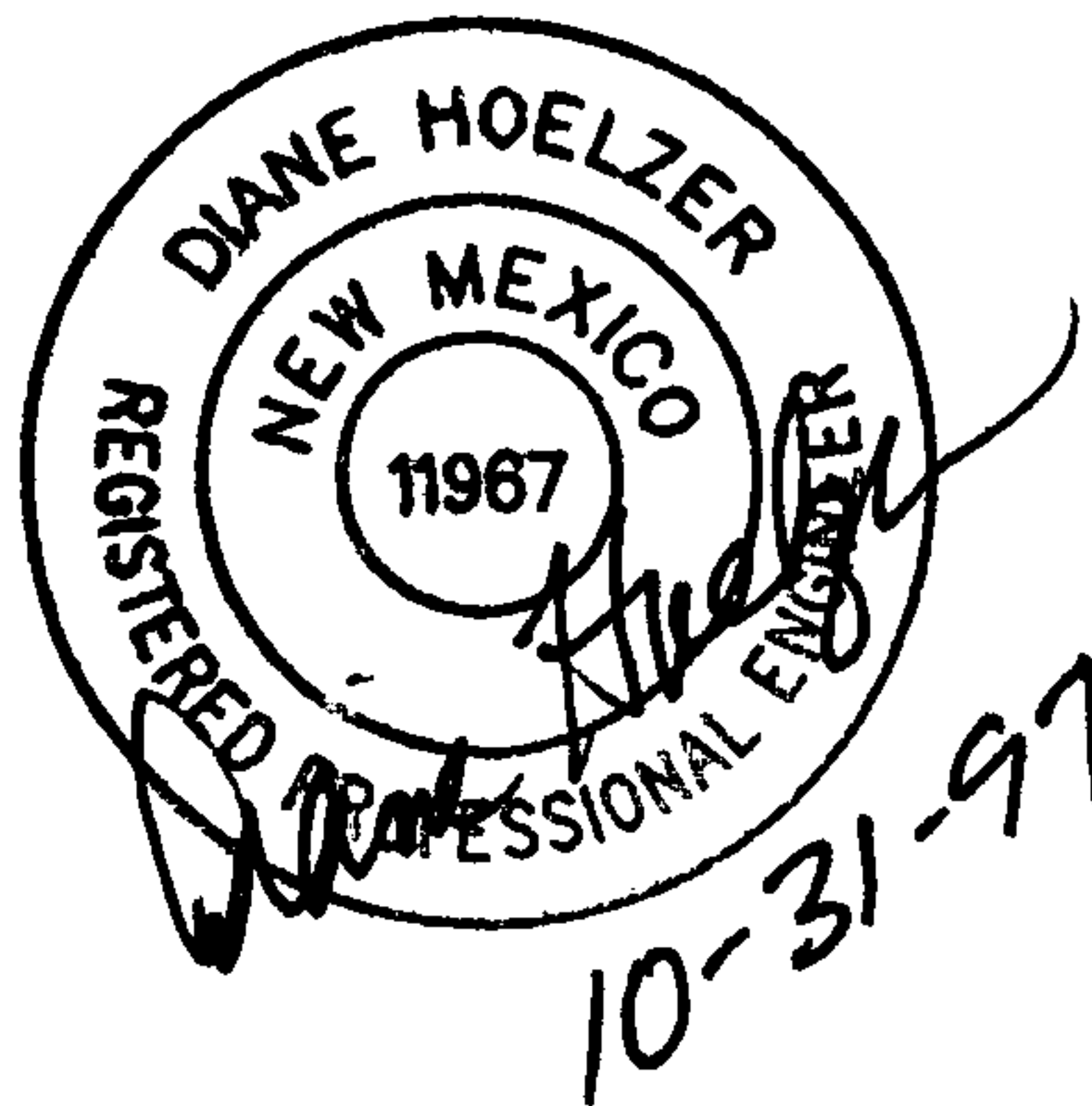
D. Mark Goodwin & Associates, P.A.
Consulting Engineers and Surveyors

PROJECT Storm Drain Unit III
SUBJECT Prel. Calcs.
BY DLH DATE 12-5-97
CHECKED _____ DATE _____
SHEET _____ OF _____



	SLOPE	Q	Depth	
#1L - #2L	1.0 %	6.4 cfs	.95'	18" RCP
#2L - #3L	1.0 %	10 cfs	1.5'	18" RCP
#3L - #3R	1.0 %	11.8 cfs	pressure	18" RCP
#1R - #2R	1.0 %	6.4 cfs	.95'	18" RCP
#2R - #3R	1.0 %	10 cfs	1.5'	18" RCP
#3R - MH2	1.5 %	23.6 cfs	1.6'	24" RCP
MH2 - MH1	3.5 %	47.2 cfs	1.55'	30" RCP
MH1 - END	3.5 %	53.1 cfs	1.7'	30" RCP

DRAINAGE REPORT
for
EAGLE ROCK ESTATES UNIT 3



OCTOBER 1997

I. LOCATION AND DESCRIPTION

The proposed Eagle Rock Estates Unit 3 is comprised of approximately 6.55 acres and is located in North Albuquerque Acres just west of the proposed Eagle Rock Estates Unit 2 (Figure 1). Proposed development includes the infrastructure to support the development of 36 single family residential homes.

The topographic relief in the area is in an westerly direction at a slope of approximately 3.5 percent.

The FEMA map indicates that all of the site is within the 100-year floodplain (Figure 2). AMAFCA has submitted a LOMR to FEMA to remove the floodplain based on the dike constructed at Wyoming and Louisiana. To date, AMAFCA has not heard from FEMA.

II. DRAINAGE DESIGN CRITERIA AND PREVIOUS REPORTS

The design criteria used in this report was in accordance with Section 22.2 Hydrology of the Development Process Manual, Volume 2, Design Criteria, January 1993 edition. A master drainage management plan for this area in North Albuquerque Acres which included the Unit 3 property was prepared and approved by Hydrology this year and was called the Eagle Rock Subdivision Conceptual Drainage Master Plan Report, (C18/D39) dated April, 1997 with supplemental information dated June 12, 1997. The results for their "existing drainage conditions" analysis were assumed still valid for purposes of this report. Their proposed interim and future drainage conditions presented in their report are being modified in this submittal because the project limits for the Eagle Rock Estates have been changed since the time of their approved plan.

III. EXISTING DRAINAGE CONDITIONS

Under existing drainage conditions, runoff flows in a westerly direction through the site in one well defined arroyo. Offsite flows enter the site from the east. Flows in Eagle Rock Avenue to the north continue in a westerly direction within and along side the street section.

IV. FUTURE DRAINAGE CONDITIONS

A. INTERIM CONDITIONS

For the interim condition a temporary retention pond will be constructed on two lots in the northwest corner of Unit 3 and will retain a minimum of 1.58 acre-feet. The pond will have 2:1 side slopes and be protected by a 2" thick gravel mulch, will be 14.0 feet deep and will be fenced. All the onsite drainage will be intercepted by a series of inlets at the north end of Obsidian Street. A storm drain in Eagle Rock Avenue will be built from the Obsidian Street intersection west to the property line for the future connection into the proposed storm drain system in Eagle Rock Avenue. For the interim condition, the storm drain runoff will be diverted into the temporary retention pond. A waterblock at the Olivine/Oakland intersection will prevent offsite flows from entering the site.

1. Louisiana Blvd. and Oakland Avenue

Offsite flows generated east of the proposed Louisiana Blvd. centerline will be intercepted by the inlets in Louisiana Blvd. at Modesto Avenue and Oakland Avenue and by the temporary retention ponds in Unit 1 and 2. Any nuisance flows not intercepted will be prevented from crossing over Louisiana Blvd. by the crown section and instead will be directed south or north along the Louisiana Blvd. east flowline. Most of the offsite flows generated in the west half of Louisiana Blvd. will be directed south and then west along Oakland Avenue. Since Oakland Avenue west of Louisiana Blvd. does not presently exist,

all offsite flows in Oakland will be intercepted by the offsite Oakland-Louisiana temporary retention Pond along Unit 3 west property boundary as shown on the grading and drainage plan.

2. Eagle Rock Avenue

Offsite runoff in Eagle Rock Avenue adjacent to Unit 3 will continue to flow in a westerly direction following the same historical flow pattern. Eagle Rock Avenue is currently paved all the way to San Pedro Avenue. The proposed arterial Louisiana Blvd. crown section will prevent any offsite flows in Eagle Rock east of Louisiana Blvd. from crossing over Louisiana Blvd. Offsite flows in Eagle Rock Avenue east of Louisiana Blvd. will be intercepted by inlets in Eagle Rock at Louisiana Blvd.

B. ULTIMATE CONDITIONS

For the ultimate drainage conditions, the onsite temporary retention pond will disappear when the downstream storm drain improvements have been built. The temporary Oakland-Louisiana Retention pond will disappear when the adjacent property owner develops his/her site. At that time when development of their property occurs, these offsite flows will need to be addressed.

TABLE 1
EXISTING CONDITIONS
HYDROLOGIC CHARACTERISTICS
AND
100-YEAR FLOW RATES

BASIN	AREA SQ.MI.	% A	% B	% C	% D	TP HRS	V100 AC-FT	Q100 CFS
101	.0094	85	0	5	10	.13	.41	12.8
102	.0247	85	0	5	10	.13	1.07	33.5
103	.0112	85	0	5	10	.13	.49	15.2
201	.0254	85	0	5	10	.13	1.10	34.4
202	.0179	85	0	5	10	.13	.78	24.3
203	.005	85	0	5	10	.13	.22	6.8
204	.0095	85	0	5	10	.13	.41	12.9

TABLE 2
SUMMARY OF INLET CALCULATIONS

Eagle Rock Estates, Unit 3

LOCATION	CURB	WIDTH ft.	SLOPE %	Q cfs	DEPTH ft	EG ft	Q INLET cfs	#/TYPE of INLETS	REMAIN Q (cfs)
Limestone Street	MTB	26' FF	5.3	5.4	0.17	0.39	N/A		
Shale Street	MTB	28' FF	5.0	7.7	0.23	0.37	N/A		
Obsidian Street	STD	32' FF	0.6	24.6	0.51	0.63	6.4	2 DBLA	11.80
Obsidian Street	STD	32' FF	0.6	11.8	0.42	0.48	3.6	2 SGL C	4.60
Obsidian Street	STD	32' FF	0.6	4.6	0.31	0.36	1.8	2 SGL C	1.00

Louisiana and Eagle Rock

to Eagle Rock

MTB = Mountable Curb
STD = Standard Curb

f:\eaglerck\inlet3.cal



D. Mark Goodwin & Associates, P.A.
Consulting Engineers and Surveyors

PROJECT Eagle Rock - Unit 3
SUBJECT Retention Pond Calcs.
BY DLH DATE 10-16
CHECKED _____ DATE _____
SHEET _____ OF _____

Retention Pond Volume For 10 day storm

$$V_{10DAY} = V_{360} + A_D \cdot \left(\frac{P_{100} - P_{360}}{12} \right)$$

$$\begin{aligned} V_{360} &= .92(.234) + 1.29(.234) + 2.36(.532) \\ &= \frac{1.7727 \text{ inches (6.3135 acres)}}{12} = 0.9327 \text{ AF} \end{aligned}$$

$$P_{360} = 2.60" \quad P_{10} = 4.9"$$

$$A_D = .532(6.3135 \text{ acres}) = 3.359$$

$$V_{10DAY} = 0.933 \text{ AF} + 3.359 \cdot \left(\frac{4.9 - 2.6}{12} \right) = 1.576 \text{ AF}$$

Retention Pond For Oakland and Louisiana (Offsite)

$$\begin{aligned} V_{360} &= .92(.11575) + 1.29(.11575) + 2.36(.7685) \\ &= \frac{2.06447 (1.1443 \text{ ac.})}{12} = .1973 \text{ AF} \end{aligned}$$

$$A_D = .7685(1.1443 \text{ ac.}) = .8794 \text{ AF}$$

$$V_{10DAY} = .1973 + .8794 \left(\frac{4.9 - 2.6}{12} \right) = .3659 \text{ AF}$$



D. Mark Goodwin & Associates, P.A.
Consulting Engineers and Surveyors

PROJECT Eagle Rock Unit 3
SUBJECT LAND TREATMENT
BY _____ DATE _____
CHECKED _____ DATE _____
SHEET _____ OF _____

Eagle Rock Unit 3
OFFSITE

NORTH LOUISIANA

$$PAV (37)(160) = 5920 (75.5)$$

$$TR.B (12)(160) = 1920 (24.5)$$

$$7840 SF = .00028 SQMI$$

SOUTH LOUISIANA

$$PAV (37)(364) = 13468 (75.5)$$

$$TR.B (12)(364) = 4368 (24.5)$$

$$17,836 SF = .00064 SQMI$$

OAKLAND + LOT 13-PI OLIVINE

$$PAV (29)(620) + 3993 + 36(85) = 25033 = .782$$

$$TR.B (6)(620) + 2400 + 10(85) = 6970 = .218$$

$$21,700 SF + \underbrace{6393 SF + 3,910 SF}_{10,303 SF = .2365} = 32,003 SF = .001148$$

(.73469 AC.)

EAGLE ROCK

$$PAV (24)(620) = 14880 (80\%)$$

$$TR.B (6)(620) = 3720 (20\%)$$

$$18,600 SF = .00067$$

$$ONSITE \quad 6.55 \text{ ac.} - .2365 \text{ ac.} = 6.3135 \text{ ac.} = .009865 \text{ SQMI}$$

$$SINGLE FAMILY \quad N = 36 \text{ UNITS} / 6.55 \text{ AC} = 5.5$$

$$Tr. D = 7 \left[5.5^2 + 5(5.5) \right]^{.5} = 53.2\%$$

$$Tr. B = 23.4\%$$

$$Tr. C = 23.4\%$$

AHYMO SUMMARY TABLE (AHYMO194) - AMAFCA Hydrologic Model - January, 1994

RUN DATE

(MON/DAY/YR) =10/28/1997

INPUT FILE = eagle3.dat

USER NO.= M_GOODWN.I01

COMMAND	HYDROGRAPH ID IDENTIFICATION	AREA (SQ MI)	PEAK DISCH (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE
START					TIME=	.00	
RAINFALL TYPE= 1					RAIN6=	2.450	
COMPUTE NM HYD	100.00 - 1	.00987	24.64	.871	1.65541	1.500	3.903 PER IMP= 53.20
COMPUTE NM HYD	101.00 - 1	.00028	.80	.029	1.92318	1.500	4.447 PER IMP= 75.50
COMPUTE NM HYD	102.00 - 1	.00064	1.80	.066	1.92318	1.500	4.386 PER IMP= 75.50
COMPUTE NM HYD	103.00 - 1	.00115	3.25	.120	1.95560	1.500	4.422 PER IMP= 78.20
COMPUTE NM HYD	104.00 - 1	.00067	1.92	.071	1.97721	1.500	4.475 PER IMP= 80.00
FINISH							

CITY OF ALBUQUERQUE



April 3, 2007

Mr. Larry Read, P.E.
Larry Read & Associates, Inc
2430 Midtown Pl. NE Ste. C
Albuquerque, NM 87107

**Re: Pond Recovery, 6800 Limestone Ave. NE,
Approval of Permanent Certificate of Occupancy (C.O.)
Engineer's Stamp dated 2/13/2006 (C-18/D039B)
Certification dated 03/30/2007**

Based upon the information provided in your submittal received 3/30/2007, the above referenced certification is approved for release of Permanent Certificate of Occupancy by Hydrology.

P.O. Box 1293

If you have any questions, you can contact me at 924-3982.

Albuquerque

Sincerely,

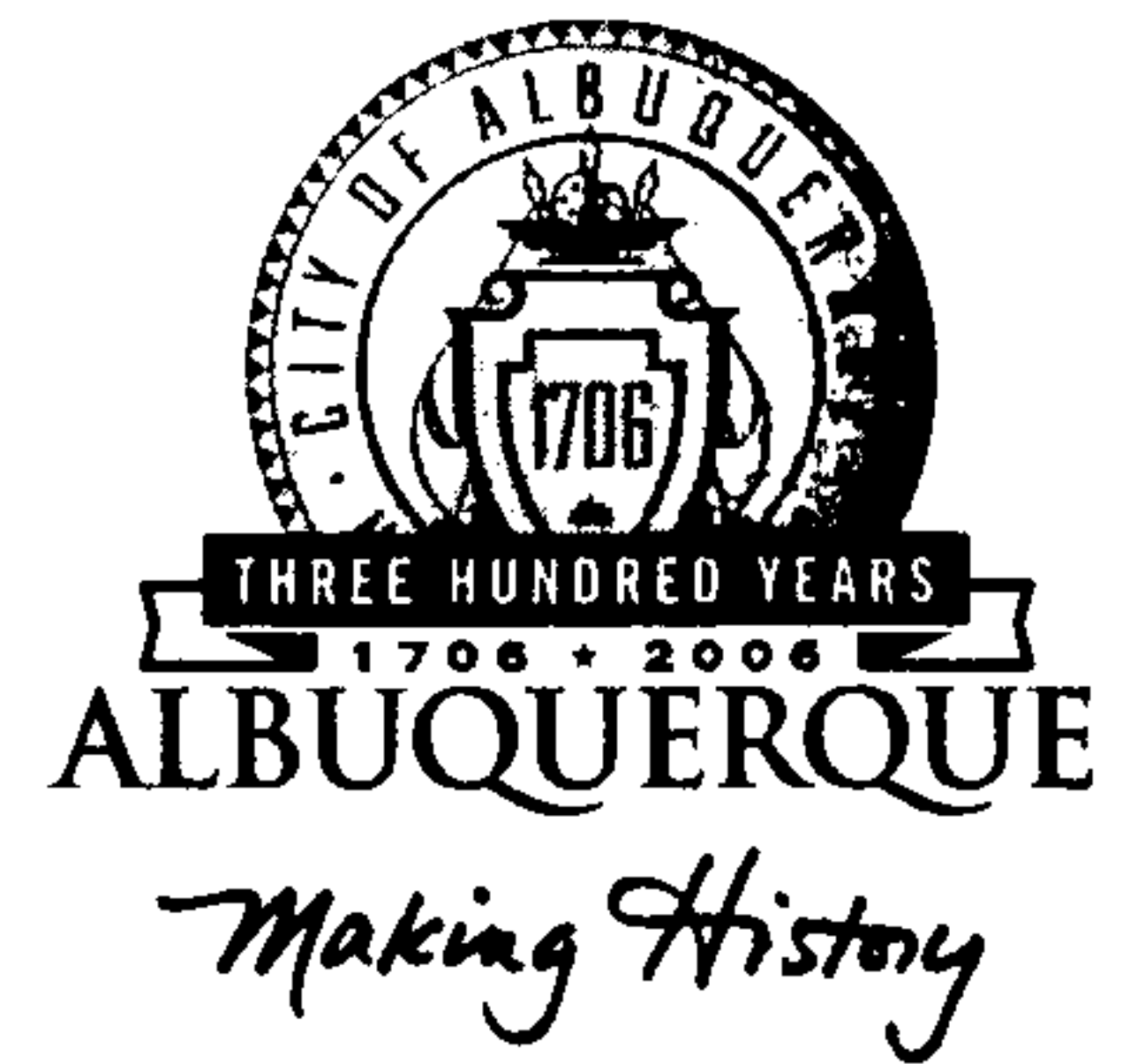
Timothy Sims
Plan Checker-Hydrology, Planning Dept.
Development and Building Services

New Mexico 87103

www.cabq.gov

C: CO Clerk-Katrina Sigala
File

CITY OF ALBUQUERQUE



March 14, 2006

Larry D. Read, PE
Larry Read & Associates
2430 Midtown Place NE, Ste C
Albuquerque, NM 87107

**Re: Eagle Rock Estates Unit 3 Pond Recovery (Limestone) Grading Plan
Engineer's Stamp dated 2-13-06, (C18/D39B)**

Dear Mr. Read,

Based upon the information provided in your submittal dated 2-13-06, the above referenced plan is approved for Grading Permit, SO#19 Permit and Preliminary Plat. Prior to Final Plat signoff by City Engineer, please submit an Engineer's certification of this grading plan and provide acceptance of the modification to the sidewalk culvert in Oakland.

If you have any questions, please contact me at 924-3986.

Sincerely,

Bradley L. Bingham
Bradley L. Bingham, PE

Principal Engineer, Planning Dept.
Development and Building Services

C: Ed Elwell, DMD
file

P.O. Box 1293

Albuquerque

New Mexico 87103

www.cabq.gov