

City of Albuquerque

Planning Department Development & Building Services Division DRAINAGE AND TRANSPORTATION INFORMATION SHEET (REV 09/2015)

| Project Title: | Building Permit #: | City Drainage #: | | |
|---|---|----------------------------------|--|--|
| DRB#: EPC#: | | Work Order#: | | |
| Legal Description: | | | | |
| City Address: | | | | |
| Engineering Firm: | | Contact: | | |
| Address: | | | | |
| Phone#: Fax#: | | E-mail: | | |
| Owner: | | Contact: | | |
| Address: | | | | |
| Phone#: Fax#: | | _ E-mail: | | |
| Architect: | | Contact: | | |
| Address: | | | | |
| Phone#: Fax#: | | E-mail: | | |
| Other Contact: | | Contact: | | |
| Address: | | | | |
| Phone#: Fax#: | | E-mail: | | |
| TRAFFIC/ TRANSPORTATION MS4/ EROSION & SEDIMENT CONTROL | | ERMIT APPROVAL E OF OCCUPANCY | | |
| TYPE OF SUBMITTAL: | | | | |
| ENGINEER/ ARCHITECT CERTIFICATION | PRELIMINARY PLAT APPROVAL SITE PLAN FOR SUB'D APPROVAL | | | |
| | SITE PLAN FOR BLDG. PERMIT APPROVAL | | | |
| CONCEPTUAL G & D PLAN | FINAL PLAT | FINAL PLAT APPROVAL | | |
| GRADING PLAN | SIA/ RELEA | | | |
| DRAINAGE MASTER PLAN | FOUNDATIC | FOUNDATION PERMIT APPROVAL | | |
| DRAINAGE REPORT | GRADING P | GRADING PERMIT APPROVAL | | |
| CLOMR/LOMR | | SO-19 APPROVAL | | |
| TRAFFIC CIRCULATION LAYOUT (TCL) | | PAVING PERMIT APPROVAL | | |
| TRAFFIC IMPACT STUDY (TIS) | GRADING/ P | PAD CERTIFICATION | | |
| EROSION & SEDIMENT CONTROL PLAN (ESC) | WORK ORDE | | | |
| OTHER (SPECIFY) | | | | |
| | PRE-DESIGN | | | |
| IS THIS A RESUBMITTAL?: Yes No | OTHER (SPE | ECIFY) | | |
| | | | | |
| DATE SUBMITTED:By: | | | | |
| | | | | |

COA STAFF: ELECTRONIC SUBMITTAL RECEIVED: ____



March 17, 2016

Mr. Abiel Carrillo, P.E. Principal Engineer-Hydrology City of Albuquerque 600 2nd Street NW Albuquerque, NM 87102

RE: LOT 2A & 3A DEL NORTE PLAZA GRADING AND DRAINAGE PLAN ENGINEER'S STAMP DATE 2-17-2016 (C18-D042D)

Dear Mr. Carrillo:

Per your correspondence dated March 9, 2016, please find the following responses addressing the comments listed below:

1. An Erosion and Sediment Control Plan needs to be approved by the Storm Water Quality Engineer.

Response: An erosion and sediment control plan is being submitted along with this drainage resubmittal.

- 2. If Transportation Development will require a sidewalk along the south frontage, the openchannel curb cuts proposed will need to be sidewalk culverts that comply with City Standard Details. The metal plate will need to extend 1-foot behind the sidewalk. Response: Transportation Development is not requiring any further sidewalk along the frontage, therefore there will only be one sidewalk culvert for the Pond 2 outfall where there is existing sidewalk.
- 3. The existing inlet in the proposed "Pond 1" is called to be modified. Clarify what the modification involves.
 - a. The grate is shown on the slope of the pond; it is assumed that the box will be perched to ensure that the inlet elevation is level and set at EL 45.60 as called out.

Response: The modification to the existing inlet is to raise the grate elevation from 42.45 to 45.60. This modification has been called out on the plan to be raised and perched.

- Provide benchmark information, unless it is called out on a different sheet of the construction set.
 Response: Benchmark Information has been added and can be seen on the SE corner of the site.
- 5. Detail B describes the use of a trench drain to provide an outfall for the roof drains of the larger building. A type of grate should be called out that complies with ADA standards and is "heel safe", since it is proposed along the center of the sidewalk. If the sidewalk is intended to drain to the west (without needing to enter the trench drain) we recommend calling out a solid metal top.

Response: As discussed with you on the phone, Section A-A has been updated to call out the metal grate to be ADA compliant and "Heel Safe". Because the sidewalk would be draining into the trench drain, the metal grate has also been called out to be an open grate.

SON

- Label the intended cross slope of the sidewalk on Section A-A.
 Response: The cross slope of the sidewalk has been called out on Section A-A as 1.0% sloped towards the trench drain.
- 7. We recommend shifting the 12:1 wheelchair ramp that is in front of the easternmost retail space of the larger building slightly to the west so that the top of the ramp lines up with the parking curb; this would remove a potential tripping hazard and provide a better turning space for wheelchairs at the top of the ramp.
 Response: The wheelchair ramp in the montioned location has been abifted elightly.

Response: The wheelchair ramp in the mentioned location has been shifted slightly towards the west to line up with the parking curb.

If you have any questions or need additional information regarding this matter, please do not hesitate to contact me.

Sincerely,

Ronald R. Bohannan, PE

JN: 2015064 RRB/vp DRAINAGE REPORT

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For

Lot 2-A and 3-A Del Norte Plaza

Prepared by:

Tierra West, LLC 5571 Midway Park Place NE Albuquerque, New Mexico 87109

March 17, 2016

I certify that this report was prepared under my supervision, and I am a registered professional engineer in the State of New Mexico in good standing.



Job No. 2015064

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| Hydrology Calculations | APPENDIX B |
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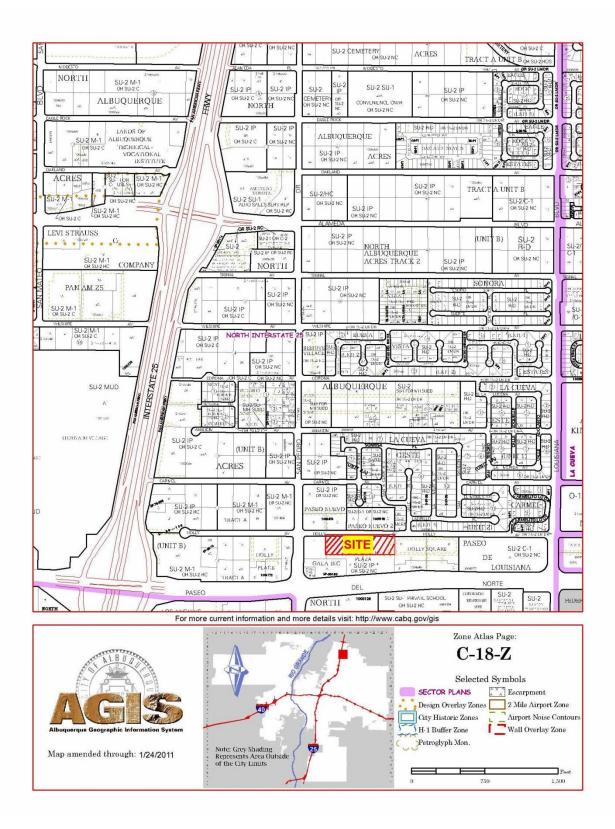
Purpose

The purpose of this report is to develop a Drainage Management Plan for a 2.99 acre parcel of land, which is a all of Lot 2A & a portion of Lot 3A Del Norte Plaza. The 2.99 acres of development will be for retail/restaurant and parking use within the Del Norte Plaza Shopping Center.

Location

The site is located near the northeast corner of the Paseo Del Norte Blvd/San Pedro Dr. Intersection. The site is bounded by a paved private road for the Del Norte Plaza Shopping Center along the east, west and south property lines and is bounded by Holly Ave. to the north. The site location is shown on the Zone Atlas Page, C-18-Z found in Exhibit A.

Exhibit A – Vicinity Map



Existing Conditions

The site is undeveloped and rough graded with the overall surface drainage flowing from northeast to southeast. There are two existing drainage basins for the lot which essentially splits the lot in half.

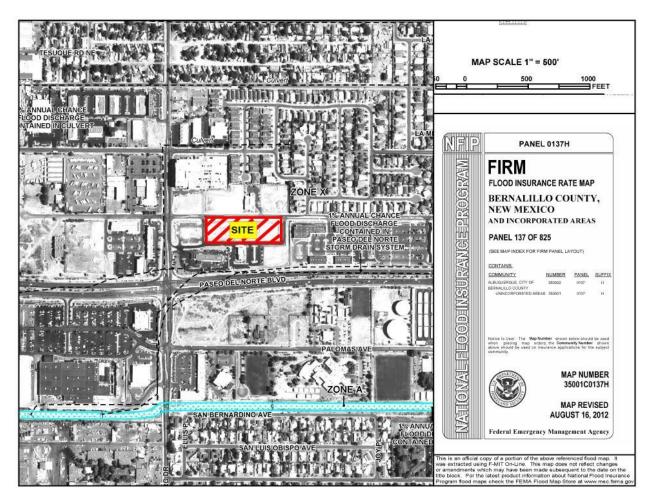
The western drainage basin (Existing Basin 1) surface flows to the southwest corner of the site towards an existing Single D drop inlet. The inlet conveys the collected flows via storm drain towards the public storm drain underneath Holly Avenue. The eastern drainage basin (Existing Basin 2) surface flows south towards the private roadway directly south of the site and flows west, making its way towards Holly Ave. and entering the public storm drain system.

There is an approved drainage report for the site titled "Lot 1A Block 35 Tract A Unit B of North Albuquerque Acres" stamped 11-1-06 (C18/D42). The drainage report calls for allowing a discharge of 11.15 cfs from lots 2A & 3A to the Holly Avenue storm drain system. This report was approved prior to the change in the drainage ordinance.

Flood Plain

The site is located on FIRM Map 35001C0137H. The map indicates that the site does not lie within a 100-year flood plain. This FIRM Map can be found in Exhibit B.

Exhibit B – FIRM Map



Proposed Conditions

The site will be built in its entirety for all paving and landscape improvements. There are three developed basins to the site which all send the drainage flow from northeast to southwest. The furthest western basin (Basin 1) includes all three buildings and the parking area south and east of the furthest west building. All flows within this basin will be conveyed towards a landscaped pond in the southwest corner of the property with a raised single D inlet for discharge and first flush retention with a total discharge of 6.96 cfs. The landscaped pond will be depressed enough to retain the first flush volume of this basin (1666 cubic feet) before all remaining runoff will discharge into the raised single D inlet.

The two easternmost buildings and patios will drain on the north side of the respective buildings via roof drain and area drain connections to a grated trench drain that will run along the span of the proposed sidewalk behind these buildings. The trench drain will daylight along the parking area west of these buildings and flow towards the landscaped pond with raised inlet. Capacity calculations for the trench drain can be found in Appendix C.

Basin 2 consists of the paved and parking area directly south of the middle building. All flows in this basin flow towards a depressed landscaped area for first flush retention before being conveyed through a 2 foot wide sidewalk culvert to the existing private road with a discharge of 1.06 cfs. The landscaped pond in this basin will be depressed enough to retain the first flush volume of this basin (283 cubic feet) before the remaining runoff discharges through the sidewalk culvert towards the private road.

Basin 3 consists of the paved parking area directly south and east of the easternmost building. All flows will flow similar to Basin 2 towards a landscaped depressed area onsite for first flush retention before being conveyed through a 2 foot sidewalk culvert towards the existing private road with a discharge of 3.13 cfs. The landscaped pond will be depressed enough to retain the first flush volume of this basin (735 cubic feet) before the remaining runoff discharges through the sidewalk culvert towards the private road.

All runoff from this developed site will ultimately be directed towards the storm drain system in Holly Avenue with a total discharge of 11.15 cfs, which is the allowable developed discharge rate for this site per the approved drainage report (C18/D42) for this subdivision.

Calculations and Water Quality

The Weighted E Method from the "City of Albuquerque Development Process Manual Volume I – Design Criteria, 2006 Revision" was used to calculate the runoff and volume for the site, the hydrology table can be found in Appendix B. Drainage capacities for the sidewalk culverts, single D inlet, and trench drain can be found in Appendix C. Also included on the weighted E table is the first flush retention volume calculations for the fully developed site calculated per the City of Albuquerque drainage ordinance as 0.44" of the impervious area.

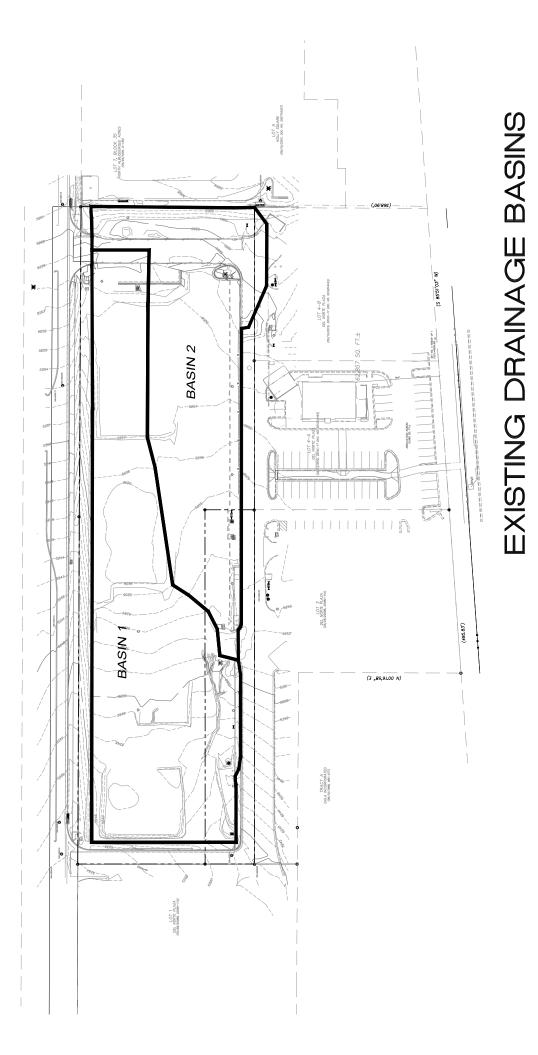
Summary

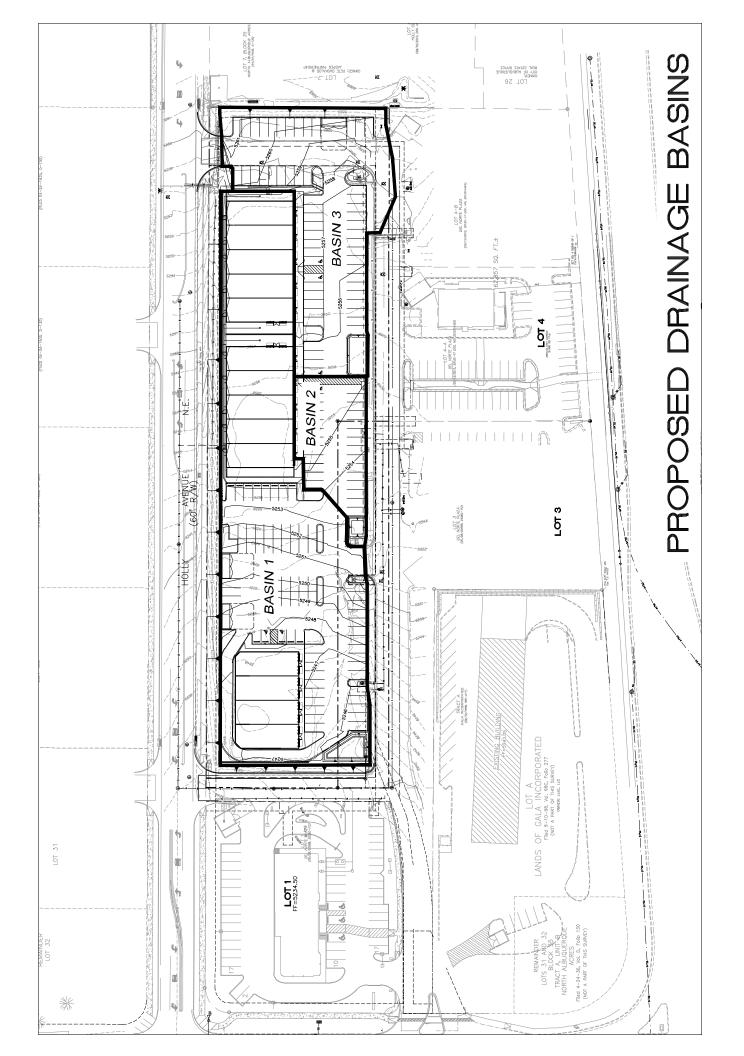
The entire site will be graded and all of the surface improvements will be built out in their entirety. The enclosed grading plan shows the grades for the entire project.

The proposed development consists of three drainage basins that all flow from northeast to southwest towards a landscaped ponding area within each respective basin. Basin 1 will convey the non-retained flows through an existing single D inlet towards the storm drain in Holly Avenue. Basins 2 and 3 will convey the non-retained flows through a proposed sidewalk culvert for each basin and free discharge to the private roadway that bounds the property. The total discharge of the proposed development will be 11.15 cfs which is the allowable discharge per the approved drainage report entitled "Lot 1A Block 35 Tract A Unit B of North Albuquerque Acres" stamped 11-1-06 (C18/D42).

APPENDIX A:

Drainage Basin Maps





APPENDIX B: Hydrology Calculations

DPM Weighted E Method

2/15/2016 Precipitation Zone 3 SE Corner of San Pedro Dr. and Holly Ave. Lot 2 Del Norte Plaza TWLLC Date 2/15/2016

Existing Conditions

| | | | | 3 | Basin Descr | criptions | | | | | | 100 | 100-Year, 6-Hr | | 10- | 10-Year, 6-Hr | |
|-------|------------|---------|-------------------|-------------|--------------------|-----------|-------------|-------------|---------|--------|-------------|------------|----------------|------|------------|---------------|------|
| Basin | Area | Area | Area | Treatment A | nent A | Treatn | Freatment B | Treatment (| tent C | Treatn | Treatment D | Weighted E | Volume | Flow | Weighted E | Volume | Flow |
| ٩ | (sf) | (acres) | acres) (sq miles) | % | (acres) | % | (acres) | % | (acres) | % | (acres) | (ac-ft) | (ac-ft) | cfs | (ac-ft) | (ac-ft) | cfs |
| 1 | 63,540.68 | 1.459 | 0.00228 | %0 | 0.000 | 97% | 1.415 | %0 | 0.000 | 3% | 0.044 | 0.963 | 0.117 | 3.90 | 0.394 | 0.048 | 1.83 |
| 2 | 46,090.02 | 1.058 | 0.00165 | %0 | 0.000 | 88% | 0.931 | %0 | 0.000 | 12% | 0.127 | 1.093 | 0.096 | 3.06 | 0.497 | 0.044 | 1.54 |
| Total | 109,630.70 | 2.517 | 0.00393 | | | | | | | | | | 0.213 | 6.96 | | 0.092 | 3.37 |

Proposed Conditions

| | | | | Ш | Basin Descr | escriptions | | | | | | 100 | 100-Year, 6-Hr | | 10- | 10-Year, 6-Hr | |
|-------|------------------------|---------|------------|-------------|--------------------|-------------|-------------|-------------|---------|-------------|---------|------------|----------------|-------|------------|---------------|------|
| Basin | Area | Area | Area | Treatment A | nent A | Treatn | Treatment B | Treatment C | nent C | Treatment D | ient D | Weighted E | Volume | Flow | Weighted E | Volume | Flow |
| ٩ | (sf) | (acres) | (sq miles) | % | (acres) | % | (acres) | % | (acres) | % | (acres) | (ac-ft) | (ac-ft) | cfs | (ac-ft) | (ac-ft) | cfs |
| 1 | 64,767.60 | 1.487 | 0.00232 | %0 | 0.000 | 14% | 0.208 | %0 | 0.000 | 86% | 1.279 | 2.158 | 0.267 | 6.96 | 1.340 | 0.166 | 4.58 |
| 2 | 9,800.50 | 0.225 | 0.00035 | %0 | 0.000 | 13% | 0.029 | %0 | 0.000 | 87% | 0.196 | 2.173 | 0.041 | 1.06 | 1.352 | 0.025 | 0.70 |
| ŝ | 29,877.37 | 0.686 | 0.00107 | %0 | 0.000 | 19% | 0.130 | %0 | 0.000 | 81% | 0.556 | 2.086 | 0.119 | 3.13 | 1.283 | 0.073 | 2.04 |
| Total | Total 104,445.47 2.398 | 2.398 | 0.00375 | | | | | | | | | | 0.427 | 11.15 | | 0.265 | 7.32 |

Equations:

Weighted E = Ea*Aa + Eb*Ab + Ec*Ac + Ed*Ad / (Total Area)

Volume = Weighted D * Total Area

 $Flow = Qa^*Aa + Qb^*Ab + Qc^*Ac + Qd^*Ad$

| Excess P | Excess Precipitation, E (in.) | n, E (in.) |
|----------|-------------------------------|------------|
| Zone 1 | 100-Year | 10-Year |
| Ea | 0.44 | 0.08 |
| Eb | 0.67 | 0.22 |
| Ec | 0.99 | 0.44 |
| Ed | 1.97 | 1.24 |

| (cfs/acre) | 10-Year | 0.24 | 0.76 | 1.49 | 2.89 |
|------------------|----------|------|------|------|------|
| Peak Discharge (| 100-Year | 1.29 | 2.03 | 2.87 | 4.37 |
| Peak | Zone 1 | Qa | qD | Qc | Qd |

First Flush

Total Impervious Area = 2.03 acres = 88,426.8 SF

Retainage depth = 0.44" -IA = 0.44" - 0.1" = 0.34"=0.028'

Retention Volume = 0.028 * 88426.8 = 2502.5 CF = 0.058 ac-ft

APPENDIX C:

Trench Drain, Inlet, and Sidewalk Culvert Capacities

| Worksheet | for | Trench | Drain | Capacity |
|-----------|-----|--------|-------|----------|
| | | | | |

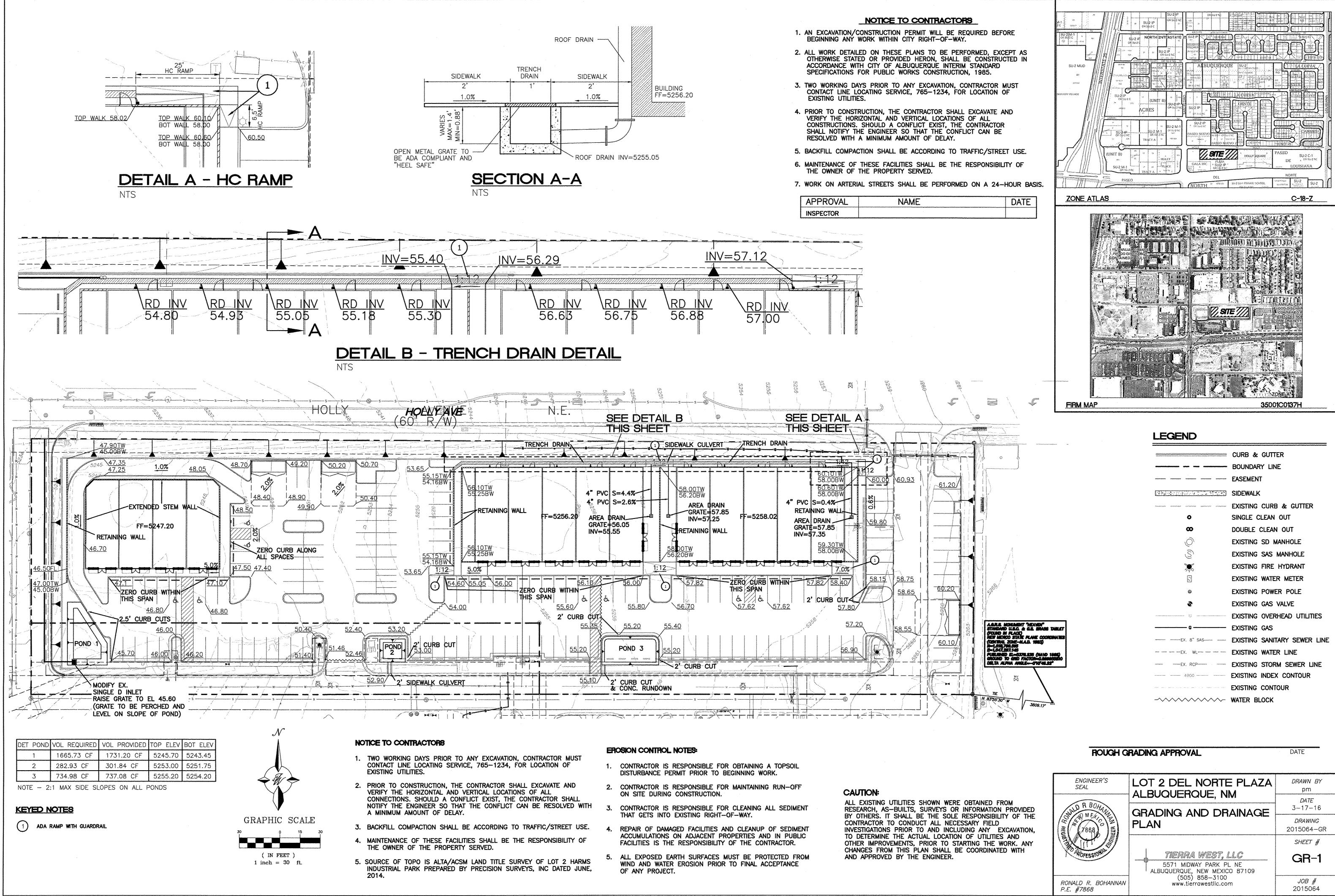
| Project Description | | | | |
|-----------------------|-----------------|----------|--------------------|--|
| Friction Method | Manning Formula | | | |
| Solve For | Discharge | | | |
| Input Data | | | | |
| Roughness Coefficient | | 0.013 | | |
| Channel Slope | | 0.00500 | ft/ft | |
| Normal Depth | | 0.88 | ft | |
| Bottom Width | | 1.00 | ft | |
| Results | | | | |
| Discharge | | 3.32 | ft ³ /s | Although the trench drain varies in height, |
| Flow Area | | 0.88 | ft² | the capacity was calculated using the |
| Wetted Perimeter | | 2.76 | ft | minimum height (0.88 ft) as a conservative |
| Hydraulic Radius | | 0.32 | ft | approach. The maximum discharge capaci |
| Top Width | | 1.00 | ft | with these parameter (3.32 cfs) is greater than the maximum discharge of the easter |
| Critical Depth | | 0.70 | ft | buildings' roofs, patios, and rear sidewalk |
| Critical Slope | | 0.00891 | ft/ft | (2.24 cfs), therefore the trench drain |
| Velocity | | 3.77 | ft/s | capacity is OK. |
| Velocity Head | | 0.22 | ft | |
| Specific Energy | | 1.10 | ft | |
| Froude Number | | 0.71 | | |
| Flow Type | Subcritical | | | |
| GVF Input Data | | | | |
| Downstream Depth | | 0.00 | ft | |
| Length | | 0.00 | ft | |
| Number Of Steps | | 0 | | |
| GVF Output Data | | | | |
| Upstream Depth | | 0.00 | ft | |
| Profile Description | | | | |
| Profile Headloss | | 0.00 | ft | |
| Downstream Velocity | | Infinity | ft/s | |
| Upstream Velocity | | Infinity | ft/s | |
| Normal Depth | | 0.88 | ft | |
| Critical Depth | | 0.70 | ft | |
| Channel Slope | | 0.00500 | ft/ft | |
| Critical Slope | | 0.00891 | ft/ft | |
| | | | | |

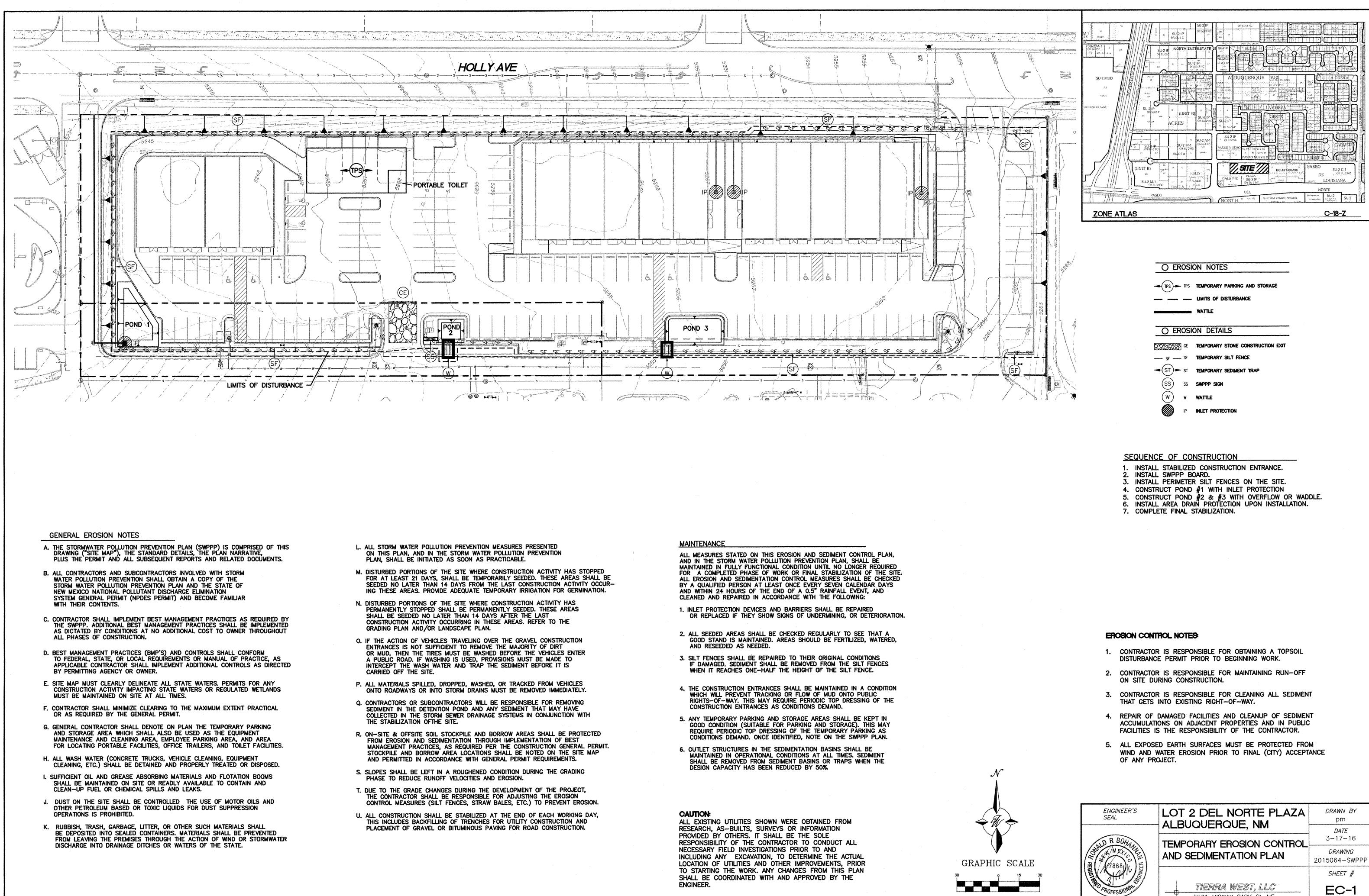
Worksheet for Basin 1 Curb Cut Capacities

| Project Description | | | | |
|-----------------------|-----------------|-----------------|--------------------|--|
| Friction Method | Manning Formula | | | |
| Solve For | Discharge | | | |
| Input Data | | | | |
| Roughness Coefficient | | 0.013 | | |
| Channel Slope | | 0.01000 | ft/ft | |
| Normal Depth | | 0.50 | ft | |
| Bottom Width | | 2.50 | ft | |
| Results | | | | |
| Discharge | | 7.19 | ft ³ /s | Discharge capacity of a 2.5' curb cut (7.1 |
| Flow Area | | 1.25 | ft² | cfs) is greater than the discharge of Bas |
| Wetted Perimeter | | 3.50 | ft | 1 (6.96 cfs), therefore curb cuts within |
| Hydraulic Radius | | 0.36 | ft | these respective basins are OK. |
| Top Width | | 2.50 | ft | |
| Critical Depth | | 0.64 | ft | |
| Critical Slope | | 0.00496 | ft/ft | |
| Velocity | | 5.75 | ft/s | |
| Velocity Head | | 0.51 | ft | |
| Specific Energy | | 1.01 | ft | |
| Froude Number | | 1.43 | | |
| Flow Type | Supercritical | | | |
| GVF Input Data | | | | |
| Downstream Depth | | 0.00 | ft | |
| Length | | 0.00 | ft | |
| Number Of Steps | | 0 | | |
| GVF Output Data | | | | |
| Upstream Depth | | 0.00 | ft | |
| Profile Description | | | | |
| Profile Headloss | | 0.00 | ft | |
| Downstream Velocity | | Infinity | ft/s | |
| Upstream Velocity | | Infinity | ft/s | |
| Normal Depth | | 0.50 | ft | |
| | | | | |
| Critical Depth | | 0.64 | ft | |
| | | 0.64 0.01000 | ft ft/ft | |

Worksheet for Basin 2 and 3 Curb Cut Capacities

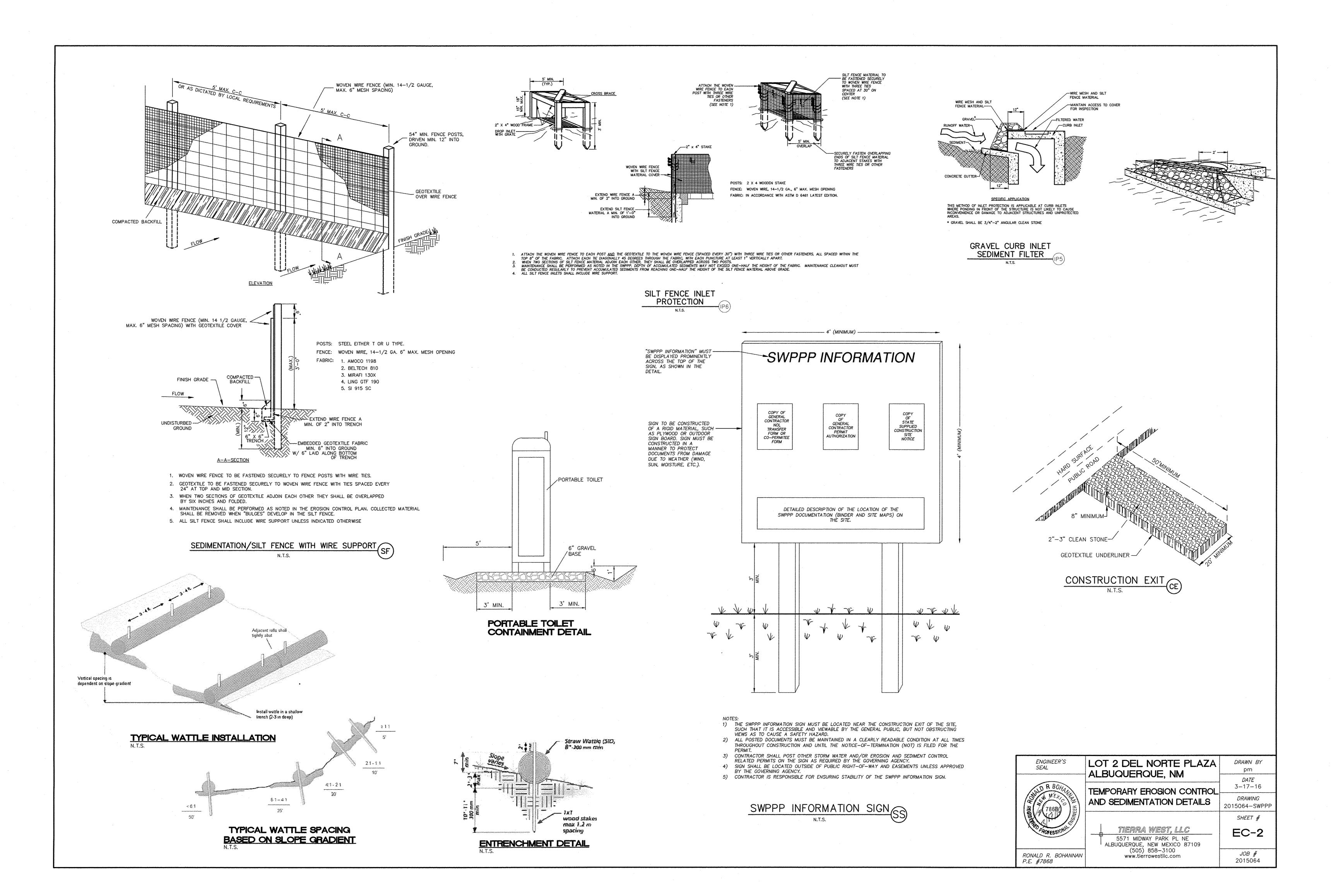
| Project Description | | | | |
|-----------------------|-----------------|----------|--------------------|---|
| Friction Method | Manning Formula | | | |
| Solve For | Discharge | | | |
| Input Data | | | | |
| Roughness Coefficient | | 0.013 | | |
| Channel Slope | | 0.01000 | ft/ft | |
| Normal Depth | | 0.50 | ft | |
| Bottom Width | | 2.00 | ft | |
| Results | | | | |
| | | | | |
| Discharge | | 5.50 | ft ³ /s | Discharge capacity of a 2' curb cut (5.5 cfs) |
| Flow Area | | 1.00 | ft² | is greater than discharge of Basin 2 (1.06 |
| Wetted Perimeter | | 3.00 | ft | cfs) and Basin 3 (3.13 cfs), therefore curb cuts within these respective basins are OK. |
| Hydraulic Radius | | 0.33 | ft | cuts within these respective basins are OK. |
| Top Width | | 2.00 | ft | |
| Critical Depth | | 0.62 | ft | |
| Critical Slope | | 0.00549 | ft/ft | |
| Velocity | | 5.50 | ft/s | |
| Velocity Head | | 0.47 | ft | |
| Specific Energy | | 0.97 | ft | |
| Froude Number | | 1.37 | | |
| Flow Type | Supercritical | | | |
| GVF Input Data | | | | |
| Downstream Depth | | 0.00 | ft | |
| Length | | 0.00 | ft | |
| Number Of Steps | | 0 | | |
| GVF Output Data | | | | |
| Upstream Depth | | 0.00 | ft | |
| Profile Description | | | | |
| Profile Headloss | | 0.00 | ft | |
| Downstream Velocity | | Infinity | ft/s | |
| Upstream Velocity | | Infinity | ft/s | |
| Normal Depth | | 0.50 | ft | |
| Critical Depth | | 0.62 | ft | |
| Channel Slope | | 0.01000 | ft/ft | |
| Critical Slope | | 0.00549 | ft/ft | |
| | | | | |





| | ENGINEER'S SEAL | LOT 2 DEL NORTE PLAZA ALBUQUERQUE, NM | <i>DRAWN BY</i> pm |
|----|---|---|---------------------------------|
| | BROW | · · · · · · · · · · · · · · · · · · · | <i>DATE</i> 3-17-16 |
| LE | R BOHANNELCO R BOHANNELCO R H MORELO R H MORELO R H MORELO R H MORELO R H MORELO R H MORELO R BOHANNEL R H M MELCO R BOHANNE R | TEMPORARY EROSION CONTROL AND SEDIMENTATION PLAN | <i>DRAWING</i> 2015064-SWPPP |
| 30 | | | SHEET # |
| | PRO PROFESSIONAL | 5571 MIDWAY PARK PL NE ALBUQUERQUE, NEW MEXICO 87109 | EC-1 |
| | RONALD R. BOHANNAN P.E. #7868 | (505) 858—3100 www.tierrawestllc.com | <i>JOB #</i> 2015064 |

(IN FEET) 1 inch = 30 ft.



CITY OF ALBUQUERQUE



Richard J. Berry, Mayor

March 23, 2016

Ronald R. Bohannan Tierra West, LLC 5571 Midway Park Pl, NE Albuquerque, NM, 87109

RE: Lot 2-A & 3A Del Norte Plaza Drainage Report, Grading and Drainage Plan Engineer's Stamp Date 3-17-2016 (File: C18D042D)

Dear Mr. Bohannan:

Based upon the information provided in your submittal received 3-18-2016, the abovereferenced plan is approved for Grading Permit (ESC Permit) and Building Permit.

PO Box 1293 Please attach a copy of this approved plan in the construction sets when submitting for a building permit. Prior to Certificate of Occupancy release, Engineer Certification per the DPM checklist will be required.

Albuquerque

If you have any questions, you can contact me at 924-3986.

New Mexico 87103

Sincerely,

www.cabq.gov

Abiel Carrillo, P.E. Principal Engineer, Planning Department Development Review Services

Orig: Drainage file