

TIERRA WEST, LLC

File

8509 Jefferson NE
Albuquerque, NM 87113

Phone: 505-858-3100
800-245-3102
Fax: 505-858-1118

FACSIMILE TRANSMITTAL

TO: Mr. Brad Bingham
Mr. Joe Janusz

FAX NO: (505) 924-3864
FAX NO: (262) 703-7105

DATE: 12/8/2005

FROM: Jonathan D. Niski, EI

TOTAL OF PAGES: (3) INCLUDING COVER SHEET

SUBJECT: Kohl's - Louisiana & Paseo Del Norte

JN: 25017

Gentlemen:

Attached, please find the following documents regarding the above referenced project site.

If you have any questions regarding this fax, you may contact Jon Niski @ (505) 858-3100.

Thank you,

Debbie Gale for
Jonathan D. Niski, EI

IF YOU DO NOT RECEIVE ALL PAGES OF THIS TRANSMITTAL, OR IF MATERIAL IS NOT
LEGIBLE, PLEASE CONTACT OUR OFFICE FROM 7:00 AM TO 5:00 PM, MST, MONDAY
THROUGH THURSDAY OR 8:00 AM TO 12:00 PM, MST, FRIDAY. THANK YOU

November 28, 2005

Mr. Jason Daskalos
Pete Daskalos Properties
5321 Menaul Boulevard NE
Albuquerque, NM 87110

RE: Permission to Grade on Adjacent Property

Dear Mr. Daskalos:

Tierra West LLC, on behalf of Kohl's Departments Stores Inc., is requesting permission to grade within 30 feet along the east property line of Lots 10 and 23, Unit B, North Albuquerque Acres, which you currently own. This will allow Kohl's Department Stores Inc. to grade a 3:1 maximum slope from there development down to the existing grade on your property, filling in an existing arroyo. The extents of the fill are shown on the enclosed exhibit. The slope area would be grubbed of all organic material and filled in with compacted clean fill generated from the Kohl's site. The compaction would be completed using normal construction methods and compacted to 95% of maximum density as determined by ASTM D1557. Once the slope is graded it would be reseeded with a native seed mixture to prevent erosion. This work will also be shown on the Storm Water Pollution Prevention Plans for the Kohl's Development.

If you agree to grant permission to grade within your property please sign below.

If you have any questions or need additional information regarding this matter, please do not hesitate to call me.

Sincerely,

Jonathan D. Niski, EI

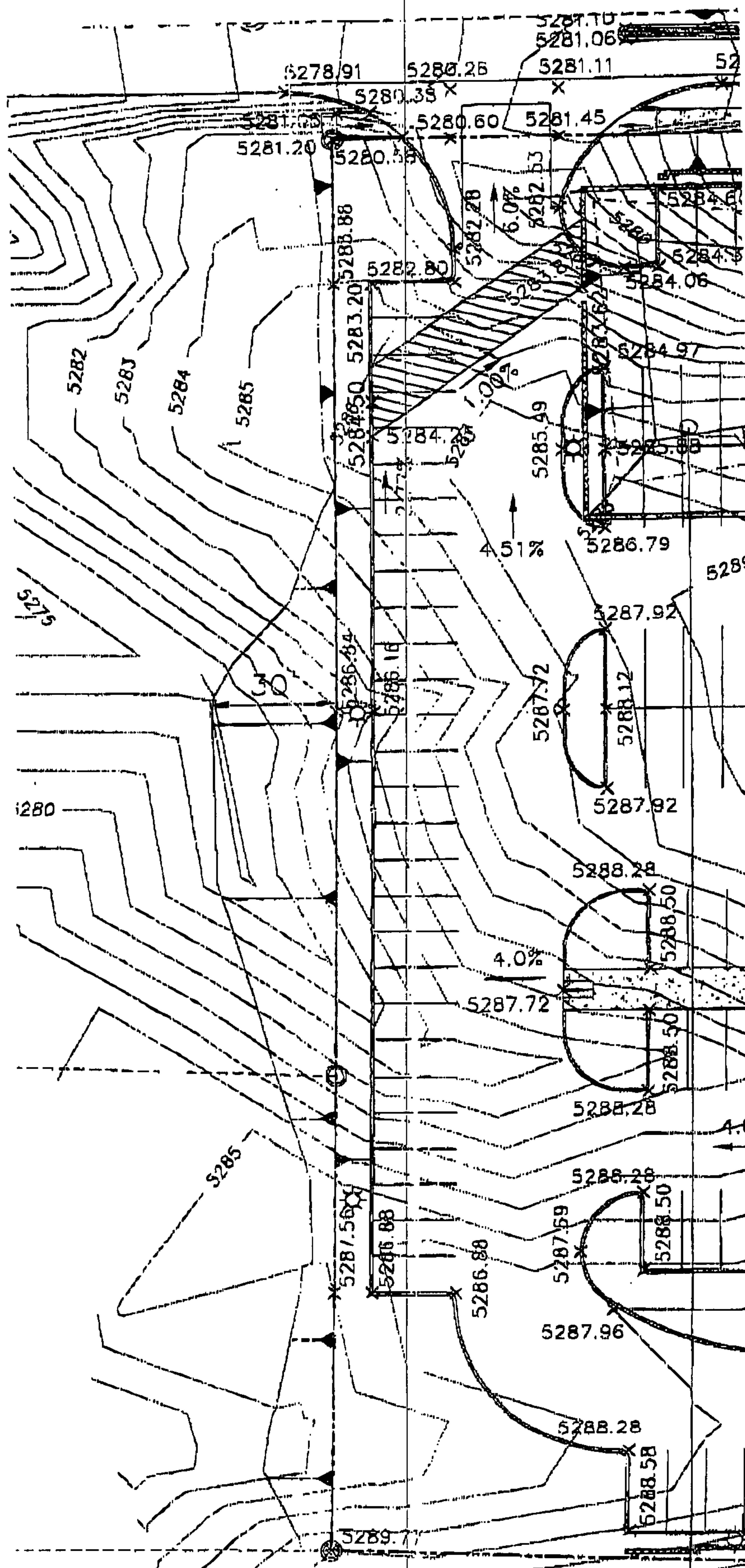

Jason Daskalos

12-7-05
Date

Enclosure/s

cc: Joe Janusz

JN: 25017
JDN/dg



CITY OF ALBUQUERQUE



**Planning Department
Transportation Development Services Section**

November 16, 2006

Ronald R. Bohannon, P.E.
5571 Midway Park Place NE
Albuquerque, NM 87109

Re: Certification Submittal for Final Building Certificate of Occupancy for
Kohl's, [C-18 / D57]
6800 Holly Ave. NE
Engineer's Stamp Dated 11/16/06

Dear Mr. Bohannon:

P.O. Box 1293

The TCL / Letter of Certification submitted on November 16, 2006 is sufficient for acceptance by this office for final Certificate of Occupancy (C.O.). Notification has been made to the Building and Safety Section.

Albuquerque

Sincerely,

New Mexico 87103


Nilo E. Salgado-Fernandez, P.E.
Senior Traffic Engineer
Development and Building Services
Planning Department

www.cabq.gov

c: Engineer
Hydrology file
CO Clerk

DRAINAGE AND TRANSPORTATION SHEET

(REV. 1/28/2003rd)

PROJECT TITLE: Kohl's @ Paseo Del Norte & Louisiana
DRB #: 1000579 EPC #: _____

ZONE MAP/DRG. FILE #: C18/D57
WORK ORDER #: _____

LEGAL DESCRIPTION: Tract A, Paseo De Louisiana
CITY ADDRESS: 6800 Holly Avenue NE

ENGINEERING FIRM: Tierra West, LLC
ADDRESS: 8509 Jefferson NE
CITY, STATE: Albuquerque, NM

CONTACT: Jonathan Niski
PHONE: (505) 858-3100
ZIP CODE: 87113

OWNER: Kohl's Department Stores
ADDRESS: N56 W17000 Ridgewood Drive
CITY, STATE: Menomonee Falls, WI

CONTACT: Joseph Janusz
PHONE: 262-703-1632
ZIP CODE: 53051

ARCHITECT: Perkowitz & Ruth Architects
ADDRESS: 111 W. Ocena Blvd, 21st Floor
CITY, STATE: Long Beach, CA

CONTACT: Zohair Vajihuddin
PHONE: 562-628-8000
ZIP CODE: 90802

SURVEYOR: Precision Surveys
ADDRESS: 8500 Jefferson St. NE
CITY, STATE: Albuquerque, NM

CONTACT: Larry Medrano
PHONE: 505-856-5700
ZIP CODE: 87113

CONTRACTOR: Whiting-Turner Construction
ADDRESS: 300 East Joppa Road
CITY, STATE: Baltimore, MD

CONTACT: Nick Cizauskas
PHONE: (410) 494-7427
ZIP CODE: 21286

CHECK TYPE OF SUBMITTAL:

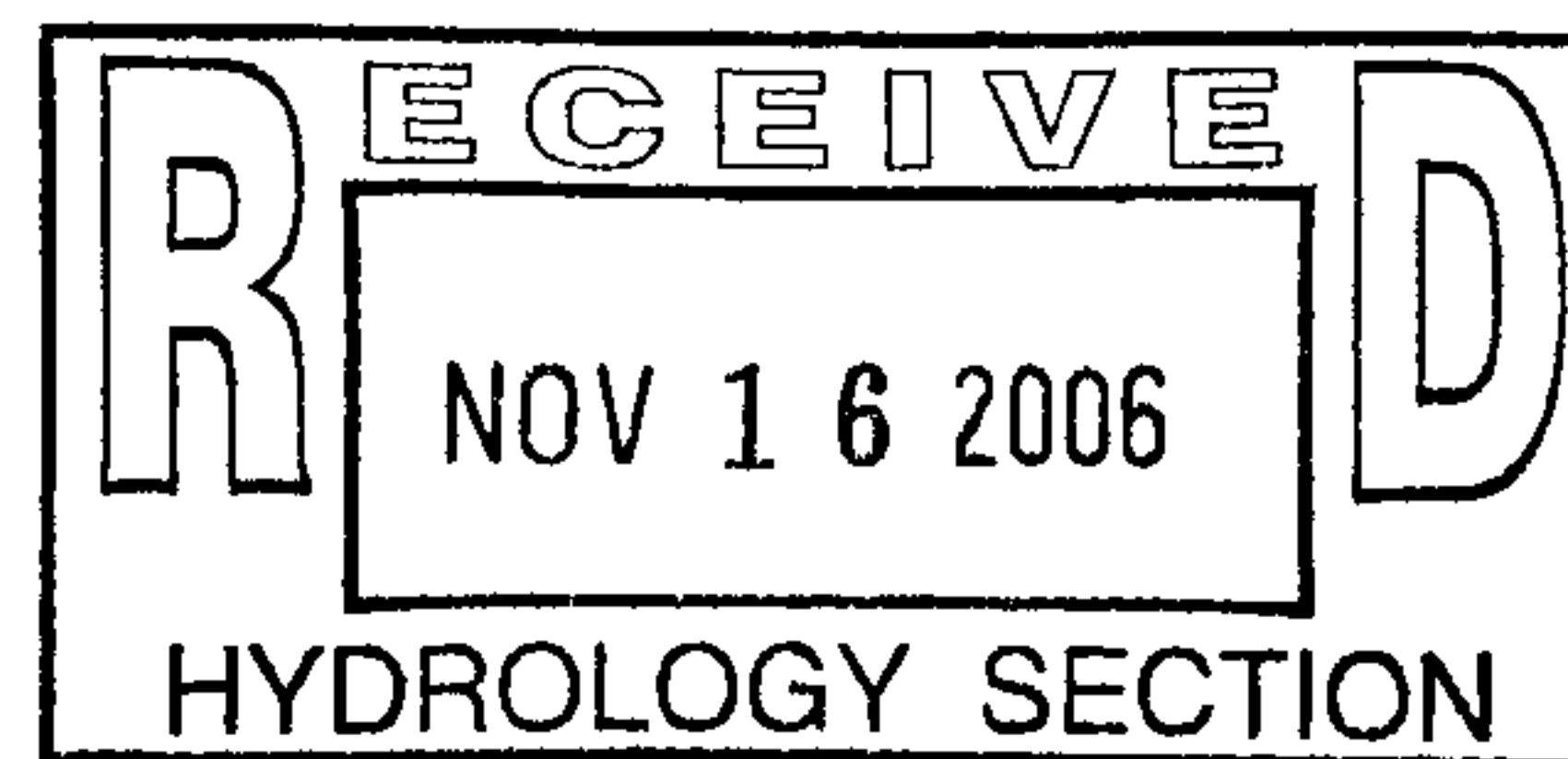
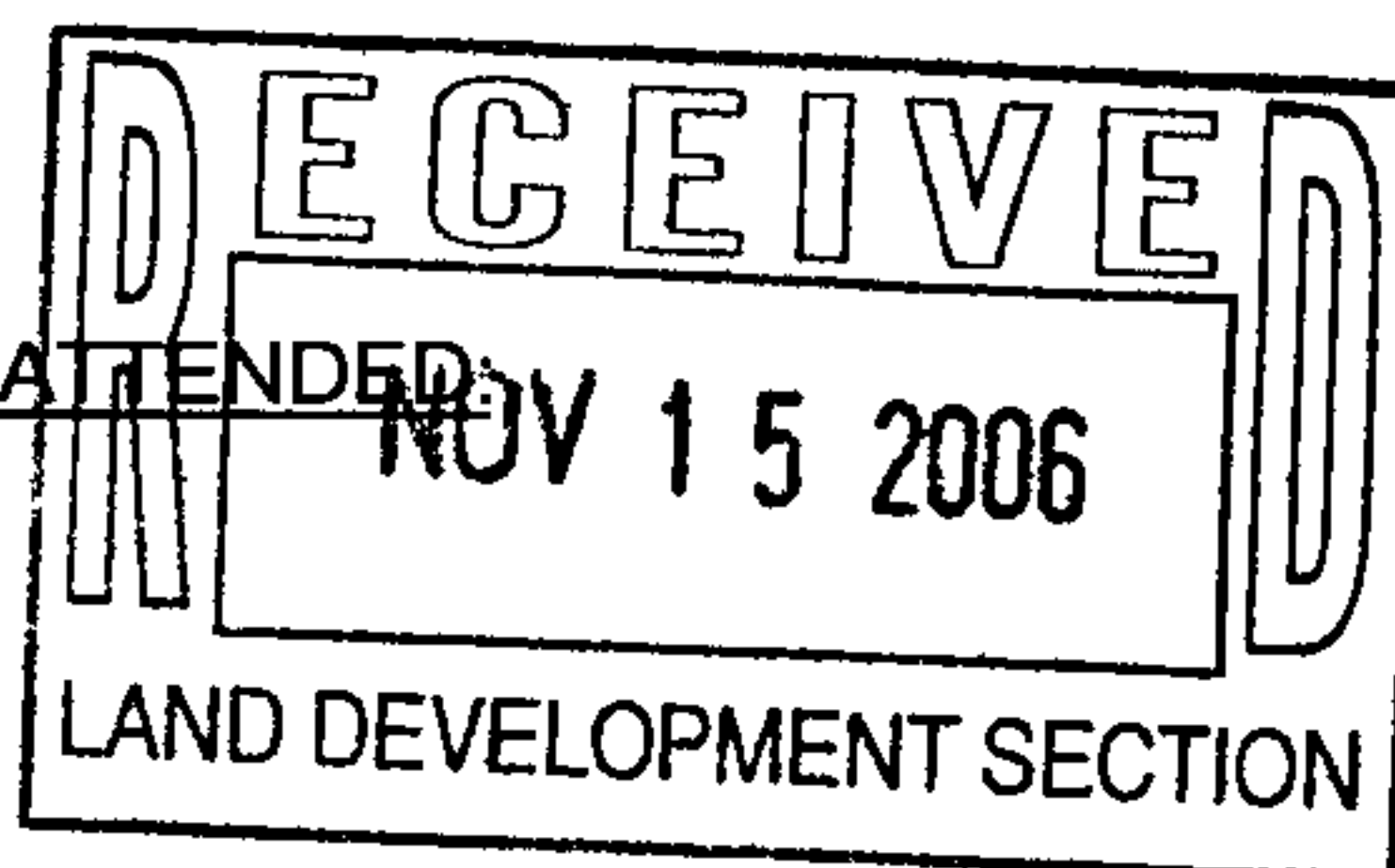
- ☐ DRAINAGE REPORT
- ☐ DRAINAGE PLAN 1st SUBMITTAL, **REQUIRES TCL or equal**
- ☐ DRAINAGE PLAN RESUBMITTAL
- ☐ CONCEPTUAL GRADING & DRAINAGE PLAN
- ☐ GRADING PLAN
- ☐ EROSION CONTROL PLAN
- ☐ ENGINEER'S CERTIFICATION (HYDROLOGY)
- ☐ CLOMR/LOMR
- ☐ TRAFFIC CIRCULATION LAYOUT (TCL)
- ☐ ENGINEERS CERTIFICATION (TCL)
- ☒ ENGINEERS CERTIFICATION (DRB APPR. SITE PLAN)
- ☐ OTHER

CHECK TYPE OF APPROVAL SOUGHT:

- ☐ SIA / FINANACIAL GUARANTEE RELEASE
- ☐ PRELIMINARY PLAT APPROVAL
- ☐ S. DEV. PLAN FOR SUB'D. APPROVAL
- ☐ S. DEV. PLAN FOR BLDG. PERMIT APPROVAL
- ☐ SECTOR PLAN APPROVAL
- ☐ FINAL PLAT APPROVAL
- ☐ FOUNDATION PERMIT APPROVAL
- ☐ BUILDING PERMIT APPROVAL
- ☒ CERTIFICATE OF OCCUPANCY (PERM.)
- ☐ CERTIFICATE OF OCCUPANCY (TEMP.)
- ☐ GRADING PERMIT APPROVAL
- ☐ PAVING PERMIT APPROVAL
- ☐ WORK ORDER APPROVAL
- ☐ SO-19

WAS A PRE-DESIGN CONFERENCE ATTENDED:

- ☐ YES
- ☐ NO
- ☐ COPY PROVIDED



DATE SUBMITTED: 11/16/2006 BY: Jon Niski

Requests for approvals of Site Development Plans and/or Subdivision Plats shall be accompanied by a drainage submittal. The particular nature, location and scope of the proposed development defines the degree of drainage detail. One or more of the following levels of submittal may be required based on the following:

1. **Conceptual Grading and Drainage Plans** Required for approval of Site Development Plans greater than five (5) acres and Sector Plans.
2. **Drainage Plans** Required for building permits, grading permits, paving permits and site plans less than five (5) acres.
3. **Drainage Report** Required for subdivisions containing more than ten (10) lots or constituting five (5) acres or more.

TIERRA WEST, LLC

5571 Midway Park Place NE
Albuquerque, NM 87109

(505) 858-3100
fax (505) 858-1118

twllc@tierrawestllc.com
1-800-245-3102

November 16, 2006

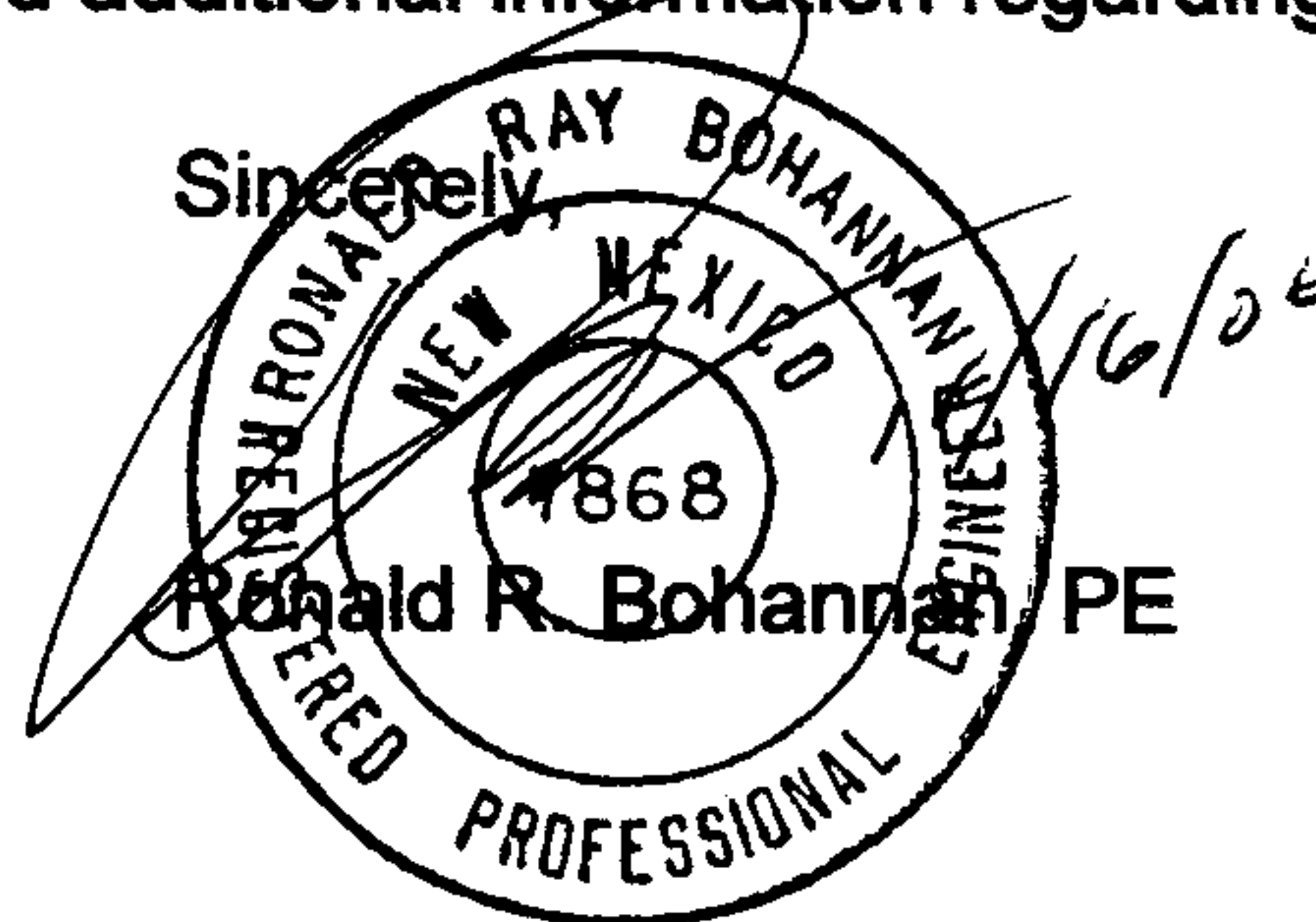
Mr. Nilo Salgado-Fernandez, PE
Development and Building Services
Public Works Department
PO Box 1293
Albuquerque, NM 87103

**RE: DRB Approved Site Plan Certification for Permanent Certificate of Occupancy
Kohl's Department Stores
6800 Holly Avenue NE**

Dear Mr. Salgado-Fernandez:

Tierra West, LLC requests a Permanent Certification of the DRB approved Site Plan for Building Permit for the Kohl's Department Store located at 6800 Holly Avenue NE. Enclosed please find the information sheet, the As-Built Administrative Amendment Site Plan for Building Permit and the Approved Site Plan for Building Permit (for reference only). All punchlist items have been completed and the project has been constructed in substantial compliance with the approved plan. Therefore, we request Certification of the As-Built Administrative Amendment Site Plan for Building Permit for a Permanent Certificate of Occupancy.

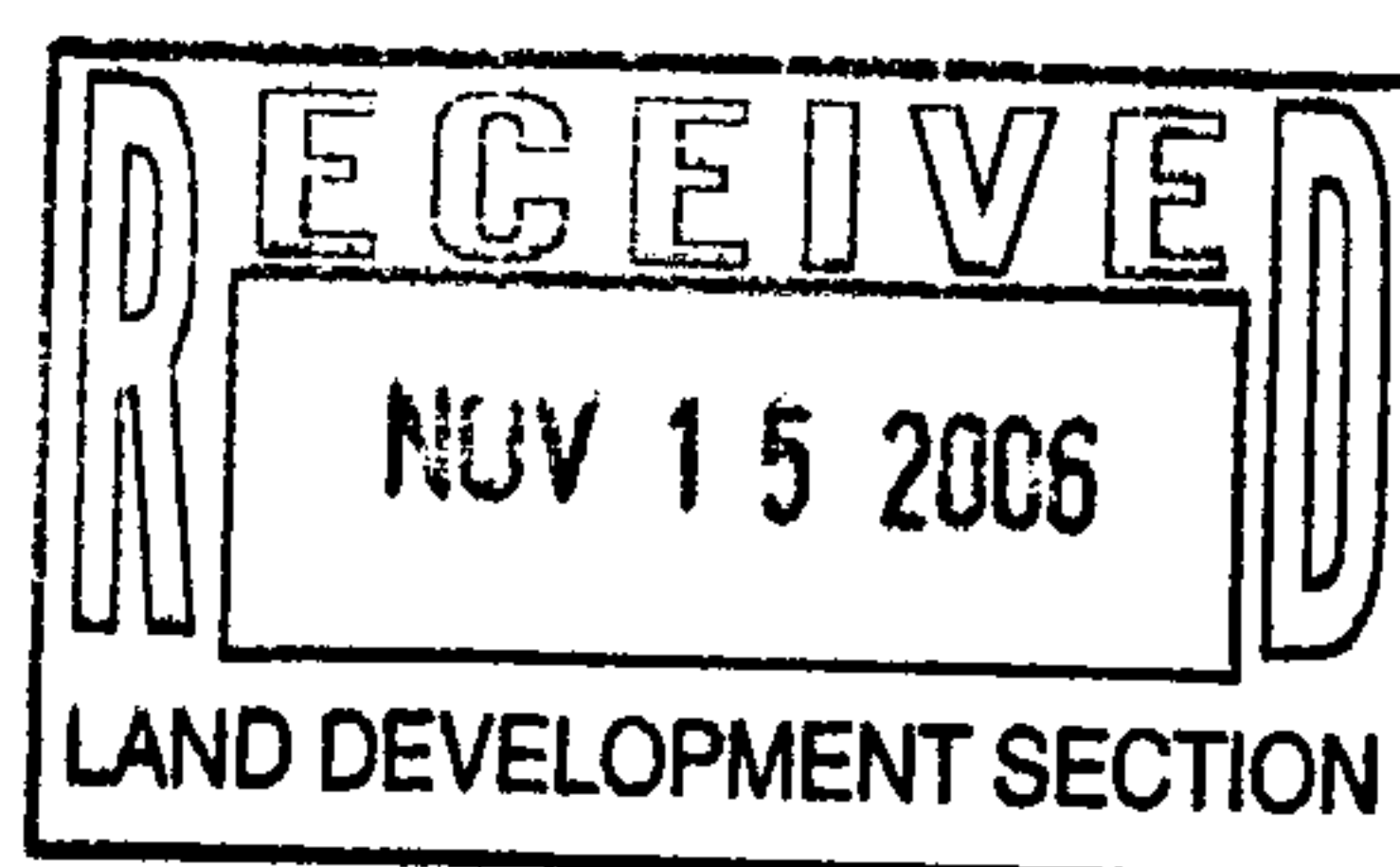
If you have any questions or need additional information regarding this matter, please do not hesitate to contact me.



Enclosure/s

cc: Josphe Janusz

JN: 25017
RRB/bf/kdk



2005: 25017 Nilo Temp CO 092706

CITY OF ALBUQUERQUE



November 17, 2006

Mr. Ron Bohannon, PE
TIERRA WEST, LLC
5571 Midway Park Place NE
Albuquerque, NM 87109

Re: KOHL'S @ LOUISIANA BLVD. NE
6800 Holly Ave. NE
Approval of ^{Permanent} Temporary Certificate of Occupancy (C.O.)
Engineer's Stamp dated 11/02/2005 (C-18/D57)
Certification dated 11/16/2006

Dear Ron,

P.O. Box 1293

Based upon the information provided in your submittal received 11/16/2006 the above referenced certification is approved for release of Permanent Certificate of Occupancy by Hydrology.

Albuquerque

If you have any questions, you can contact me at 924-3982.

New Mexico 87103

Sincerely,

Arlene V. Portillo

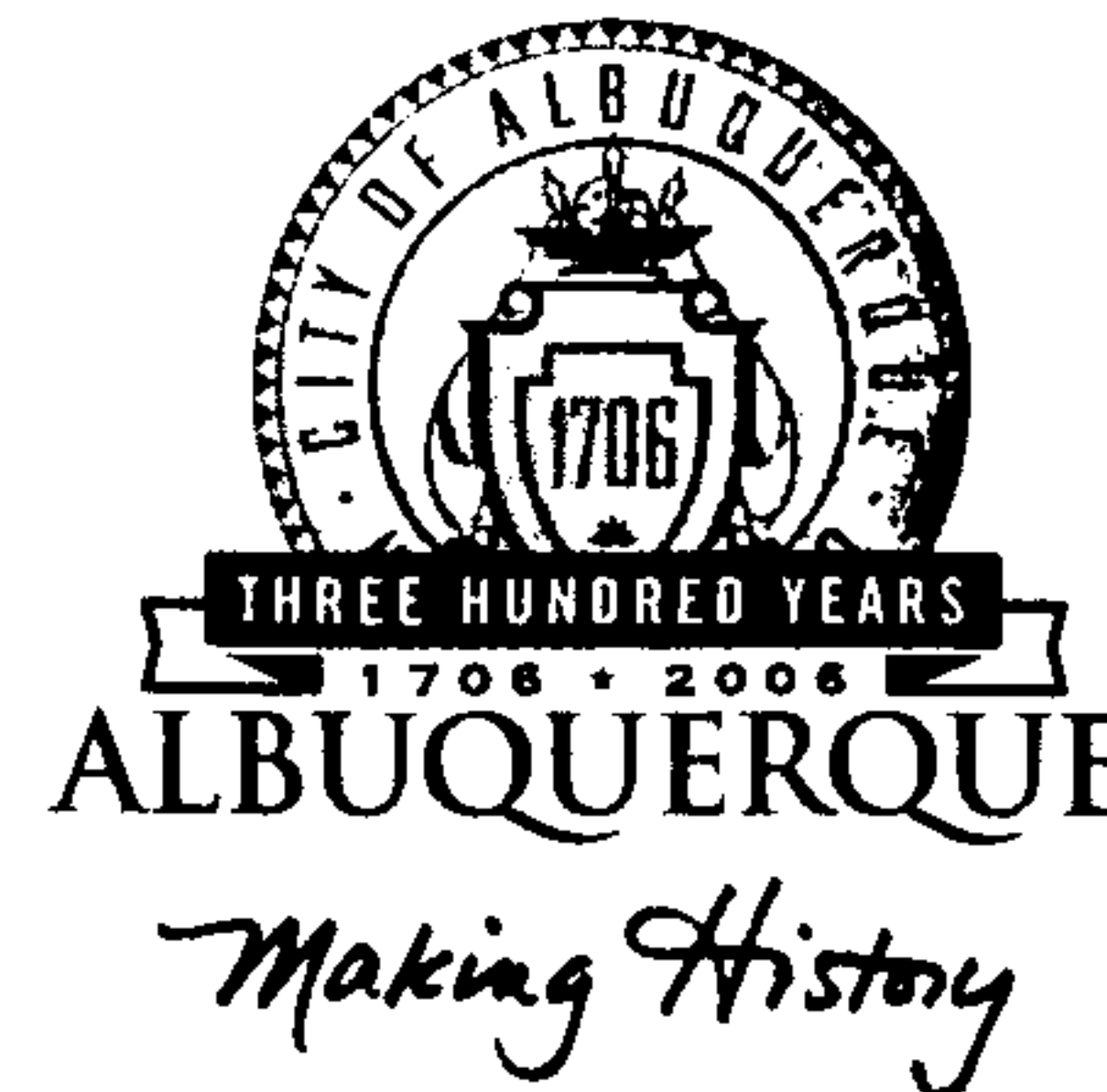
Arlene V. Portillo
Plan Reviewer, Planning Dept.-Hydrology
Development and Building Services

www.cabq.gov

C:

CO Clerk
File *✓*

CITY OF ALBUQUERQUE



January 4, 2006

Ronald R. Bohannon, P.E.
Tierra West, LLC
8509 Jefferson NE
Albuquerque, NM 87113

**Re: Kohl's Department Store, NW Corner of Paseo Del Norte and Louisiana
Grading and Drainage Plan**

Engineer's Stamp dated 11-2-05 (C18-D57)

Dear MR. Bohannon,

Based upon the information provided in your submittal received 9-21-05 and subsequent approval of the adjoining property owner to grade on his property, the above referenced plan is approved for Building Permit. Please attach a copy of this approved plan to the construction sets prior to sign-off by Hydrology.

This project requires a National Pollutant Discharge Elimination System (NPDES) permit. Refer to the attachment that is provided with this letter for details. If you have any questions please feel free to call the Municipal Development Department, Hydrology section at 768-3654 (Charles Caruso).

Also, prior to Certificate of Occupancy release, Engineer Certification of the grading plan per the DPM checklist will be required.

If you have any questions, you can contact me at 924-3986.

Sincerely,

Bradley L. Bingham, PE
Principal Engineer, Planning Dept.
Development and Building Services

C: Chuck Caruso, DMD
file

P.O. Box 1293

Albuquerque

New Mexico 87103

www.cabq.gov

**DRAINAGE REPORT
FOR**

***Tract A
Paseo De Louisiana
Albuquerque, NM***

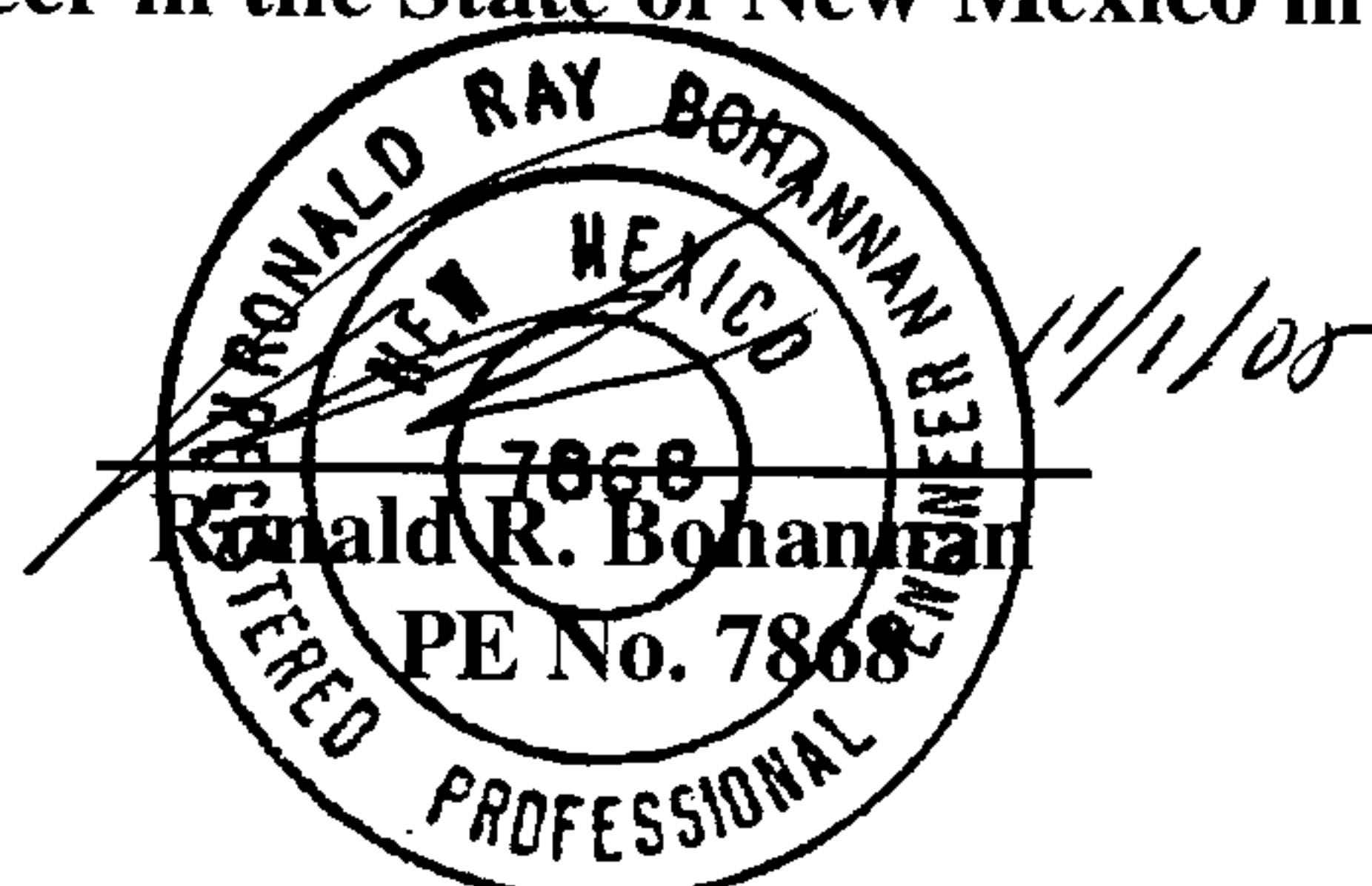
Prepared by:

**Tierra West, LLC
8509 Jefferson NE
Albuquerque, New Mexico 87113**

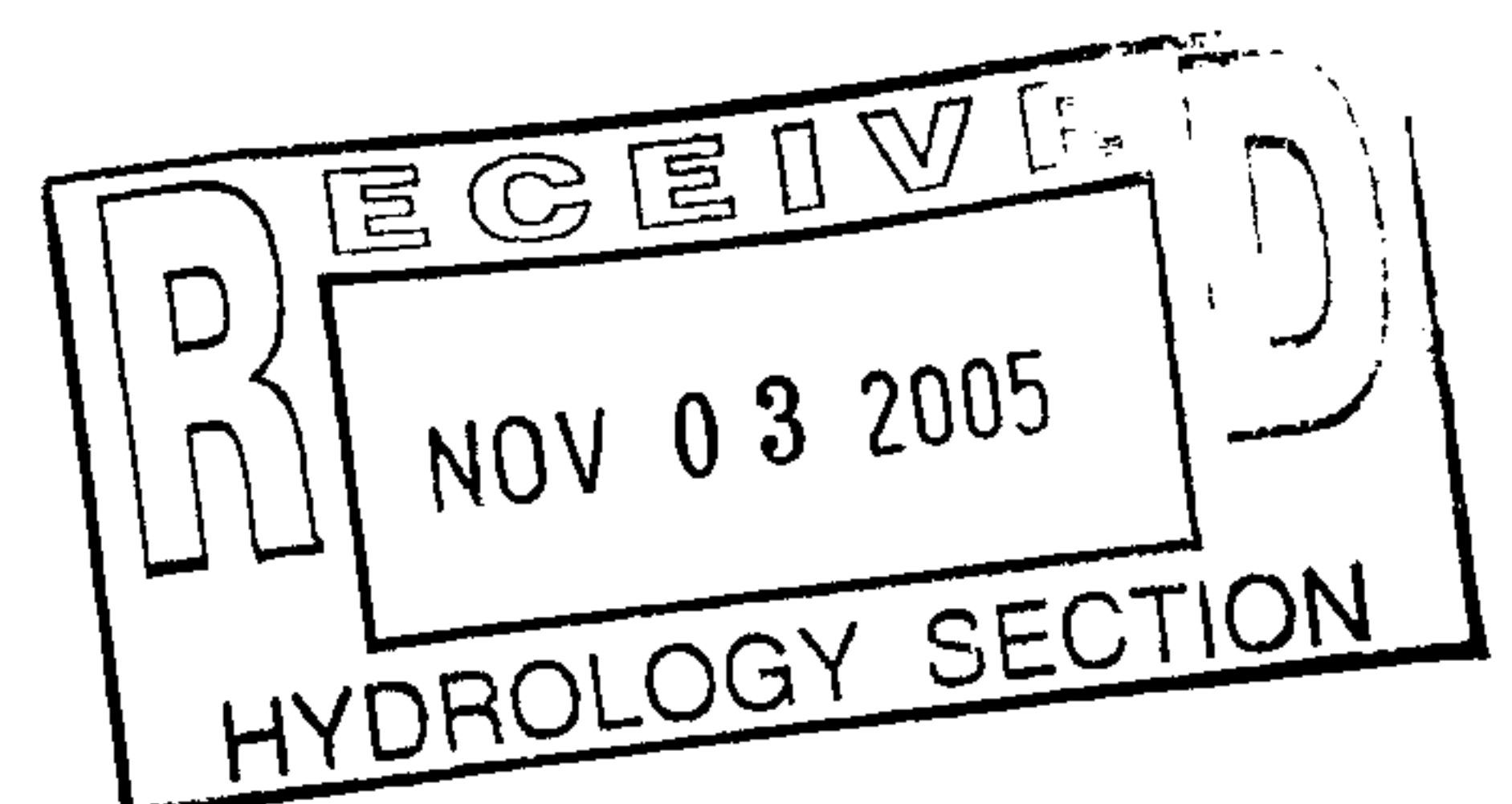
**Prepared for:
Kohl's Department Stores
N56 W17000 Ridgewood Dr.
Menomonee Falls, WI 53051**

**September 2005
(Revised)**

I certify that this report was prepared under my supervision, and I am a registered professional engineer in the State of New Mexico in good standing.



Job No 25017



Prelude

This report is being prepared at the request of the current owner, Kohl's Department Stores, who proposes to develop a single retail store on the subject property.

Location

The subject site is located on the northwest corner of Paseo Del Norte and Louisiana Boulevard and consists of Tract A, Paseo De Louisiana. The exact location of the site is shown highlighted on the enclosed Zone Atlas page number C-18. The site will be built in one phase and contains 7.3572 acres, more or less.

Existing Drainage Conditions

The site is currently undeveloped and is bordered by Paseo Del Norte on the south, Holly Avenue on the north, Louisiana Boulevard on the east and a vacant tract of land to the west.

The flows from the site sheet flow to the northwest corner of the site, eventually flowing to Holly Avenue, which is currently undeveloped. From this point the flows continue to the west eventually reaching San Pedro Drive. All flow from the east of Louisiana is intercepted by the existing storm sewer in the street. There are minimal flows currently entering the site from the north, south and east.

Flood Plain

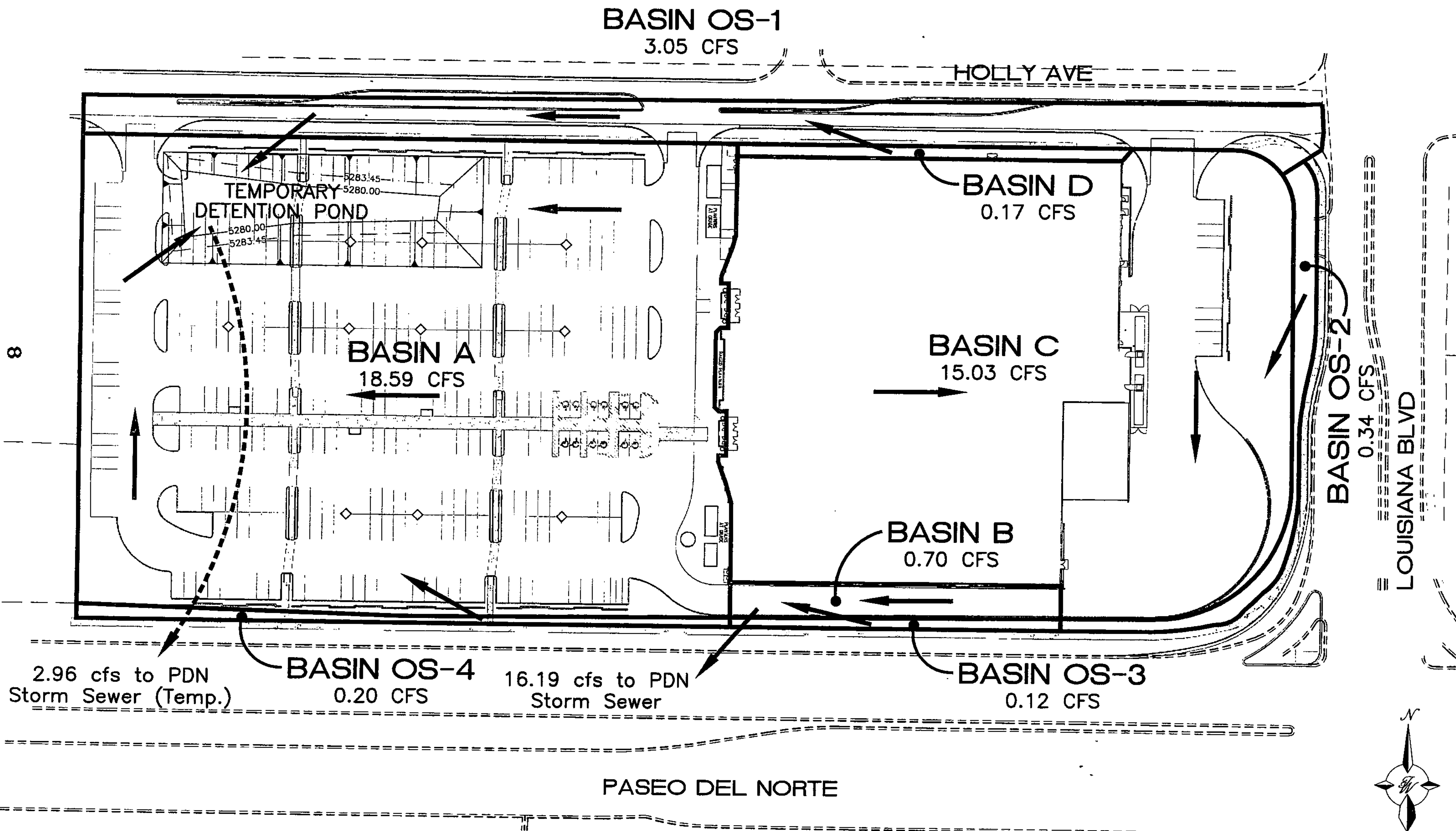
The site is located on FIRM Map 35001C0137F as shown on the attached excerpt. The map shows that the site does not lie within a flood plain; however it does lie within an area that has a 1% annual chance of flood discharge contained in the Paseo Del Norte storm sewer system.

Proposed Drainage Management Plan

As shown on the attached exhibit, the proposed site is divided into 4 onsite basins and 4 off-site basins. Based on the approved Conceptual Grading and Drainage Plan for the Sanchez Properties (C18/D57) dated March 2000, this site is allowed to discharge 20 cfs to the Paseo Del Norte storm sewer system that runs along the south boundary of the site. The site is also allowed free discharge to the Holly Avenue and San Pedro Drive storm sewer system, which is currently not built. This information is further outlined in letters found in Appendix A of this report.

Based on this information Basins B, C, OS-2 and OS-3 will drain to a storm sewer system on site which will be connected to the Paseo Del Norte storm sewer system at an existing man-hole located adjacent to the southern property line mid-way through the property. The total discharge from these basins is 16.19 cfs.

The remainder of the site (Basins A, D, OS-1 and OS-4) will be allowed free discharge to the Holly Avenue storm sewer system that is not currently built. As an interim solution, these basins sheet flow to a temporary detention pond located along the northern property line. Basin D drains to Holly Avenue via a 24" sidewalk culvert. Basin OS-1 (Holly Avenue) will drain to the temporary detention pond via a temporary sidewalk culvert. The temporary detention pond will be allowed to discharge at a rate of 2.96 cfs to the Paseo Del Norte storm sewer system bringing the total discharge to the Paseo Del Norte storm sewer system to 19.15 cfs, which is below the 20 cfs allowed. When construction of the Holly Avenue storm sewer system is complete, the temporary detention pond will be removed and the flows allowed to free discharge to Holly Avenue. These flows will be collected in drop inlets proposed in Holly Avenue and conveyed to the storm sewer located at San Pedro Drive.



PROPOSED BASIN MAP

Calculations

The weighted E method from the “City of Albuquerque Development Process Manual Volume 11 – Design Criteria, 1997 Revision” was used to calculate the runoff and volume for the site. The site was further analyzed using the AHYMO program routing the flows through the site.

Summary

This site will discharge a total of 38.20 cfs; with 19.15 cfs going into the existing storm sewer along Paseo Del Norte as a interim solution. A total of 16.19 cfs (Basins B, C, OS-2 and OS-3) will be a permanent discharge to this storm sewer system. The remaining 22.01 cfs (Basins A, D, OS-1 and OS-4) being a temporary discharge to the Paseo Del Norte storm sewer system at a rate of 2.96 cfs until the storm sewer in Holly Avenue is constructed. Once the Holly Avenue storm sewer is complete, the temporary detention pond can then be eliminated and the flows allowed to freely discharge into Holly Avenue.

The development of this site is consistent with the DPM, Chapter 22, Hydrology section. Since this site encompasses more than 1 acre an NPDES permit will be required in addition to a Top Soil Disturbance Permit prior to any construction activity.

Weighted E Method

Developed On-Site Basins

| Basin | Area (sf) | Area (acres) | Treatment A | | Treatment B | | Treatment C | | Treatment D | | 100-Year | | | 10-Year | | |
|-------|--------------|-----------------|-------------|---------|-------------|---------|-------------|---------|-------------|---------|-----------------------|-------------------|-------------|-----------------------|-------------------|-------------|
| | | | % | (acres) | % | (acres) | % | (acres) | % | (acres) | Weighted E (ac-ft) | Volume (ac-ft) | Flow cfs | Weighted E (ac-ft) | Volume (ac-ft) | Flow cfs |
| A | 173,014 | 3.97 | 0% | 0 | 14% | 0.56 | 0% | 0.00 | 86% | 3.42 | 2.158 | 0.714 | 18.59 | 1.340 | 0.444 | 12.24 |
| B | 6,341 | 0.15 | 0% | 0 | 9% | 0.01 | 0% | 0.00 | 91% | 0.13 | 2.230 | 0.027 | 0.70 | 1.397 | 0.017 | 0.46 |
| C | 138,415 | 3.18 | 0% | 0 | 12% | 0.38 | 0% | 0.00 | 88% | 2.80 | 2.187 | 0.579 | 15.03 | 1.363 | 0.361 | 9.93 |
| D | 2,786 | 0.06 | 0% | 0 | 100% | 0.06 | 0% | 0.00 | 0% | 0.00 | 0.920 | 0.005 | 0.17 | 0.360 | 0.002 | 0.08 |

Developed Off-Site Basins

| Basin | Area (sf) | Area (acres) | Treatment A | | Treatment B | | Treatment C | | Treatment D | | 100-Year | | | 10-Year | | |
|-------|--------------|-----------------|-------------|---------|-------------|---------|-------------|---------|-------------|---------|-----------------------|-------------------|-------------|-----------------------|-------------------|-------------|
| | | | % | (acres) | % | (acres) | % | (acres) | % | (acres) | Weighted E (ac-ft) | Volume (ac-ft) | Flow cfs | Weighted E (ac-ft) | Volume (ac-ft) | Flow cfs |
| OS-1 | 30,460 | 0.70 | 0% | 0 | 27% | 0.19 | 0% | 0.00 | 73% | 0.51 | 1.971 | 0.115 | 3.05 | 1.192 | 0.069 | 1.96 |
| OS-2 | 5,697 | 0.13 | 0% | 0 | 100% | 0.13 | 0% | 0.00 | 0% | 0.00 | 0.920 | 0.010 | 0.34 | 0.360 | 0.004 | 0.16 |
| OS-3 | 1,985 | 0.05 | 0% | 0 | 100% | 0.05 | 0% | 0.00 | 0% | 0.00 | 0.920 | 0.003 | 0.12 | 0.360 | 0.001 | 0.05 |
| OS-4 | 3,374 | 0.08 | 0% | 0 | 100% | 0.08 | 0% | 0.00 | 0% | 0.00 | 0.920 | 0.006 | 0.20 | 0.360 | 0.002 | 0.09 |

Equations:

Weighted E = $E_a \cdot A_a + E_b \cdot A_b + E_c \cdot A_c + E_d \cdot A_d$ / (Total Area)

Volume = Weighted E * Total Area

Flow = $Q_a \cdot A_a + Q_b \cdot A_b + Q_c \cdot A_c + Q_d \cdot A_d$

| Excess Precipitation, E (inches) | | |
|----------------------------------|----------|-----------|
| Zone3 | 100-Year | 10 - Year |
| E _a | 0.66 | 0.19 |
| E _b | 0.92 | 0.36 |
| E _c | 1.29 | 0.62 |
| E _d | 2.36 | 1.50 |

| Peak Discharge (cfs/acre) | | |
|---------------------------|----------|-----------|
| Zone 3 | 100-Year | 10 - Year |
| Q _a | 1.87 | 0.58 |
| Q _b | 2.60 | 1.19 |
| Q _c | 3.45 | 2.00 |
| Q _d | 5.02 | 3.39 |

Weighted E Method

Undeveloped On-Site Basins

| Basin | Area (sf) | Area (acres) | Treatment A | | Treatment B | | Treatment C | | Treatment D | | 100-Year | | | 10-Year | | |
|-------|--------------|-----------------|-------------|---------|-------------|---------|-------------|---------|-------------|---------|-----------------------|-------------------|-------------|-----------------------|-------------------|-------------|
| | | | Treatment A | | Treatment B | | Treatment C | | Treatment D | | Weighted E (ac-ft) | Volume (ac-ft) | Flow cfs | Weighted E (ac-ft) | Volume (ac-ft) | Flow cfs |
| | | | % | (acres) | % | (acres) | % | (acres) | % | (acres) | | | | | | |
| A | 173,014 | 3.97 | 100% | 3.97 | 0% | 0.00 | 0% | 0.00 | 0% | 0.00 | 0.660 | 0.218 | 7.43 | 0.190 | 0.063 | 2.30 |
| B | 6,341 | 0.15 | 100% | 0.15 | 0% | 0.00 | 0% | 0.00 | 0% | 0.00 | 0.660 | 0.008 | 0.27 | 0.190 | 0.002 | 0.08 |
| C | 138,415 | 3.18 | 100% | 3.18 | 0% | 0.00 | 0% | 0.00 | 0% | 0.00 | 0.660 | 0.175 | 5.94 | 0.190 | 0.050 | 1.84 |
| D | 2,786 | 0.06 | 100% | 0.06 | 0% | 0.00 | 0% | 0.00 | 0% | 0.00 | 0.660 | 0.004 | 0.12 | 0.190 | 0.001 | 0.04 |

Undeveloped Off-Site Basins

| Basin | Area (sf) | Area (acres) | Treatment A | | Treatment B | | Treatment C | | Treatment D | | 100-Year | | | 10-Year | | |
|-------|--------------|-----------------|-------------|---------|-------------|---------|-------------|---------|-------------|---------|-----------------------|-------------------|-------------|-----------------------|-------------------|-------------|
| | | | Treatment A | | Treatment B | | Treatment C | | Treatment D | | Weighted E (ac-ft) | Volume (ac-ft) | Flow cfs | Weighted E (ac-ft) | Volume (ac-ft) | Flow cfs |
| | | | % | (acres) | % | (acres) | % | (acres) | % | (acres) | | | | | | |
| OS-1 | 30,460 | 0.70 | 60% | 0.42 | 12% | 0.08 | 0% | 0.00 | 28% | 0.20 | 1.167 | 0.068 | 1.99 | 0.577 | 0.034 | 1.01 |
| OS-2 | 5,697 | 0.13 | 0% | 0.00 | 100% | 0.13 | 0% | 0.00 | 0% | 0.00 | 0.920 | 0.010 | 0.34 | 0.360 | 0.004 | 0.16 |
| OS-3 | 1,985 | 0.05 | 0% | 0.00 | 100% | 0.05 | 0% | 0.00 | 0% | 0.00 | 0.920 | 0.003 | 0.12 | 0.360 | 0.001 | 0.05 |
| OS-4 | 3,374 | 0.08 | 0% | 0.00 | 100% | 0.08 | 0% | 0.00 | 0% | 0.00 | 0.920 | 0.006 | 0.20 | 0.360 | 0.002 | 0.09 |

Equations:

Weighted E = $E_a \cdot A_a + E_b \cdot A_b + E_c \cdot A_c + E_d \cdot A_d$ / (Total Area)

Volume = Weighted D * Total Area

Flow = $Q_a \cdot A_a + Q_b \cdot A_b + Q_c \cdot A_c + Q_d \cdot A_d$

| Excess Precipitation, E (inches) | | |
|----------------------------------|----------|-----------|
| Zone3 | 100-Year | 10 - Year |
| E _a | 0.66 | 0.19 |
| E _b | 0.92 | 0.36 |
| E _c | 1.29 | 0.62 |
| E _d | 2.36 | 1.50 |

| Peak Discharge (cfs/acre) | | |
|---------------------------|----------|-----------|
| Zone 3 | 100-Year | 10 - Year |
| Q _a | 1.87 | 0.58 |
| Q _b | 2.60 | 1.19 |
| Q _c | 3.45 | 2.00 |
| Q _d | 5.02 | 3.39 |

VOLUME CALCULATIONS

POND

Ab - Bottom Of The Pond Surface Area

At - Top Of The Pond Surface Area

D - Water Depth

Dt - Total Pond Depth

C - Change In Surface Area / Water Depth

$$\text{Volume} = \text{Ab} * \text{D} + 0.5 * \text{C} * \text{D}^2$$

$$\text{C} = (\text{At} - \text{Ab}) / \text{Dt}$$

$$\text{Ab} = 5,813.00$$

$$\text{At} = 11,832.00$$

$$\text{Dt} = 4.00$$

$$\text{C} = 1504.75$$

| ACTUAL ELEV. | DEPTH (FT) | VOLUME (AC-FT) | Q (CFS) |
|-----------------|---------------|-------------------|------------|
| 5280.00 | 0.00 | 0.00 | 0.0000 |
| 5281.00 | 1.00 | 0.1334 | 1.3723 |
| 5282.00 | 2.00 | 0.2842 | 2.1698 |
| 5283.00 | 3.00 | 0.4694 | 2.7446 |
| 5284.00 | 4.00 | 0.6892 | 3.2184 |

Orifice Equation

$$Q = \text{CA} \text{ SQRT}(2gH)$$

$$\text{C} = 0.6$$

$$\text{Diameter (in)} = 8$$

$$\text{Area (ft}^2\text{)} = 0.349$$

$$g = 32.2$$

$$\text{H (Ft)} = \text{Depth of water above center of orifice}$$

$$\text{Q (CFS)} = \text{Flow}$$

AHYMO PROGRAM SUMMARY TABLE (AHYMO_97) -
 =10/19/2005
 INPUT FILE = C:\AHYMO~1\25017D~1.TXT
 AHYMO-S-9702d1Tierraw-AH

AHYMO.SUM

- VERSION: 1997.02d

RUN DATE (MON/DAY/YR)

USER NO.=

| COMMAND NOTATION | HYDROGRAPH IDENTIFICATION | FROM ID NO. | TO ID NO. | AREA (SQ MI) | PEAK DISCHARGE (CFS) | RUNOFF VOLUME (AC-FT) | RUNOFF (INCHES) | TIME TO PEAK (HOURS) | CFS PER ACRE | PAGE |
|-----------------------------------|---------------------------|-------------|-----------|--------------|----------------------|-----------------------|-----------------|----------------------|--------------|--------|
| START .00 | | | | | | | | | | TIME= |
| RAINFALL TYPE= 1 | | | | | | | | | | RAIN6= |
| 2.600 | | | | | | | | | | |
| *S BASIN A | | | | | | | | | | |
| IMP= 86.00 | COMPUTE NM HYD | 101.00 | - 1 | .00621 | 18.61 ✓ | .714 | 2.15447 | 1.500 | 4.682 | PER |
| *S BASIN B | | | | | | | | | | |
| IMP= 91.00 | COMPUTE NM HYD | 102.00 | - 2 | .00023 | .72 ✓ | .027 | 2.22619 | 1.500 | 4.895 | PER |
| *S BASIN C | | | | | | | | | | |
| IMP= 88.00 | COMPUTE NM HYD | 103.00 | - 3 | .00497 | 15.05 ✓ | .579 | 2.18316 | 1.500 | 4.732 | PER |
| *S BASIN D | | | | | | | | | | |
| IMP= .00 | COMPUTE NM HYD | 104.00 | - 4 | .00010 | .18 ✓ | .005 | .92096 | 1.500 | 2.752 | PER |
| *S BASIN OS-1 | | | | | | | | | | |
| IMP= 73.00 | COMPUTE NM HYD | 105.00 | - 5 | .00109 | 3.06 ✓ | .114 | 1.96801 | 1.500 | 4.391 | PER |
| *S BASIN OS-2 | | | | | | | | | | |
| IMP= .00 | COMPUTE NM HYD | 106.00 | - 6 | .00020 | .34 ✓ | .010 | .92096 | 1.500 | 2.673 | PER |
| *S BASIN OS-3 | | | | | | | | | | |
| IMP= .00 | COMPUTE NM HYD | 107.00 | - 7 | .00007 | .13 ✓ | .003 | .92096 | 1.500 | 2.819 | PER |
| *S BASIN OS-4 | | | | | | | | | | |
| IMP= .00 | COMPUTE NM HYD | 108.00 | - 8 | .00012 | .21 ✓ | .006 | .92096 | 1.500 | 2.727 | PER |
| *S ADD BASINS A AND OS-4 | | | | | | | | | | |
| ADD HYD | | 151.00 | 1& 8 9 | .00633 | 18.82 ✓ | .719 | 2.13104 | 1.500 | 4.645 | |
| *S ADD BASINS B AND OS-3 | | | | | | | | | | |
| ADD HYD | | 152.00 | 2& 7 10 | .00030 | .85 ✓ | .031 | 1.92051 | 1.500 | 4.411 | |
| *S ADD BASINS C AND OS-2 | | | | | | | | | | |
| ADD HYD | | 153.00 | 3& 6 11 | .00517 | 15.39 ✓ | .588 | 2.13426 | 1.500 | 4.652 | |
| *S ADD BASINS D AND OS-1 | | | | | | | | | | |
| ADD HYD | | 154.00 | 4& 5 12 | .00119 | 3.24 ✓ | .119 | 1.87972 | 1.500 | 4.253 | |
| *S ADD BASINS A, D, OS-1 and OS-4 | | | | | | | | | | |
| ADD HYD | | 155.00 | 9&12 13 | .00752 | 22.06 ✓ | .839 | 2.09127 | 1.500 | 4.583 | |
| *S ADD BASINS B, C, OS-2 AND OS-3 | | | | | | | | | | |
| ADD HYD | | 156.00 | 10&11 14 | .00547 | 16.24 ✓ | .619 | 2.12253 | 1.500 | 4.639 | |
| *S POND A | | | | | | | | | | |

| | | | | | | | | | |
|-----------------|--------|----|----|-----------|------|------|---------|-------|-------------|
| ROUTE RESERVOIR | 201.00 | 13 | 15 | AHYMO.SUM | | | | | |
| .568 | | | | .00752 | 2.96 | .839 | 2.09126 | 2.133 | .614 AC-FT= |
| FINISH | | | | | | | | | |

25017DRAIN.txt

```
*****
*
*          KOHL'S AT PASEO DRAINAGE ANALYSIS
*          LOUISIANA & PASEO DEL NORTE (25017)
*          PROPOSED CONDITIONS (100-YEAR, 6-HR STORM)
*
*****
```

```
START          TIME=0.0 HR
RAINFALL       TYPE=1 RAIN QUARTER=0.0 IN
               RAIN ONE=2.14 IN RAIN SIX=2.60 IN
               RAIN DAY=3.10 IN DT=0.03333 HR
```

```
*S BASIN A
```

```
COMPUTE NM HYD      ID=1 HYD NO=101 AREA=0.00621 SQ MI
                    PER A=0 PER B=14.00 PER C=0.00 PER D=86.00
                    TP=0 HR MASS RAINFALL=-1
PRINT HYD           ID=1 CODE=1
```

```
*S BASIN B
```

```
COMPUTE NM HYD      ID=2 HYD NO=102 AREA=0.00023 SQ MI
                    PER A=0 PER B=9.00 PER C=0.00 PER D=91.00
                    TP=0 HR MASS RAINFALL=-1
PRINT HYD           ID=2 CODE=1
```

```
*S BASIN C
```

```
COMPUTE NM HYD      ID=3 HYD NO=103 AREA=0.00497 SQ MI
                    PER A=0 PER B=12.00 PER C=0.00 PER D=88.00
                    TP=0 HR MASS RAINFALL=-1
PRINT HYD           ID=3 CODE=1
```

```
*S BASIN D
```

```
COMPUTE NM HYD      ID=4 HYD NO=104 AREA=0.0001 SQ MI
                    PER A=0 PER B=100.00 PER C=0.00 PER D=0.00
                    TP=0 HR MASS RAINFALL=-1
PRINT HYD           ID=4 CODE=1
```

```
*S BASIN OS-1
```

```
COMPUTE NM HYD      ID=5 HYD NO=105 AREA=0.00109 SQ MI
                    PER A=0 PER B=27.00 PER C=0.00 PER D=73.00
                    TP=0 HR MASS RAINFALL=-1
PRINT HYD           ID=5 CODE=1
```

```
*S BASIN OS-2
```

```
COMPUTE NM HYD      ID=6 HYD NO=106 AREA=0.00020 SQ MI
                    PER A=0 PER B=100.00 PER C=0.00 PER D=0.00
                    TP=0 HR MASS RAINFALL=-1
PRINT HYD           ID=6 CODE=1
```

```
*S BASIN OS-3
```

```
COMPUTE NM HYD      ID=7 HYD NO=107 AREA=0.00007 SQ MI
                    PER A=0 PER B=100.00 PER C=0.00 PER D=0.00
                    TP=0 HR MASS RAINFALL=-1
PRINT HYD           ID=7 CODE=1
```

25017DRAIN.txt

```

*
*S BASIN OS-4
*
COMPUTE NM HYD      ID=8 HYD NO=108 AREA=0.00012 SQ MI
                    PER A=0 PER B=100.00 PER C=0.00 PER D=0.00
                    TP=0 HR MASS RAINFALL=-1
PRINT HYD           ID=8 CODE=1

*
*S ADD BASINS A AND OS-4
*
ADD HYD             ID=9 HYD NO=151.00 ID=1 ID=8
PRINT HYD           ID=9 CODE=1

*
*S ADD BASINS B AND OS-3
*
ADD HYD             ID=10 HYD NO=152.00 ID=2 ID=7
PRINT HYD           ID=10 CODE=1

*
*S ADD BASINS C AND OS-2
*
ADD HYD             ID=11 HYD NO=153.00 ID=3 ID=6
PRINT HYD           ID=11 CODE=1

*
*S ADD BASINS D AND OS-1
*
ADD HYD             ID=12 HYD NO=154.00 ID=4 ID=5
PRINT HYD           ID=12 CODE=1

*
*S ADD BASINS A, D, OS-1 and OS-4
*
ADD HYD             ID=13 HYD NO=155.00 ID=9 ID=12
PRINT HYD           ID=13 CODE=1

*
*S ADD BASINS B, C, OS-2 AND OS-3
*
ADD HYD             ID=14 HYD NO=156.00 ID=10 ID=11
PRINT HYD           ID=14 CODE=1

*
*S POND A
*
ROUTE RESERVOIR     ID=15 HYD NO=201.00 INFLOW ID=13 CODE=24
                    OUTFLOW(CFS)      STORAGE(AC-FT)  ELEVATION(FT)
                    0.0000             0.0000         5280.00
                    1.3723             0.1334         5281.00
                    2.1698             0.2842         5282.00
                    2.7446             0.4694         5283.00
                    3.2184             0.6892         5284.00

PRINT HYD           ID=15 CODE=1
*

*
FINISH

```


AHYMO PROGRAM (AHYMO_97) - - Version:
 1997.02d
 RUN DATE (MON/DAY/YR) = 10/19/2005
 START TIME (HR:MIN:SEC) = 14:04:29 USER NO.= AHYMO-S-
 9702d1TierraW-AH
 INPUT FILE = C:\AHYMOW~1\25017D~1.TXT

 *
 * KOHL'S AT PASEO DRAINAGE ANALYSIS *
 * LOUISIANA & PASEO DEL NORTE (25017) *
 * PROPOSED CONDITIONS (100-YEAR, 6-HR STORM) *
 *

START TIME=0.0 HR
 RAINFALL TYPE=1 RAIN QUARTER=0.0 IN
 RAIN ONE=2.14 IN RAIN SIX=2.60 IN
 RAIN DAY=3.10 IN DT=0.03333 HR

COMPUTED 6-HOUR RAINFALL DISTRIBUTION BASED ON NOAA
 ATLAS 2 - PEAK AT 1.40 HR.
 DT = .033330 HOURS END TIME = 5.999400

| | | | | | | |
|--------|--------|--------|--------|--------|--------|--------|
| HOURS | .0000 | .0027 | .0055 | .0084 | .0113 | .0143 |
| .0173 | .0204 | .0236 | .0269 | .0302 | .0337 | .0372 |
| .0408 | .0445 | .0484 | .0523 | .0564 | .0606 | .0649 |
| .0694 | .0741 | .0789 | .0839 | .0892 | .0946 | .1003 |
| .1063 | .1126 | .1192 | .1262 | .1322 | .1385 | .1452 |
| .1597 | .1922 | .2422 | .3139 | .4119 | .5407 | .7049 |
| .9093 | 1.1588 | 1.3904 | 1.4871 | 1.5687 | 1.6414 | 1.7074 |
| 1.7683 | 1.8247 | 1.8775 | 1.9270 | 1.9735 | 2.0174 | 2.0589 |
| 2.0982 | 2.1354 | 2.1707 | 2.2041 | 2.2359 | 2.2661 | 2.2737 |
| 2.2807 | 2.2875 | 2.2939 | 2.3001 | 2.3060 | 2.3117 | 2.3172 |
| 2.3226 | 2.3277 | 2.3328 | 2.3376 | 2.3423 | 2.3470 | 2.3514 |
| 2.3558 | 2.3601 | 2.3643 | 2.3683 | 2.3723 | 2.3762 | 2.3801 |
| 2.3838 | 2.3875 | 2.3911 | 2.3947 | 2.3982 | 2.4016 | 2.4050 |
| 2.4083 | 2.4115 | 2.4147 | 2.4179 | 2.4210 | 2.4241 | 2.4271 |
| 2.4301 | 2.4330 | 2.4359 | 2.4388 | 2.4416 | 2.4444 | 2.4472 |
| 2.4499 | 2.4526 | 2.4553 | 2.4579 | 2.4605 | 2.4631 | 2.4656 |
| 2.4681 | 2.4706 | 2.4731 | 2.4755 | 2.4779 | 2.4803 | 2.4827 |
| 2.4850 | 2.4873 | 2.4896 | 2.4919 | 2.4942 | 2.4964 | 2.4986 |
| 2.5008 | | | | | | |

| | | | | | | |
|--------|--------|--------|--------|--------|--------|--------|
| 2.5157 | 2.5030 | 2.5052 | 2.5073 | 2.5094 | 2.5115 | 2.5136 |
| 2.5297 | 2.5177 | 2.5198 | 2.5218 | 2.5238 | 2.5258 | 2.5277 |
| 2.5430 | 2.5317 | 2.5336 | 2.5355 | 2.5374 | 2.5393 | 2.5412 |
| 2.5557 | 2.5449 | 2.5467 | 2.5486 | 2.5504 | 2.5522 | 2.5540 |
| 2.5679 | 2.5575 | 2.5593 | 2.5610 | 2.5627 | 2.5645 | 2.5662 |
| 2.5795 | 2.5696 | 2.5713 | 2.5729 | 2.5746 | 2.5762 | 2.5779 |
| 2.5907 | 2.5811 | 2.5828 | 2.5844 | 2.5860 | 2.5876 | 2.5891 |
| | 2.5923 | 2.5938 | 2.5954 | 2.5969 | 2.5984 | 2.6000 |

*

*S BASIN A

*

COMPUTE NM HYD ID=1 HYD NO=101 AREA=0.00621 SQ MI
 PER A=0 PER B=14.00 PER C=0.00 PER D=86.00
 TP=0 HR MASS RAINFALL=-1

*****Warning*****A value for the Time to Peak was not
 previously computed; use Tp=0.133333 hrs.

TIME TO PEAK (hrs)= .1333

K = .072666HR TP = .133333HR K/TP RATIO = .545000
 SHAPE CONSTANT, N = 7.106420
 UNIT PEAK = 21.080 CFS UNIT VOLUME = .9988 B =
 526.28 P60 = 2.1400
 AREA = .005341 SQ MI IA = .10000 INCHES INF =
 .04000 INCHES PER HOUR
 RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER
 METHOD - DT = .033330

K = .133689HR TP = .133333HR K/TP RATIO = 1.002670
 SHAPE CONSTANT, N = 3.520804
 UNIT PEAK = 2.0986 CFS UNIT VOLUME = .9937 B =
 321.84 P60 = 2.1400
 AREA = .000869 SQ MI IA = .50000 INCHES INF =
 1.25000 INCHES PER HOUR
 RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER
 METHOD - DT = .033330

PRINT HYD ID=1 CODE=1

PARTIAL HYDROGRAPH 101.00

RUNOFF VOLUME = 2.15447 INCHES = .7136 ACRE-FEET
 PEAK DISCHARGE RATE = 18.61 CFS AT 1.500 HOURS BASIN
 AREA = .0062 SQ. MI.

*

*S BASIN B

*

COMPUTE NM HYD ID=2 HYD NO=102 AREA=0.00023 SQ MI
 PER A=0 PER B=9.00 PER C=0.00 PER D=91.00
 TP=0 HR MASS RAINFALL=-1
 TIME TO PEAK (hrs)= .1333
 *****Warning*****This Tp value was used for a previously
 computed hydrograph.

A new Tp value should be computed.

K = .072666HR TP = .133333HR K/TP RATIO = .545000
 SHAPE CONSTANT, N = 7.106420
 UNIT PEAK = .82612 CFS UNIT VOLUME = .9862 B =
 526.28 P60 = 2.1400
 AREA = .000209 SQ MI IA = .10000 INCHES INF =
 .04000 INCHES PER HOUR
 RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER
 METHOD - DT = .033330

K = .133689HR TP = .133333HR K/TP RATIO = 1.002670
 SHAPE CONSTANT, N = 3.520804
 UNIT PEAK = .49967E-01CFS UNIT VOLUME = .8691 B =
 321.84 P60 = 2.1400
 AREA = .000021 SQ MI IA = .50000 INCHES INF =
 1.25000 INCHES PER HOUR
 RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER
 METHOD - DT = .033330

PRINT HYD ID=2 CODE=1

PARTIAL HYDROGRAPH 102.00

RUNOFF VOLUME = 2.22619 INCHES = .0273 ACRE-FEET
 PEAK DISCHARGE RATE = .72 CFS AT 1.500 HOURS BASIN
 AREA = .0002 SQ. MI.

*
 *S BASIN C
 *

COMPUTE NM HYD ID=3 HYD NO=103 AREA=0.00497 SQ MI
 PER A=0 PER B=12.00 PER C=0.00 PER D=88.00
 TP=0 HR MASS RAINFALL=-1
 TIME TO PEAK (hrs)= .1333
 *****Warning*****This Tp value was used for a previously
 computed hydrograph.

A new Tp value should be computed.

K = .072666HR TP = .133333HR K/TP RATIO = .545000
 SHAPE CONSTANT, N = 7.106420
 UNIT PEAK = 17.263 CFS UNIT VOLUME = .9988 B =
 526.28 P60 = 2.1400
 AREA = .004374 SQ MI IA = .10000 INCHES INF =
 .04000 INCHES PER HOUR
 RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER
 METHOD - DT = .033330

K = .133689HR TP = .133333HR K/TP RATIO = 1.002670
 SHAPE CONSTANT, N = 3.520804
 UNIT PEAK = 1.4396 CFS UNIT VOLUME = .9903 B =
 321.84 P60 = 2.1400
 AREA = .000596 SQ MI IA = .50000 INCHES INF =
 1.25000 INCHES PER HOUR
 RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER
 METHOD - DT = .033330

PRINT HYD ID=3 CODE=1

PARTIAL HYDROGRAPH 103.00

RUNOFF VOLUME = 2.18316 INCHES = .5787 ACRE-FEET
 PEAK DISCHARGE RATE = 15.05 CFS AT 1.500 HOURS BASIN
 AREA = .0050 SQ. MI.

*S BASIN D

*

COMPUTE NM HYD ID=4 HYD NO=104 AREA=0.0001 SQ MI
 PER A=0 PER B=100.00 PER C=0.00 PER D=0.00
 TP=0 HR MASS RAINFALL=-1

TIME TO PEAK (hrs)= .1333

*****Warning*****This Tp value was used for a previously
computed hydrograph.

A new Tp value should be computed.

K = .133689HR TP = .133333HR K/TP RATIO = 1.002670
 SHAPE CONSTANT, N = 3.520804
 UNIT PEAK = .24138 CFS UNIT VOLUME = .9432 B =
 321.84 P60 = 2.1400
 AREA = .000100 SQ MI IA = .50000 INCHES INF =
 1.25000 INCHES PER HOUR
 RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER
 METHOD - DT = .033330

PRINT HYD ID=4 CODE=1

PARTIAL HYDROGRAPH 104.00

RUNOFF VOLUME = .92096 INCHES = .0049 ACRE-FEET
 PEAK DISCHARGE RATE = .18 CFS AT 1.500 HOURS BASIN
 AREA = .0001 SQ. MI.

*

*S BASIN OS-1

*

COMPUTE NM HYD ID=5 HYD NO=105 AREA=0.00109 SQ MI
 PER A=0 PER B=27.00 PER C=0.00 PER D=73.00
 TP=0 HR MASS RAINFALL=-1

TIME TO PEAK (hrs)= .1333

*****Warning*****This Tp value was used for a previously
computed hydrograph.

A new Tp value should be computed.

K = .072666HR TP = .133333HR K/TP RATIO = .545000
SHAPE CONSTANT, N = 7.106420
UNIT PEAK = 3.1407 CFS UNIT VOLUME = .9955 B =
526.28 P60 = 2.1400
AREA = .000796 SQ MI IA = .10000 INCHES INF =
.04000 INCHES PER HOUR
RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER
METHOD - DT = .033330

K = .133689HR TP = .133333HR K/TP RATIO = 1.002670
SHAPE CONSTANT, N = 3.520804
UNIT PEAK = .71039 CFS UNIT VOLUME = .9809 B =
321.84 P60 = 2.1400
AREA = .000294 SQ MI IA = .50000 INCHES INF =
1.25000 INCHES PER HOUR
RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER
METHOD - DT = .033330

PRINT HYD ID=5 CODE=1

PARTIAL HYDROGRAPH 105.00

RUNOFF VOLUME = 1.96801 INCHES = .1144 ACRE-FEET
PEAK DISCHARGE RATE = 3.06 CFS AT 1.500 HOURS BASIN
AREA = .0011 SQ. MI.

*

*S BASIN OS-2

*

COMPUTE NM HYD ID=6 HYD NO=106 AREA=0.00020 SQ MI
PER A=0 PER B=100.00 PER C=0.00 PER D=0.00
TP=0 HR MASS RAINFALL=-1

TIME TO PEAK (hrs)= .1333

*****Warning*****This Tp value was used for a previously
computed hydrograph.

A new Tp value should be computed.

K = .133689HR TP = .133333HR K/TP RATIO = 1.002670
SHAPE CONSTANT, N = 3.520804
UNIT PEAK = .48277 CFS UNIT VOLUME = .9709 B =
321.84 P60 = 2.1400
AREA = .000200 SQ MI IA = .50000 INCHES INF =
1.25000 INCHES PER HOUR
RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER
METHOD - DT = .033330

PRINT HYD ID=6 CODE=1

PARTIAL HYDROGRAPH 106.00

RUNOFF VOLUME = .92096 INCHES = .0098 ACRE-FEET
PEAK DISCHARGE RATE = .34 CFS AT 1.500 HOURS BASIN
AREA = .0002 SQ. MI.

*

*S BASIN OS-3

*

COMPUTE NM HYD ID=7 HYD NO=107 AREA=0.00007 SQ MI
PER A=0 PER B=100.00 PER C=0.00 PER D=0.00
TP=0 HR MASS RAINFALL=-1

TIME TO PEAK (hrs)= .1333

*****Warning*****This Tp value was used for a previously
computed hydrograph.

A new Tp value should be computed.

K = .133689HR TP = .133333HR K/TP RATIO = 1.002670
SHAPE CONSTANT, N = 3.520804
UNIT PEAK = .16897 CFS UNIT VOLUME = .9206 B =
321.84 P60 = 2.1400
AREA = .000070 SQ MI IA = .50000 INCHES INF =
1.25000 INCHES PER HOUR
RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER
METHOD - DT = .033330

PRINT HYD

ID=7 CODE=1

PARTIAL HYDROGRAPH 107.00

RUNOFF VOLUME = .92096 INCHES = .0034 ACRE-FEET
PEAK DISCHARGE RATE = .13 CFS AT 1.500 HOURS BASIN
AREA = .0001 SQ. MI.

*

*S BASIN OS-4

*

COMPUTE NM HYD ID=8 HYD NO=108 AREA=0.00012 SQ MI
PER A=0 PER B=100.00 PER C=0.00 PER D=0.00
TP=0 HR MASS RAINFALL=-1

TIME TO PEAK (hrs)= .1333

*****Warning*****This Tp value was used for a previously
computed hydrograph.

A new Tp value should be computed.

K = .133689HR TP = .133333HR K/TP RATIO = 1.002670
SHAPE CONSTANT, N = 3.520804
UNIT PEAK = .28966 CFS UNIT VOLUME = .9519 B =
321.84 P60 = 2.1400
AREA = .000120 SQ MI IA = .50000 INCHES INF =
1.25000 INCHES PER HOUR
RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER
METHOD - DT = .033330

PRINT HYD ID=8 CODE=1

PARTIAL HYDROGRAPH 108.00

RUNOFF VOLUME = .92096 INCHES = .0059 ACRE-FEET
PEAK DISCHARGE RATE = .21 CFS AT 1.500 HOURS BASIN
AREA = .0001 SQ. MI.

*

*S ADD BASINS A AND OS-4

*

ADD HYD ID=9 HYD NO=151.00 ID=1 ID=8
PRINT HYD ID=9 CODE=1

PARTIAL HYDROGRAPH 151.00

RUNOFF VOLUME = 2.13104 INCHES = .7194 ACRE-FEET
PEAK DISCHARGE RATE = 18.82 CFS AT 1.500 HOURS BASIN
AREA = .0063 SQ. MI.

*

*S ADD BASINS B AND OS-3

*

ADD HYD ID=10 HYD NO=152.00 ID=2 ID=7
PRINT HYD ID=10 CODE=1

PARTIAL HYDROGRAPH 152.00

RUNOFF VOLUME = 1.92051 INCHES = .0307 ACRE-FEET
PEAK DISCHARGE RATE = .85 CFS AT 1.500 HOURS BASIN
AREA = .0003 SQ. MI.

*

*S ADD BASINS C AND OS-2

*

ADD HYD ID=11 HYD NO=153.00 ID=3 ID=6
PRINT HYD ID=11 CODE=1

PARTIAL HYDROGRAPH 153.00

RUNOFF VOLUME = 2.13426 INCHES = .5885 ACRE-FEET
PEAK DISCHARGE RATE = 15.39 CFS AT 1.500 HOURS BASIN
AREA = .0052 SQ. MI.

*
*S ADD BASINS D AND OS-1
*

ADD HYD ID=12 HYD NO=154.00 ID=4 ID=5
PRINT HYD ID=12 CODE=1

PARTIAL HYDROGRAPH 154.00

RUNOFF VOLUME = 1.87972 INCHES = .1193 ACRE-FEET
PEAK DISCHARGE RATE = 3.24 CFS AT 1.500 HOURS BASIN
AREA = .0012 SQ. MI.

*
*S ADD BASINS A, D, OS-1 and OS-4
*

ADD HYD ID=13 HYD NO=155.00 ID=9 ID=12
PRINT HYD ID=13 CODE=1

PARTIAL HYDROGRAPH 155.00

RUNOFF VOLUME = 2.09127 INCHES = .8387 ACRE-FEET
PEAK DISCHARGE RATE = 22.06 CFS AT 1.500 HOURS BASIN
AREA = .0075 SQ. MI.

*
*S ADD BASINS B, C, OS-2 AND OS-3
*

ADD HYD ID=14 HYD NO=156.00 ID=10 ID=11
PRINT HYD ID=14 CODE=1

PARTIAL HYDROGRAPH 156.00

RUNOFF VOLUME = 2.12253 INCHES = .6192 ACRE-FEET
PEAK DISCHARGE RATE = 16.24 CFS AT 1.500 HOURS BASIN
AREA = .0055 SQ. MI.

*
*S POND A
*

ROUTE RESERVOIR ID=15 HYD NO=201.00 INFLOW ID=13 CODE=24
OUTFLOW(CFS) STORAGE(AC-FT) ELEVATION(FT)
0.0000 0.0000 5280.00
1.3723 0.1334 5281.00

| | | |
|--------|--------|---------|
| 2.1698 | 0.2842 | 5282.00 |
| 2.7446 | 0.4694 | 5283.00 |
| 3.2184 | 0.6892 | 5284.00 |

* * * * *

| TIME (HRS) | INFLOW (CFS) | ELEV (FEET) | VOLUME (AC-FT) | OUTFLOW (CFS) |
|---------------|-----------------|----------------|-------------------|------------------|
| .00 | .00 | 5280.00 | .000 | .00 |
| .80 | .00 | 5280.00 | .000 | .00 |
| 1.60 | 15.37 | 5282.44 | .366 | 2.42 |
| 2.40 | .98 | 5283.32 | .539 | 2.90 |
| 3.20 | .24 | 5282.58 | .391 | 2.50 |
| 4.00 | .17 | 5281.80 | .254 | 2.01 |
| 4.80 | .16 | 5281.12 | .151 | 1.46 |
| 5.60 | .17 | 5280.63 | .085 | .87 |
| 6.40 | .01 | 5280.36 | .048 | .50 |
| 7.20 | .00 | 5280.18 | .025 | .25 |
| 8.00 | .00 | 5280.09 | .012 | .13 |
| 8.80 | .00 | 5280.05 | .006 | .06 |
| 9.60 | .00 | 5280.02 | .003 | .03 |
| 10.40 | .00 | 5280.01 | .002 | .02 |
| 11.20 | .00 | 5280.01 | .001 | .01 |
| 12.00 | .00 | 5280.00 | .000 | .00 |

PEAK DISCHARGE = 2.957 CFS - PEAK OCCURS AT HOUR 2.13

MAXIMUM WATER SURFACE ELEVATION = 5283.449

MAXIMUM STORAGE = .5681 AC-FT INCREMENTAL TIME=
.033330HRS

PRINT HYD ID=15 CODE=1

PARTIAL HYDROGRAPH 201.00

RUNOFF VOLUME = 2.09126 INCHES = .8387 ACRE-FEET
 PEAK DISCHARGE RATE = 2.96 CFS AT 2.133 HOURS BASIN
 AREA = .0075 SQ. MI.

*

*

FINISH

NORMAL PROGRAM FINISH

END TIME (HR:MIN:SEC) = 14:04:29

SIDEWALK CULVERTS

Orifice Equation:

$$Q = CA\sqrt{2gH}$$

Where:

$$C = 0.6$$

$$A = 0.5833 \times 2 = 1.167 \text{ ft}^2$$

$$g = 32.2$$

H = Height of water measured from center of orifice

$$Q = 0.6 \times 1.1678 \sqrt{2 \times 32.2 \times 0.2917}$$

$$Q = 3.035 \text{ cfs}$$

Basin D

$$3.04 \text{ cfs} > 0.17 \text{ cfs}$$

Use one 24" sidewalk culvert

Basin OS-1

$$6.07 \text{ cfs} > 3.22 \text{ cfs}$$

Use two 24" sidewalk culverts

