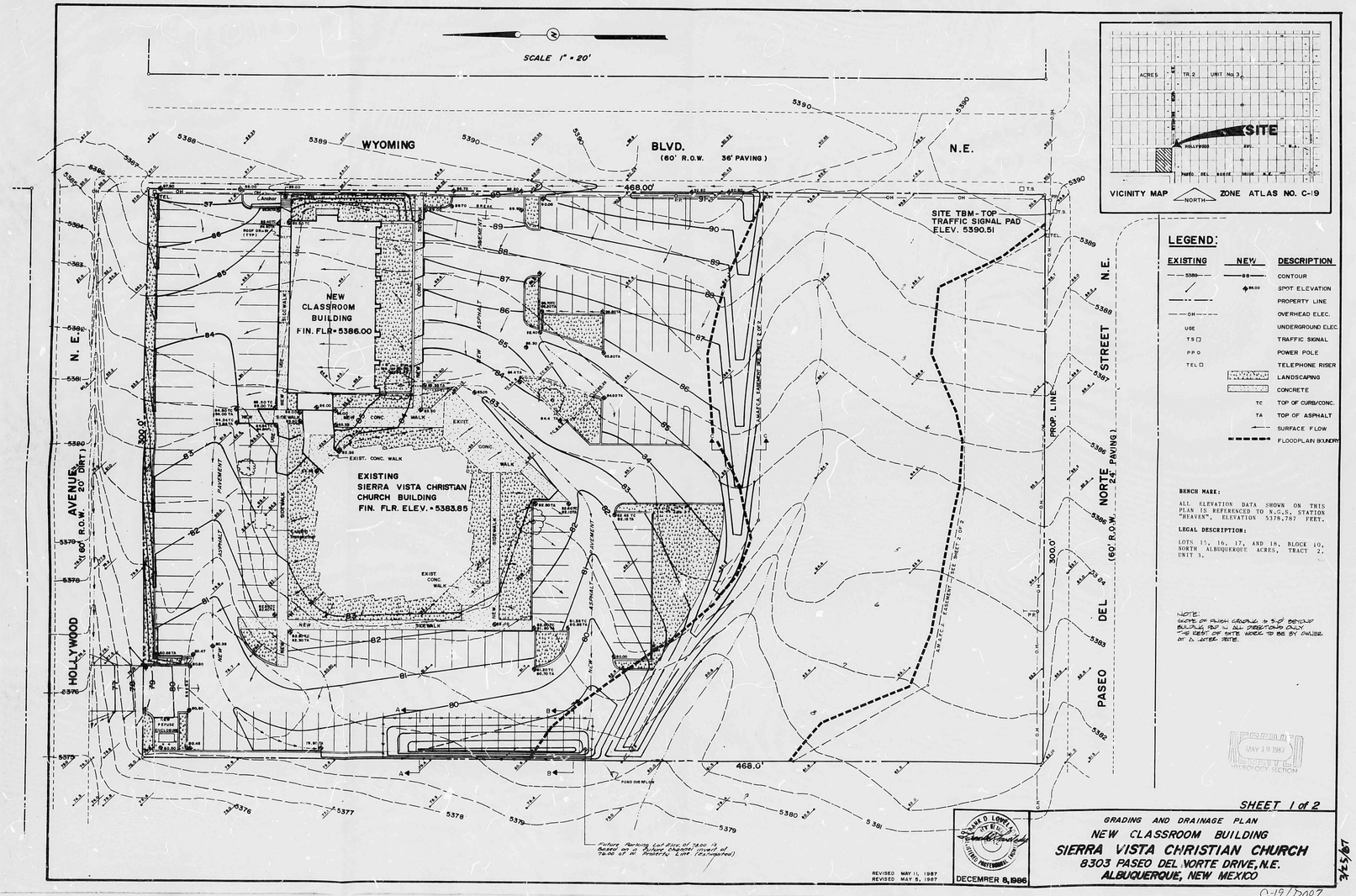
Hydrology File No. C-19/D007 1 of 1



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CITY OF ALBUQUERQUE

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PLANS/PLATS ON FILE

FILE DESC: C-19/D007

PLANS/PLATS 2

F 30 D 24 C 20 B 16 A 12

CITY OF ALBUQUERQUE

C D F

DRAINAGE CALCULATIONS

I. REFERENCES:

A. CITY OF ALBUQUERQUE DEVELOPMENT PROCESS MANUAL (DPM) VOLUME 2, DESIGN CRITERIA, CHAPTER 22.

B. FLOODWAY MAP, PANEL 10 OF 50, (FEMA), OCTOBER 14, 1983.

C. SOIL SURVEY OF BERNALILLO COUNTY, NEW MEXICO. U.S. SOIL CONSERVATION SERVICE. JUNE, 1977.

II. ON-SITE RUNOFF CHARACTERISTICS:

A. UNDEVELOPED RUNOFF COEFFICIENT:

TYPE OF SURFACE	AREA	"C"VALUE	AREA("C")
STREETS, DRIVES, WALKS	16314	0.95	15498
ROOFS	6018	0.90	5416
LAWNS & LANDSCAPING	350	0.25	88
UNDEVELOPED	117718	0.40	47087
TOTAL AREA	140400 SF = 3.223 ACRES		68089

WEIGHTED "C" VALUE = 68089 / 140400 = 0.49

PERCENT IMPERVIOUS = 16%

B. DEVELOPED RUNOFF COEFFICIENT:

TYPE OF SURFACE	AREA	"C"VALUE	AREA("C")
STREETS, DRIVES, WALKS	59334	0.95	56367
ROOFS	10876	0.90	9788
LAWNS & LANDSCAPING	19530	0.25	4883
UNDEVELOPED	50657	0.40	20262
TOTAL AREA	140400		91300

WEIGHTED "C" VALUE = 91300 / 140400 = 0.65

PERCENT IMPERVIOUS = 30 %

C. RAINFALL, 100-YR.,6-HR.:(REF. A., PLATE 22.2 D-1)
P_6 = 2.4 INCHES

D. TIME OF CONCENTRATION: TEN MINUTES.

E. RAINFALL INTENSITY,I (REF. A., PLATE 22.2 D-2)
$I=P_6 X 6.84(T_c)^{-0.51}=(2.4)6.84(10)^{-0.51}=5.07IN/HR$

F. SOIL TYPE (REF. I.C., SHEET NO. 11) SOIL IS EtC, EMBUDO TIJERAS COMPLEX, HYDROLOGIC SOIL GROUP "B".

G. RUNOFF CURVE NUMBER (CN)-(REF.I.A., PLATE 22.2 C-2) (SEE SECTION IV BELOW)

III. ON-SITE PEAK DISCHARGE BY RATIONAL EQUATION:

(NOTE: THIS IS THE DISCHARGE FOR THE ENTIRE SITE INCLUDING THE SOUTHERN PORTION OF THE SITE WHICH IS NOT BEING DEVELOPED.)

A. EXISTING CONDITIONS:
Q_{100} = 0.49 X 5.07 X 3.223 = 8.01 CFS
Q_{10} = 0.657 X 8.01 = 5.26 CFS

B. DEVELOPED CONDITIONS:
Q_{100} = 0.65 X 5.07 X 3.223 = 10.62 CFS
Q_{10} = 0.657 X 10.62 = 6.98 CFS

IV. ON-SITE VOLUME BY S.C.S. METHOD:

A. AREA:
CONSIDER ONLY THE AREA NORTH OF CHANNEL.

TOTAL SITE AREA	140,400
LESS UNDEVELOPED	50,660
DEVELOPED AREA	89,740

89,740 / 43560 = 2.06 ACRES

B. WEIGHTED CURVE NUMBER (CN)

TYPE OF SURFACE	AREA (SF)
ASPHALT PAVEMENT AND SIDEWALKS	39,334
ROOFS	10,876
LAWNS AND LANDSCAPING	19530

S.C.S. CURVE NUMBER FOR ROOFS AND PAVEMENT = 98
S.C.S. CURVE NUMBER FOR LANDSCAPING = 61

WEIGHTED CN $\frac{(39334 + 10876)98 + 19530 X 61}{89743}$ = 89.94 = 90

C. DIRECT RUNOFF
DIRECT RUNOFF IS EQUAL TO 1.5 INCHES (REF. I.A. PL 22.2 C-4)

D. VOLUME:
VOLUME = 89743 (1.5/12) = 11,218 CF

V. PEAK DISCHARGE, DEVELOPED AREA ONLY:

A. WEIGHTED "CV" FACTOR
"C_V" = $\frac{(39334 X 0.95) + (10876 X 0.90) + (19530 X 0.25)}{89743}$ = 0.79

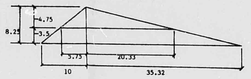
B. PEAK DISCHARGE:
Q = CIA = 0.79 X 5.07 X 2.06 = 8.25 CFS

VI. POND VOLUME BY HYDROGRAPH:

A. DURATION:
T = 2V/SQ = 2 X 11218 / 60 X 8.25 = 45.32 MINUTES.

B. ALLOWABLE DISCHARGE: USE 1.7 CFS PER ACRE IN ACCORDANCE WITH LA CUEVA HIGH SCHOOL LAND USE GUIDE.
1.7 X 2.06 = 3.50 CFS TOTAL ALLOWABLE DISCHARGE.

C. HYDROGRAPH:



D. POND VOLUME:
V = (3.75 + 20.33)4.75 X 0.5 X 60 = 3716 CF

VII. ACTUAL POND VOLUME:

SEE SECTIONS A-A AND B-B:

AREA OF CROSS SECTION A-A = 40.4 SF
AREA OF CROSS SECTION B-B = 49.45 SF
AVERAGE AREA = 44.93 SF

REQUIRED POND LENGTH = 3716 / 44.93 = 82.7 FEET.

VIII. OFF-SITE FLOW:

A. REFERENCE - REVIEW AND REFINEMENT OF NORTHEAST HEIGHTS DRAINAGE MANAGEMENT PLAN, NORTH DOMINGO BACA ARROYO, ALBUQUERQUE METROPOLITAN ARROYO FLOOD CONTROL AUTHORITY, MAY, 1980.

B. PEAK OFF-SITE FLOW:
NODE 25, NB-C2 400 CFS
NODE 35, NB-A5 140 CFS
TOTAL 540 CFS

C. 100-YEAR FLOODPLAIN:

THE 100-YEAR FLOODPLAIN WAS CALCULATED BY USING MANNING'S EQUATION AT SECTIONS TAKEN AT POINTS WHERE CONTOUR LINES CROSS THE FLOWLINE OF THE CHANNEL. THE FOLLOWING ASSUMPTIONS WERE MADE:

1. MANNING'S "N" = 0.35.

2. IT IS ASSUMED THE CHANNEL BOTTOM HAS A UNIFORM SLOPE BETWEEN THE CONTOUR AT THE CENTER OF THE CHANNEL AND THE NEXT HIGHER CONTOUR THAT THE SECTION INTERSECTS. THIS SLOPE IS CALCULATED FOR EACH SIDE OF THE CHANNEL. THE WIDTH OF FLOODPLAIN IS CALCULATED BY EXTENDING THIS SLOPE TO THE POINT OF INTERSECTION WITH THE REQUIRED FLOODPLAIN LEVEL FOR THE FLOW OF 540 CFS.

3. ON THE NORTH SIDE, THE DIKE ENCROACHES SLIGHTLY ON THE FLOODPLAIN AS CALCULATED IN THE MANNER DESCRIBED ABOVE. IT IS ASSUMED THAT THE LOSS OF CAPACITY OR EFFECT ON THE SOUTHERN FLOODPLAIN DUE TO THIS ENCROACHMENT IS NEGLIGABLE.

THE COMPUTATIONS WERE MADE BY TRIAL A'D ERROR USING AN HP41CV PROGRAM. EACH SIDE OF THE CHANNEL WAS CALCULATED SEPARATELY AND THE SUM OF THE TWO WAS COMPARED WITH THE REQUIRED PEAK FLOW OF 540 CFS. THE RESULTS ARE TABULATED BELOW:

SECTION NUMBER	AVE. SLOPE AT SECTION	TOTAL Q	LEFT SIDE OF CHANNEL Z	Z'	D	RIGHT SIDE OF CHANNEL Q	Z	Z'	D	Q
1.	0.0364	540	29	38	1.31	152	74	96	1.31	388
2.	0.0307	546	30	47	1.57	234	40	63	1.57	312
3.	0.0257	548	37	59	1.60	278	36	58	1.60	270
4.	0.0257	543	38	57	1.50	240	48	72	1.50	303
5.	0.0284	541	27	41	1.51	183	53	80	1.51	358
6.	0.0311	540	9.5	15	1.60	78	56	90	1.60	462
7.	0.0343	543	12	18	1.52	90	60	91	1.52	453
8.	0.0272	560	14	27	1.90	170	32	61	1.90	170
9.	0.0280	545	37	58	1.58	280	35	55	1.58	265

DEFINITION OF TERMS USED IN ABOVE TABULATION:

SECTION NUMBER - SEE SHEET 1 OF 2

AVE. SLOPE - AVERAGE OF THE SLOPE BETWEEN CONTOURS UPSTREAM AND DOWNSTREAM FROM SECTION.

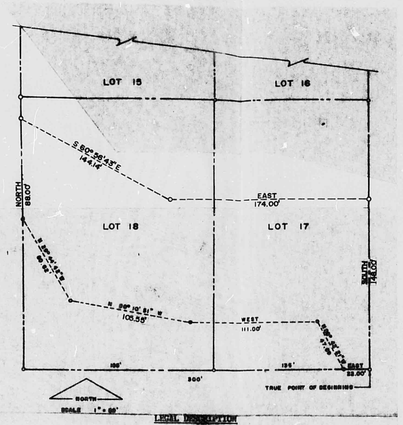
TOTAL Q - THE SUM OF THE FLOW AS CALCULATED FOR THE LEFT SIDE AND RIGHT SIDE OF THE CHANNEL, CFS.

Z - HORIZONATAL DISTANCE BETWEEN CONTOUR LINES MEASURED ALONG THE SECTION.

Z' - HORIZONTAL DISTANCE BETWEEN CENTER OF CHANNEL AND EDGE OF FLOODPLAIN, CALCULATED BY MULTIPLYING Z BY THE DEPTH D AT THE CHANNEL FLOWLINE.

D - DEPTH OF FLOW AT THE CHANNEL FLOWLINE FOR EACH SECTION.

A.M.A.F.C.A. EASEMENT



LEGAL DESCRIPTION

Beginning at the S.E. corner of Lot 17, Block 10, of North Albuquerque Acres Tract 2, Unit 3, which corner is the TRUE POINT OF BEGINNING;

THENCE, N. 90° 00' 00" W., a distance of 22.00 feet;

THENCE, N. 28° 42' 21" W., a distance of 47.89 feet;

THENCE, N. 90° 00' 00" W., a distance of 111.00 feet;

THENCE, N. 80° 10' 51" W., a distance of 105.55 feet;

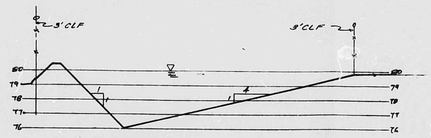
THENCE, N. 29° 44' 42" W., a distance of 80.62 feet;

THENCE, N. 00° 00' 00" W., a distance of 88.00 feet;

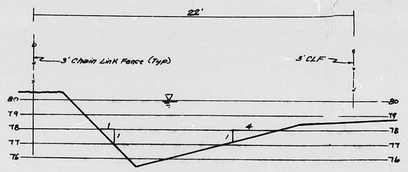
THENCE, S. 60° 56' 43" E., a distance of 144.14 feet;

THENCE, S. 90° 00' 00" E., a distance of 148.00' to the TRUE POINT OF BEGINNING, containing 0.9793 acres.

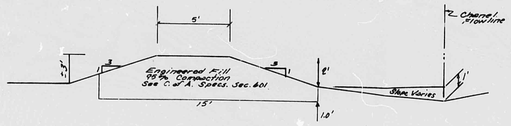
FLOODWAY MAP PANEL NO. 10 OF 50
NORTH



SECTION A-A POND
1/4" = 1'-0"



SECTION B-B POND
1/4" = 1'-0"



SECTION C-C DIKE
1/4" = 1'-0"

THIS PLAN SHOT TWICE ONCE ON LIGHT AND ONCE ON DARK.

CITY OF ALBUQUERQUE
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RECEIVED MAY 19 1987 HYDROLOGY SECTION

SHEET 2 of 2

RANK D. LOVE JR. NEW MEXICO 6512 REGISTERED PROFESSIONAL ENGINEER

DRAINAGE CALCULATIONS & OTHER DATA
NEW CLASSROOM BUILDING
SIERRA VISTA CHRISTIAN CHURCH
8303 PASEO DEL NORTE DRIVE, N.E.
ALBUQUERQUE, NEW MEXICO

DECEMBER 8,1986

REVISED MAY 11, 1987
REVISED MAY 5, 1987

C-14/D007

D 24 C 20 B 16 A 12 CITY OF ALBUQUERQUE C D

DRAINAGE CALCULATIONS

I. REFERENCES:

A. CITY OF ALBUQUERQUE DEVELOPMENT PROCESS MANUAL (DPM) VOLUME 2, DESIGN CRITERIA, CHAPTER 22.

B. FIRM/FIA MAP, PANEL 10 OF 50, DATED OCTOBER 16, 1982.

C. SOIL SURVEY OF BERNALILLO COUNTY, N.M. U.S. DEPT. OF SOIL CONSERVATION SERVICE, JAN. 1977.

II. ON-SITE RUNOFF CHARACTERISTICS:

A. UNDEVELOPED RUNOFF COEFFICIENT:

TYPE OF SURFACE	AREA	"C" VALUE	AREA (%)
STREETS, DRIVES, WALKS	(ACS)	(C)	(%)(C)
ROOFS	(ACS)	(C)	(%)(C)
LAWNS & LANDSCAPING	(ACS)	(C)	(%)(C)
UNDEVELOPED	(ACS)	(C)	(%)(C)
TOTAL AREA	(ACS)		(%)(C)

WEIGHTED "C" VALUE = (%)(C) = (%)(C) = (%)(C)

PERCENT IMPERVIOUS = (%)

B. DEVELOPED RUNOFF COEFFICIENT:

TYPE OF SURFACE	AREA	"C" VALUE	AREA (%)
STREETS, DRIVES, WALKS	(ACS)	(C)	(%)(C)
ROOFS	(ACS)	(C)	(%)(C)
LAWNS & LANDSCAPING	(ACS)	(C)	(%)(C)
UNDEVELOPED	(ACS)	(C)	(%)(C)
TOTAL AREA	(ACS)		

WEIGHTED "C" VALUE = (%)(C) = (%)(C) = (%)(C)

PERCENT IMPERVIOUS = (%)

C. RAINFALL INTENSITY: I_{6-HR} (REF. A, FIGURE 22.2 (6-HR)) $I_{6} = 2.$ (INCHES)

D. TIME OF CONCENTRATION: TEN MINUTES

E. RAINFALL INTENSITY: (REF. A, FIGURE 22.2 (6-HR))

$I = I_{6}/6.84 (T^{-0.5}) = (__)/6.84 (__^{-0.5}) = __$ (INCHES PER HR)

F. SOIL TYPE (REF. C, SHEET NO. ; SOIL TYPE: IMMOND TIFFRAS COMPLEX, HYDROLOGIC SOIL GROUP "B")

G. RUNOFF CURVE NUMBER (CN) (REF. A, FIGURE 22.5 (CN)) SEE SECTION IV BELOW

III. ON-SITE PEAK DISCHARGE BY RATIONAL EQUATION:

NOTE: THIS IS THE DISCHARGE FOR THE ENTIRE SITE INCLUDING THE SOUTHERN PORTION OF THE SITE WHICH IS NOT being DEVELOPED.

A. EXISTING CONDITIONS:
$Q_{100} = C_{100} \times I_{100} \times A = 0.41 \times 5.07 \times 2.22 = 4.61$ CFS
$Q_{10} = C_{10} \times I_{10} \times A = 0.07 \times 4.02 \times 2.22 = 0.62$ CFS

B. DEVELOPED CONDITIONS:
$Q_{100} = C_{100} \times I_{100} \times A = 0.63 \times 5.07 \times 2.22 = 10.62$ CFS
$Q_{10} = C_{10} \times I_{10} \times A = 0.37 \times 4.02 \times 2.22 = 4.96$ CFS

IV. ON-SITE VOLUME BY S.C.S. METHOD:

A. AREA:
CONSIDER ONLY THE AREA NORTH OF CHANNEL.
TOTAL SITE AREA 140,000
LESS UNDEVELOPED 50,260
DEVELOPED AREA 89,740 / 43560 = 2.06 ACRES

B. WEIGHTED CURVE NUMBER (CN)

TYPE OF SURFACE	AREA (S.F.)
STREETS, DRIVEWAY AND SIDEWALKS	39,935
ROOFS	22,435
LAWNS AND LANDSCAPING	27,370

S.C.S. CURVE NUMBER FOR ROOFS AND PAVEMENT = 98
S.C.S. CURVE NUMBER FOR LANDSCAPING = 61
WEIGHTED CN = (39,935 + 22,435) / 89,740 = (98 x 62,370 / 89,740) = 68.1

C. DIRECT RUNOFF:
DIRECT RUNOFF IN REAL TERMS (INCHES) (REF. A, EQ. 22.2 (Q))

D. VOLUME:
VOLUME = (Q)(A) = (0.7)(89,740) = 52,353 CF

V. PEAK DISCHARGE, DEVELOPED AREA ONLY:

A. WEIGHTED "C" FACTOR
"C" = (.95)(39,935 / 89,740) + (.35)(22,435 / 89,740) + (.35)(27,370 / 89,740) = 0.63

B. PEAK DISCHARGE:
Q = CIA = (0.63) x 5.07 x 2.06 = 8.23 CFS

VI. POND VOLUME BY HYDROGRAPH:

A. DURATION:
T = 2V/Q = 2 x 12218 / 60 x 8.23 = 4.9 MINUTES.

B. ALLOWABLE DISCHARGE: USE 12.7 CFS PER ACRE IN ACCORDANCE WITH A 100 YEAR DEVELOPED LAND USE SITE.
12.7 CFS x 2.06 = 26.15 CFS (TOTAL ALLOWABLE DISCHARGE).

C. INTERPOLATING:

D. POND VOLUME:
V = (T/2) x (Q - 2Q) = (4.9/2) x (8.23 - 2 x 26.15) = -215 CF

VII. ACTUAL POND VOLUME:

SEE SECTION A-A AND B-B:

AREA OF POND SECTION A-A = 4,114 SF
AREA OF POND SECTION B-B = 4,114 SF
TOTAL AREA = 8,228 SF
PROJECTED POND LENGTH = 2,7[illegible] = 44.[illegible] x 44.[illegible] (FEET)

VIII. ON-SITE PLAN:

A. REFERENCE: "REVISION AND REEXAMINATION OF NORTHEAST HEIGHTS DRAINAGE MANAGEMENT PLAN, NORTH DOMESTIC AREA AND NORTH ALBUQUERQUE METROPOLITAN ARROYO FLOOD CONTROL AUTHORITY", MAY, 1986.

B. PEAK ON-SITE FLOW:
NORTH 100-YR. = 5.07 CFS
WEST 100-YR. = 3.40 CFS
TOTAL = 8.47 CFS

C. ARROYO FLOODPLAIN:

THE 100-YEAR FLOODPLAIN WAS CALCULATED BY USING MANNING'S EQUATION AT SECTIONS TAKEN AT POINTS WHERE CONTOUR LINES CROSS THE FLOWLINE OF THE CHANNEL. THE FOLLOWING ASSUMPTIONS WERE MADE:

1. MANNING'S "N" = 0.35.

2. IT IS ASSUMED THE CHANNEL BOTTOM HAS A UNIFORM SLOPE BETWEEN THE CONTOUR AT THE CENTER OF THE CHANNEL AND THE NEXT HIGHER CONTOUR THAT THE SECTION INTERSECTS. THIS SLOPE IS CALCULATED FOR EACH SIDE OF THE CHANNEL. THE WIDTH OF FLOODPLAIN IS CALCULATED

BY EXTENDING THIS SLOPE TO THE POINT OF INTERSECTION WITH THE VIRGINIAN FLOODPLAIN LEVEL FOR THE SIDE OF THE CHANNEL.

3. ON THE NORTH SIDE, THE FIRM ELEVATIONS SLIGHTLY ON THE FLOODPLAIN AS CALCULATED IN THE MANNER DESCRIBED ABOVE. IT IS ASSUMED THAT THE LOSS OF CAPACITY ON EFFECT ON THE SOUTHERN FLOODPLAIN DUE TO THIS ENCROACHMENT IS NEGLIGIBLE.

THE COMPUTATIONS WERE MADE BY TRIAL AND ERROR USING AN HP41CV PROGRAM. EACH SIDE OF THE CHANNEL WAS CONSIDERED SEPARATELY AND THE SUM OF THE TWO WAS COMPARED WITH THE PEAK 100-YR. FLOW OF 5.07 CFS. TWO RESULTS WERE TABULATED BELOW.

SECTION A-A: SLOPE TOTAL LEFT SIDE OF CHANNEL; RIGHT SIDE OF CHANNEL

NUMBER	ELEVATION	"N"	"A"	"P"	"R"	"R"	"S"	"Q"
1.	5[illegible]	.35	[illegible]	[illegible]	[illegible]	[illegible]	[illegible]	[illegible]
2.	5[illegible]	.35	[illegible]	[illegible]	[illegible]	[illegible]	[illegible]	[illegible]
3.	5[illegible]	.35	[illegible]	[illegible]	[illegible]	[illegible]	[illegible]	[illegible]
4.	5[illegible]	.35	[illegible]	[illegible]	[illegible]	[illegible]	[illegible]	[illegible]
5.	5[illegible]	.35	[illegible]	[illegible]	[illegible]	[illegible]	[illegible]	[illegible]
6.	5[illegible]	.35	[illegible]	[illegible]	[illegible]	[illegible]	[illegible]	[illegible]
7.	5[illegible]	.35	[illegible]	[illegible]	[illegible]	[illegible]	[illegible]	[illegible]
8.	5[illegible]	.35	[illegible]	[illegible]	[illegible]	[illegible]	[illegible]	[illegible]
9.	5[illegible]	.35	[illegible]	[illegible]	[illegible]	[illegible]	[illegible]	[illegible]
10.	5[illegible]	.35	[illegible]	[illegible]	[illegible]	[illegible]	[illegible]	[illegible]

DEFINITION OF TERMS USED IN ABOVE TABULATION:

SECTION NUMBER - SEE SHEET 1 OF

"S" - SLOPE - AVERAGE OF THE SLOPE BETWEEN CONTOURS UPSTREAM AND DOWNSTREAM FROM SECTION.

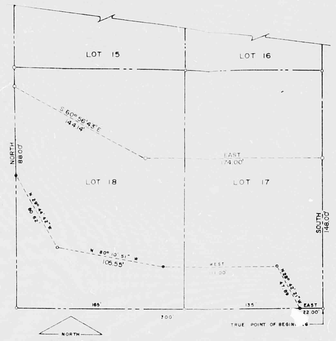
TOTAL Q - THE SUM OF THE FLOW AS CALCULATED FOR THE LEFT SIDE AND RIGHT SIDE OF THE CHANNEL, CFS.

1. HORIZONTAL DISTANCE BETWEEN CONTOUR LINES MEASURED ON THE SECTION.

2. VERTICAL DISTANCE BETWEEN CENTER OF CHANNEL AND THE 100-YR FLOODPLAIN, ASSUMED TO BE EQUIVALENT TO THE DEPTH OF FLOW IN THE CHANNEL.

3. DEPTH OF FLOW AT THE CHANNEL FLOWLINE FOR EACH SECTION.

A.M.A.F.C.A. EASEMENT



LEGAL DESCRIPTION

Beginning at the S.E. corner of Lot 17, Block 10, of North Albuquerque Acres Tract 2, Unit 3, which corner is the TRUE POINT OF BEGINNING;

THENCE, N. 90° 00' 00" W., a distance of 22.00 feet;

THENCE, N. 28° 42' 17" W., a distance of 47.[illegible] feet;

THENCE, N. 90° 00' 00" W., a distance of 11.[illegible] feet;

THENCE, N. 80° 10' 37" W., a distance of 10[illegible] feet;

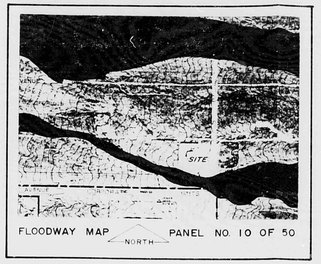
THENCE, N. 20° 44' [illegible]" W., a distance of 50.[illegible] feet;

THENCE, N. 00° 00' 00" W., a distance of 88.[illegible] feet;

THENCE, N. 42° 56' 4[illegible]" E., a distance of 146.[illegible] feet;

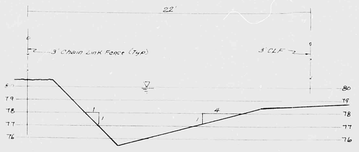
THENCE, N. 90° 00' 00" E., a distance of 174.[illegible] feet;

THENCE, N. 00° 00' 00" E., a distance of 148.00' to the TRUE POINT OF BEGINNING, containing 0.97[illegible] acres.



FLOODWAY MAP PANEL NO. 10 OF 50

SECTION A-A POND
1/4" = 1'-0"



SECTION B-B POND
1/4" = 1'-0"

SECTION C-C DIKE
1/4" = 1'-0"

SHEET 2 of 2

REVISED MAY 11, 1987
REVISED MAY 5, 1987

DECEMBER 8, 1986

DRAINAGE CALCULATIONS & OTHER DATA
NEW CLASSROOM BUILDING
SIERRA VISTA CHRISTIAN CHURCH
8303 PASEO DEL NORTE DRIVE, N.E.
ALBUQUERQUE, NEW MEXICO

THIS PLAN SHOT TWICE
ONCE ON LIGHT AND
ONCE ON DARK.

CITY OF ALBUQUERQUE
THIS MICROIMAGE IS THE BEST POSSIBLE REPRODUCTION DUE TO THE POOR QUALITY OF THE ORIGINAL DOCUMENT.

NO. 21 DATE 2-23-87 TO 2-27-87

CITY OF ALBUQUERQUE

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