

TO EAGLE ROCK AVENUE AND WILL DRAIN TO THE NORTH LA CUEVA CHANNEL.

PROVIDED ON EACH LOT. THE FIRST FLUSH VOLUME FOR A 0.44-INCH RAIN IS

THE FIRST FLUSH FROM ON-SITE BASIN 1 WILL BE RETAINED IN THE PONDS

304 CUBIC—FEET.

LOT 4-P1, TOP AREA=312 SF, BOTTOM AREA=120 SF, DEPTH= 1.0'

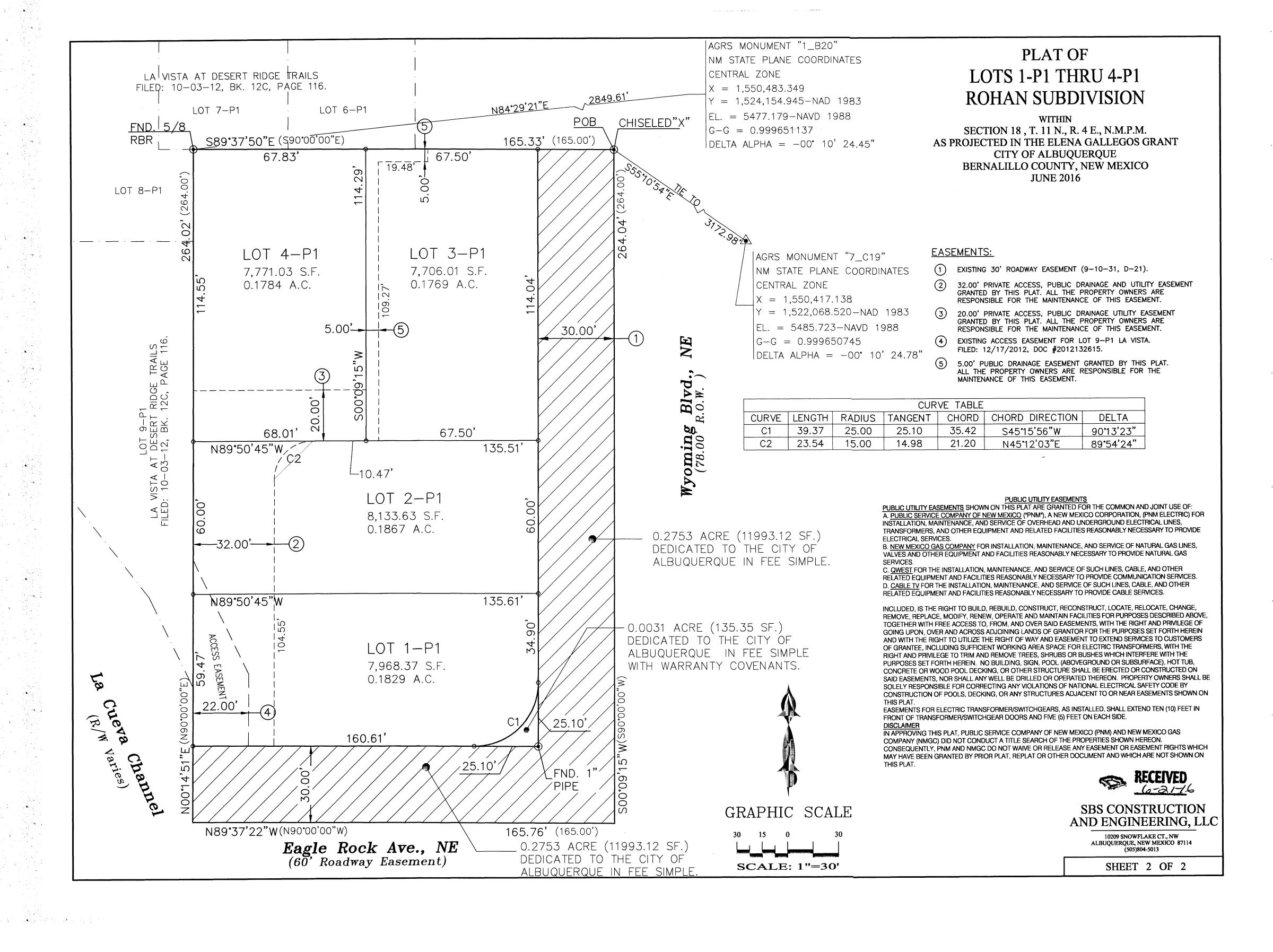
PONDING VOLUME PROVIDED = (312 + 120)/2 * 1 = 216.00 CF

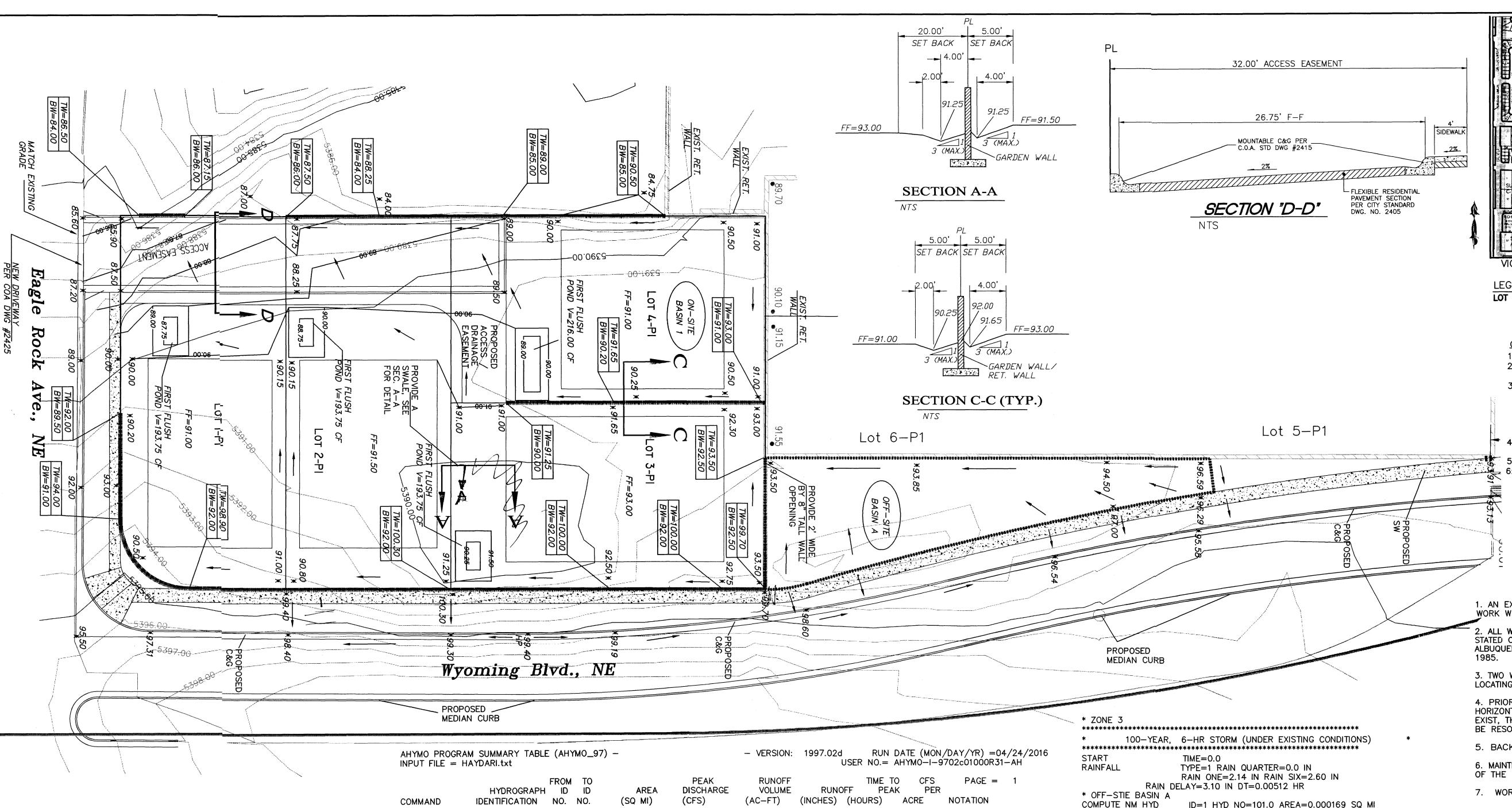
GRADING AND DRAINAGE PLAN DRAWN BY: DRAWING: DATE: SHEET# 201605-GR.DWG SH-B 03-22-2016

LAST REVISION: 06-10-10

TP=0.1333 HR MASS RAINFALL=-1

FINISH





DRAINAGE PLAN:

LEGAL DESCRIPTION: LOT 17, BLOCK 1, NAA TRACT 2, UNIT 3

SITE AREA: 0.7277 ACRES

FLOOD HAZARD STATEMENT: F.E.M.A. FLOODWAY BOUNDARY AND FLOODWAY MAP DATED AUGUST 16, 2012 (PANEL NO. 35001C0133G & 35001C0141G) INDICATES A FLOOD HAZARD ZONE X WHICH IS AN AREA DETERMINED TO BE OUTSIDE THE 500-YEAR FLOODPLAIN. THE NORTH LA CUEVA CHANNEL AT THE SOUTHWEST CORNER HAS A FLOOD HAZARD ZONE A WITH A 1% ANNUAL CHANCE FLOOD DISCHARGE CONTAINED IN CHANNEL

LOCATION AND DESCRIPTION: THE LOT IS LOCATED ON EAGLE ROCK AVENUE WEST OF WYOMING BOULEVARD. AT THE SOUTHWEST CORNER OF THE PROPERTY IS THE NORTH LA CUEVA CHANNEL.

EXISTING DRAINAGE CONDITIONS:

THE DRAINAGE ANALYSIS FOR THIS SITE IS IN ACCORDANCE WITH THE CITY OF ALBUQUERQUE DEVELOPMENT PROCESS MANUAL SECTION 22.2, HYDROLOGY. THE PROPERTY IS LOCATED IN ZONE 3. THE 100-YEAR, 6-HOUR STORM IS 2.60 INCHES.

THE SITE CURRENTLY DRAINS FROM EAST TO WEST TO THE NORTH LA CUEVA CHANNEL. CURRENTLY THERE ARE OFFSITE FLOWS FROM THE EAST AND

DEVELOPED DRAINAGE CONDITIONS:

THERE IS ONE OFF-SITE BASINS DRAINING ONTO LOT 17. OFF-SITE BASIN A IS LOCATED NORTH OF LOT 17. RUNOFF FROM OFF-SITE BASIN B WILL DRAIN TO THE POND PROVIDED JUST NORTH OF THE LOT 17, 377.50 CFS. THE PEAK FLOW OF 0.71 CFS WILL BE CONVEYED TO THE POND

RUNOFF FROM ON-SITE BASIN 1, WHICH INCLUDES ALL OF LOT 17, WILL BE DIRECTED TO EAGLE ROCK AVENUE THROUGH THE PAVED DRIVEWAY. THE ROOF OF THE BUILDINGS WILL DRAIN TO THE WEST AND EAST. EACH LOT WILL HAVE A PONDING AREA FOR THE FIRST FLUSH. AFTER SENDING THE WATER INTO THE PONDS, THEN THE FLOWS WILL BE DIRECTD INTO PAVED RAOD AND FINALLY INTO EAGLE ROACK. A TOTAL OF 2.63 CFS FROM LOT 17 WILL DRAIN TO EAGLE ROCK AVENUE AND WILL DRAIN TO THE NORTH LA CUEVA CHANNEL. THE FIRST FLUSH FROM ON-SITE BASIN 1 WILL BE RETAINED IN THE PONDS PROVIDED ON EACH LOT. THE FIRST FLUSH VOLUME FOR A 0.44-INCH RAIN IS 304 CUBIC-FEET.

	HYDROGRAPH		ID	AREA	PEAK DISCHARGE	RUNOFF VOLUME	RUNOFF	TIME TO PEAK	PER	PAGE	
COMMAND	IDENTIFICATION	NO.	NO.	(SQ MI)	(CFS)	(AC-FT)	(INCHES) (H	OURS)	ACRE	NOTATIO	N
START RAINFALL TYPE= COMPUTE NM HYE	101.00	_	1	.00017	.30	.008	.92230	1.526	2.731 Pi	TIME= RAIN6= ER IMP=	.00 2.600 .00
COMPUTE NM HYD START RAINFALL TYPE=			1	.00114	1.93	.056	.92024	1.526		ER IMP= TIME= RAIN6=	.00 .00 1.730
COMPUTE NM HYD		_	1	.00017	.13	.003	.35547	1.533	1.209 PE	R IMP=	.00
COMPUTE NM HYD		-	1	.00114	1.43	.037	.60463	1.500		R IMP= TIME=	.00
RAINFALL TYPE= COMPUTE NM HYD START		_	1	.00114	3.18	.117	1.93606	1.500	4.368 PE	TIME=	2.600 68.00 .00
RAINFALL TYPE= COMPUTE NM HYD		_	1	.00114	1.99	.070	1.15661	1.500	2.739 PE	RAIN6= R IMP=	1.730 68.00

REQUIRED FIRST FLUSH PONDING CALCULATIONS

LOT 1-P1, IMPERVIOUS AREA = 5,430 CF

PONDING VOLUME REQUIRED = $5,430 \times 0.34/12 = 153.85$ CF

LOT 2-P1, IMPERVIOUS AREA = 5,770 CF PONDING VOLUME REQUIRED = $5,770 \times 0.34/12 = 163.48 \text{ CF}$

LOT 3-P1, IMPERVIOUS AREA = 4,547.50 CF

PONDING VOLUME REQUIRED = $4,547.50 \times 0.34/12 = 153.85 \text{ CF}$

LOT 4-P1, IMPERVIOUS AREA = 5,142.50 CF

PONDING VOLUME REQUIRED = $5,142.50 \times 0.34/12 = 145.70 \text{ CF}$

PROPOSED FIRST FLUSH PONDING VOLUME CALCULATIONS

LOT 1-P1, TOP AREA=260 SF, BOTTOM AREA=50 SF, DEPTH= 1.25' PONDING VOLUME PROVIDED = (260 + 50)/2 * 1 = 193.75 CF LOT 2-P1, TOP AREA=260 SF, BOTTOM AREA=50 SF, DEPTH= 1.25'

PONDING VOLUME PROVIDED = (260 + 50)/2 * 1 = 193.75 CF LOT 3-P1, TOP AREA=260 SF, BOTTOM AREA=50 SF, DEPTH= 1.25' PONDING VOLUME PROVIDED = (260 + 50)/2 * 1 = 193.75 CF

LOT 4-P1, TOP AREA=312 SF, BOTTOM AREA=120 SF, DEPTH= 1.0' PONDING VOLUME PROVIDED = (312 + 120)/2 * 1 = 216.00 CF

PER A=0.00 PER B=100.00 PER C=0.00 PER D=0.00 TP=0.1333 HR MASS RAINFALL=-1 * ON-SITE

ID=1 HYD NO=103.0 AREA=0.001137 SQ MI

PER A=0.00 PER B=0.00 PER C=100.00 PER D=0.00

PER A=0.00 PER B=100.00 PER C=0.00 PER D=0.00 TP=0.1333 HR MASS RAINFALL=-1 10-YEAR, 6-HR STORM (UNDER EXISTING CONDITIONS)

START RAINFALL TYPE=1 RAIN QUARTER=0.0 IN RAIN ONE=1.43 IN RAIN SIX=1.73 IN

RAIN DAY=2.07 IN DT=0.03333 HR * OFF-STIE BASIN A COMPUTE NM HYD ID=1 HYD NO=111.0 AREA=0.000169 SQ MI

PER A=0.00 PER B=100.00 PER C=0.00 PER D=0.00 TP=0.1333 HR MASS RAINFALL=-1 * ON-SITE COMPUTE NM HYD ID=1 HYD NO=113.0 AREA=0.001137 SQ MI

TP=0.1333 HR MASS RAINFALL=-1 100-YEAR, 6-HR STORM (UNDER PROPOSED CONDITIONS)

START TIME=0.0

RAINFALL TYPE=1 RAIN QUARTER=0.0 IN RAIN ONE=2.14 IN RAIN SIX=2.60 IN RAIN DELAY=3.10 IN DT=0.03333 HR * ON-SITE

COMPUTE NM HYD ID=1 HYD NO=103.1 AREA=0.001137 SQ MI PER A=0.00 PER B=20.00 PER C=12.00 PER D=68.00 TP=0.1333 HR MASS RAINFALL=-1

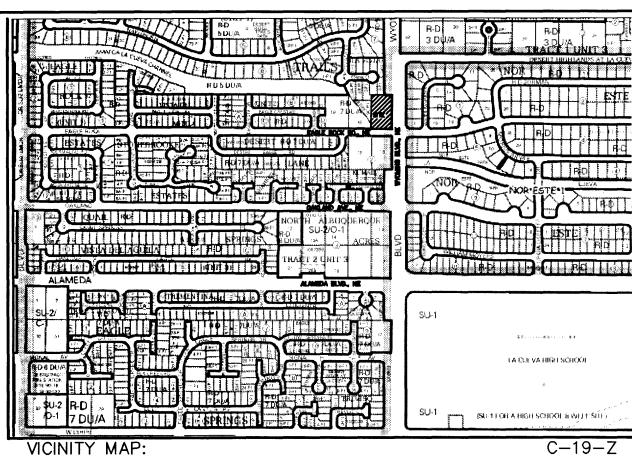
10-YEAR, 6-HR STORM (UNDER PROPOSED CONDITIONS) START

RAINFALL TYPE=1 RAIN QUARTER=0.0 IN RAIN ONE=1.43 IN RAIN SIX=1.73 IN RAIN DAY=2.07 IN DT=0.03333 HR

* ON-SITE COMPUTE NM HYD ID=1 HYD NO=113.1 AREA=0.001137 SQ MI PER A=0.00 PER B=20.00 PER C=12.00 PER D=68.00 TP=0.1333 HR MASS RAINFALL=-1

FINISH

COMPUTE NM HYD



LEGAL DESCRIPTION: LOT 17, BLOCK 1, NAA, TRACT 2, UNIT 3, CONTAINING 0.7277 ACRE

GENERAL NOTES:

1: CONTOUR INTERVAL IS HALF (1.00) FOOT 2: ELEVATIONS ARE BASED ON CITY OF ALBUQUERQUE CONTROL STATION

9_D15, HAVING AN ELEVATION OF 4988.802 FEET ABOVE SEA LEVEL 3: UTILITIES SHOWN HEREON ARE IN THEIR APPROXIMATE LOCATION BASED ONLY ON ABOVE GROUND EVIDENCE FOUND IN THE FIELD AND AS-BUILT INFORMATION PROVIDED BY THE CLIENT. UTILITIES SHOWN HEREON, WHETHE INDICATED AS ABANDONED OR NOT, SHALL BE VERIFIED BY OTHERS FOR EXACT LOCATION AND/ OR DEPTH PRIOR TO EXCAVATION OR DESIGN CON

4: THIS IS <u>NOT</u> A BOUNDARY SURVEY, BEARINGS ARE ASSUMED, DISTANCES AND FOUND PROPERTY CORNERS ARE FOR INFORMATIONAL PURPOSES ONL' SLOPES ARE AT 3:1 MAXIMUM.

IF THE DRIVEWAY LOCATIONS CONFLICTS WITH THE PROPOSED LOCATION OF A FIRST FLUSH PONDS, CONTRACTOR MUST RELOCATE THE PONDS WITH THE SAME VOLUME ELSEWHERE ON THE LOT, EXCEPT IN THE REAR LOT

NOTICE TO CONTRACTORS

 AN EXCAVATION/CONSTRUCTION PERMIT WILL BE REQUIRED BEFORE BEGINNING ANY WORK WITHIN CITY RIGHT-OF-WAY.

ALL WORK DETAILED ON THESE PLANS TO BE PERFORMED, EXCEPT AS OTHERWISE STATED OR PROVIDED HEREON, SHALL BE CONSTRUCTED IN ACCORDANCE WITH CITY OF ALBUQUERQUE INTERIM STANDARD SPECIFICATIONS FOR PUBLIC WORKS CONSTRUCTION

3. TWO WORKING DAYS PRIOR TO ANY EXCAVATION, CONTRACTOR MUST CONTACT LINE LOCATING SERVICE, 765-1234, FOR LOCATION OF EXISTING UTILITIES.

4. PRIOR TO CONSTRUCTION, THE CONTRACTOR SHALL EXCAVATE AND VERIFY THE HORIZONTAL AND VERTICAL LOCATIONS OF ALL CONSTRUCTIONS. SHOULD A CONFLICT EXIST, THE CONTRACTOR SHALL NOTIFY THE ENGINEER SO THAT THE CONFLICT CAN BE RESOLVED WITH A MINIMUM AMOUNT OF DELAY.

5. BACKFILL COMPACTION SHALL BE ACCORDING TO TRAFFIC/STREET USE. 6. MAINTENANCE OF THESE FACILITIES SHALL BE THE RESPONSIBILITY OF THE OWNER OF THE PROPERTY SERVED.

7. WORK ON ARTERIAL STREETS SHALL BE PERFORMED ON A 24-HOUR BASIS.

LEGEND

_____5100_____ EXISTING CONTOUR (MAJOR) EXISTING CONTOUR (MINOR) BOUNDARY LINE PROPOSED SPOT ELEVATION ¥ *85.46*

EXISTING GRADE



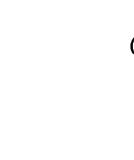
PROPOSED RET. WALL

(11814)

REZA AFAGHPOUR

P.E. #11814

LAST REVISION: 06-10-16



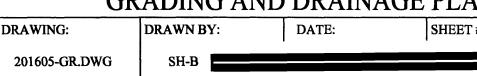
GRAPHIC SCALE

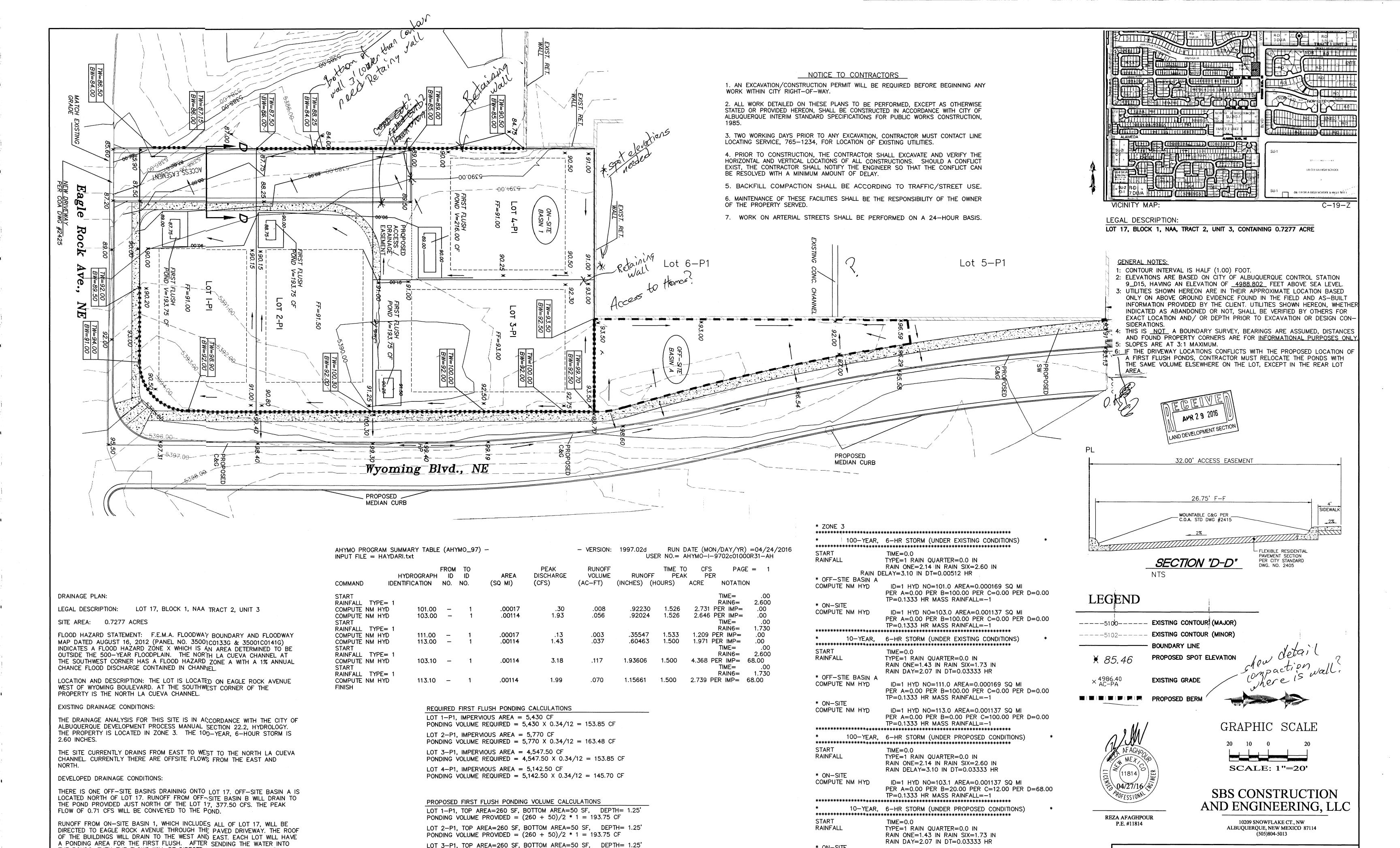
SBS CONSTRUCTION AND ENGINEERING, LLC

SCALE: 1"=20'

10209 SNOWFLAKE CT., NW ALBUQUERQUE, NEW MEXICO 87114 (505)804-5013

LOTS 1 THRU 4, ROHAN SUBDIVISION GRADING AND DRAINAGE PLAN DRAWN BY:





PONDING VOLUME PROVIDED = (260 + 50)/2 * 1 = 193.75 CF

PONDING VOLUME PROVIDED = (312 + 120)/2 * 1 = 216.00 CF

LOT 4-P1, TOP AREA=312 SF, BOTTOM AREA=120 SF, DEPTH= 1.0'

THE PONDS, THEN THE FLOWS WILL BE DIRECTD INTO PAVED RAOD AND

304 CUBIC-FEET.

FINALLY INTO EAGLE ROACK. A TOTAL OF 2.63 CFS FROM LOT 17 WILL DRAIN

TO EAGLE ROCK AVENUE AND WILL DRAIN TO THE NORTH LA CUEVA CHANNEL.

PROVIDED ON EACH LOT. THE FIRST FLUSH VOLUME FOR A 0.44-INCH RAIN IS

THE FIRST FLUSH FROM ON-SITE BASIN 1 WILL BE RETAINED IN THE PONDS

* ON-SITE

FINISH

COMPUTE NM HYD

ID=1 HYD NO=113.1 AREA=0.001137 SQ MI

TP=0.1333 HR MASS RAINFALL=-1

PER A=0.00 PER B=20.00 PER C=12.00 PER D=68.00

LOTS 1 THRU 4, ROHAN SUBDIVISION GRADING AND DRAINAGE PLAN DRAWING: DRAWN BY: DATE: SHEET# 201605-GR.DWG 03-22-2016

DRAINAGE PLAN:

LEGAL DESCRIPTION: LOT 17, BLOCK 1, NAA TRACT 2, UNIT 3

SITE AREA: 0.7277 ACRES

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RUNOFF FROM ON-SITE BASIN 1, WHICH INCLUDES ALL OF LOT 17, WILL BE DIRECTED TO EAGLE ROCK AVENUE THROUGH THE PAVED DRIVEWAY. THE ROOF OF THE BUILDINGS WILL DRAIN TO THE WEST AND EAST. EACH LOT WILL HAVE A PONDING AREA FOR THE FIRST FLUSH. AFTER SENDING THE WATER INTO THE PONDS, THEN THE FLOWS WILL BE DIRECTD INTO PAVED RAOD AND FINALLY INTO EAGLE ROACK. A TOTAL OF 2.63 CFS FROM LOT 17 WILL DRAIN TO EAGLE ROCK AVENUE AND WILL DRAIN TO THE NORTH LA CUEVA CHANNEL. THE FIRST FLUSH FROM ON-SITE BASIN 1 WILL BE RETAINED IN THE PONDS PROVIDED ON EACH LOT. THE FIRST FLUSH VOLUME FOR A ().44-INCH RAIN IS 304 CUBIC-FEET.

* * ZONE 3 100-YEAR. 6-HR STORM (UNDER EXISTING CONDITIONS)

START TYPE=1 RAIN QUARTER=0.0 IN RAIN ONE=2.14 IN RAIN SIX=2.60 IN RAINFALL

RAIN DELAY=3.10 IN DT=0.00512 HR * OFF-STIE A COMPUTE NM HYD

ID=1 HYD NO=101.0 AREA=0.000396 SQ MI PER A=0.00 PER B=100.00 PER C=0.00 PER D=0.00 TP=0.1333 HR MASS RAINFALL=-1

* OFF-SITE B COMPUTE NM HYD ID=1 HYD NO=102.0 AREA=0.000592 SQ MI PER A=0.00 PER B=100.00 PER C=0.00 PER D=0.00 TP-0.1333 HR MASS RAINFALL=-1 * ON-SITE ID=1 HYD NO=103.0 AREA=0.001137 SQ MI COMPUTE NM HYD

PER A=0.00 PER B=100.00 PER C=0.00 PER D=0.00 TP=0.1333 HR MASS RAINFALL=-1

10-YEAR, 6-HR STORM (UNDER EXISTING CONDITIONS) START TIME=0.0

TYPE=1 RAIN QUARTER=0.0 IN RAINFALL RAIN ONE=1.43 IN RAIN SIX=1.73 IN RAIN DAY=2.07 IN DT=0.03333 HR * OFF-STIE A

COMPUTE NM HYD ID=1 HYD NO=111.C) AREA=0.000396 SQ MI PER A=0.00 PER B=100.00 PER C=0.00 PER D=0.00 TP=0.1333 HR MASS RAINFALL=-1

* OFF-SITE B ID=1 HYD NO=112.6 AREA=0.000592 SQ MI COMPUTE NM HYD PER A=0.00 PER B=100.00 PER C=0.00 PER D=0.00 TP=0.1333 HR MASS RAINFALL=-1

* ON-SITE ID=1 HYD NO=113.() AREA=0.001137 SQ MI COMPUTE NM HYD PER A=0.00 PER B=0.00 PER C=100.00 PER D=0.00 TP=0.1333 HR MASS RAINFALL=-1

*********** 100-YEAR, 6-HR STORM (UNDER PROPOSED CONDITIONS) START TYPE=1 RAIN QUARTER=0.0 IN RAINFALL

RAIN ONE=2.14 IN RAIN SIX=2.60 IN RAIN DELAY=3.10 IN DT=0.03333 HR * ON-SITE ID=1 HYD NO=103.1 AREA=0.001137 SQ MI COMPUTE NM HYD PER A=0.00 PER B=0.00 PER C=27.00 PER D=46.00

10-YEAR, 6-HR STORM (UNDER PROPOSED CONDITIONS) ***********************

START TYPE=1 RAIN QUARTER=0.0 IN RAINFALL RAIN ONE=1.43 IN RAIN SIX=1.73 IN RAIN DAY=2.07 IN DT=0.03333 HR ID=1 HYD NO=113.1 AREA=0.001137 SQ MI COMPUTE NM HYD PER A=0.00 PER B=0.00 PER C=27.00 PER D=46.00 TP=0.1333 HR MASS RAINFALL=-1

TP=0.1333 HR MASS RAINFALL=-1

*********************** FINISH

BASIN A 5/388.84 TOE 5390.98 POND V=377.50 CF 398\63 5387.29 ~EP 5399.36 EXIST. RET. ----91.00 -—89.00-₩ 5391.43 91.50 🎢 X 91.00 90.50 X X 92.00 EXIST. RET. TOE 5395.03 TW=98.00 BW=92.00 5389.95 EP 5399.86 5387,69 Lot 3-PI TONP 15131991221 TOP 5392.32 DE | \$392.00 5386.53 89.80 TW = 89.00FF=91.50 FF=90.00 BW=85.00 ON-SITE BASIN 1 5389.26 TOE 5385.17 EP 5400.26 TW=88.50 ---TOE 5388.98 BW=84.80 88.80 ---89.00 ---| X 90.25 FIRST FLUSH FIRST FLUSH 89 00 POND V= 75.75 CF POND V=95.75 CF 5399 53 ---90.25---89.65 x TOP 5390.46 TOP 5385.16 TOE 5388 TW=97.00 TW=87.60 BW=92.00 BW=84.00 FF=91.50 TOE 5386.01 Coordinate FIRST FLUSH POND V=90.00 CF Show how Lot 2-PI p (an v this area TW=87.50 Dave Thompson's 90.80 BW=86.00 should be 87.75 88.25 **→** √OE 5393.33 \ /× ×90.15 91.00 X /TW=97.00 BW=92.00 FF=91.00 FIRS/T FLŲSH 5391.65 POND V=124.00 CF 5384.46 89.50-TW = 94.00Eagle Rock Ave., NE EP 5395.59 BW=91.00 - What are the limits of construction? How are the concent The Eagle Rock

AHYMO PROGRAM SUMMARY TABLE (AHYMO_97) -

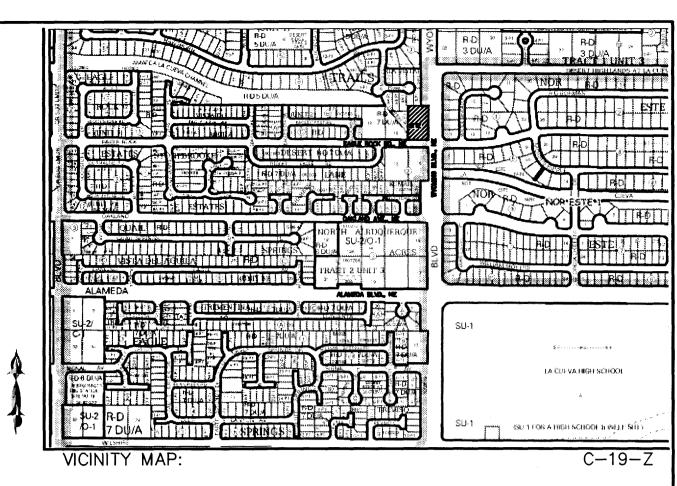
Blocks

- VERSION: 1997.02d RUN DATE (MON/DAY/YR) =03/22/2016 USER NO.= AHYMO-I-9702c01000R31-AH INPUT FILE = HAYDARI.TXT TIME TO CFS PAGE = 1FROM TO RUNOFF HYDROGRAPH ID ID DISCHARGE VOLUME RUNOFF PEAK (INCHES) (HOURS) ACRE (SQ MI) (CFS) (AC-FT) COMMAND IDENTIFICATION NO. NO. START RAIN6= 2.600 RAINFALL TYPE= 1 1.526 2.673 PER IMP= 101.00 COMPUTE NM HYD _ .029 1.526 1.526 .00059 .92212 .92024 1.01 2.659 PER IMP= COMPUTE NM HYD 102.00 2.646 PER IMP= 1.93 .00 103.00 COMPUTE NM HYD TIME= START RAIN6= 1.730 RAINFALL TYPE= .008 1.183 PER IMP= .00040 111.00 COMPUTE NM HYD .00059 1.533 .35547 1.178 PER IMP= COMPUTE NM HYD 112.00 1.500 .60463 1.971 PER IMP= 113.00 .00114 COMPUTE NM HYD TIME= START RAIN6= 2.600 RAINFALL TYPE= 1 4.464 PER IMP= .00114 3.25 103.10 COMPUTE NM HYD START TIME= RAINFALL TYPE= 1 RAIN6= 1.730 .00114 2.07 2.842 PER IMP= 63.01 113.10 COMPUTE NM HYD

drawing needs to

be built enough to handle the concentrated flows

REQUIRED FIRST FLUSH PONDING SAMPLE CALCULATIONS (LOT 3-P1) IMPERVIOUS AREA = 3,540 CF PONDING VOLUME REQUIRED = $3,540 \times 0.34/12 = 100.30 \text{ CF}_{2}$



LEGAL DESCRIPTION: LOT 17, BLOCK 1, NAA, TRACT 2, UNIT 3, CONTAINING 0.7277 ACRE

NOTICE TO CONTRACTORS

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7. WORK ON ARTERIAL STREETS SHALL BE PERFORMED ON A 24-HOUR BASIS.

GENERAL NOTES:

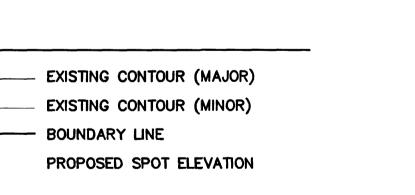
1: CONTOUR INTERVAL IS HALF (1.00) FOOT.

2: ELEVATIONS ARE BASED ON CITY OF ALBUQUERQUE CONTROL STATION 20_C18, HAVING AN ELEVATION OF <u>5393.48</u> FEET ABOVE SEA LEVEL.

3: UTILITIES SHOWN HEREON ARE IN THEIR APPROXIMATE LOCATION BASED ONLY ON ABOVE GROUND EVIDENCE FOUND IN THE FIELD AND AS-BUILT INFORMATION PROVIDED BY THE CLIENT. UTILITIES SHOWN HEREON, WHETHER INDICATED AS ABANDONED OR NOT, SHALL BE VERIFIED BY OTHERS FOR EXACT LOCATION AND/ OR DEPTH PRIOR TO EXCAVATION OR DESIGN CON-

4: THIS IS NOT A BOUNDARY SURVEY, BEARINGS ARE ASSUMED, DISTANCES AND FOUND PROPERTY CORNERS ARE FOR INFORMATIONAL PURPOSES ONLY.

5: SLOPES ARE AT 3:1 MAXIMUM.



EXISTING GRADE

(11814)

REZA AFAGHPOUR

P.E. #11814

PROPOSED BERM

GRAPHIC SCALE

SCALE: 1"=20'

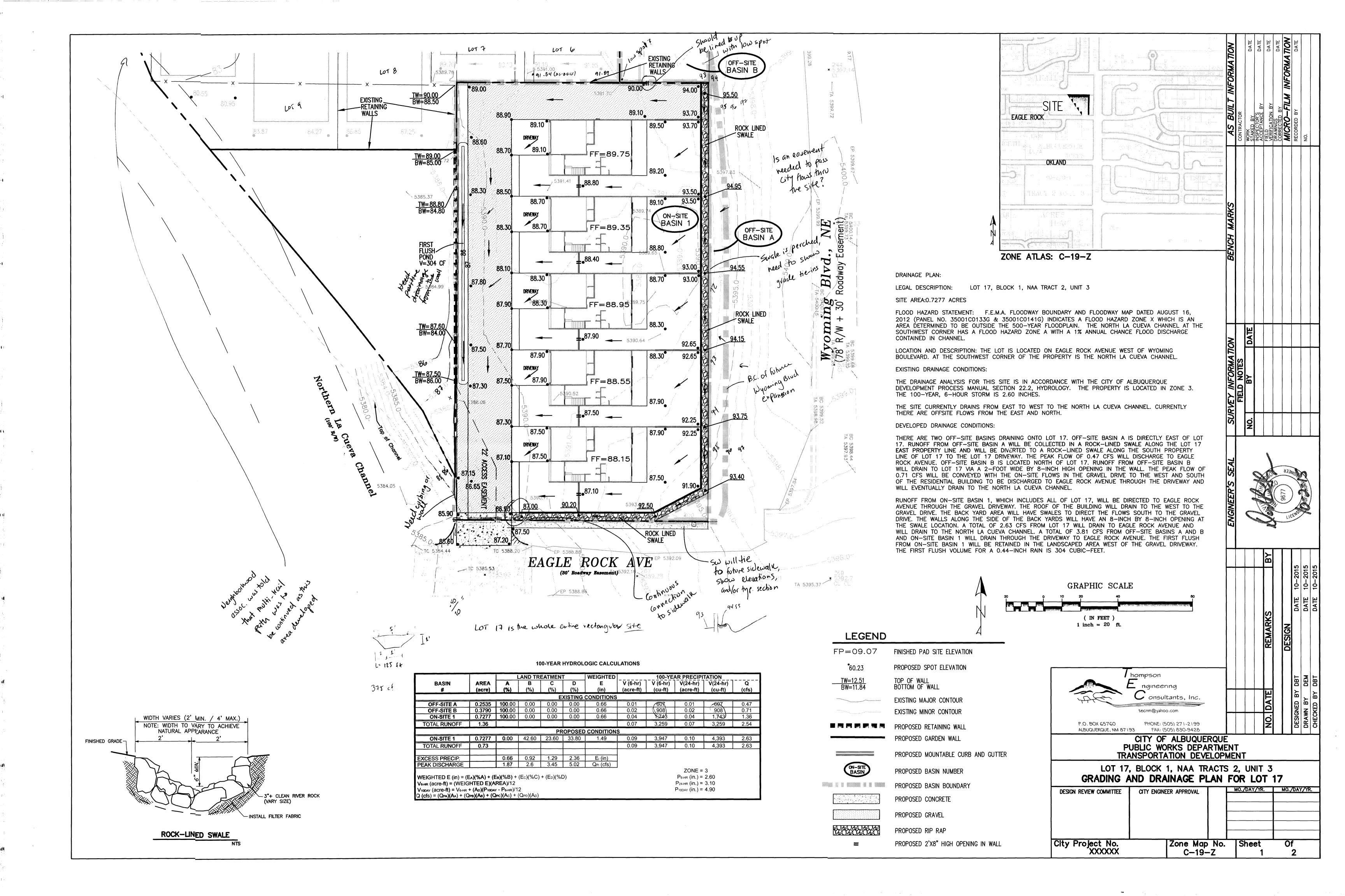


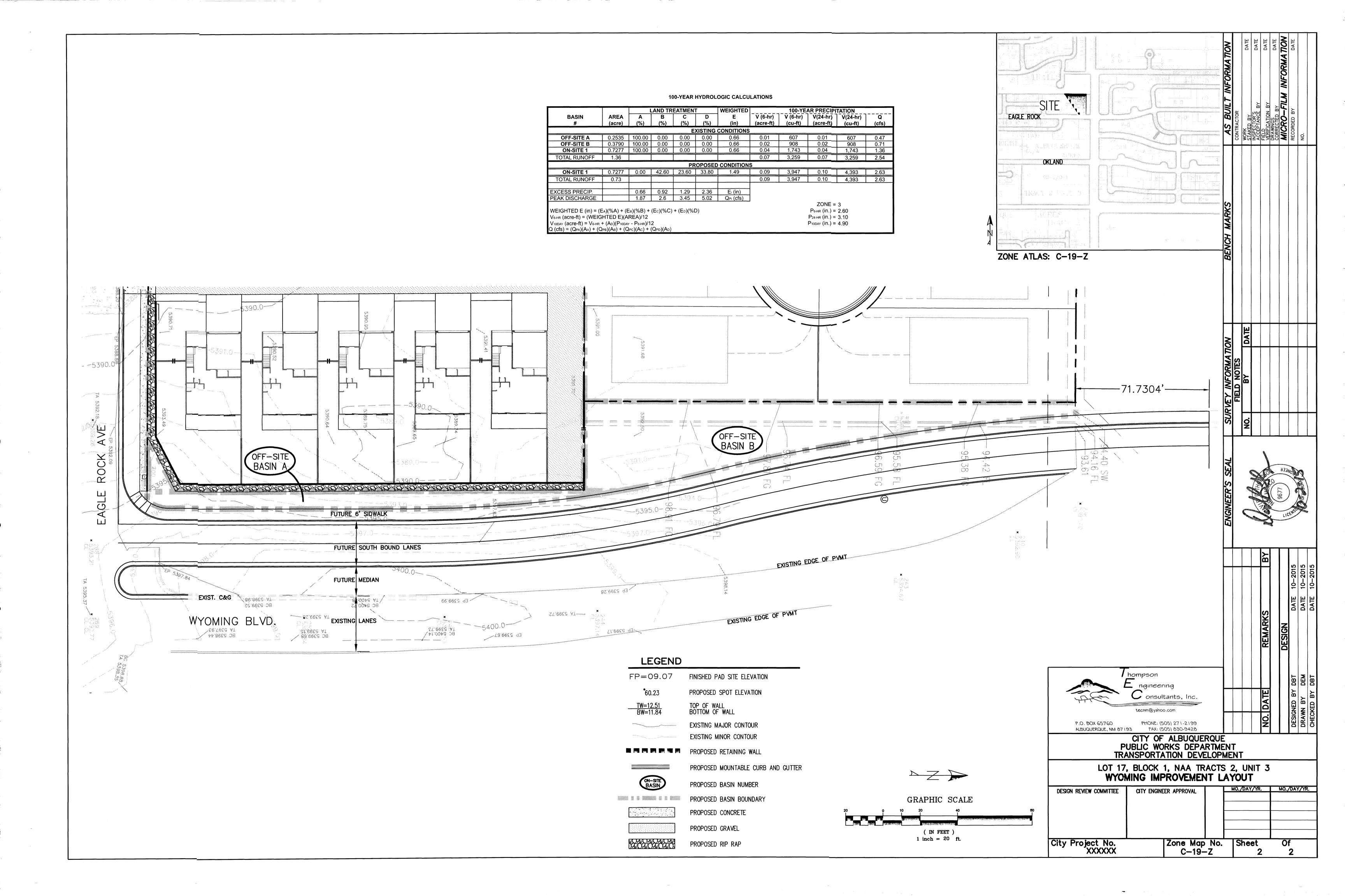
MAP 2 4 2016 LAND DEVELOPMENT SECTION

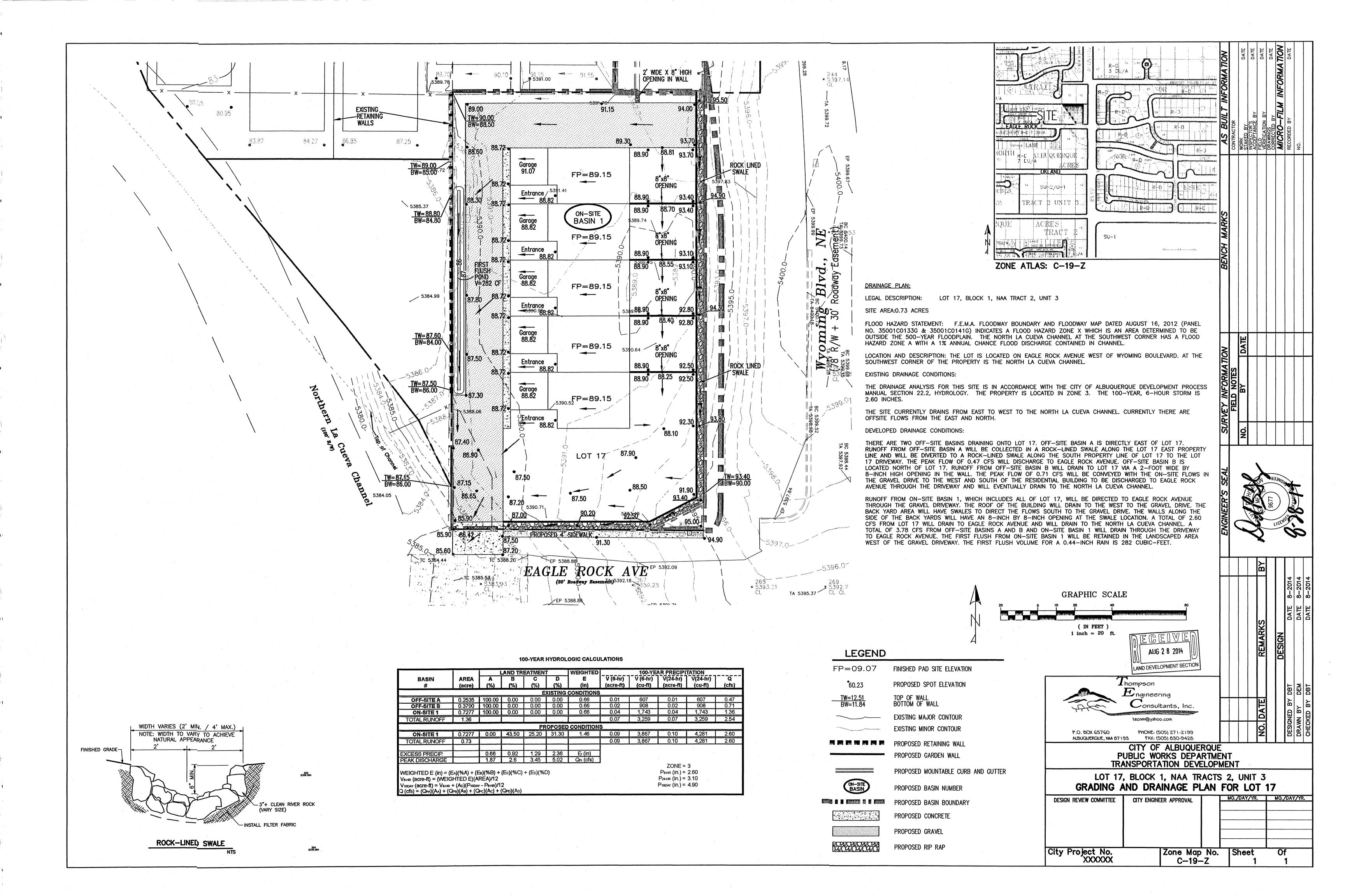
SBS CONSTRUCTION AND ENGINEERING, LLC

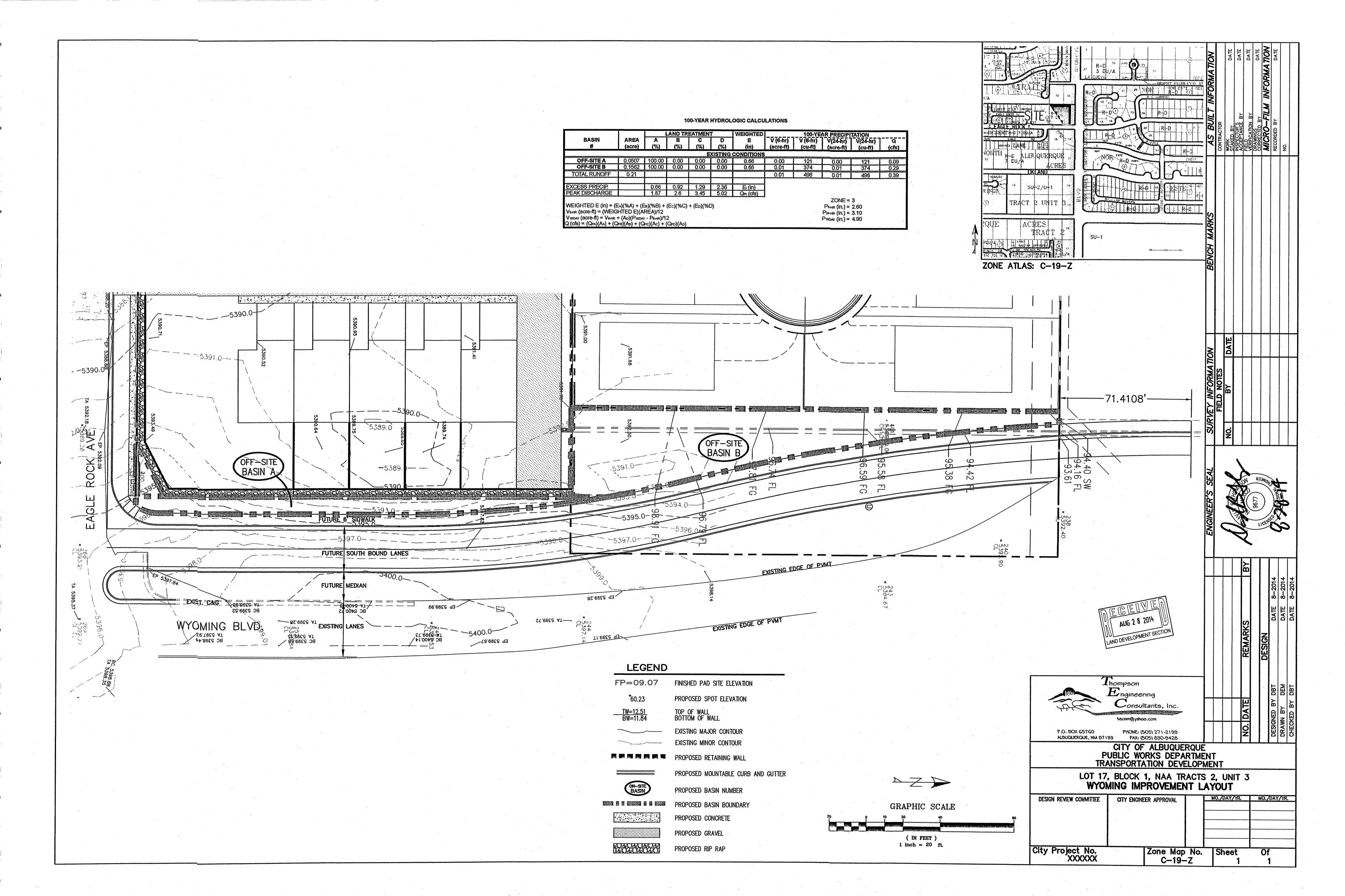
> 10209 SNOWFLAKE CT., NW ALBUQUERQUE, NEW MEXICO 87114 (505)804-5013

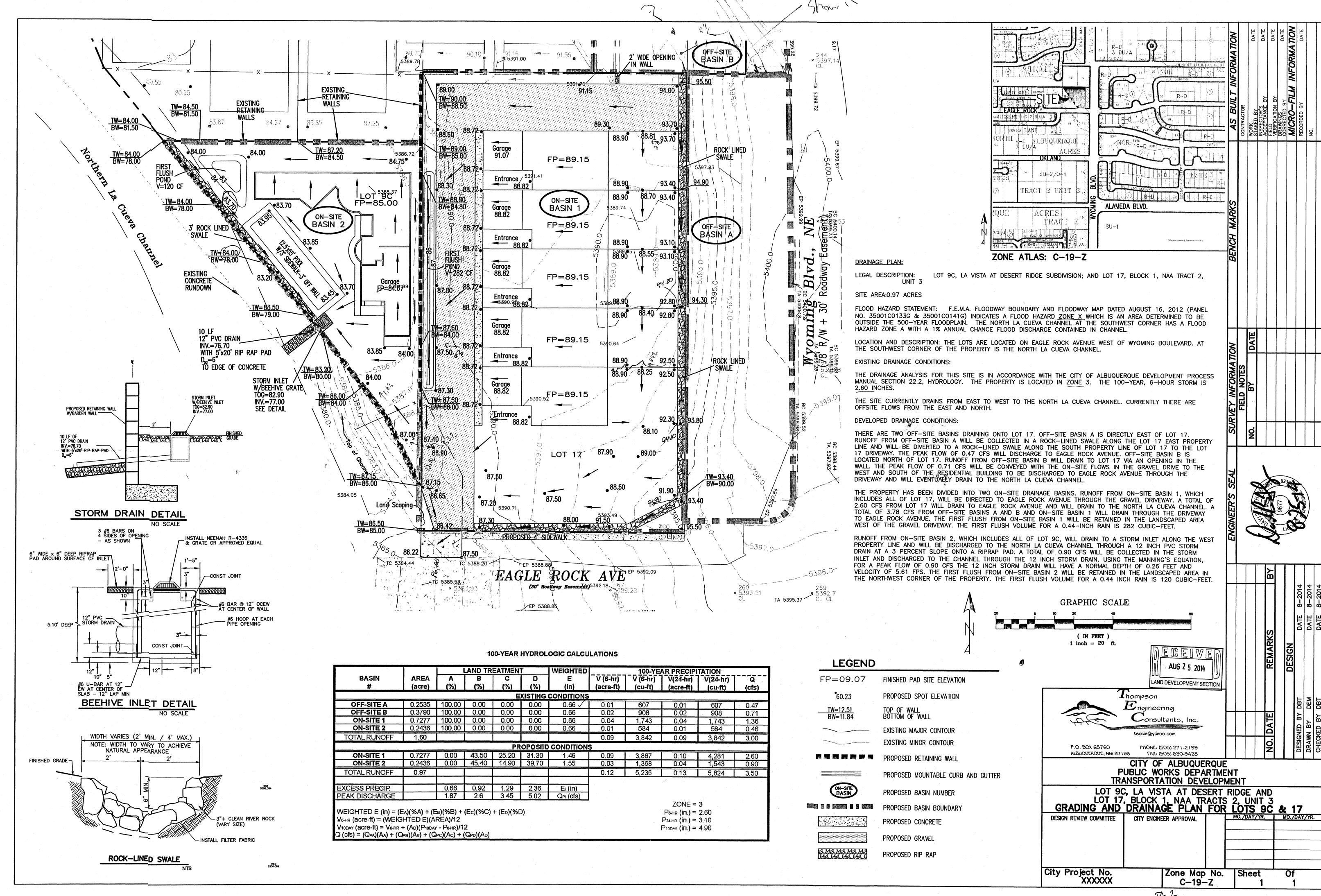
			· · · · · · · · · · · · · · · · · ·					
LOTS 1 THRU 4, ROHAN SUBDIVISION GRADING AND DRAINAGE PLAN								
DRAWING:	DRAWN BY:	DATE:	SHEET#					
201605-GR.DWG	SH-B	03-22-2016	1					











C19 D042A