



# CITY OF ALBUQUERQUE

## DEPARTMENT OF MUNICIPAL DEVELOPMENT

### NORTH DOMINGO BACA PARK PHASE 5

### MASTER GRADING PLAN

#### Introduction

The project site is located in the Northeast Heights of Albuquerque between Corona and Camel, west of Wyoming Boulevard, and east of Kinney Dam. The site is currently undeveloped. There is an existing police and fire station located on the northeast corner of the site at the intersection of Corona and Wyoming Boulevard. However, this facility is not part of the park development. A twin concrete box culvert (CBC) was recently constructed to convey flows from the Window G Channel (at Wyoming Boulevard) through the site to the Kinney Dam (an AMAFCA facility). This eliminated all flood plains that existing on the site prior to the construction of the box culvert. The existing terrain generally slopes to the west and the ground cover is primarily desert vegetation.

#### Proposed Development

The information shown on this Master Drainage Plan (MDP) is the proposed grading and layout for Phase 5 of the overall park project. Phase 5 will be split into two phases, 5A and 5B. 5A will consist of the dog run area on the west side of the site and the new parking lot south of Corona. 5B will consist of the play area east of the Corona parking lot and the skate park located east of the play area. This MDP addresses the grading of the entire site. Grading and drainage plans for each phase of the project will be submitted to COA Hydrology Development for review prior to the start of construction for that phase.

#### Hydrology

Chapter 22 of the City of Albuquerque (COA) Development Process Manual (DPM) was used to determine the proposed conditions hydrology for the project site. The design storm is the 100-yr, 6-hour event. The DPM allows the use of the Rational Method for determining runoff rates and volumes for areas that are 40-acre or less in size. This analysis uses the Rational Method. A cfs/acre and excess precipitation coefficient is used to determine the discharge and volume, respectively, depending on the location of the project. The project site is located in zone 3 (between San Mateo and Eubank).

The project site was sub-divided into 22 basins designated 1 through 22 with limits as shown. The total area of the basins is approximately 10 acres. Basins 1 through 18 and 20 will drain to existing and proposed storm drain systems and then into the existing CBC located on the site. Basins 21 and 22 will drain to the existing Kinney Dam via historic drainage paths. Only Drainage Basin 19 will drain to Corona with the flow from this basin being 0.4 cfs.

#### Proposed Storm Drain Systems

The inlets, both proposed and existing, are shown on the plan and labeled as such. There are five storm drain systems proposed for the site. Two of these systems use existing catch basins, manholes, and storm drain pipe in their layouts. The other three systems are new systems. Each system is designated on the plan by a letter (e.g. "A") for the system and a number for each storm drain pipe that comprises that system. All systems intercept the flows from the site and convey these flows to the recently constructed box culvert. New connections to the existing box culvert will be connected with the storm drain inverts placed above the 100-year water surface elevation in the box culvert. Table 2 shows the system designations and all the properties of the pipes within each system. All pipes are sized to carry the flow that will reach the system at each leg. The exception to this is System "E". Pipe "E4" (an existing 18-inch RCP) has less full flow capacity than is reaching the pipe. However, the invert of the pipe is 4.5 feet below the grate elevation. Therefore, the system will operate under pressure and will convey the flows.

Table 1 - COA DPM Hydrology Data, Proposed Conditions

Hydrology Calcs using COA DPM, Chapter 22, Section 2

Precipitation Zone:	3	(from Table A-1)
Excess Precipitation, E (in):	A	Land Treatment B C D (from Table A-8, 100-yr storm)
	0.68	0.92 1.29 2.36
Peak Discharge, Q (cfs/acre):	A	Land Treatment B C D (from Table A-9, 100-yr storm)
	1.87	2.60 3.45 5.02

100-YR STORM HYDROLOGY

Basin	Area (SF)	Area (acres)	Land Treatment Percent				Volume, V (acre-ft)	Discharge, Q (cfs)
	(a)		A	B	C	D	(c)	(d)
1	15213	0.349	0	55	45	0	0.0316	1.0
2	11123	0.259	0	75	15	10	0.0251	0.8
3	19908	0.457	0	20	10	70	0.0748	2.0
4	22672	0.520	0	0	20	80	0.0931	2.4
5	15798	0.363	0	75	15	10	0.0338	1.1
6	29223	0.672	0	90	0	10	0.0596	1.9
7	2760	0.063	0	80	10	10	0.0058	0.2
8	11709	0.270	0	80	10	10	0.0248	0.8
9	13994	0.319	0	25	50	25	0.0389	1.2
10	13823	0.313	0	25	50	25	0.0382	1.1
11	17413	0.400	0	70	20	10	0.0379	1.2
12	23311	0.535	0	15	85	20	0.0846	1.9
13	10464	0.240	0	0	100	0	0.0258	0.8
14	68405	1.572	0	0	20	80	0.2811	7.4
15	31988	0.734	0	85	15	0	0.0597	2.0
16	8478	0.195	0	85	15	0	0.0158	0.5
17	18909	0.435	0	75	15	10	0.0405	1.3
18	16947	0.381	0	90	5	5	0.0194	0.6
19	6132	0.141	0	95	5	0	0.0110	0.4
20	47750	1.086	0	90	5	5	0.0823	3.0
21	31667	0.727	0	85	5	10	0.0656	2.1
22	7041	0.162	0	90	5	5	0.0136	0.4
TOTAL	438388	10.064					1.1533	34.3

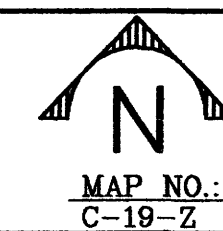
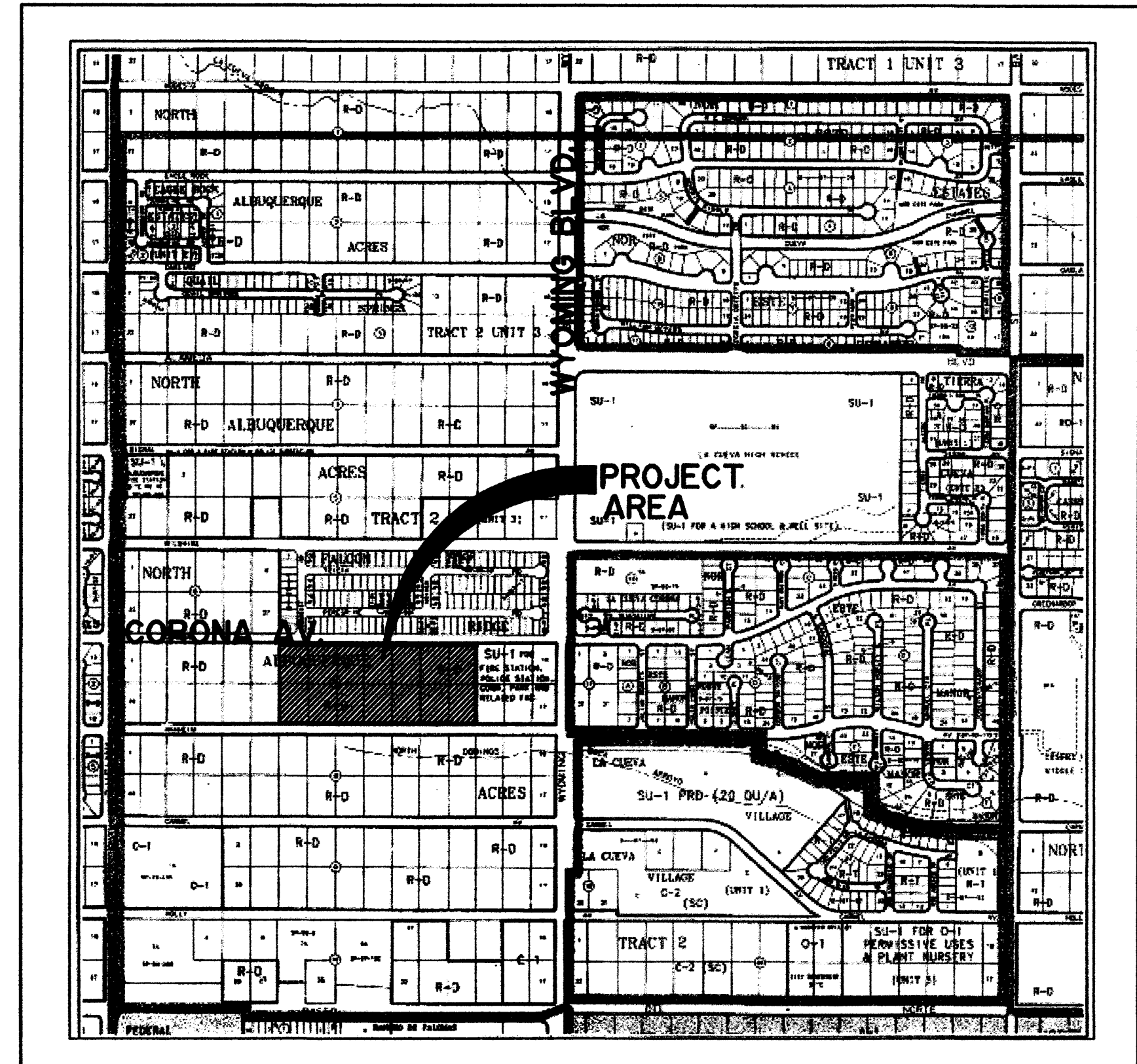
#### NOTES:

- Areas determined using Basin Map.
- Land Treatment Percent visually estimated from Basin Map.
- Volume determined using the following equations a-5 and a-6 from the DPM:  
$$Volume, V = ((E_A \cdot A_A + E_B \cdot A_B + E_C \cdot A_C + E_D \cdot A_D) / A_{TOTAL}) \cdot A_{TOTAL} / 12$$
- Discharge determined using the following equation a-10 from the DPM:  
$$Discharge, Q = Q_{PA} \cdot A_A + Q_{PB} \cdot A_B + Q_{PC} \cdot A_C + Q_{PD} \cdot A_D$$

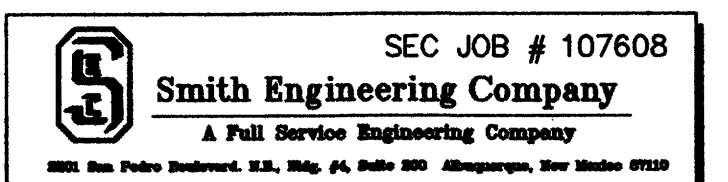
Table 2 - Storm Drain Data

Name/System	Size (in.)	Material	Existing/Proposed	Length (ft)	Slope (ft/ft)	Flow in Pipe (cfs)	Capacity of Pipe (cfs)	Capacity > Flow?
System "A"								
A1	18	PVC	Proposed	53	0.0139	1.3	5.5	Yes
A2	18	RCP	Existing	210	0.0100	1.9	10.5	Yes
A3	18	RCP	Existing	47	0.0100	4.9	10.5	Yes
A4	18	RCP	Existing	36	0.0150	4.9	12.9	Yes
System "B"								
B1	12	PVC	Proposed	89	0.0060	0.8	3.3	Yes
B2	18	PVC	Proposed	82	0.0100	10.2	13.7	Yes
B3	18	PVC	Proposed	85	0.0100	10.7	13.7	Yes
B4	24	DIP	Proposed	10	0.0100	10.7	24.5	Yes
System "C"								
C1	12	PVC	Proposed	76	0.0100	0.8	4.6	Yes
C2	12	PVC	Proposed	139	0.0100	2.0	4.6	Yes
C3	12	PVC	Proposed	85	0.0060	1.1	3.3	Yes
C4	24	PVC	Proposed	175	0.0100	5.0	29.4	Yes
C5	24	DIP	Proposed	10	0.0100	5.0	24.5	Yes
System "D"								
D1	12	PVC	Proposed	160	0.0100	1.1	4.6	Yes
D2	12	PVC	Proposed	61	0.0100	1.1	4.6	Yes
D3	18	DIP	Proposed	24	0.0100	4.1	11.4	Yes
System "E"								
E1	2-24	RCP	Existing	N/A	N/A	11.9	N/A	N/A
E2	12	PVC	Proposed	116	0.0100	1.0	4.6	Yes
E3	12	PVC	Proposed	24	0.0200	3.4	6.5	Yes
E4*	18	RCP	Existing	136	0.0100	17.3	10.5	No
E5	18	RCP	Existing	27	0.0380	17.5	20.5	Yes

\* System will operate under pressure.



ZONE ATLAS MAP  
NOT TO SCALE



REV.	SHEETS	CITY ENGINEER	DATE	USER DEPARTMENT	DATE	USER DEPARTMENT	DATE
ENGINEERS STAMP & SIGNATURE		APPROVED	ENGINEER		DATE	APPROVED FOR CONSTRUCTION	
		DRG Chairman				DEC 19 2008	
		Transportation					
		Water/Wastewater					
		Hydrology					
		CIP					
		AMAFCA					
		Constr. Coord.				CITY ENGINEER DATE	
PROJECT NO.		713893		SHEET NO.		DRAWING NO. 1 OF 4	



