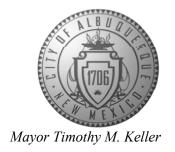
CITY OF ALBUQUERQUE

Planning Department Alan Varela, Director



March 10, 2023

Kent A. Delph, P.E. Delph Engineering 3620 Wyoming Blvd. NE, #203 Albuquerque, NM 87111

RE: Precision Eye Center
7521 Alameda Blvd. NE
Grading & Drainage Plans
Engineer's Stamp Date: 02/28/23

Hydrology File: C19D070

Dear Mr. Delph:

PO Box 1293

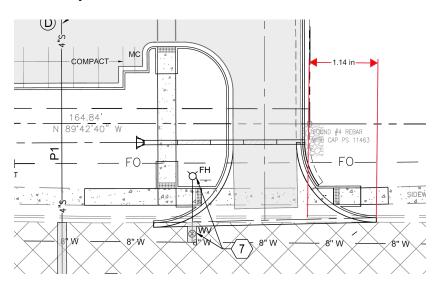
Based upon the information provided in your submittal received 03/02/2023, the Grading & Drainage Plans **are not** approved for Building Permit, Grading Permit and SO19 Permit. The following comments need to be addressed for approval of the above referenced project:

Albuquerque

1. Please do not provide Sheets C-001 (Civil Notes), C-101 (Utility Plan), C-301 (Trash Enclosure), and D-101 to D-105 (Civil Details). Hydrology only approves Sheets C-102 (Grading Plan), C-103 (Drainage Plan), and C-201 (Wall Plan & Profile).

NM 87103

- www.cabq.gov
- 2. The project curb returns cannot extend beyond the property line. The curb return must become tangent at or before the property line, so please move the entrance about 23 ft to the west. Also, please make the other curb return tangent to the existing curb because it is currently not buildable as shown.

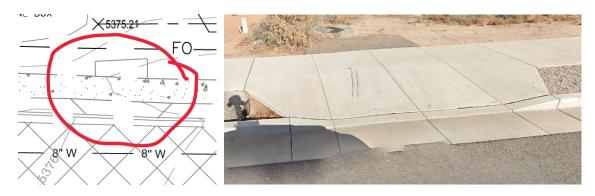


CITY OF ALBUQUERQUE

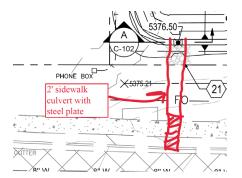
Planning Department Alan Varela, Director



3. The existing drive pad at the southwest corner of the property needs to be called out as "Remove existing drive pad and replace with curb & gutter and sidewalk.".



- 4. On Sheet 103, under the Existing Conditions. Please state, "Project is with North Albuquerque Acres Master Drainage Plan by Resource Technology, Inc. October 1998 which this site has an allowable discharge rate of 3.60 cfs/ac".
- 5. Please note that any existing remanence of a natural arroyo is just what is left over from when La Cueva Arroyo was placed in a concrete channel, so when the adjacent lot is developed, it will be discharging into Alameda which is reflected in the above NAA Master Drainage Plan. Therefore, accommodating for this is not needed, but any existing flows can be just handled with a curb cut in the curb return to allow the flows to go into Alameda Blvd. So, the curvet that shown does not need to be installed and the grades for the drive needs to follow CoA Detail 2426 with a water block.
- 6. On Sheet 103, the Stormwater Quality Pond volume is incorrect. Per DPM Article 6-12 Stormwater Quality and Low-Impact Development for the sizing calculations, 0.42 inches for new development sites and 0.26 inches for redevelopment sites. This site is a new development and not a redevelopment.
- 7. On Sheet 102, please provide a sidewalk culvert for the spillway for the stormwater quality pond to Alameda Blvd. and label per CoA Detail 2236. Please also show the invert at the pond (this is the SWQ water surface elevation) and the invert at the existing gutter on Alameda Blvd.



PO Box 1293

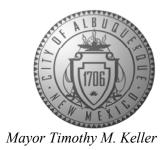
Albuquerque

NM 87103

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CITY OF ALBUQUERQUE

Planning Department Alan Varela, Director



- 8. On Sheet 103, please provide the weir calculations, per DPM Article 6-16(A), for the curb cuts and sidewalk culverts. A coefficient of 2.7 is typically used for the weir equation $Q = CLH^{2/3}$.
- 9. On Sheet 103, please provide a cross section of the SWQ pond with the bottom of pond elevation, top of pond elevation, and SWQ water surface elevation.
- 10. On Sheet 102, please provide the Benchmark information (location, description and elevation) for the survey contour information provided.
- 11. On Sheet 102, please provide the SO-19 Standard Notes, latest version.

As a reminder, if the project total area of disturbance (including the staging area and any work within the adjacent Right-of-Way) is 1 acre or more, then an Erosion and Sediment Control (ESC) Plan and Owner's certified Notice of Intent (NOI) is required to be submitted to the Stormwater Quality Engineer (Doug Hughes, PE, jhughes@cabq.gov, 924-3420) 14 days prior to any earth disturbance.

PO Box 1293

If you have any questions, please contact me at 924-3995 or rbrissette@cabq.gov .

Albuquerque

Sincerely,

NM 87103

Renée C. Brissette, P.E. CFM Senior Engineer, Hydrology Planning Department

Renée C. Brissette

www.cabq.gov



City of Albuquerque

Planning Department Development & Building Services Division

DRAINAGE AND TRANSPORTATION INFORMATION SHEET (REV 11/2018)

Project Title: Precision Eye Center	Building Permi	t #: BP-2022-39517
DRB#:	EPC#:	Work Order#:
	Гract 2, Unit 3 North Abuqu	nerque Acres; Section 18 T 11 N, R 4 E
Applicant: Insight Construction Address: 3909 12th, Street, Albuquero		Contact: Damian Chimenti
Phone#: (505) 506-9144 (m)	Fax#: <u>n/a</u>	E-mail: damian@insightnm.co
Owner: _ Dr. Rob Lavoie		Contact: Dr. Rob Lavoie
Address: 8000 Paseo Del Norte NE		
Phone#: (505) 291-1711	Fax#: <u>n/a</u>	E-mail: robisgolfing@gmail.com
IS THIS A RESUBMITTAL?:	Yes X	
DEPARTMENT: X TRAFFIC/ TRA	ANSPORTATION \underline{X}	HYDROLOGY/ DRAINAGE
Check all that Apply: TYPE OF SUBMITTAL: ENGINEER/ARCHITECT CERTIFY PAD CERTIFICATION CONCEPTUAL G & D PLAN X GRADING PLAN DRAINAGE MASTER PLAN DRAINAGE REPORT FLOODPLAIN DEVELOPMENT P ELEVATION CERTIFICATE CLOMR/LOMR TRAFFIC CIRCULATION LAYOU TRAFFIC IMPACT STUDY (TIS) OTHER (SPECIFY) PRE-DESIGN MEETING? Note: Fire 1 & TCL to be submitted separate	ERMIT APPLIC JT (TCL)	TYPE OF APPROVAL/ACCEPTANCE SOUGHT: BUILDING PERMIT APPROVALCERTIFICATE OF OCCUPANCYPRELIMINARY PLAT APPROVALSITE PLAN FOR SUB'D APPROVALSITE PLAN FOR BLDG. PERMIT APPROVALFINAL PLAT APPROVALSIA/ RELEASE OF FINANCIAL GUARANTEEFOUNDATION PERMIT APPROVALX GRADING PERMIT APPROVALX GRADING PERMIT APPROVALX PAVING PERMIT APPROVAL
DATE SUBMITTED: 2/28/2023	Ву:	
COA STAFE.	ELECTRONIC GUI	DMITTAL DECEIVED.

FEE PAID:___

NOTES

- 1. THE CONTRACTOR AGREES THAT HE SHALL ASSUME SOLE AND COMPLETE RESPONSIBILITY FOR JOB SITE CONDITIONS DURING THE COURSE OF CONSTRUCTION OF THIS PROJECT, INCLUDING SAFETY OF ALL PERSONS AND PROPERTY: THAT THIS REQUIREMENT SHALL APPLY CONTINUOUSLY AND NOT BE LIMITED TO NORMAL WORKING HOURS; AND THAT THE CONTRACTOR SHALL DEFEND, INDEMNIFY, AND HOLD THE OWNER, ARCHITECT, AND ENGINEER HARMLESS FROM ANY AND ALL LIABILITY, REAL OR ALLEGED, ON CONNECTION WITH THE PERFORMANCE OF WORK ON THIS PROJECT, EXCEPTING FOR LIABILITY ARISING FROM THE SOLE NEGLIGENCE OF THE OWNER, ARCHITECT, OR ENGINEER.
- EXISTING SURVEY DATA PROVIDED TO DELPH ENGINEERING BY OWNER, DELPH ENGINEERING ACCEPTS NO LIABILITY FOR POTENTIAL ERRORS FROM THE DATA PROVIDED CONTRACTOR IS ENCOURAGED TO POTHOLE FOR UTILITIES PRIOR TO CONSTRUCTION IN AREAS TO BE EXCAVATED.

GRADING & CONCRETE NOTES

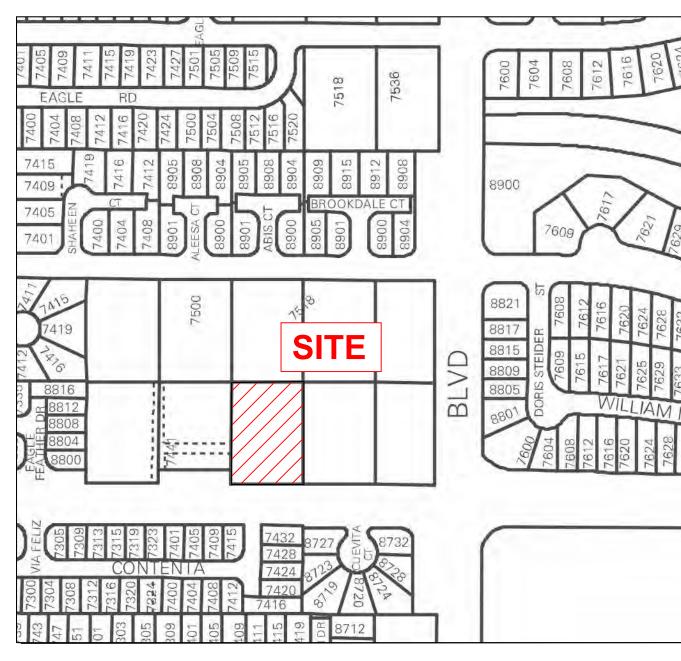
- 1. ALL WORK SHALL BE IN ACCORDANCE WITH CITY OF ALBUQUERQUE STANDARD SPECIFICATIONS FOR PUBLIC WORKS, 2020 (CABQ STD. SPEC.), UNLESS OTHERWISE NOTED.
- 2. DUST SHALL BE CONTROLLED AS REQUIRED NY CABQ.
- 3. THE LOCATION AND PROTECTION OF ALL UTILITIES IS THE RESPONSIBILITY OF THE
- 4. APPROVED PROTECTIVE MEASURES AND TEMPORARY DRAINAGE PROVISION SHALL BE USED TO PROTECT ADJOINING DRAINAGE AREAS DURING THE CONSTRUCTION OF THE PROJECT.
- 5. THE AREAS OF PROPOSED IMPROVEMENTS SHALL BE CLEARED OF ALL VEGETATION AND DELETERIOUS MATER PRIOR TO BEGINNING THE GRADING OPERATION.
- 6. AREAS FOR OF PROPOSED FILL OR PAVING SHALL BE SCARIFIED, MOISTENED &
- RECOMPACTED FOR A MINIMUM DEPTH OF 10".
- AGGREGATE BASE COURSE SHALL CONFORM TO CABQ STD. SPEC. SECTION 302, TYPE II.
 ASPHALTIC CONCRETE SHALL CONFORM TO CABQ STD. SPEC. SECTION 116, SP-IV (Table 116.B) WITH A PG70-22 BINDER (Table 116.C.1).
- 9. SITE WORK CONCRETE (sidewalks, curbs, gutter) SHALL MEET CABQ STD. SPEC. SECTIONS 101 AND 340, FOR A 28-DAY COMPRESSIVE STRENGTH OF 3,000 PSI.
- 8. EARTHWORK SHALL BE IN COMPLIANCE WITH WESTERN TECHNOLOGIES INC. "GEOTECHNICAL EVALUATION REPORT PRECISION EYE CENTER" JANUARY 6, 2020. THIS COMPLIANCE INCLUDES REMOVING EXISTING SOILS A MINIMUM DEPTH OF 3' BELOW THE BOTTOM OF PROPOSED FOOTINGS INCLUDING A HORIZONTAL AREA OF 2 BEYOND THE FOOTING EDGE. THE FILL MATERIAL (IMPORT, NATIVE OR BLENDED) SHALL MEET THE FOLLOWING SPECIFICATION:

GRADATION (ASTM C136):	PERCENT FINER BY WEIGHT
6"	100
4"	85-00
3 ₁₁	70-100
NO. 4 SIEVE	50-100
NO. 200 SIEVE	20 (MAX)
MAXIMUM SOLUBLE SULFATES (%)	0.10
MAXIMUM PLASTICITY INDEX	

UTILITY NOTES

- 1. VERIFY DEPTH AND LOCATION OF ALL EXISTING UTILITIES PRIOR TO CONSTRUCTING ANY NEW UTILITY LINES.
- 2. REFER TO SITE ELECTRICAL PLANS FOR DETAIL AND LOCATION OF ELECTRICAL IMPROVEMENTS
- 3. CONTRACTOR IS ENCOURAGED TO POTHOLE FOR UTILITIES PRIOR TO CONSTRUCTION IN AREAS TO BE EXCAVATED AND SPECIFICALLY AT THE CROSSING OF THE PROPOSED RETAINING WALL AND EXISTING FIBER OPTIC LINE, IN ALAMEDA BLVD. NE RIGHT-OF-WAY.
- 4. WATER SERVICE CONNECTION MATERIALS AND METHODS SHALL COMPLY WITH CABQ STD. SPEC. SECTION 802.
- 5. SANITARY SEWER MAIN CONNECTION, SERVICE LINE AND CLEANOUTS SHALL COMPLY WITH CABQ STD. SPEC. SECTIONS 901 AND 905.





REGIONAL

SITE

CIVIL SHEET INDEX						
Number	Title					
C-001	CIVIL NOTES					
C-101	UTILITY PLAN					
C-102	GRADING PLAN					
C-103	DRAINAGE PLAN					
C-201	WALL PLAN & PROFILE					
C-301	TRASH ENCLOSURE, ACCESS & DETAILS					
D-101	CIVIL DETAILS					
D-102	CIVIL DETAILS					
D-103	CIVIL DETAILS					
D-104	CIVIL DETAILS					
D-105	CIVIL DETAILS					

ADDRESS:

7521 Alameda Blvd NE, Albuquerque, NM 87113

Robert Lavoie
Precision Eye Center
(505) 291-1711
robisgolfing@gmail.com

OWNER:

ENGINEER:

DELPH ENGINEERING
Kent Delph, PE, CFM
3620 Wyoming Blvd NE, Suite 203
Albuquerque, NM 87111
(505) 206-8385
kent@delpheng.com





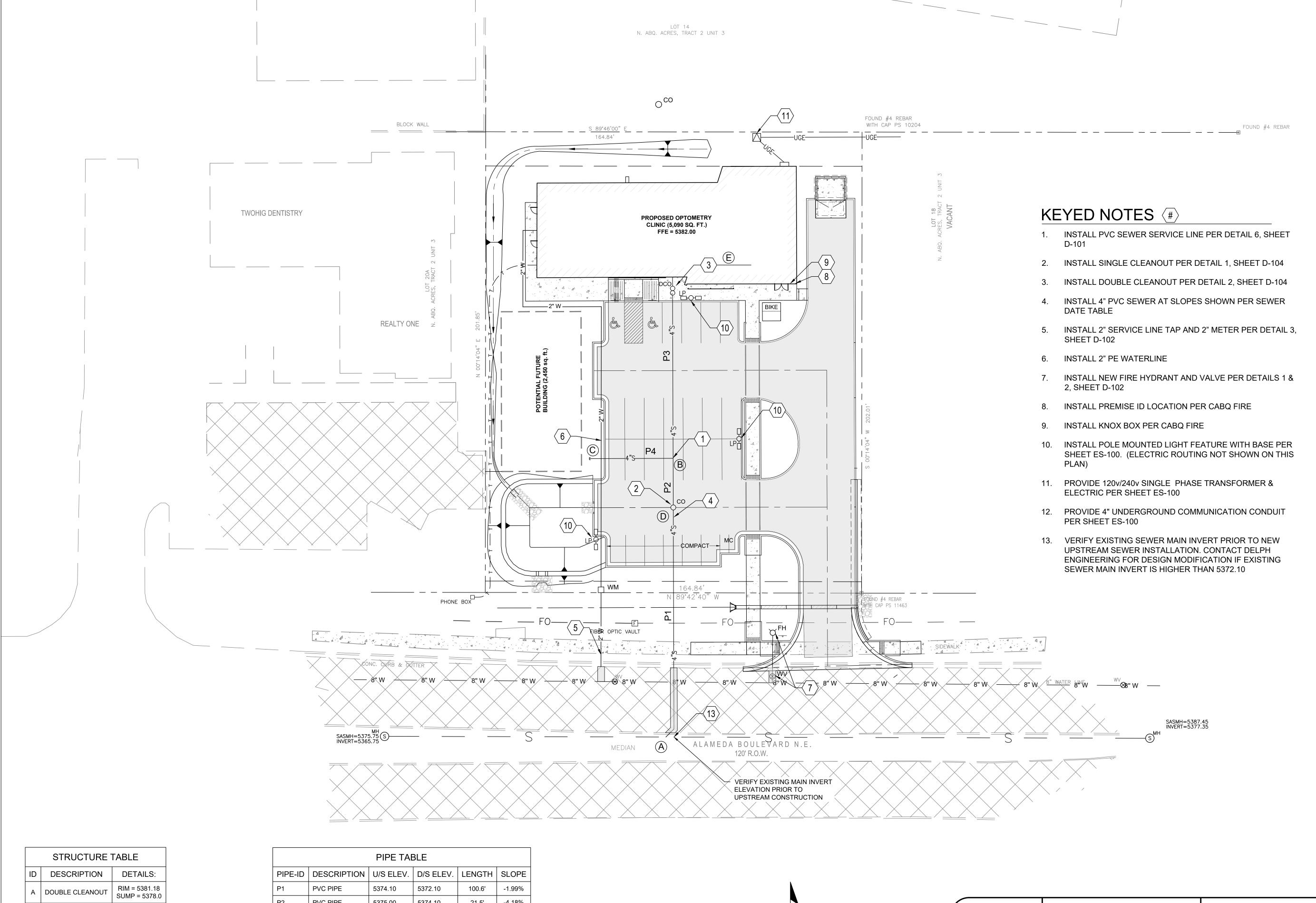
3620 Wyoming Blvd. NE, # 203 Albuquerque, New Mexico 87111 Phone. (505) 206-8385 kent@delpheng.com PRECISION EYE CENTER

SITE IMPROVEMENT PLANS

7521 ALAMEDA BLVD. NE ALBUQUERQUE, NM 87113 CIVIL NOTES

CABQ # BP-2022-39517

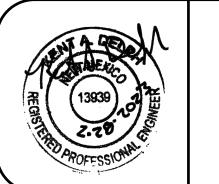
C-001



STRUCTURE TABLE						
ID	DESCRIPTION	DETAILS:				
Α	DOUBLE CLEANOUT	RIM = 5381.18 SUMP = 5378.0				
В	4" WYE	RIM = 5378.18 SUMP = 5375.0				
С	4" STUB OUT	RIM = 5379.68 SUMP = 5376.5				
D	SINGLE CLEANOUT	RIM = 5377.28 SUMP = 5374.1				
E	MAIN CONNECTION	RIM = 5375.28 SUMP = 5372.1				

THETADLE									
PIPE-ID	DESCRIPTION	U/S ELEV.	D/S ELEV.	LENGTH	SLOPE				
P1	PVC PIPE	5374.10	5372.10	100.6'	-1.99%				
P2	PVC PIPE	5375.00	5374.10	21.5'	-4.18%				
P3	PVC PIPE	5378.00	5375.00	74.5'	-4.03%				
P4	PVC PIPE	5376.50	5375.00	36.7'	-4.09%				

0' 10' 20' Scale: 1"=20'





3620 Wyoming Blvd. NE, # 203 Albuquerque, New Mexico 87111 Phone. (505) 206-8385 kent@delpheng.com

PRECISION EYE CENTER SITE IMPROVEMENT PLANS

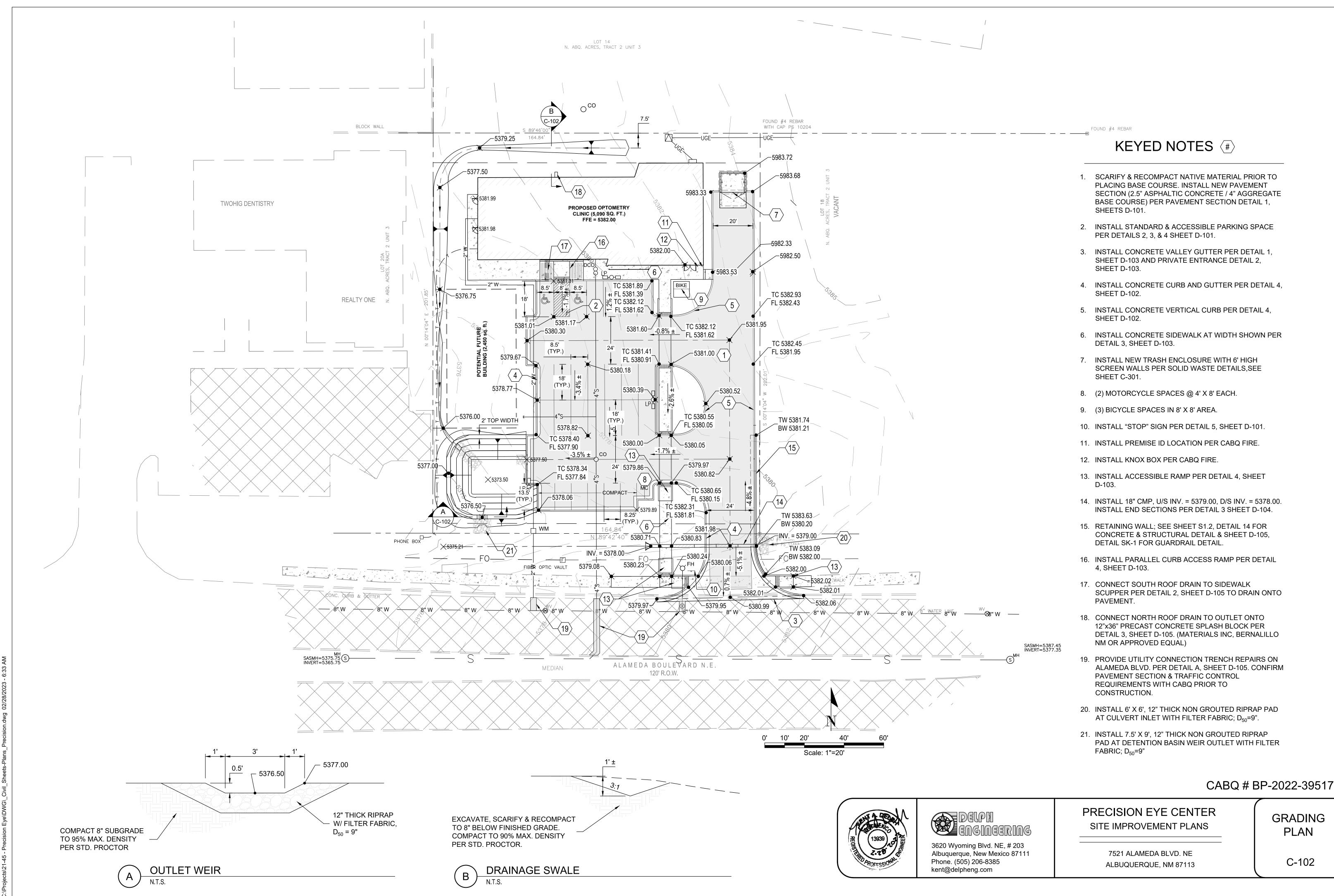
> 7521 ALAMEDA BLVD. NE ALBUQUERQUE, NM 87113

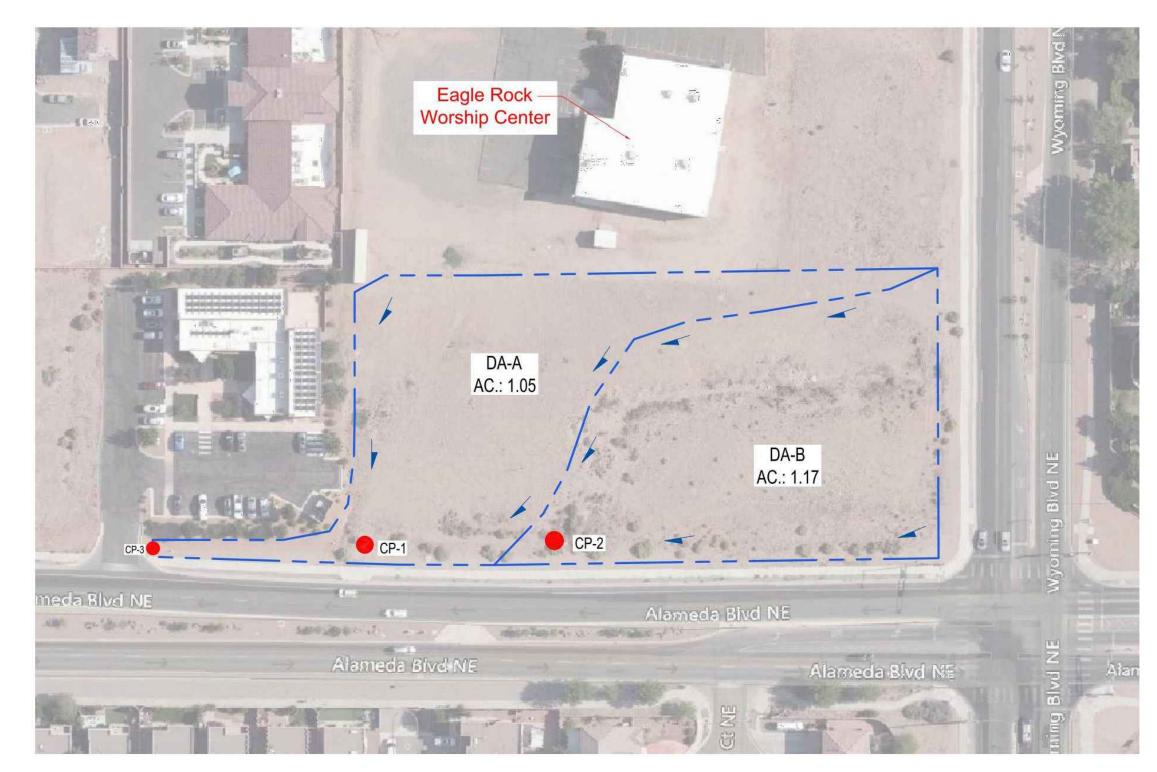
UTILTIY PLAN

CABQ # BP-2022-39517

C-101

UNKNOWN VERIFY PER PLAN NOTE; 5372.10 MAX.





PROPOSED DRAINAGE

SCALE: 1" = 40'

NARRATIVE

THE PROPOSED PROJECT IS LOCATED ON PARCEL AT 7521 ALAMEDA BOULEVARD NORTHEAST, ALBUQUERQUE NEW MEXICO ON LOT 19, BLOCK 3, TRACT 2, UNIT 3 NORTH ALBUQUERUQE ACRES (5,090 SQ FT²) AND AN ADJOINING PAKRING LOT. FOR THE PURPOSE OF RETENTION BASIN DESIGN, A FUTURE SECOND STRUCTRURE (2,450 SQ FT²) WAS CONSIDERED IN THE ANALYSIS.

REGIONAL DRAINAGE IN THE AREA IS FROM NORTHEAST TO SOUTHWEST. A MINOR EXISTING FLOWPATH IMPACTS THE SOUTHEAST CORNER OF THE SITE (1.53 ACRES, Q100 = 4.86 CFS) AND OUTLETS WEST OF THE SITE ACROSS EXISTING DECOMPOSED GRANIT IN THE RIGHT-OF-WAY.

PROPOSED CONDITIONS INCLUDE A NEW CULVERT WITH A HEADWALL INTERGRATED IN THE RIGHT-OF WAY TO ALLOW EXISTING OFFSITE FLOWS TO OUTLET WEST TO MATCH EXISTING CONDITONS. ALL ONSITE FLOWS ARE ROUTED TO PROPOSED RETENTION BASIN.

FLOODPLAIN STATUS

THE PROPOSED PROJECT IS IN OUTSIDE OF A FEMEA FLOODPLAIN (ZONE X) AS PER FIRM MAP No. 35001C0141G.

HYDROLOGIC METHODOLOGY

HYDROLOGIC ANALYSIS WAS PERFORMED PER CITY OF ALBUQUERQUE DEVELOPMENT PROCESS MANUAL FOR SMALL WATERSHEDS UNDER 40 ACRES. APPLICABLE LAND TREATMENT CLASSIFICATIONS AND PRECIPITATION VALUES WERE USED, SEE TABLE ON THIS SHEET.

HYDRAULIC METHODOLOGY

A STORMWATER RETENTION BASIN HAS BEEN PROVIDED AT THE SOUTHWEST CORNER OF THE SITE TO CONTAIN THE 100-YEAR INCREASE IN RUNOFF VOLUME OF 2,550 FT². THE BASIN VOLUME AT THE OUTLET WEIR IS 2.583 FT². THE OUTLET WEIR IS 3' WIDE WITH A FLOW DEPTH OF 0.50 FT. FOR THE O100 = 2.72 CFS. THE VOLUME FOR THE POND WAS ANAYSED USING AUTOCAD CIVIL 3D STAGE STORAGE. AN 18" DIAMETER CULVERT IS PROPOSED IN THE RIGHT OF WAY TO CONVEY THE EXISTING OFFSITE FLOW WEST. THE CULVERT DESIGN CONSIDERED THE 10 & 100-YEAR PEAK FLOWRATES & WAS ANALYZED USING FHWA HY-8 SOFTWARE:

Crossing Summary Table

Headwater Elevation (ft)	Total Discharge (cfs)	Culvert 1 Discharge (cfs)	Roadway Discharge (cfs)	Iterations
79.82 Q2	2.59	2.59	0.00	1
80.21 Q100	4.86	4.86	0.00	1
82.00	1134	1134	0.00	Overtonning

A STORMWATER RETENTION BASIN IS PROSOSED AT THE SOUTHWEST CORNER OF THE SITE THAT WILL HAVE A VOLUME OF 2,583 FT^3 TO ACCOMODATE THE MINOR INCREASE IN 100-YR STORM VOLUMES AND THE REQUIRED FIRST FLUSH VOLUME. THE FIRST FLUSH VOLUME WAS CALCULATED USING THE $\mathrm{80}^{\mathrm{TH}}$ STORM PERCENTILE RUNOFF OF 0.23" ON THE NEW ROOF FOR THE PROPSED STRUCTURE AND POTENTIAL SECONDAERY STRUCTURE, WITH ASSOCIATED PAVING AND SIDEWALK. THE REQUIRED FIRST FLUSH VOLUME IS 365 FT³; CALCULATED FOLLOWING:

- ROOF AREA (PROPOSED AND SECONDARY FUTURE) = 8,282 FT²
- ASPHALT & CONCRETE AREA = 13,028 FT²
- REQUIRED FIRST FLUSH VOLUME = $21,310 \text{ FT}^2 * (0.26 \text{ INCHES}/12 \text{ INCHES}) = 462 \text{ FT}^3$
- PROVIDED FIRST FLUSH VOLUME = 462 FT³ **EROSION CONTROL NOTES**
- 1. THE AREA OF DISTURBANCE FOR THE PROJECT IS ESTIMATED AT 0.88 ACRES. THIS AREA IS LESS THAN 1.00 ACRES, WHICH WOULD NOT REQUIRE A STORMWATER POLLUTION PREVENTION PLAN
- 2. CONTRACTOR IS RESPONSIBLE FOR MAINTAINING RUN-OFF ON SITE DURING CONSTRUCTION.
- 3. CONTRACTOR IS RESPONSIBLE FOR CLEANING ALL SEDIMENT THAT GETS INTO EXISTING
- 4. REPAIR OF DAMAGED FACILITIES AND CLEANUP OF SEDIMENT ACCUMULATIONS ON ADJACENT PROPERTIES AND IN PUBLIC FACILITIES ARE THE RESPONSIBILITY OF THE CONTRACTOR.

EXISTING DRAINAGE

HYDROLOGIC DATA

Delph Engineering Job No. 21-45

100-YR STOR	RM RAINFA	LL DEPTHS		*
P _{1-HR}	P _{6-HR}	P _{24-HR}	P _{4-DAY}	P _{10-DAY}
(in)	(in)	(in)	(in)	(in)
1.84	2.43	2.75	2.84	4.1

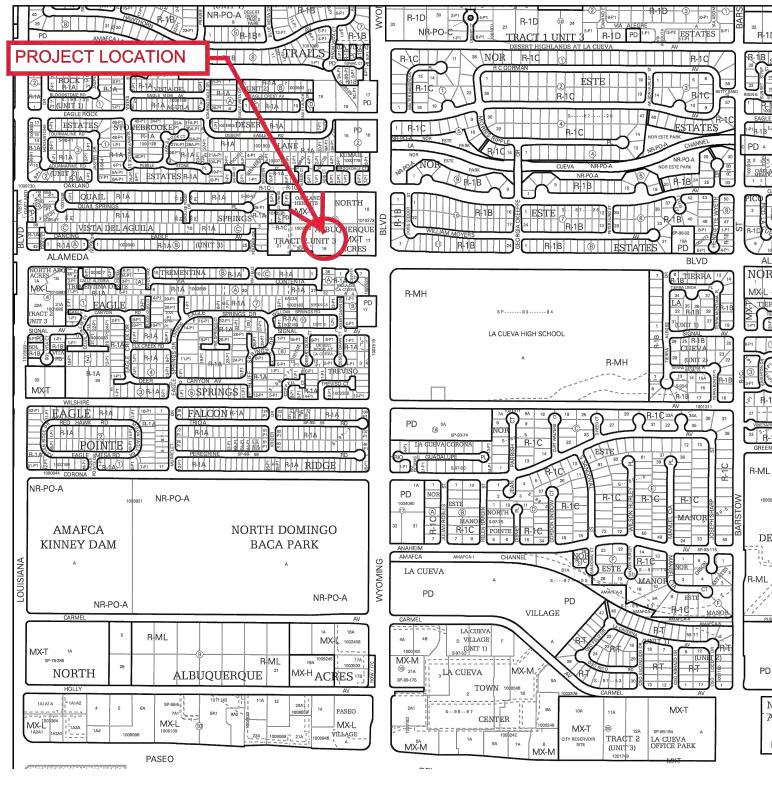
Precision Eye

	PEAK DISCHARGE FACTORS								
Land Trea	itment C	Land Treatment D							
q _{10-YR} (cfs/ac)	q _{100-YR} (cfs/ac)	q _{10-YR} (cfs/ac)	q _{100-YR} (cfs/ac)						
1.69	3.17	2.81	4.49						

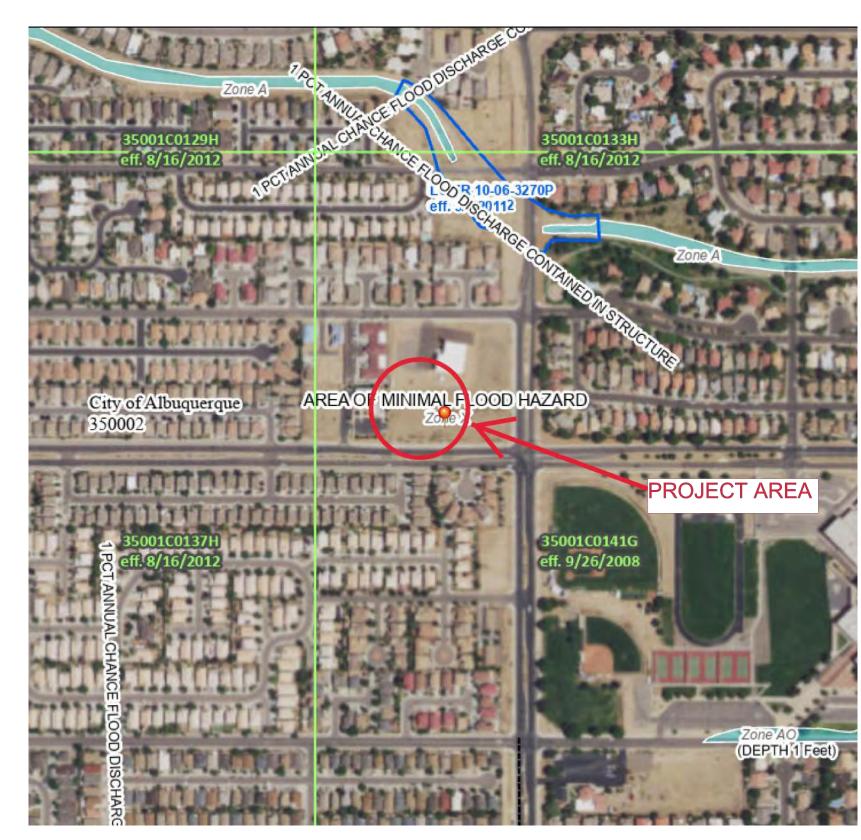
Precipitation Zone 3:	100-YR Storm:	Р _{6-нк} :	2.43	P _{24-HR} :	2.84	10-YR Storm:	Р _{6-НR} :	1.57	P _{24-HR} :	1.90			PROPOSED RUNOFF OLUMES	EXISTIN PROPOSI FLOWI	ED PEAK
	AREA	LAND	TREATME	NT AREA B	Y TYPE	EXCE		E) BY LAND TREAT	TMENT	E _{W,10-YR}	E _{W,100-YR}	V ₁₀	V ₁₀₀	Q ₁₀	Q ₁₀₀
DRAINAGE AREA	ANLA	Α	В	С	D	E _{C-10-YR}	E _{C-100-YR}	E _{D-10-YR}	E _{D-100-YR}	-w,10-YR	-W,100-YR	- 10	* 100	C 10	100
	(ac)	(ac)	(ac)	(ac)	(ac)	(in.)	(in.)	(in.)	(in.)	(in.)	(in)	(ac-ft)	(ac-ft)	(cfs)	(cfs)
Existing On & Off Site:															
CP-1	2.32	0.00	0.00	2.3227	0.00	0.52	1.09	1.64	2.58	0.52	1.09			3.93	7.36
CP-2	1.53	0.00	0.00	1.53	0.00	0.52	1.09	1.64	2.58	0.52	1.09			2.59	4.86
CP-3	2.43	0.00	0.00	2.43	0.00	0.52	1.09	1.64	2.58	0.52	1.09			4.11	7.71
											s.				
Without Development:					8										
CP-12	0.33	0.00	0.00	0.33	0.00	0.52	1.09	1.64	2.58	0.52	1.10	0.01	0.03	0.56	1.05
CP-13	0.33	0.00	0.00	0.33	0.00	0.52	1.09	1.64	2.58	0.51	1.08	<u>0.01</u>	<u>0.03</u>	<u>0.56</u>	<u>1.05</u>
									Total	' l Volume for Existing Site	e (CPs 12 & 13):	0.03	0.06	1.12	2.09
Proposed:				ĺ											
With One Building															
CP-12	0.33	0.00	0.00	0.06	0.27	0.52	1.09	1.64	2.58	1.44	2.32	0.04	0.06	0.86	1.40
CP-13	0.33	0.00	0.00	0.21	0.12	0.52	1.09	1.64	2.58	0.92	1.63	<u>0.03</u>	<u>0.05</u>	<u>0.70</u>	<u>1.22</u>
									Т	otal Volume for Site wit	th One Buidling:	0.07	0.11	1.56	2.61
					2										
With Two Buildings												· ·			
CP-12	0.33	0.00	0.00	0.06	0.27	0.52	1.09	1.64	2.58	1.44	2.32	0.04	0.06	0.86	1.40
CP-13	0.33	0.00	0.00	0.13	0.20	0.52	1.09	1.64	2.58	1.19	1.98	0.03	<u>0.06</u>	<u>0.79</u>	<u>1.32</u>
									Total Volume for Site with Two Buidlings: 0.07 0.12				0.12	1.65	2.72
	1									1				1	

INCREASE DUE TO DEVELOPMENT WITHOUT STORMWATER BASIN (CPS 12 & 13): 0.53

Basin Volume for Proposed One New Bulding Basin Volume for Proposed Two New Buldings Vol (cubic Feet



VICINITY MAP (ZONE ATLAS MAP No. C-19-Z) 7521 Alameda Boulevard NE, Albuquerque New Mexico 87113



FIRMETTE No. 35001C0141G 9/26/2008

CABQ # BP-2022-39517

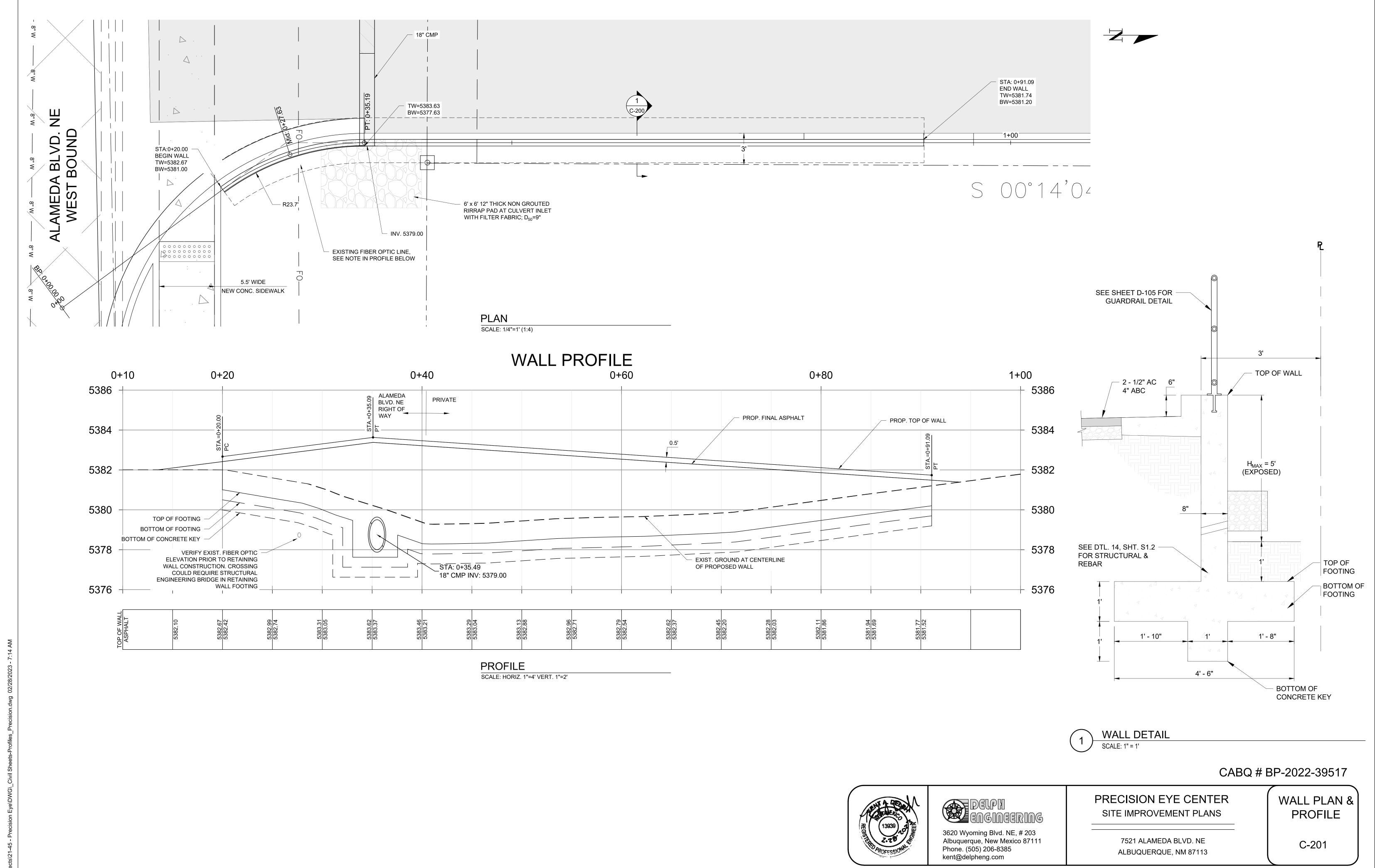


3620 Wyoming Blvd. NE, # 203 Albuquerque, New Mexico 87111 Phone. (505) 206-8385 kent@delpheng.com

PRECISION EYE CENTER SITE IMPROVEMENT PLANS

7521 ALAMEDA BLVD. NE ALBUQUERQUE, NM 87113 DRAINAGE PLAN

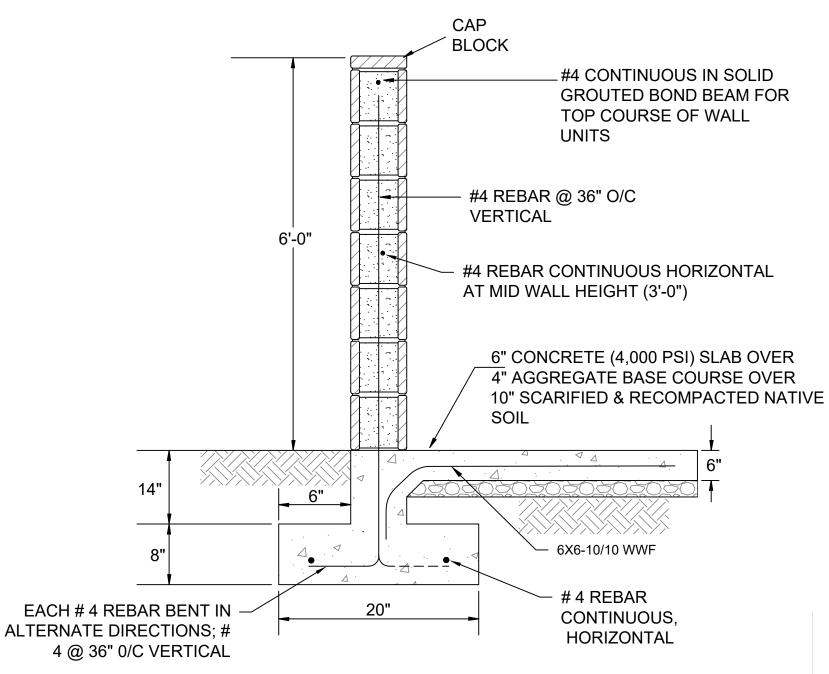
C103



KEY NOTES - TRASH ENCLOSURE

- 1. CONCRETE SLAB & APRON TO BE 6" THICK WITH 6" X 6" WELDED WIRE MESH. BOTH SHALL BE PLACED OVER 4" AGGREGATE BASE COURSE OVER 10" OF SCARIFIED AND RECOMPACTED NATIVE MATERIAL. CONCRETE FOR SLAB & APRON SHALL BE 4,000 PSI WITH \(\frac{4}{3} \)" AGGREGATE.
- 2. PROVIDE $\frac{1}{2}$ " EXPANSION JOINT BETWEEN SLAB & APRON & BETWEEN CMU WALL AND SLAB.
- 3. DRILL HOLES IN IN THE GATE OPEN & CLOSED POSITION, VERIFY LOCATION WITH GATE HARDWARE PRIOR DRILLING. (4 TOTAL).
- 4. <u>BOLLARD</u>: PROVIDE 6' LONG 6" DIAMETER STEEL PIPE FILLED WITH CONCRETE, VERTICALLY LOCATED 4' ABOVE THE SLAB & 2' BELOW TOP OF SLAB. PROVIDE CONCRETE AROUND LOWER 1.5' OF BOLLARDS, 12" BELOW & AROUND STEEL PIPE. CONCRETE FOR BOLLARD INFILL AND SUBGRADE SURROUNDING 12" SHALL HAVE A MINIMUM STRENGTH OF 2,500 PSI. (4 TOTAL).
- 5. 12" X 12" THICKENED EDGE, ALL FOUR SIDES OF APRON & SOUTH SIDE OF SLAB.
- 6. 2 EA ENCLOSURE GATES FRAME TO BE CONSTRUCTED OF $3"x3"x\frac{3}{16}"$ SQUARE STEEL TUBING W/ INTER CROSS-BRACE AND $\frac{1}{8}"$ STEEL PLATE SURFACE WELDED ON BOTH SIDES.
- 7. INSTALL CONCRETE VERTICAL CURB PER DETAIL 4, SHEET D-102.

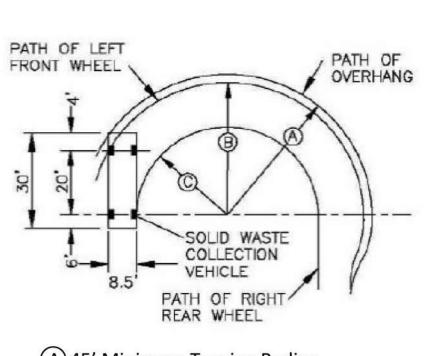
1 TRASH ENCLOSURE DETAIL



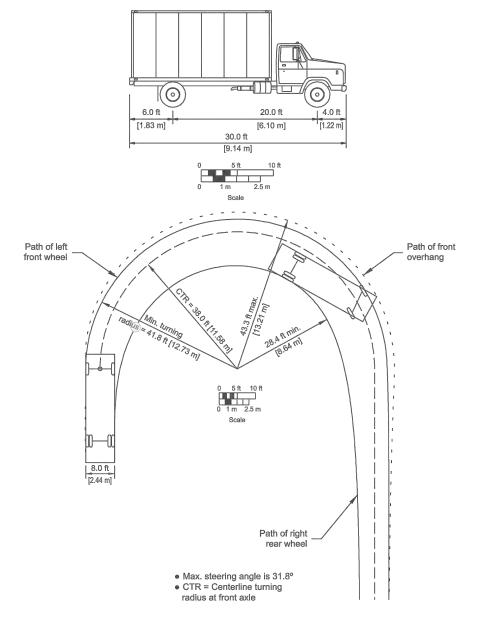
SCREEN WALL & FOOTING DETAIL

NOTES - SCREEN WALL

- 1. FOOTING TO BE PLACED ON 10" DEPTH SCARIFIED & RECOMPACTED NATIVE SOIL WITH A MIN. OF 95% OF MAX DENSITY PER STANDARD PROCTOR.
- 2. ALL CELLS CONTAINING REBAR SHALL BE SOLID GROUTED (VERTICAL & HORIZONTAL REINFORCEMENT). CELLS NOT CONTAINING REBAR SHALL NOT BE GROUTED
- 3. REBAR: ASTM STANDARD A615M GRADE 40 OR 60
- 4. 8" CMU TO BE fm = 1350 ps
- 5. ACCEPTABLE WALL MATERIALS INCLUDE BUT ARE NOT LIMITED TO, WOOD OR STUCCO OVER CMU OR OTHER STRUCTURAL MATERIALS
- 6. WALL COLOR SHALL BE OPAQUE, EARTH TONE OR MATCH THE NEW BUILDING
- 7. GROUT: ASTM TYPE S fm = 1800 psi

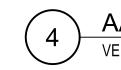


- A 45' Minimum Turning Radius
- B 42' Turning Radius
- © 30' Turning Radius



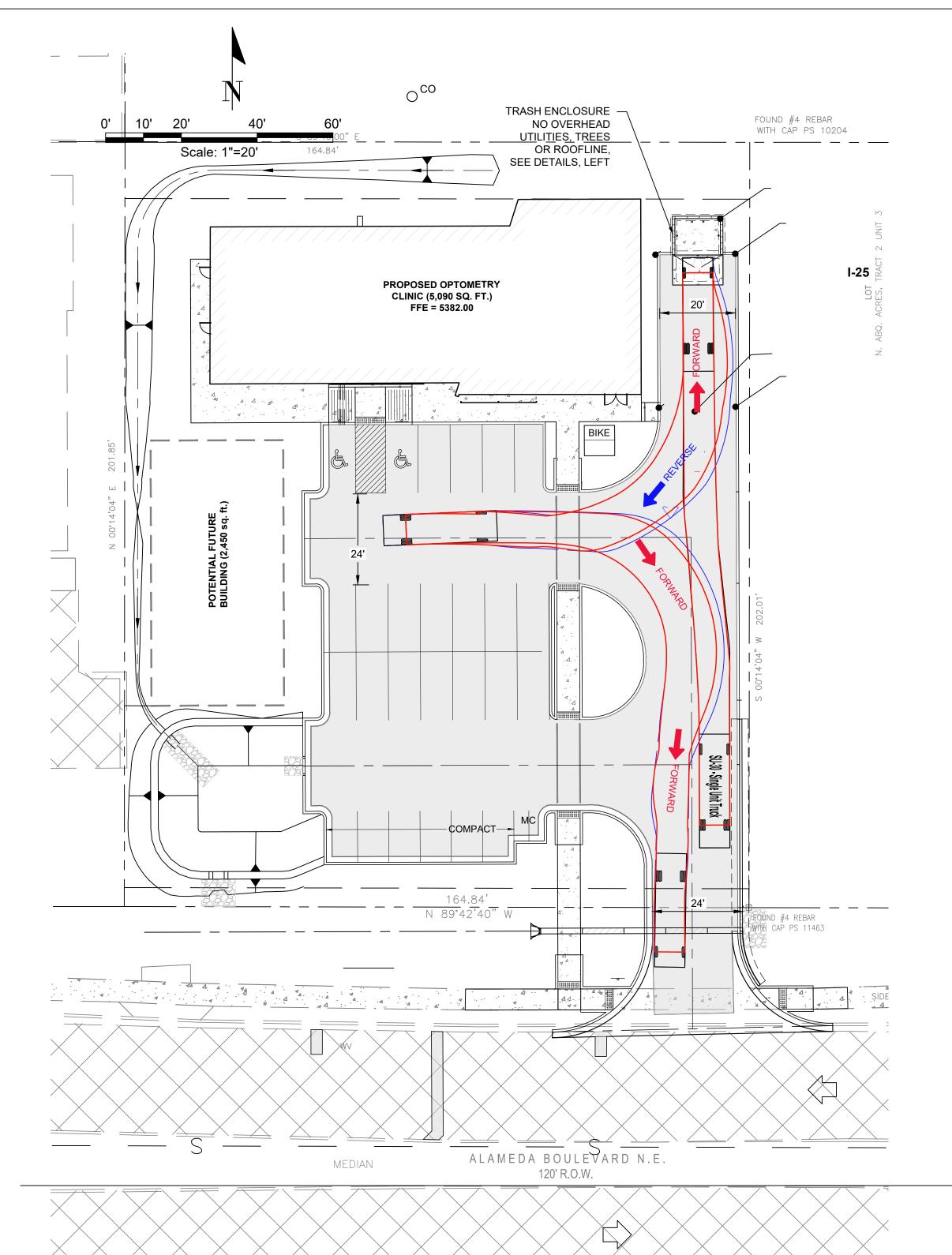
3 FROI

FRONT LOAD GARBAGE TRUCK
TYPICAL TURNING MOVEMENTS, LENGTH APPROX.,



AASHTO SU-TURNING MOVEMENTS

VEHICLE SHOWN IN CIVIL 3D VEHICLE TRACKING APPLICATION



SOLID WASTE ACCESS PLAN

SCALE: 1"=20'

SOLID WASTE DEPARTMENT - ACCESS APPROVAL
CITY OF ALBUQUERQUE

PRINTED NAME	TITLE

SIGNATURE

DATE

CABQ # BP-2022-39517



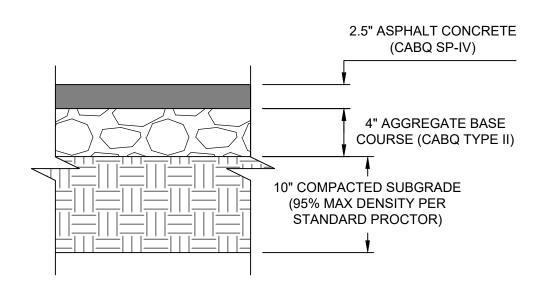
DELPH ENGINEERING

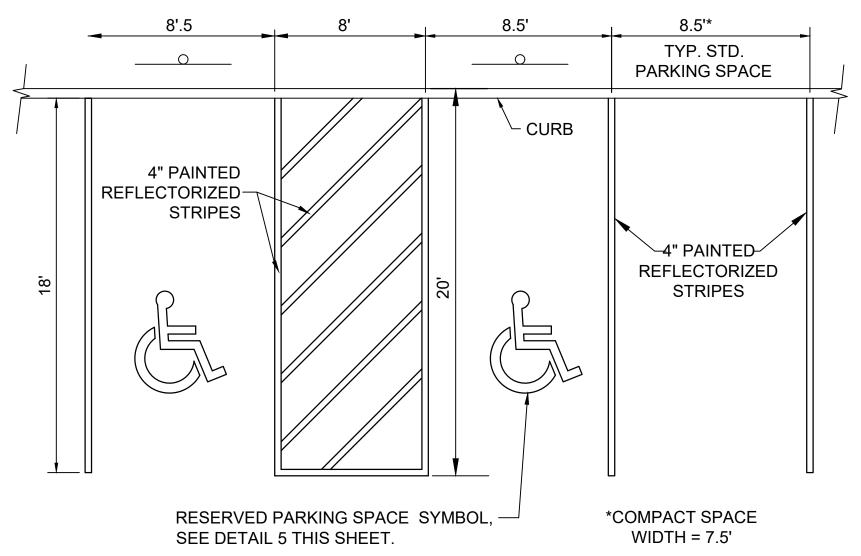
3620 Wyoming Blvd. NE, # 203 Albuquerque, New Mexico 87111 Phone. (505) 206-8385 kent@delpheng.com PRECISION EYE CENTER
SITE IMPROVEMENT PLANS

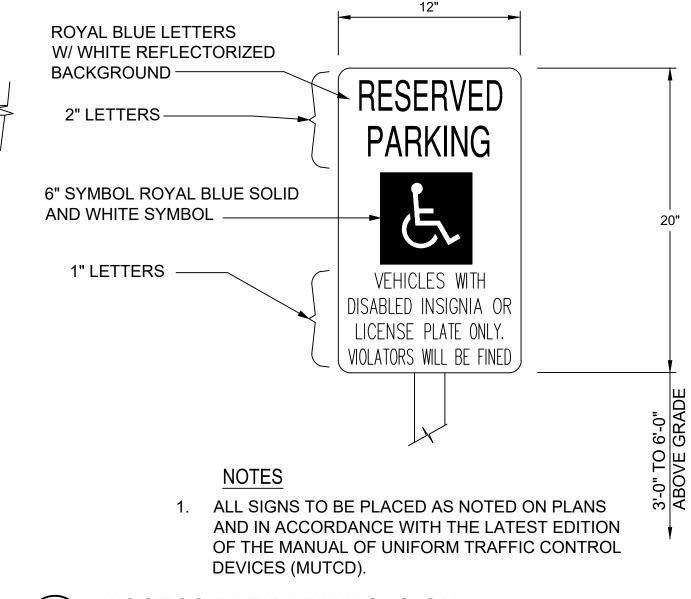
7521 ALAMEDA BLVD. NE ALBUQUERQUE, NM 87113 TRASH ENCOLOSURE, ACCESS & DETAILS

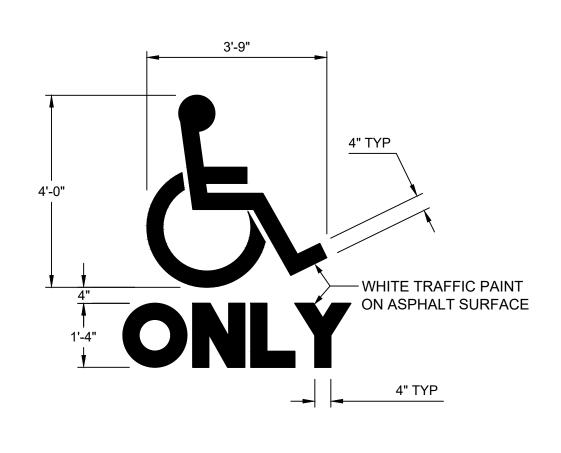
C-301

C-301







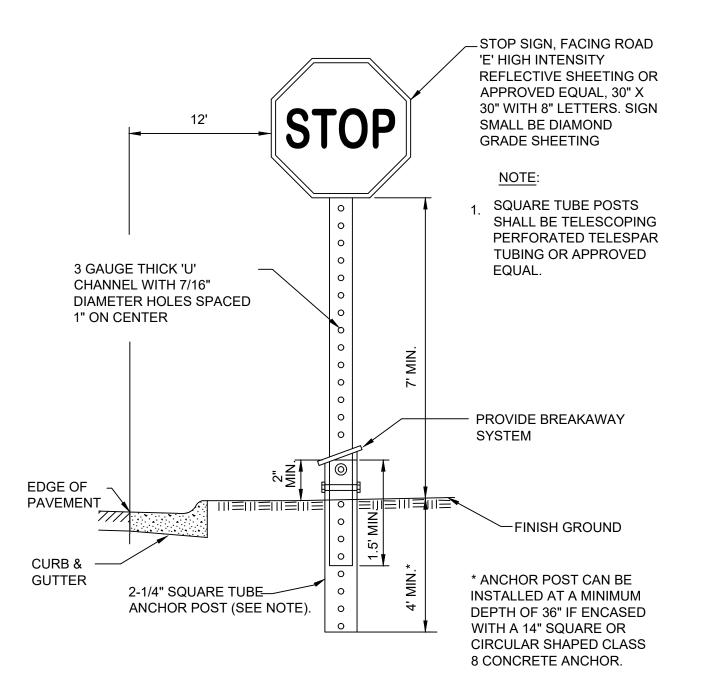


STANDARD & ACCESSIBLE PARKING SPACE

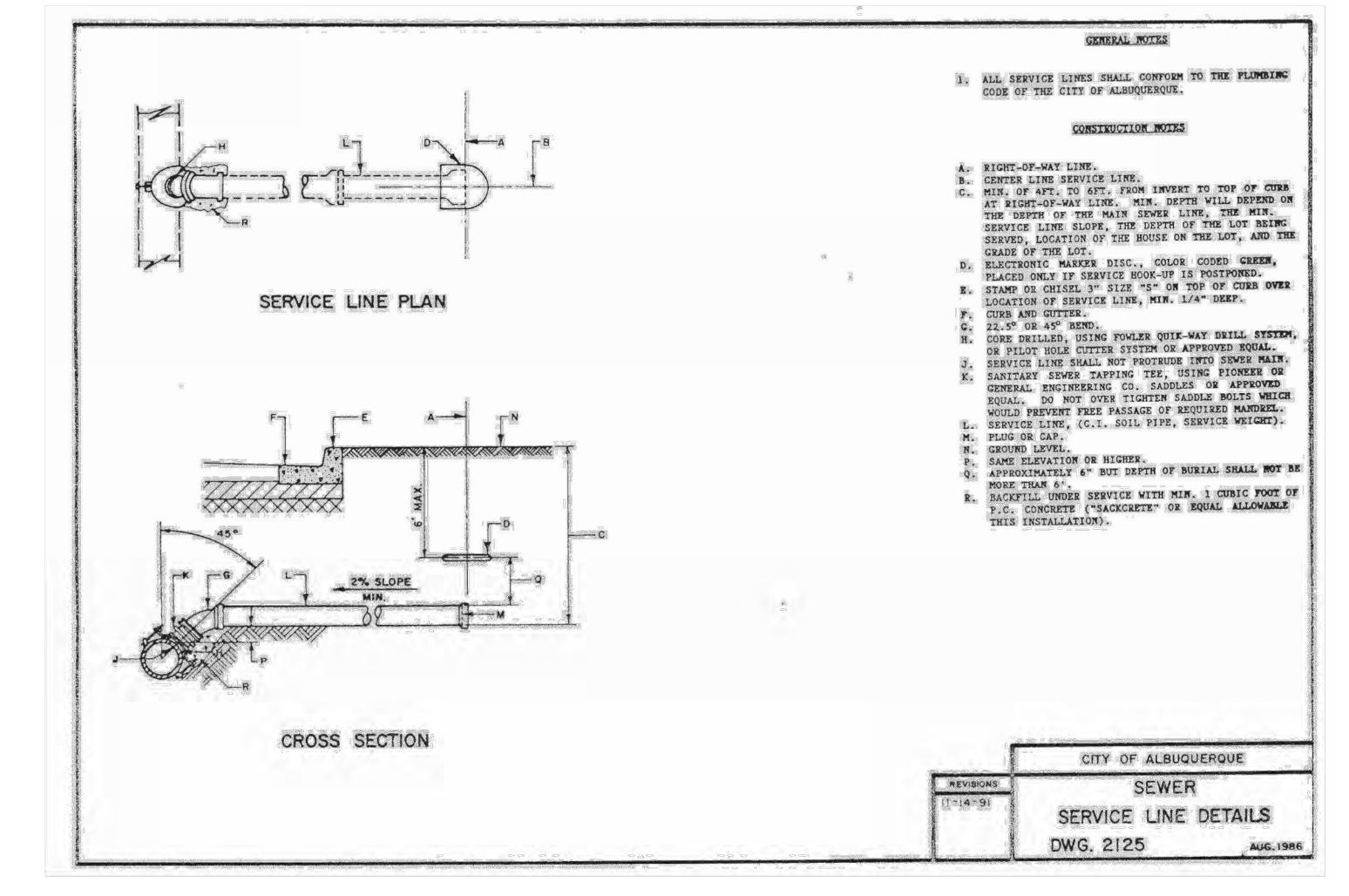
ACCESSIBLE PARKING SIGN

RESERVED PARKING SPACE SYMBOL

PAVEMENT SECTION







SEWER SERVICE LINE DETAILS - (CABQ - 2125)

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PRECISION EYE CENTER SITE IMPROVEMENT PLANS

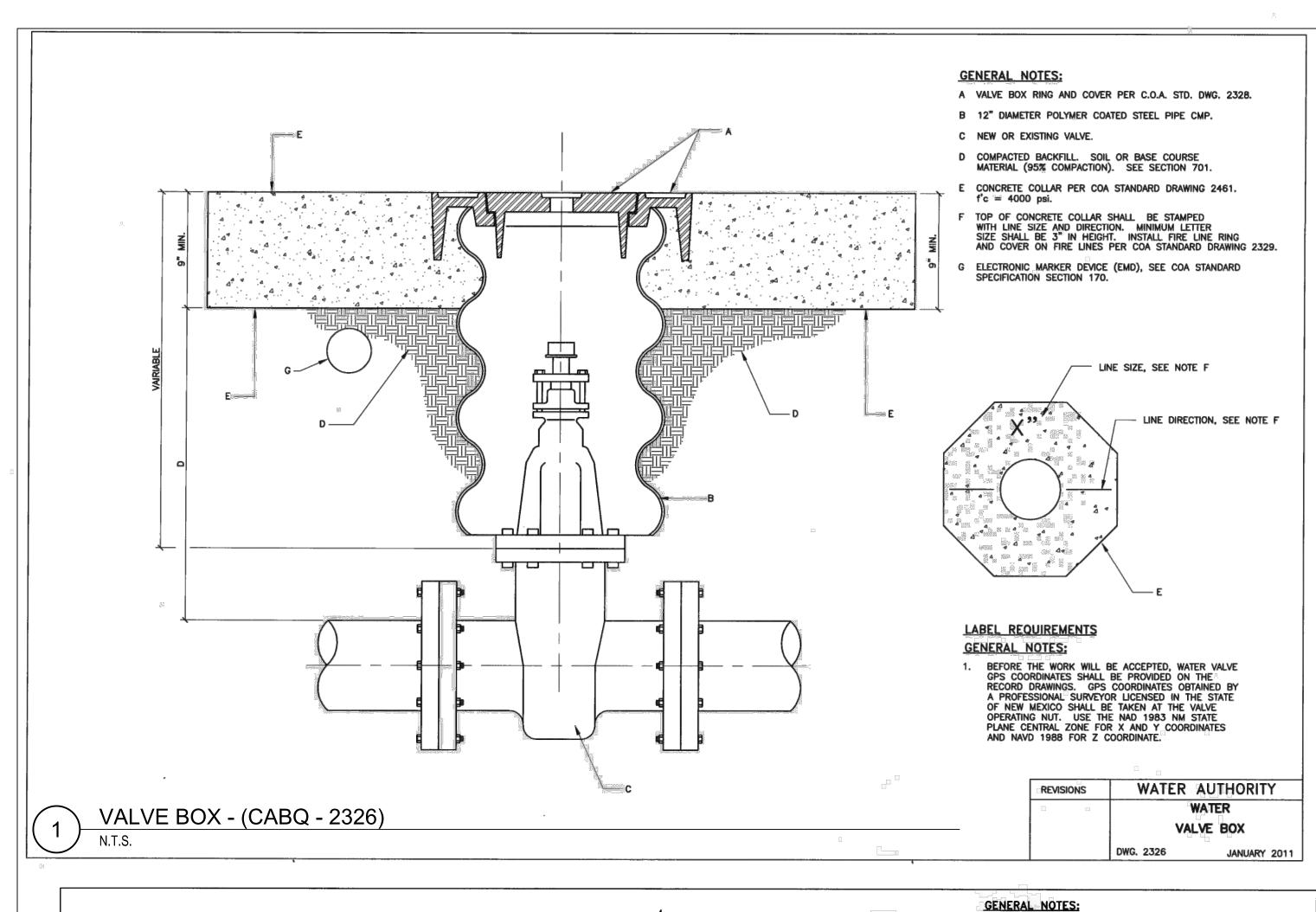
> 7521 ALAMEDA BLVD. NE ALBUQUERQUE, NM 87113

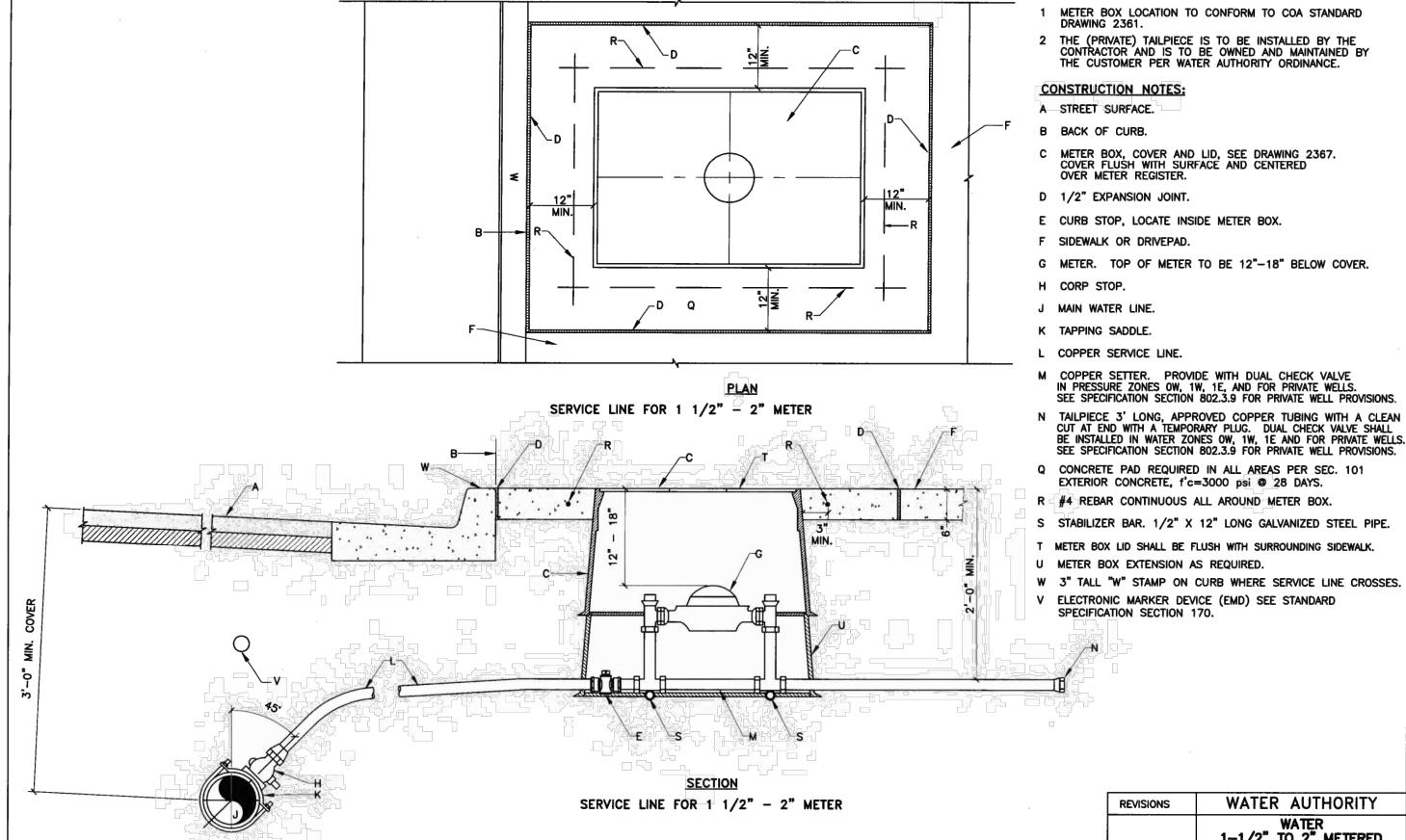
CIVIL **DETAILS**

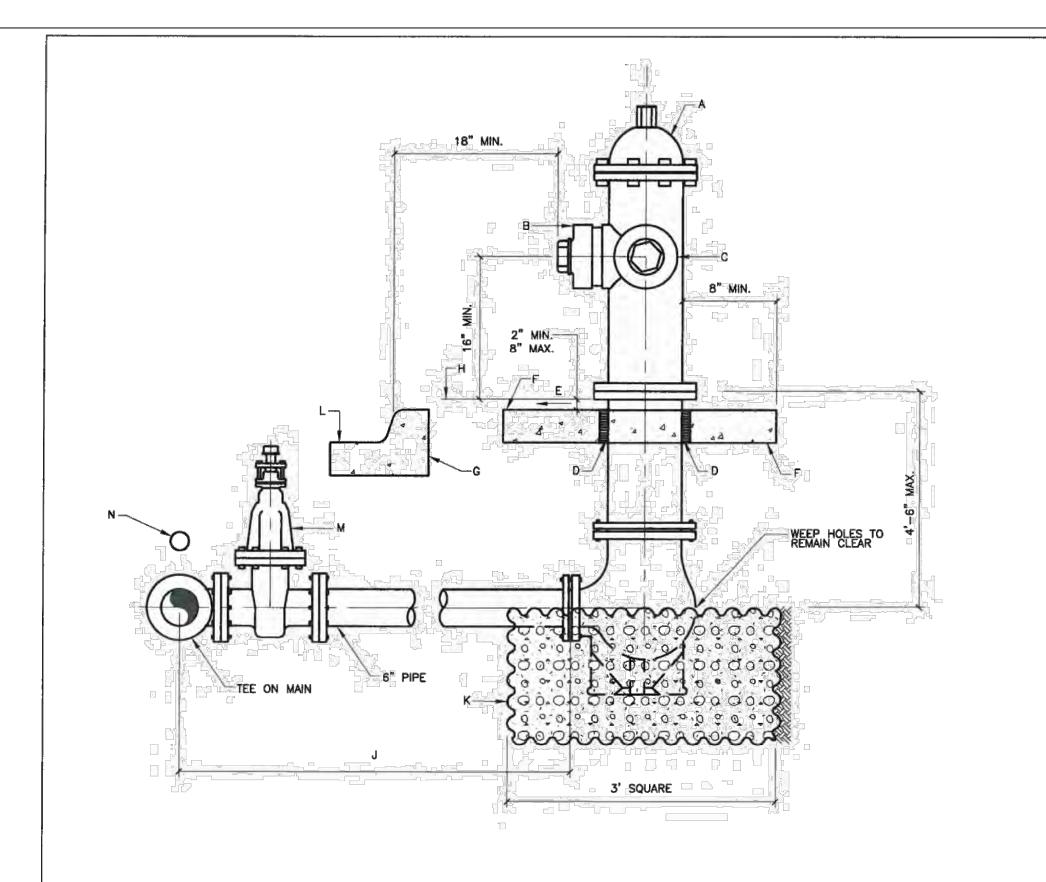
CABQ # BP-2022-39517

D-101

engineering







FIRE HYDRANT INSTALLATION - (CABQ 2340)

GENERAL NOTES:

- 1 NO OBSTRUCTIONS WILL BE PERMITTED WITHIN 3'-0" OF FIRE HYDRANT.
- 2 HYDRANT LEG SHALL BE VALVED.
- 3 CONTRACTOR SHALL BE RESPONSIBLE FOR SETTING TOP FLANGE OF FIRE HYDRANT TO THE CONTROLLED ELEVATION LINE.
- 4 FOR FIRE HYDRANT LOCATIONS, SEE DWG. 2347.
- 5 WHEN NEW OR EXISTING SIDEWALK ABUTS CURB, RECONSTRUCT SIDEWALK PER DWG. 2430, 2431.
- 6 PUMPER NOZZLE TO BE SET FACING THE TRAVELED WAY, UNLESS OTHERWISE NOTED ON PLANS.
- 7 HYDRANTS INSTALLED IN SIDEWALK AREAS SHALL MAINTAIN A MIN. 36-INCH CLEAR PEDESTRIAN PATH PER ADA STANDARD.
- 8 BEFORE THE WORK WILL BE ACCEPTED, FIRE HYDRANT GPS COORDINATES SHALL BE PROVIDED ON THE RECORD DRAWINGS. GPS COORDINATES OBTAINED BY A PROFESSIONAL SURVEYOR LICENSED IN THE STATE OF NEW MEXICO SHALL BE TAKEN AT THE FLANGE. USE THE NAD 1983 NM STATE PLANE CENTRAL ZONE FOR X AND Y COORDINATES AND NAVD 1988 FOR Z COORDINATE.

CONSTRUCTION NOTES:

- A FIRE HYDRANT PER SPECIFICATIONS.
- B PUMPER NOZZLE 4 1/2".
- C HOSE NOZZLE 2 1/2".
- D 1/2" EXPANSION JOINT MATERIAL.

f'c=3000 psi @ 28 DAYS.

- E MATCH SIDEWALK SLOPE OR SLOPE 1/4" PER FOOT.
- F 3'x3'x6" CONCRETE SQUARE PAD, TO BE CONSTRUCTED AROUND FIRE HYDRANT'S CENTER LINE WHEN NOT LOCATED WITHIN SIDEWALK OR CONCRETE AREA. CONCRETE PER SEC. 101 EXTERIOR CONCRETE,
- G BACK OF CURB.
- H CONTROLLED ELEVATION LINE, LEVEL IN ALL DIRECTIONS.
- J USE OF RESTRAINED JOINTS IS MANDATORY. ALL FIRE HYDRANT LEG PIPING AND FITTINGS INCLUDING TEE ON MAIN SHALL BE RESTRAINED JOINT.
- K GRAVEL DRAIN POCKET. USE FILTER FABRIC TO COVER AROUND GRAVEL DRAIN POCKET. ASTM C33, NO.57 GRAVEL.
- L CURB AND GUTTER. WHERE NO CURB AND GUTTER EXIST, BOLLARDS ARE REQUIRED.
- M FIRE HYDRANT ISOLATION VALVE.
- N ELECTRONIC MARKER DEVICE (EMD), SEE COA STANDARD SPECIFICATION SECTION 170.

WATER AUTHORITY REVISIONS WATER FIRE HYDRANT INSTALLATION

DWG. 2340

JANUARY 2011

GENERAL NOTES 1. CURB, GUTTER AND CUT-OFF WALL WILL BE

<u>MOUNTABLE</u>

MEDIAN CURB

8" DEPRESSED

<u>GUTTER</u>

- CONSTRUCTED OF PORTLAND CEMENT CONCRETE (PCC). 2. FOR STANDARD AND MEDIAN C & G ADJACENT TO ASPHALT CONCRETE (AC) PAVEMENT, PROVIDE CONTRACTION JOINTS AT 12' MAX. SPACING, CONTRACTION JOINTS SHALL BE EITHER SAWED OR TOOLED A MINIMUM OF 1" DEEP AT FINISHED FACES. 1/2" EXPANSION JOINTS TO BE INSTALLED AT CURB RETURNS AND AT A MAXIMUM SPACING OF 200' BETWEEN CURB RETURNS AND SEPARATELY CONSTRUCTED DRIVEWAYS.
- 3. FOR ALL OTHER C & G AND CUT-OFF WALL PROVIDE CONTRACTION JOINTS AT 10' MAX SPACING, 1/2" EXPANSION JOINTS AT CURB RETURNS & AT A MAXIMUM SPACING OF 100' BETWEEN CURB RETURNS & EACH SIDE OF SEPARATELY CONSTRUCTED DRIVEWAYS. CONTRACTION JOINTS SHALL BE EITHER SAWED OR TOOLED A MINIMUM OF 1" DEEP AT ALL FINISHED FACES. REINFORCEMENT SHALL NOT BE USED IN CUT-OFF WALLS.
- 4. FOR C & G CONSTRUCTED WITH PCC PAVEMENT, CONTRACTION JOINTS AND EXPANSION JOINTS SHALL BE THE SAME AS THE PAVEMENT JOINTS.
- 5. ALL EDGES SHALL BE EDGED WITH A 3/8" RADIUS
- 6. REMOVE & REPLACE PAVEMENT 1' WIDE ADJACENT TO LIP OF GUTTER WHEN CONSTRUCTING C & G ADJACENT
- TO EXISTING AC PAVEMENT. 7. 1/4" ISOLATION JOINT SHALL BE PLACED BETWEEN SIDEWALK AND C & G WHEN CAST ADJACENT TO EACH
- 8. ADA = AMERICANS WITH DISABILITY ACT.

CONSTRUCTION NOTES SEE COA DRAWING 2415B

> CITY OF ALBUQUERQUE REVISIONS CURB AND GUTTER DETAILS DWG. 2415A

JUNE 2019

CURB AND GUTTER DETAILS

<u>6"MEDIAN C & G</u>

<u>6" STANDARD C & G</u>

6" VERTICAL CURB



<u>6"DEPRESSED</u>

MEDIAN C & G

8" STANDARD C & G



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PRECISION EYE CENTER SITE IMPROVEMENT PLANS

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CIVIL **DETAILS**

CABQ # BP-2022-39517

D-102

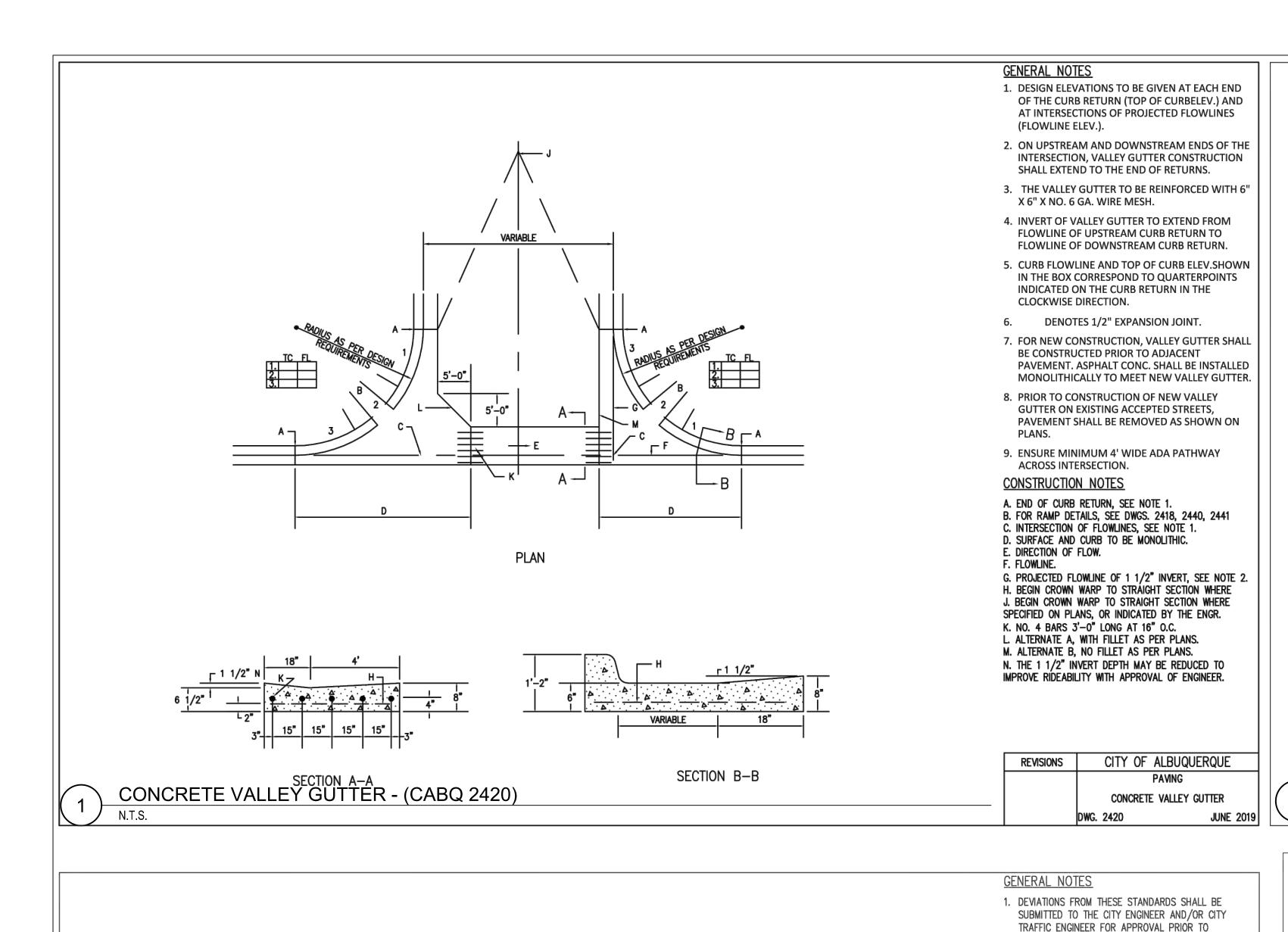
1-1/2" TO 2" METERED SERVICE LINE - (CABQ 2363)

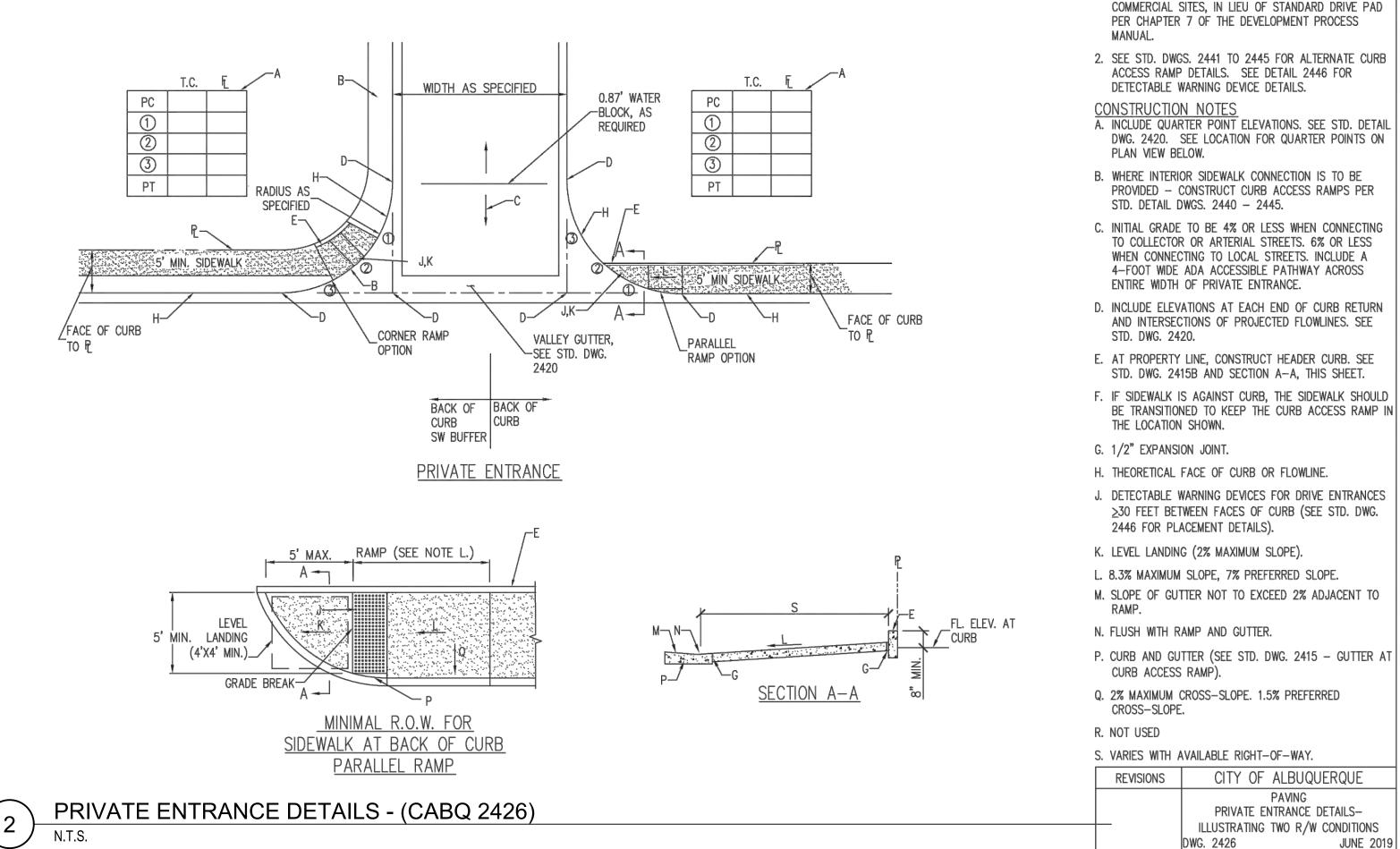
1-1/2" TO 2" METERED SERVICE LINE INSTALLATION

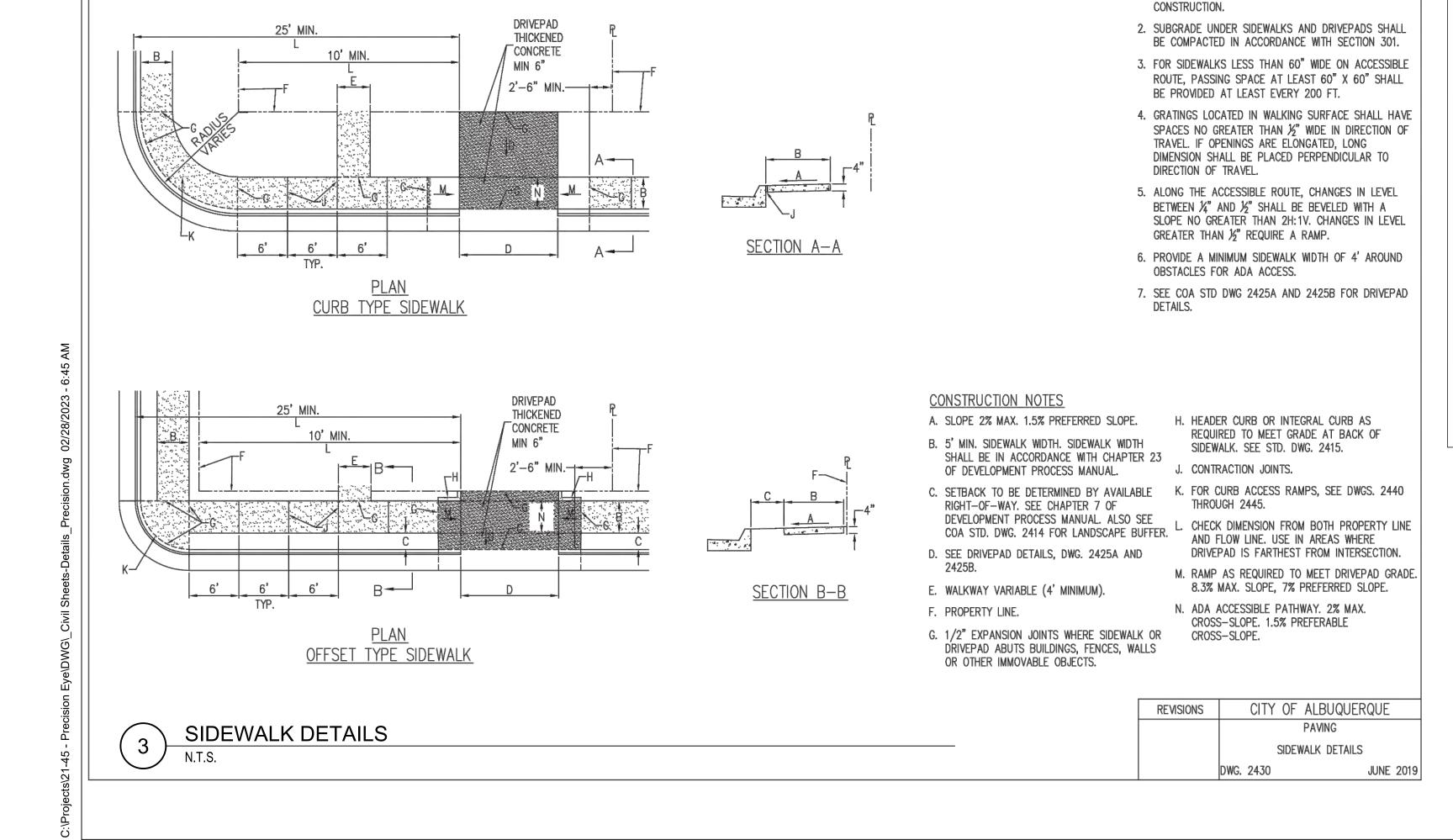
JANUARY 2011

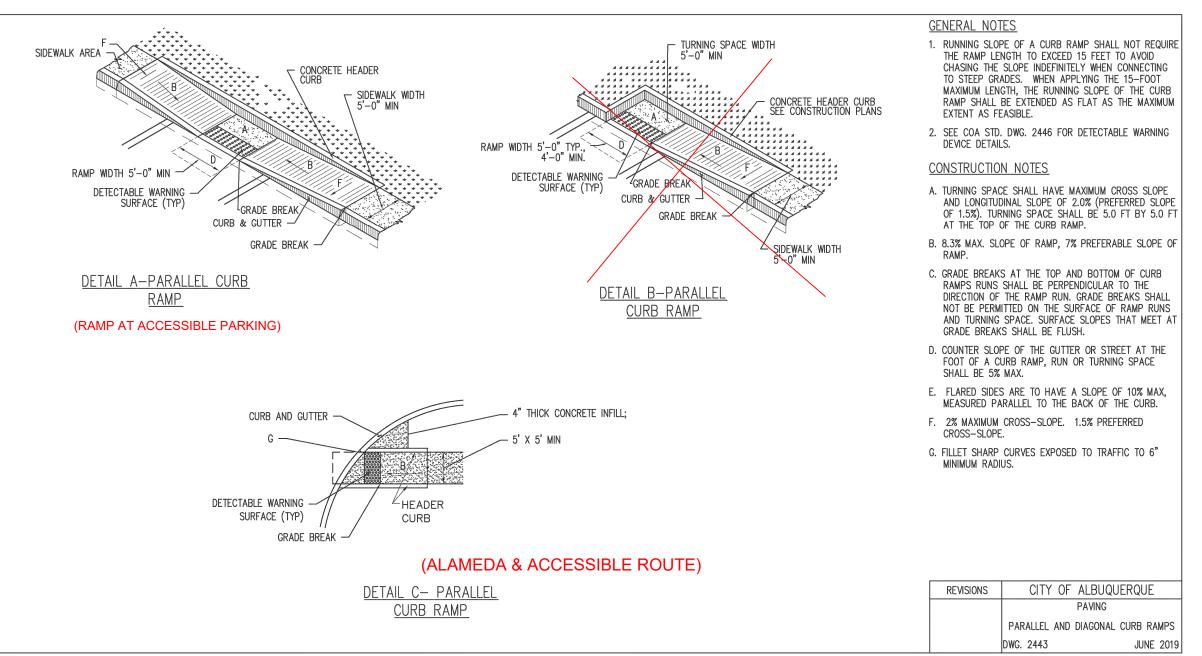
DWG. 2363

ESTATE CURB









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CABQ # BP-2022-39517

GENERAL NOTES

1. THESE DETAILS ARE PROVIDED FOR HIGH TRAFFIC

VOLUME PRIVATE ENTRANCES SUCH AS ENTRANCES TO



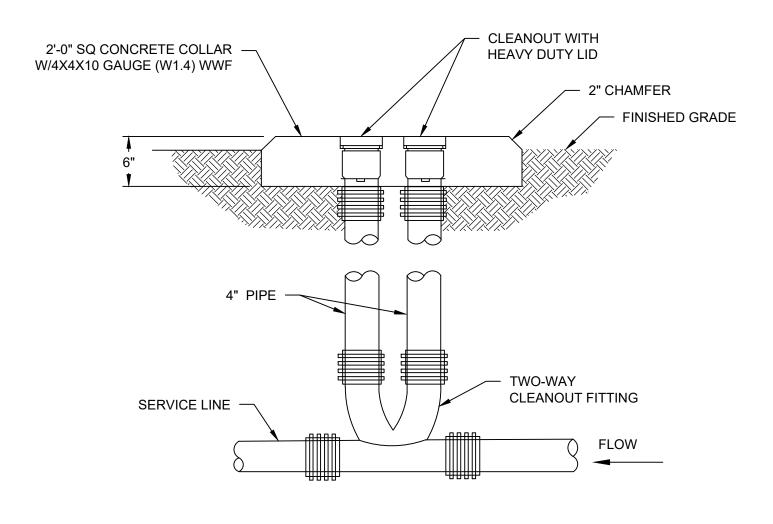
PRECISION EYE CENTER SITE IMPROVEMENT PLANS ENGINEERING

> 7521 ALAMEDA BLVD. NE ALBUQUERQUE, NM 87113

CIVIL **DETAILS**

JUNE 2019

D-103



DOUBLE CLEANOUT DETAIL

SINGLE CLEANOUT DETAIL

DIMENSIONS DIMENSIONS A (IN.) B (IN.) H (IN.) L (IN.) W (IN.) DIMENSIONS DIMENSIONS A (IN.) B (IN.) H (IN.) L (IN.) W (IN.) (±1") (MAX.) (±1") (±1½") (±2") 14 14 19 9 60 72 2 1/2:1 2 PC. 12 16 22 11 69 84 2 1/2 1 2 PC. 12 18 27 12 78 90 2 1/4.1 2 PC. 12 18 30 12 84 102 2:1 2 PC. 12 18 33 12 87 114 1 3/4:1 3 PC PLAN PLAN - REINFORCED
 72
 12
 18
 39
 12
 87
 126
 1 1/3:1
 3 PC.

 78
 12
 18
 42
 12
 87
 132
 1 1/4:1
 3 PC.

 84
 12
 18
 45
 12
 87
 138
 1 1/16:1
 3 PC.
 * THE CONTRACTOR SHALL VERIFY WITH PROVIDERS FOR CURRENT INDUSTRY SIZES. NOTE: . THE CONTRACTOR SHALL VERIFY WITH PROVIDERS FOR CURRENT INDUSTRY SIZES. 1. ALL 3 PIECE BODIES TO HAVE 12 GAUGE THICK SIDES AND 10 GAUGE THICK CENTER PANELS. WIDTH OF CENTER PANELS TO BE GREATER THAN 20% OF THE PIPE PERIPHERY MULTIPLE PANEL BODIES TO HAVE LAP SEAMS WHICH ARE TO BE TIGHTLY JOINED BY 3/8" GALVANIZED RIVETS OR BOLTS. ALL J PIECE BODIES TO HAVE 12 GAUGE THICK SIDES AND 10 GAUGE CENTER PANELS. WIDTH OF CENTER PANELS TO BE GREATER THAN 20% OF THE PIPE PERIPHERY. MULTIPLE PANEL BODIES TO HAVE LAP SEAMS WHICH ARE TO BE TIGHTLY JOINED - TOE PLATE A 2. FOR 77" X 52" AND 83" X 57" SIZES, REINFORCED EDGED TO BE SUPPLEMENTED BY L 2" X 2" X 1/4" GALVANIZED ANGLES. - * TOE PLATE 12" MAX SPA BY 3/8" Ø GALVANIZED RIVETS OR BOLTS 12" MAX. SPA. THE ANGLES TO BE ATTACHED BY 3/8" GALVANIZED NUTS AND CENTER TO CENTER FOR 60" THRU 84" SIZES, REINFORCED EDGES TO BE SUPPLEMENTED WITH GALVANIZED STIFFENER ANGLES. THE ANGLES WILL BE L 2" x 2" x 1/4" FOR 60" THRU 78" DIAMETER AND L 2 1/2" x 2 1/2" x 1/4" FOR 78" AND 84" DIAMETER. THE ANGLES TO BE ATTACHED BY 3/8" GALVANIZED NUTS AND BOLTS. CENTER TO CENTER ANGLE REINFORCEMENT WILL BE PLACED UNDER THE CENTER PANEL SEAMS ON THE 77" X 52" AND 83" X 57" SIZES. ELEVATION ELEVATION NOTE: SIZES EQUIVALENT TO THE ABOVE, USING 3" x 1"
CORRUGATIONS, MAY BE USED PROVIDING THAT THEY
MEET THE SIZES SHOWN UNDER TABLE 6 OF SERIAL . 4. TOE PLATE TO BE CONSTRUCTED WHERE SHOWN ON PLANS 206-04-1/3 THRU 206-04-3/3 * 3 TOE PLATE TO BE CONSTRUCTED WHERE SHOWN ON PLANS. TYPE 1 TYPE 2 FOR 30" THRU 84" TYPE 2 FOR 12" THRU 24" FOR 17" x 13" STANDARD CONNECTION THRU 57" x 38" STANDARD CONNECTIONS STANDARD END SECTIONS FOR PIPE-ARCH STEEL PIPE STANDARD END SECTIONS FOR ROUND STEEL PIPE 7" FOR PIPE TO 37" 12" FOR PIPE OVER 37" PIPE DIAM. DIAM. L W (IN.) (IN.)

18 19 30

21 23 36

24 28 42

30 31.5 48 GENERAL NOTES REV.BY DESCRIPTION
REVISIONS (OR CHANGE NOTICES) 1. FOR MULTIPLE INSTALLATION OF ALL TYPES, A MIN. OF A 2'-0" SPACING MEASURED ALONG THE HORIZONTAL BETWEEN FLARED END SECTIONS NEW MEXICO AT THEIR WIDEST CROSS SECTION SHALL BE USED. DEPARTMENT OF TRANSPORTATION 36 38.5 60 42 47 75 WELDING WILL NOT BE PERMITTED IN CONNECTING END SECTIONS TO CONNECTOR SECTIONS OR CONNECTOR SECTIONS TO PIPE. STANDARD DRAWING 48 54 85 60 63 96 66 70 112 72 77 128 3. TYPE 1 AND TYPE 2 MAY BE USED WITH WELDED SEAM OR LOCKSEAM CONNECTIONS HELICALLY CORRUGATED PIPE WITH REROLLED ENDS. REROLLED ENDS SHALL INCLUDE A MINIMUM OF TWO ANNULAR CORRUGATIONS OF THE SAME SIZE AS THE PIPE CORRUGATIONS. CULVERT PIPE END SECTIONS PLAN (METAL)

3 CULVERT PIPE END SECTIONS - (NMDOT 570-02-1/2)

Sheet 570-02

CABQ # BP-2022-39517

570-02-1/2

DESIGNED BY DRAWN BY SKL CHECKED BYTM/YML

13939 21 HE STORY OF THE STORY

DELPH ENGINEERING

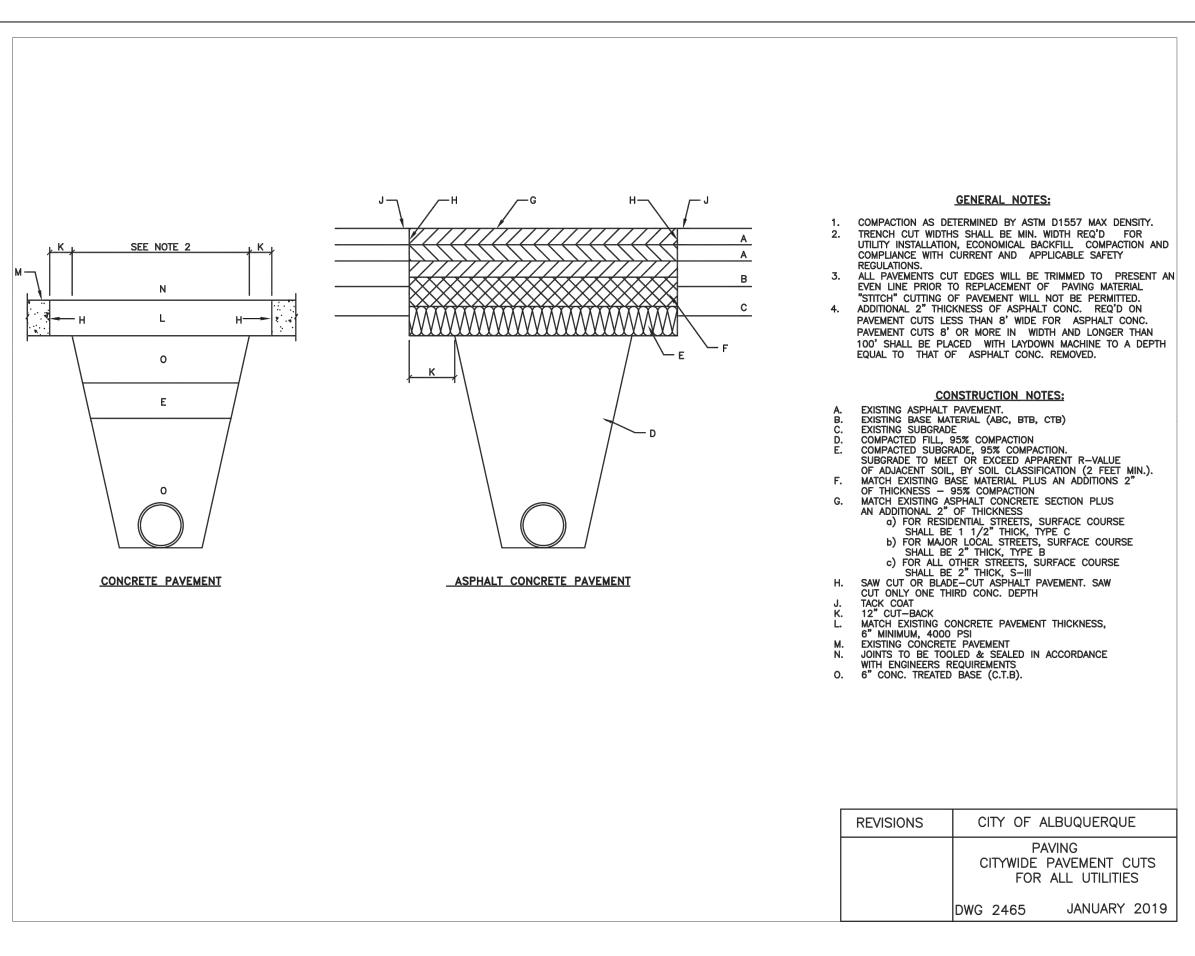
CORRUGATED ALUMINUM PIPE END SECTION

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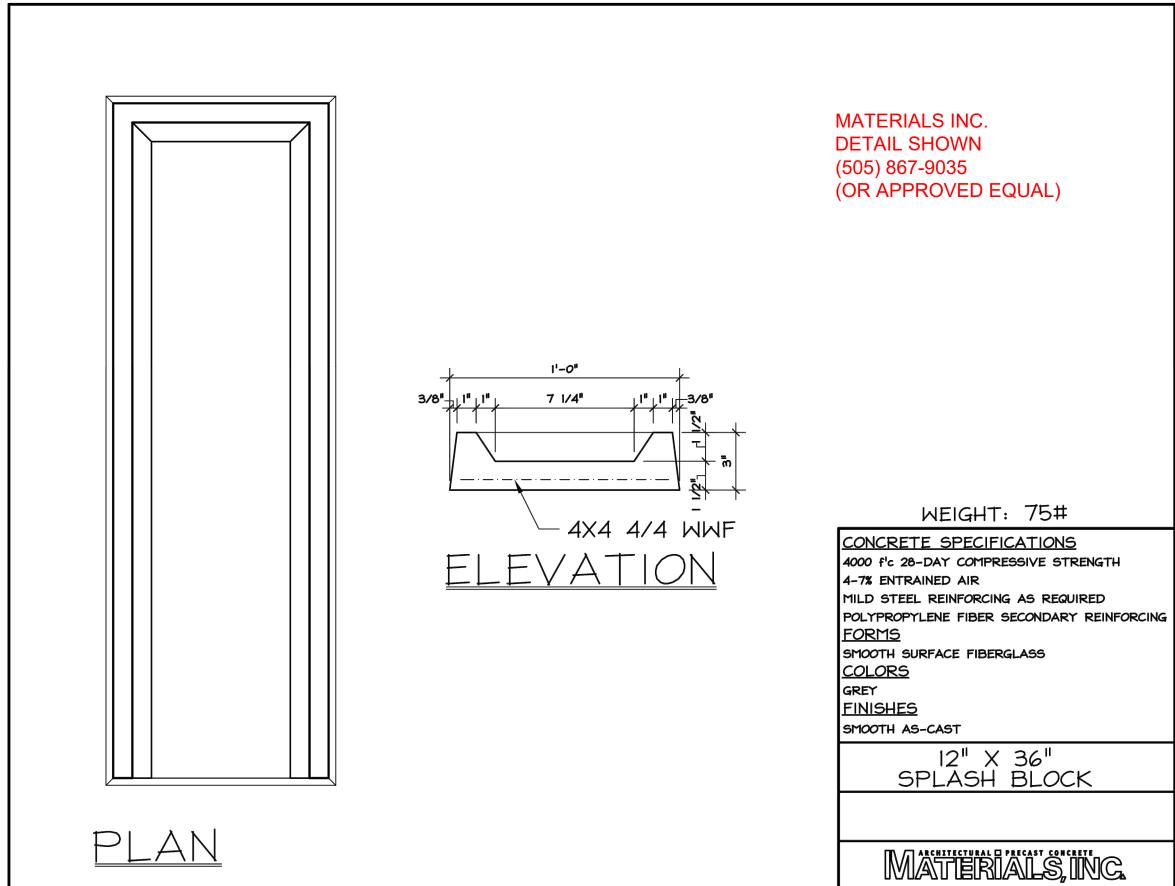
7521 ALAMEDA BLVD. NE ALBUQUERQUE, NM 87113 CIVIL DETAILS

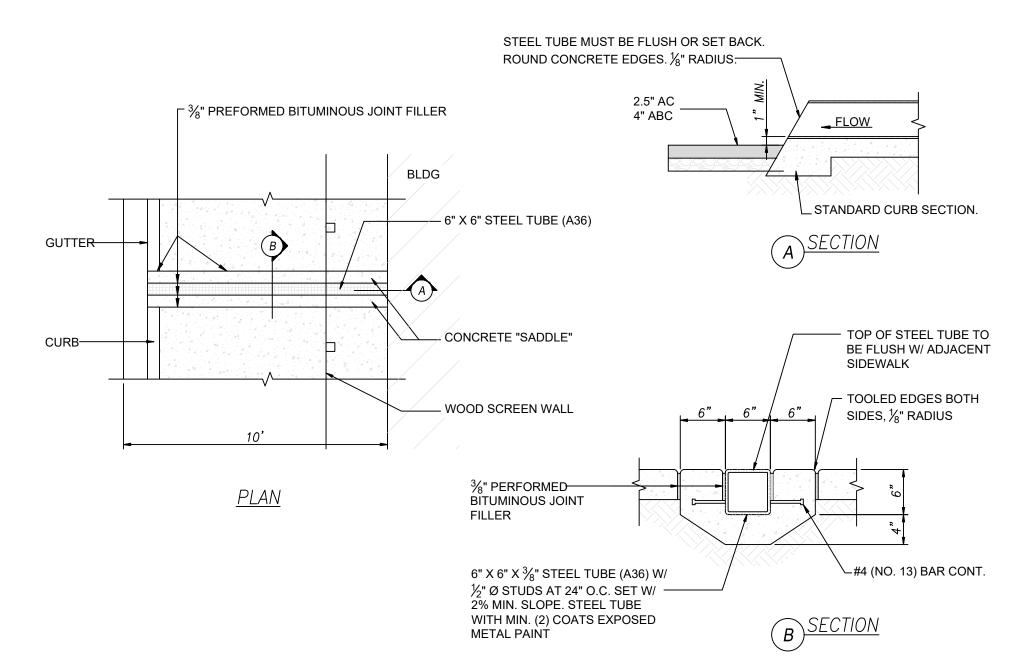
1 of 2

D-104



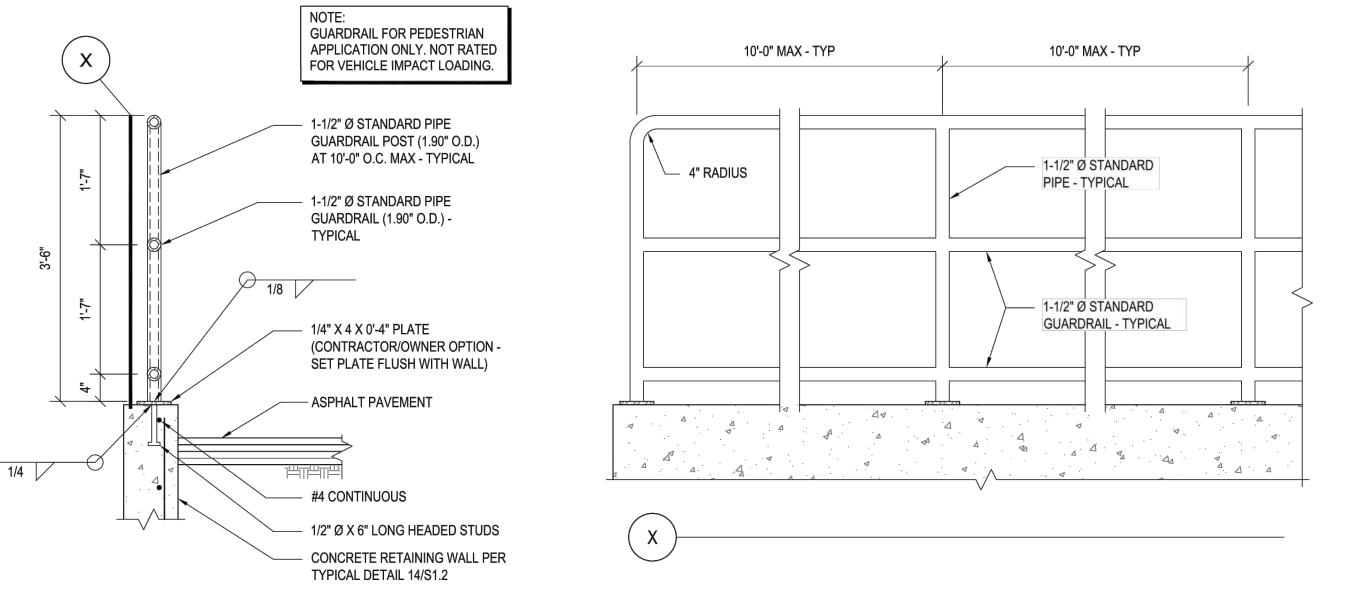
PAVEMENT CUT & REPAIR FOR UTILITIES





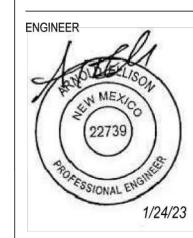
SIDEWALK SCUPPER

GUARDRAIL AT CONCRETE RETAINING WALL



(505) 360-3788 Farmington, NM

ARNOLD ELLISON, PE



PRECISION EYE (7521 ALAMEDA DE ALBUQUERQUE, NM

100% PERMIT DRAWINGS						
REVISION	DATE					
DATE	1/24/23					
PROJECT NO	02125					
SHEET NO.						
SK1						

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CIVIL **DETAILS**

D-105

CONCRETE SPLASH BLOCK