

CITY OF ALBUQUERQUE



May 18, 2006

Gary Werner, P.E.
Resource Technology, Inc.
5501 Jefferson Blvd. NE – Suite 200
Albuquerque, NM 87109

**Re: Vintner Court Subdivision, Lots 6 & 7, Block 16, Tract 3, NAA
Grading & Drainage Plan-Engineer's Stamp dated 4-23-06 (C20-D50)**

Dear Mr. Werner,

Based upon the information provided in your submittal dated 4-26-06, the above referenced plan is approved as amended. This will be the plan that must be certified for Release of Financial Guaranty.

P.O. Box 1293

If you have any other questions, you can contact me at 924-3990.

Albuquerque

Sincerely,

New Mexico 87103

Phillip J. Lovato, E.I., C.F.M.
Engineering Associate, Hydrology,
Development and Building Services,
Planning Department

www.cabq.gov

cc: file

Chapter 17 - Procedures for Drainage Submittals

DRAINAGE INFORMATION SHEET

PROJECT TITLE: Vintner Ct. Subdivision ZONE ATLAS/DRNG. FILE #: C-20-ED50
 DRB#: 05DRB-00826 EPC#: _____ WORK ORDER#: _____
 LEGAL DESCRIPTION: Lots 6 & 7, Blk 16, Tract 3, Unit 3, Vintner North Albq. Acres
 CITY ADDRESS: Vintner Ct
 ENGINEERING FIRM: Resource Technology, Inc CONTACT: Gary Werner
 ADDRESS: 5501 Jefferson Blvd NE Suite 200 PHONE: 243-7300
 CITY, STATE: Albq. NM ZIP CODE: 87109
 OWNER: Adil Rizvi CONTACT: Sama
 ADDRESS: 7049 Luella Anne, NE PHONE: 315-6484
 CITY, STATE: Albq. NM ZIP CODE: 87109
 ARCHITECT: _____ CONTACT: _____
 ADDRESS: _____ PHONE: _____
 CITY STATE: _____ ZIP CODE: _____
 SURVEYOR: Terrametrics of New Mexico CONTACT: Phil Turner
 ADDRESS: PO Box 30192 PHONE: 884-9087
 CITY, STATE: Albq. NM ZIP CODE: 87190-0192
 CONTRACTOR: _____ CONTACT: _____
 ADDRESS: _____ PHONE: _____
 CITY, STATE: _____ ZIP CODE: _____

TYPE OF SUBMITTAL

- ☒ DRAINAGE REPORT
☒ DRAINAGE PLAN
☒ CONCEPTUAL GRADING AND DRAINAGE PLAN
☐ GRADING PLAN
☐ EROSION CONTROL PLAN
☐ ENGINEER'S CERTIFICATION
☐ CLOMR/LOMR
☐ OTHER

WAS A PRE-DESIGN CONFERENCE ATTENDED:

- ☐ YES
☐ NO
☐ COPY PROVIDED

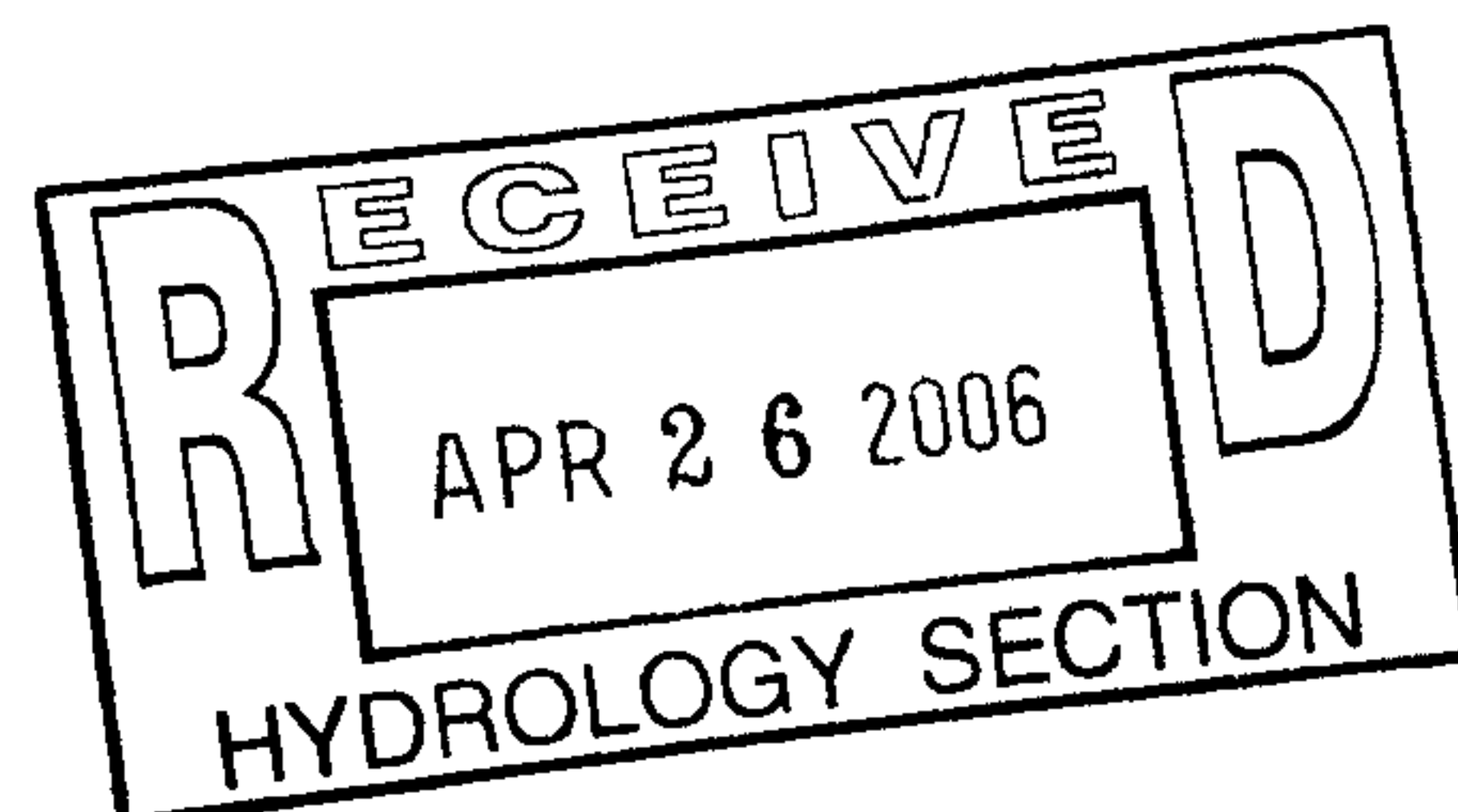
CHECK TYPE OF APPROVAL SOUGHT:

- ☐ SIA/FINANCIAL GUARANTEE RELEASE
☐ PRELIMINARY PLAT APPROVAL
☐ S. DEV. PLAN FOR SUB'D APPROVAL
☐ S. DEV. FOR BLDG. PERMIT APPROVAL
☐ SECTOR PLAN APPROVAL
☐ FINAL PLAT APPROVAL
☐ FOUNDATION PERMIT APPROVAL
☐ BUILDING PERMIT APPROVAL
☒ CERTIFICATE OF OCCUPANCY APPROVAL
☐ GRADING PERMIT APPROVAL
☐ PAVING PERMIT APPROVAL
☐ WORK ORDER APPROVAL
☐ OTHER (SPECIFY)

DATE SUBMITTED: _____ BY: Gary Werner

Requests for approvals of Site Development Plans and/or Subdivision Plats shall be accompanied by a drainage submittal. The particular nature, location and scope to the proposed development defines the degree of drainage detail. One or more of the following levels of submittal may be required based on the following:

1. **Conceptual Grading and Drainage Plan:** Required for approval of Site Development Plans greater than five (5) acres and Sector Plans.
2. **Drainage Plans:** Required for building permits, grading permits, paving permits and site plans less than five (5) acres.
3. **Drainage Report:** Required for subdivision containing more than ten (10) lots or constituting five (5) acres or more.





5501 Jefferson Blvd. NE, Ste. 200 Albuquerque, NM 87109
Telephone: (505) 243-7300
Fax: (505) 243-7400
E-Mail: rti@nm.net

TRANSMITTAL

TO: Plaza del Sol FROM: Gary Werner
Hydrology Section
DATE: 4/23/06
ATTN: Brad Bingham
RE: Vintner Ct G&D OUR PROJECT NO.: 05-190.B

WE TRANSMIT THE FOLLOWING:

- ☐ SHOP DRAWINGS ☐ SPECIFICATIONS ☐ CHANGE ORDER
☒ PRINTS ☐ COPY OF LETTER ☐ UNDER SEPARATE COVER VIA _____
☐ DOCUMENTS ☐ _____

FOR YOUR:

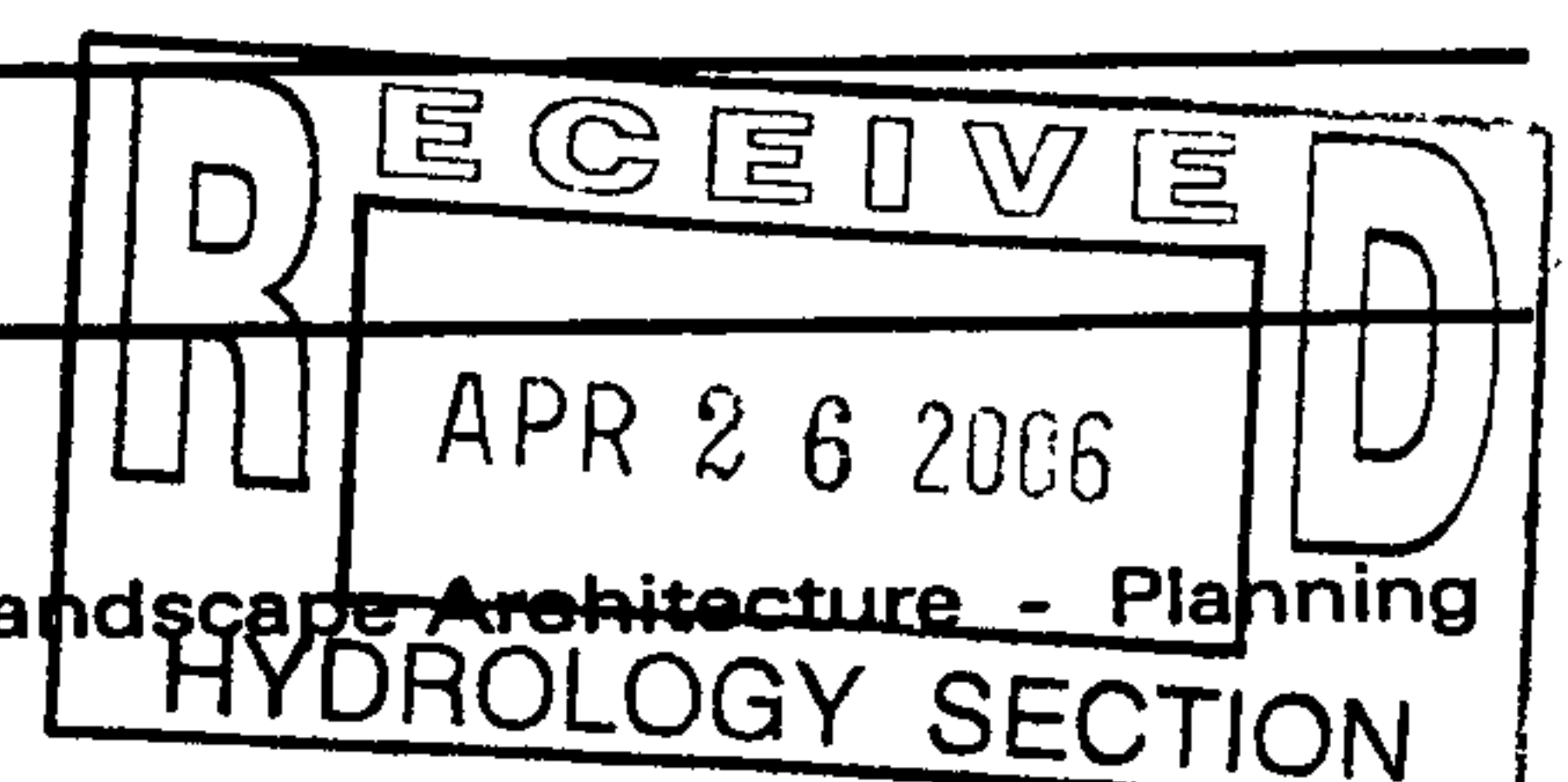
- ☒ APPROVAL ☐ DISTRIBUTION TO PARTIES
☒ REVIEW & COMMENT ☐ RECORD
☐ USE ☐ INFORMATION IF MATERIAL RECEIVED IS NOT AS LISTED, PLEASE NOTIFY US AT ONCE.

COPIES	DATE	ITEM DESCRIPTION
1	4/23/06	G&D Plans, sheets 4 & 7.
1	4/23/06	Drainage Information Sheet

REMARKS: _____

COPY TO: _____

Civil Engineering - Environmental Sciences - Water Resources - Landscape Architecture - Planning



July 1, 2002

Farkhondeh Mehrvarz
1100 St. Francis Dr., Room 2022
Joseph Montoya Building
Santa Fe, New Mexico 87502

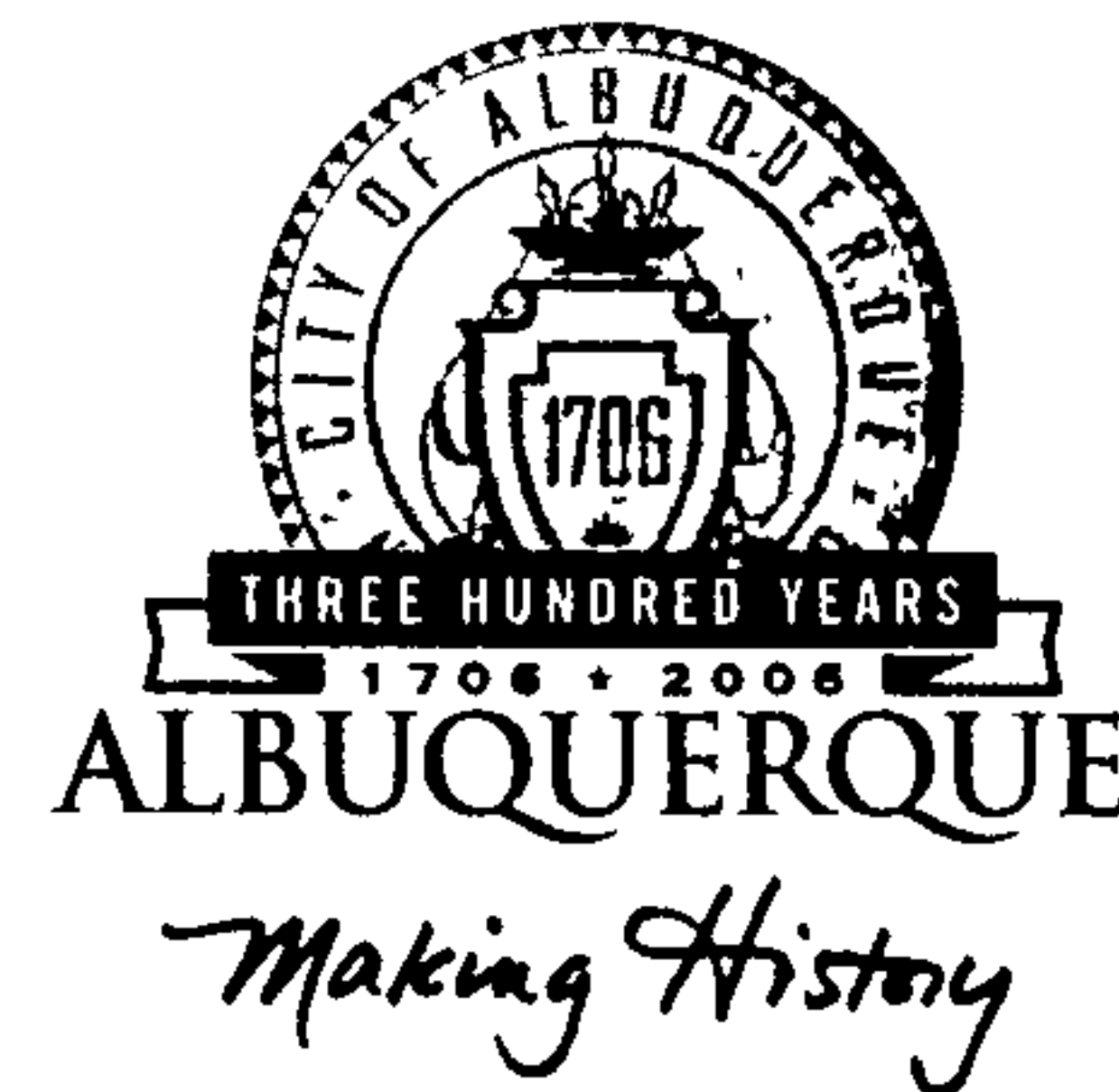
Re: Request for Company Brochure

Dear Farkhondeh,

Thank you for keeping Resource Technology, Inc. in mind for any future projects you or the Property Control Division might have. Enclosed is a brochure, as requested, describing our company and the wide variety of services we offer. I hope we have the opportunity to do more work for you in the future.

The TIWA Building will be completed shortly, pending approval of one small change order. Thanks again for all your help on that project.

CITY OF ALBUQUERQUE



October 18, 2005

Elvideo Diniz, PE
Resource Technology, Inc.
5501 Jefferson NE, Ste 200
Albuquerque, NM 87109

Re: Vintner Ct. Subdivision Grading and Drainage Plan
Engineer's Stamp dated 10-12-05, (C20/D50)

Dear Mr. Diniz,

Based on information provided with your submittal dated 10-13-05, the above referenced plan is approved for Grading Permit. I expect that the construction plans for Wilshire will be available within a few months and Work Order should commence within 6 months.

If you have any questions, you can contact me at 924-3986.

Sincerely,

Bradley L. Bingham, PE
Principal Engineer, Planning Dept.
Development and Building Services

C: file

P.O. Box 1293

Albuquerque

New Mexico 87103

www.cabq.gov

DRAINAGE AND TRANSPORTATION INFORMATION SHEET
(Rev. 06/22/2005)

VINTNER

PROJECT TITLE: VINTNER CT. SUBDIVISION ZONE MAP/DRG. FILE # D-19 C-20 / D50
DRB#: 1003369 EPC#: _____ WORK ORDER#: _____

LEGAL DESCRIPTION: Lots 6 and 7, Block 16, Tract 3, Unit 3
CITY ADDRESS: North Albuquerque Acres, Albuquerque
NM 87113

ENGINEERING FIRM: Resource Technology Inc. CONTACT: Elvidio V. Diniz
ADDRESS: 5501 Jefferson NE, PHONE: 243-7300
CITY, STATE: Suite 200, Albuquerque, ZIP CODE: 87109
New Mexico - 87109

OWNER: Adil Rizvi CONTACT: Adil Rizvi
ADDRESS: 7515 Treviso NE PHONE: 315-6484
CITY, STATE: Albuquerque, ZIP CODE: 87113

ARCHITECT: _____ CONTACT: _____
ADDRESS: _____ PHONE: _____
CITY, STATE: _____ ZIP CODE: _____

SURVEYOR: _____ CONTACT: _____
ADDRESS: _____ PHONE: _____
CITY, STATE: _____ ZIP CODE: _____

CONTRACTOR: _____ CONTACT: _____
ADDRESS: _____ PHONE: _____
CITY, STATE: _____ ZIP CODE: _____

TYPE OF SUBMITTAL:

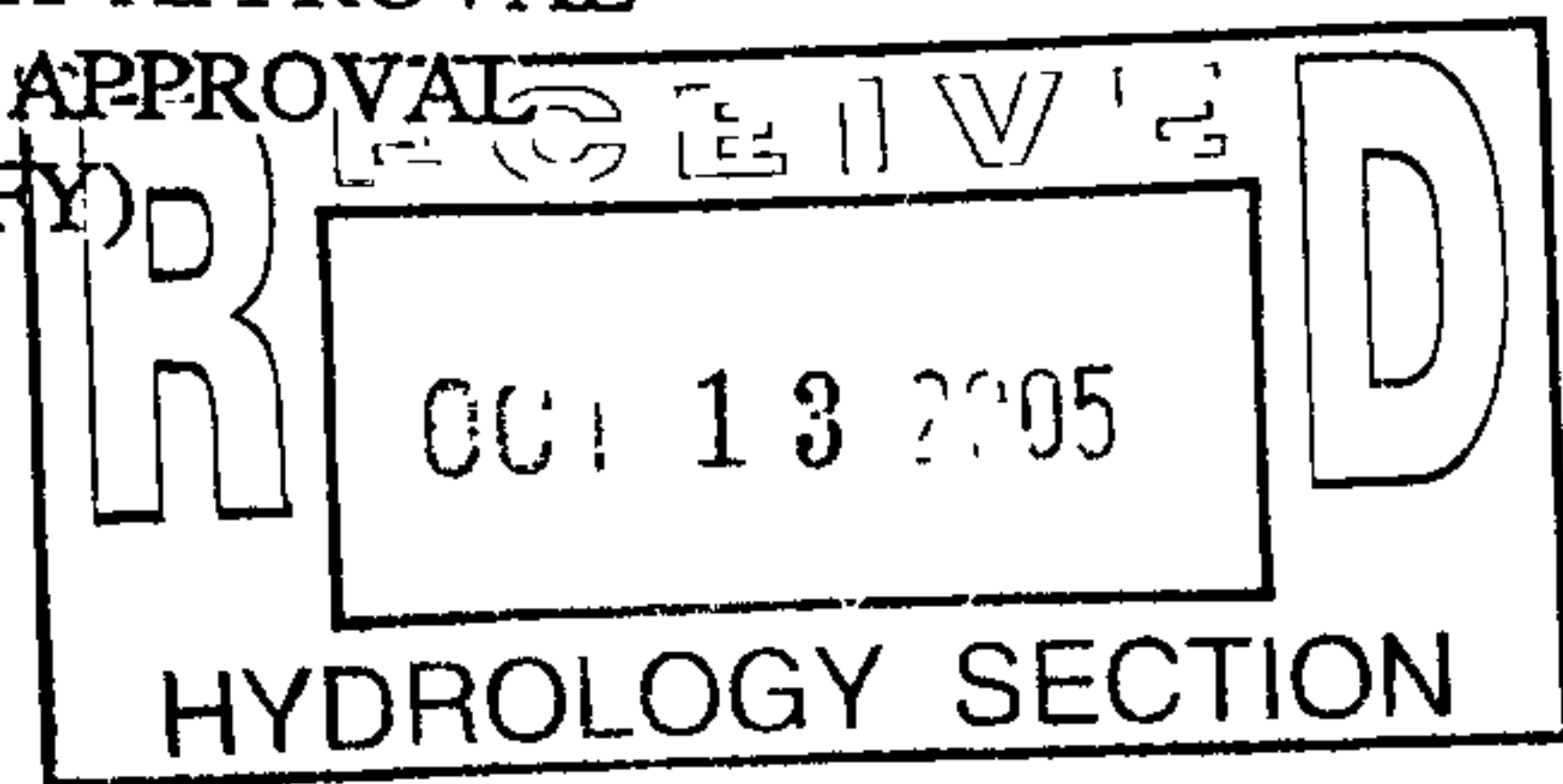
- ☐ DRAINAGE REPORT
☐ DRAINAGE PLAN 1st SUBMITTAL
☐ DRAINAGE PLAN RESUBMITTAL
☐ CONCEPTUAL G & D PLAN
☐ GRADING PLAN
☐ EROSION CONTROL PLAN
☐ ENGINEER'S CERT (HYDROLOGY)
☐ CLOMR/LOMR
☐ TRAFFIC CIRCULATION LAYOUT
☐ ENGINEER'S CERT (TCL)
☐ ENGINEER'S CERT (DRB SITE PLAN)
☒ OTHER Revised Drainage
with Proposed
Phasing of Improvements

WAS A PRE-DESIGN CONFERENCE ATTENDED:

- ☐ YES
☐ NO
☐ COPY PROVIDED

CHECK TYPE OF APPROVAL SOUGHT:

- ☐ SIA/FINANCIAL GUARANTEE RELEASE
☐ PRELIMINARY PLAT APPROVAL
☐ S. DEV. PLAN FOR SUB'D APPROVAL
☐ S. DEV. FOR BLDG. PERMIT APPROVAL
☐ SECTOR PLAN APPROVAL
☐ FINAL PLAT APPROVAL
☐ FOUNDATION PERMIT APPROVAL
☐ BUILDING PERMIT APPROVAL
☐ CERTIFICATE OF OCCUPANCY (PERM)
☐ CERTIFICATE OF OCCUPANCY (TEMP)
☒ GRADING PERMIT APPROVAL
☐ PAVING PERMIT APPROVAL
☐ WORK ORDER APPROVAL
☐ OTHER (SPECIFY) _____



SUBMITTED BY: Adil Rizvi DATE: 10/12/05

Requests for approvals of Site Development Plans and/or Subdivision Plats shall be accompanied by a drainage submittal. The particular nature, location and scope to the proposed development defines the degree of drainage detail. One or more of the following levels of submittal may be required based on the following:

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5501 Jefferson Blvd. NE, Ste. 200 Albuquerque, NM 87109

Telephone (505) 243-7300

FAX: (505) 243-7400

E-Mail: rti@nm.net

October 12, 2005

Brad Bingham
Hydrology Section
Municipal Development Dept.
City of Albuquerque
One Civic Plaza, Room 7057
Albuquerque, NM 87103

RE: Vintner Court Subdivision - Proposed Phasing of Improvements.

Dear Brad:

As discussed at our meeting on Sept. 28, 2005, this letter summarizes our interim drainage assessment for this site and outlines the procedure to design and construct phased improvements for site development.

Our drainage assessment is as follows:

The contributing drainage areas to Wilshire Blvd, and which are now partially draining into the pond, is 0.0968 sq. miles (62 ac.) as shown on figures 1A and 1B with Existing Condition peak flows and volumes as listed on Table 1. Therefore, the total volumes to the on-site existing pond are 6.96 ac.ft. (100-yr) and 2.76 ac.ft. (10-yr) for Existing Conditions. Excerpts of the AHYMO Model output are also attached.

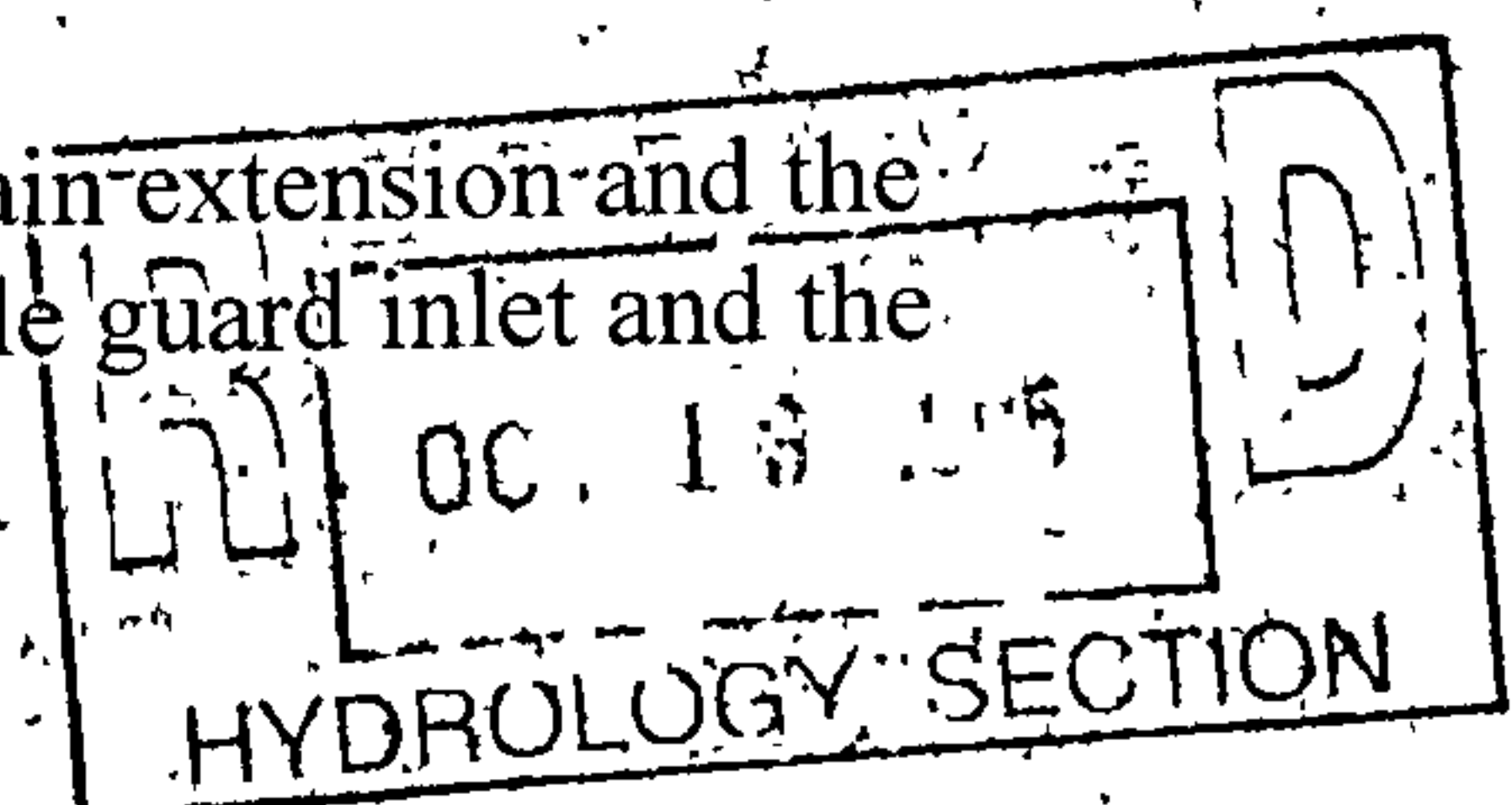
Using the 1999 Bernalillo County LIDAR topographic mapping, Figure 2, the existing emergency unreinforced spillway is at elevation 88, with potentially 2 ft. of surcharge to elev. 90. The corresponding elevation - volume data are provided on Table 2.

Therefore, it is evident that if the eastern 50% of the pond volume is filled, 100% of the Existing Condition 10 - yr volume and 57 % of the 100 - yr volume can be stored in the western 50 %, at the maximum pond volume (elev. 90) and ignoring any outflow from the pond. We anticipate that the filling and compaction of 50% of the pond will require 30 days to complete.

Assuming the following activities are agreeable with you, we propose:

A. Begin filling the east half of the pond to elevation 88, as soon as you approve this activity.

B. Simultaneously, begin design of the Wilshire storm drain extension and the Vintner Drive storm drain connection between the cattle guard inlet and the



Brad Bingham
City of Albuquerque
October 12, 2005
Page 2

existing 30-in. storm drain. This will also include grading improvements on the site to exclude off-site runoff after the improvements are constructed.

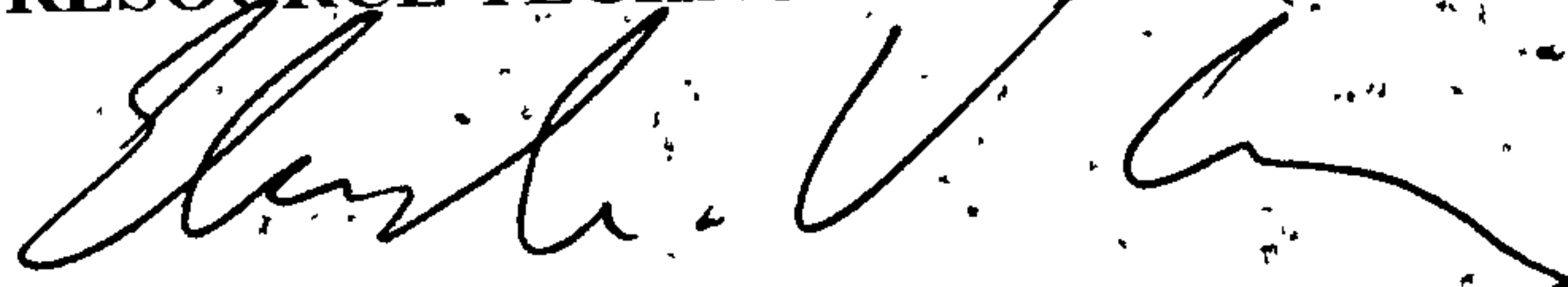
- C. We propose that the storm drain inlets will be constructed in 50 ft. of curb and gutter on each side of the street with upstream linear ponds on each side of the street to desilt 10-yr flows. Flows in excess of 10-yr flows will spill (be diverted) back onto the existing pond site, where a minimum 1 ac. ft. pond will be retained around the existing pond outlet; this small pond will remain until all storm drain and street improvements are constructed.
- D. Obtain DRC approval of the storm drain improvements as Work Order items and begin construction as soon after DRC approval as possible. Complete construction within a mutually acceptable time frame.
- E. Begin design of on-site public and private improvements (including filling in the remaining on-site 1 ac. ft. pond) and remaining off-site public improvements (Wilshire St.) and drainage to Vintner Dr.
- F. Obtain DRC approval of these on-site and remaining off-site improvements as a separate Work Order; and complete construction as soon as practical.

We hope that this sequencing of activities meets with your approval and we can start the storm drain design and partial pond filling as quickly as possible. Please let me know if any item needs to be discussed in further detail, or if any other issues are outstanding.

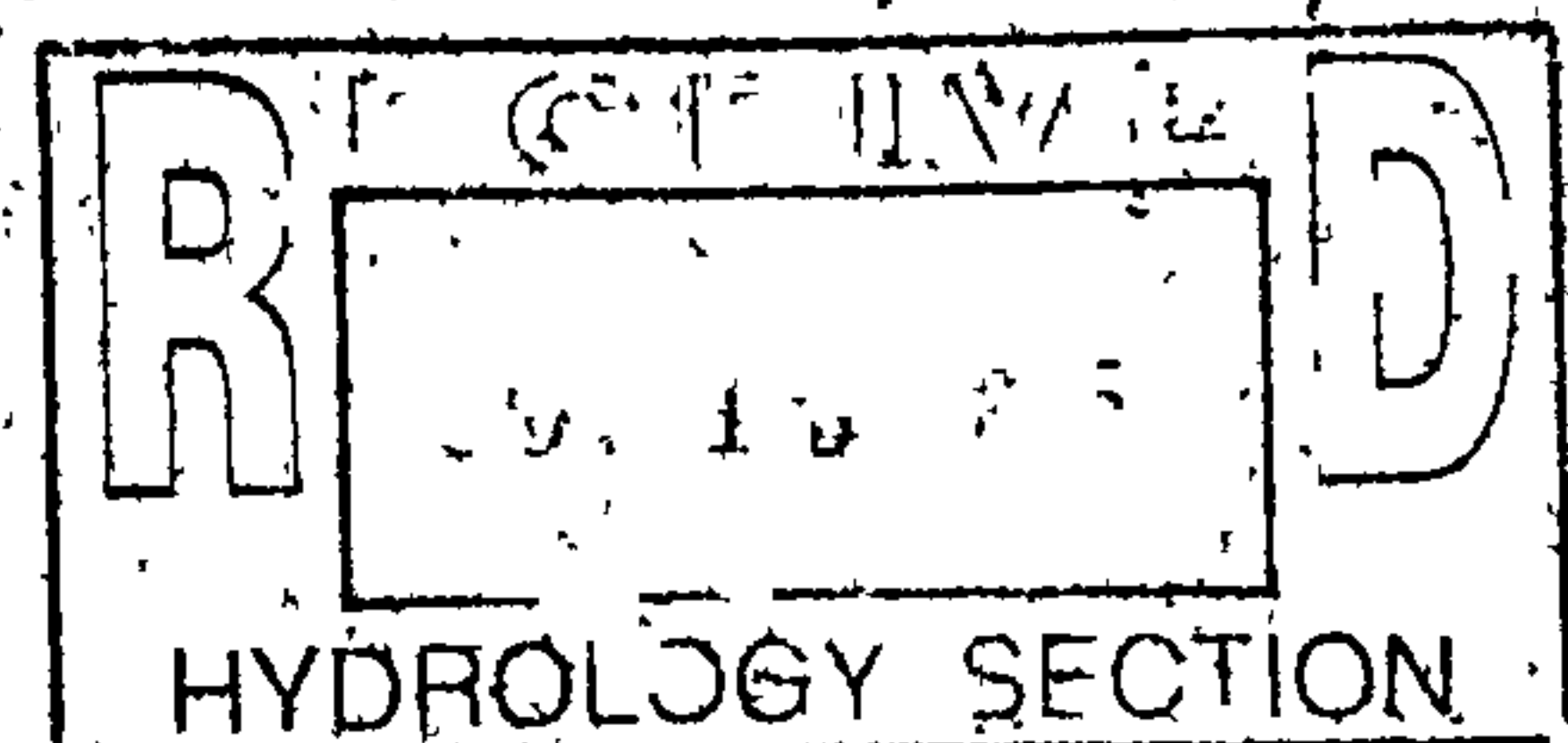
We believe that this approach is both rational and reasonable with regard to meeting both public safety and convenience, and constructing public improvements. We will work with you in meeting our common goals.

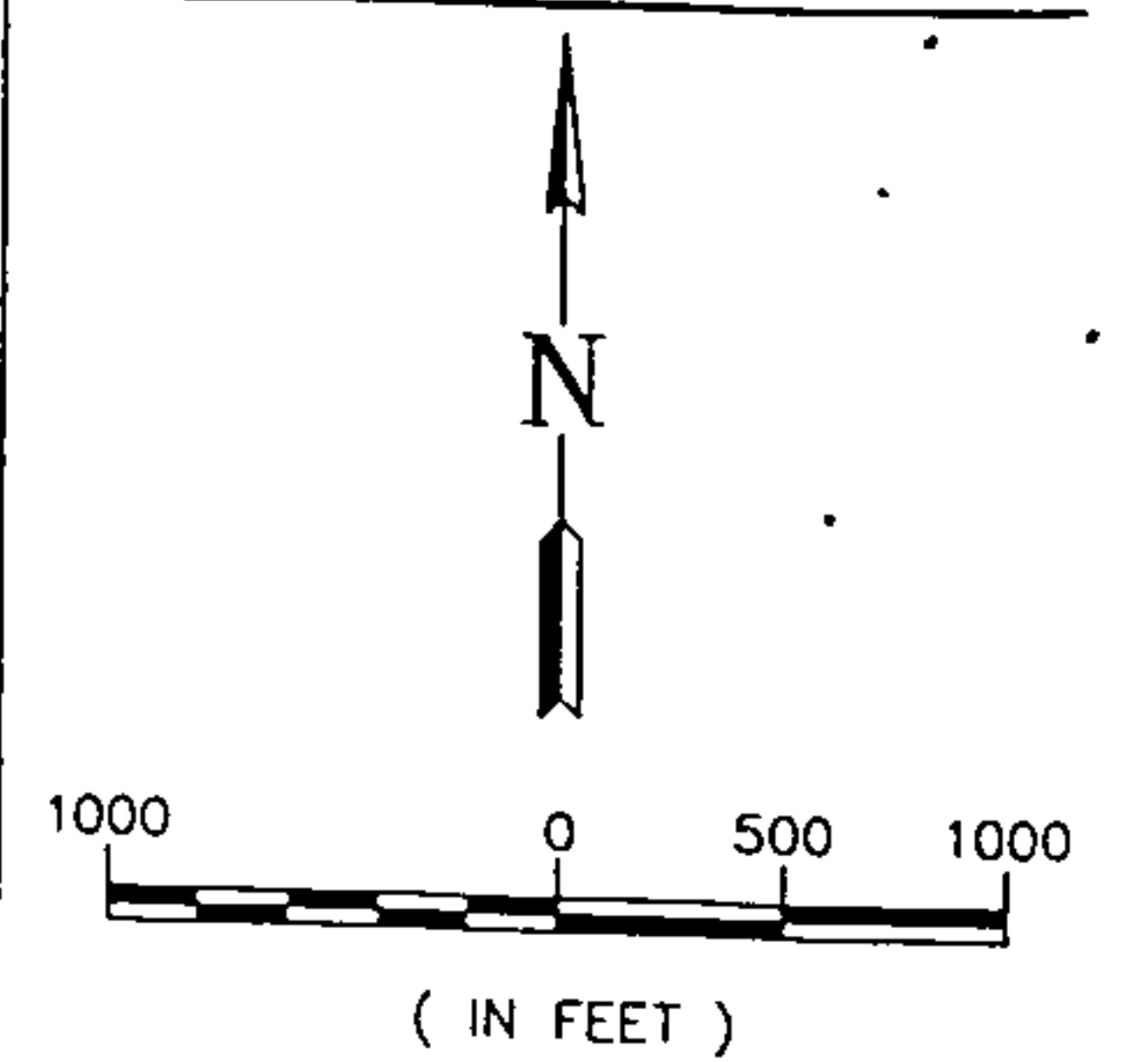
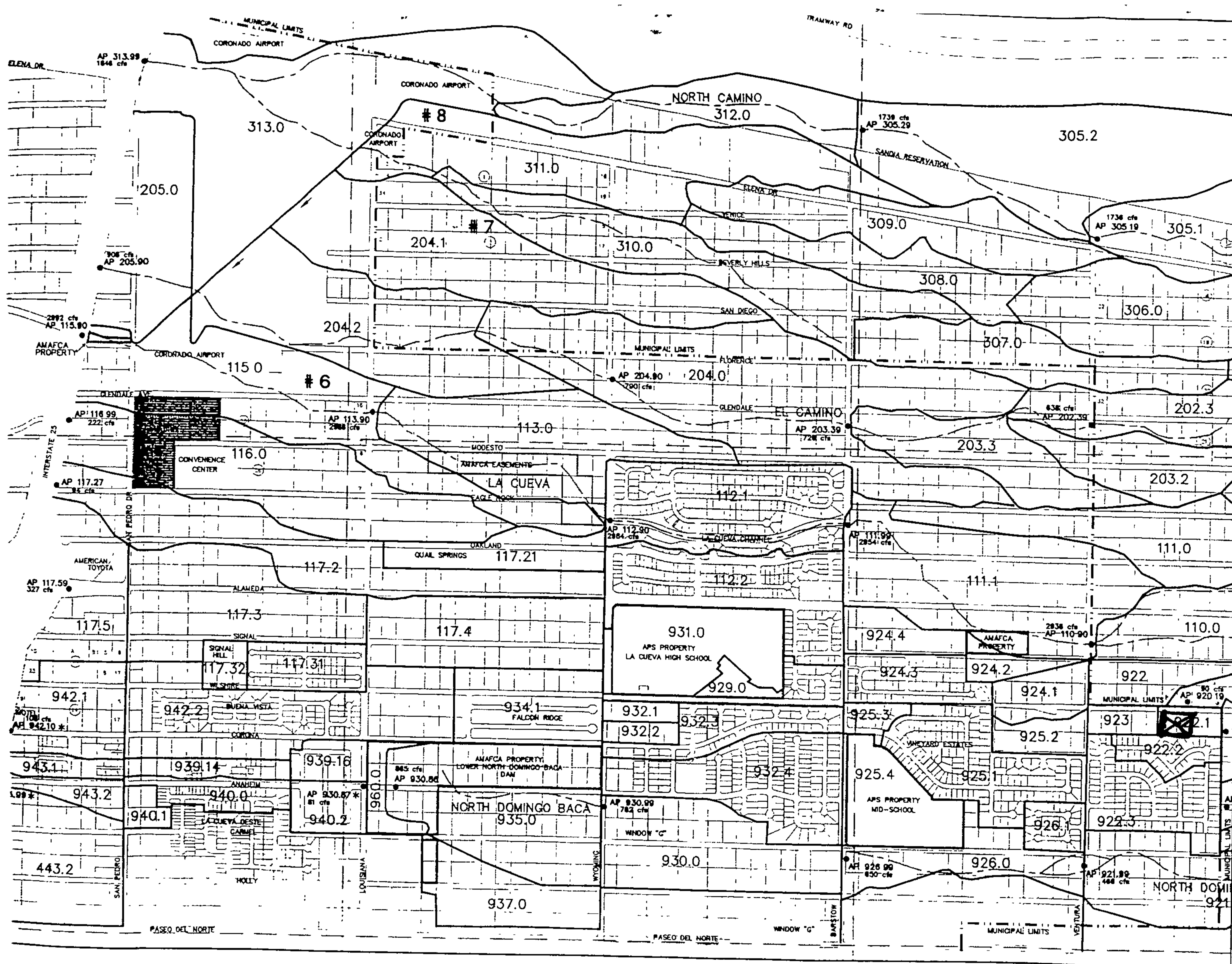
Sincerely,

RESOURCE TECHNOLOGY, INC.



Elvidio V. Diniz, P.E.
President and Principal Engineer





LEGEND

- 107.1 SUBBASIN DESIGNATION
- SUBBASIN BOUNDARY
- EXISTING PLATTING
- EXISTING ARROYO FLOW PATH
- ANALYSIS POINT AND EXISTING CONDITION FLOW RATE
- * FLOW RATE NOT BULKED FOR SEDIMENT
- # 2 POTENTIAL AVULSION LOCATION
- MUNICIPAL LIMITS

NORTH ALBUQUERQUE ACRES
MASTER DRAINAGE PLAN

EXISTING CONDITION

FIGURE 3A
CITY OF ALBUQUERQUE
PUBLIC WORKS DEPARTMENT



Resource Technology, Inc.
Civil Engineering
Environmental Sciences
Water Resources
Landscape Architecture
Planning
1720 - B Randolph Road SE
Albuquerque, New Mexico 87106
E-mail: rtech@earthlink.net
Telephone: (505) 243-7300
Facsimile: (505) 243-7400

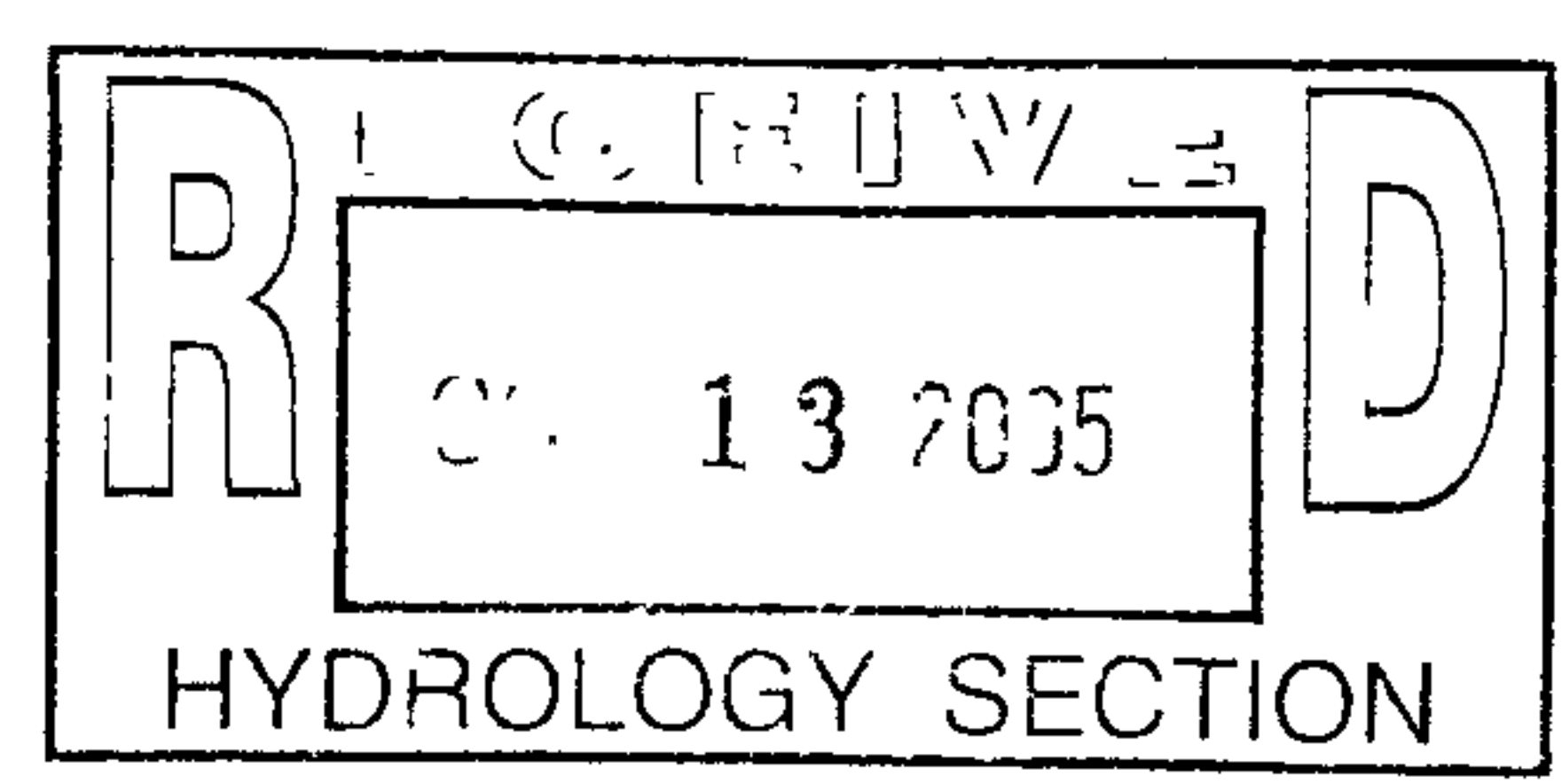
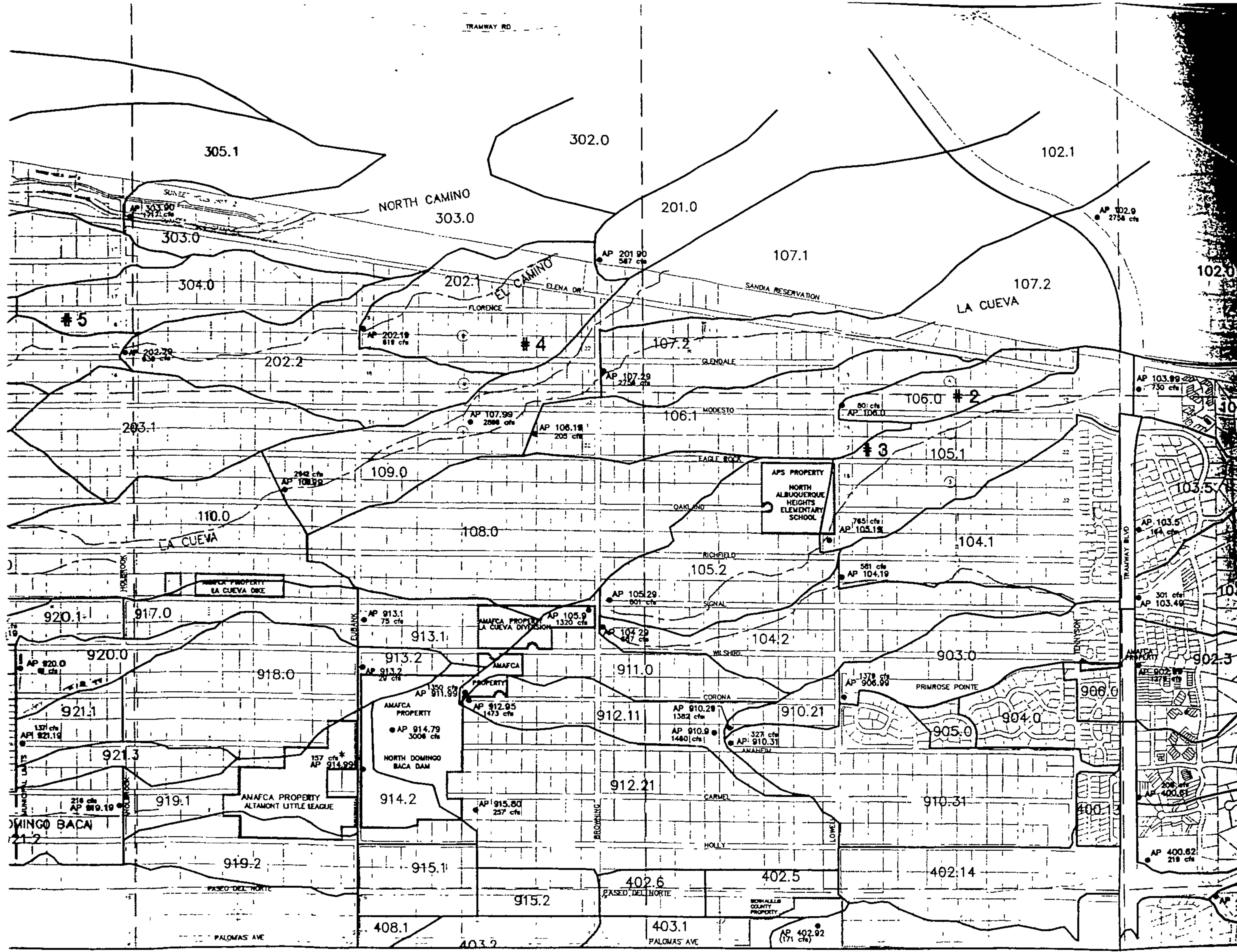


FIGURE 1A

KM Rocky Arroyo
 KO 0
 BA 1.89
 LS 65.0
 UD 1.96
 KKCPIT1
 KM Combination of Flows into Gravel Pit
 KO 0
 HC 2
 KKGPIT1
 KM Gravel Pit
 KO 0
 RS 1 STOR
 SV 0 2.13 15.66 44.97 91.36
 SE3744.8 3748 3752 3756 3760
 SL
 SS 3752 0 3.0 1.5
 ST 3752 10 3.0 1.5
 SW 10 80 230 600
 SE 3752 3756 3759 3760
 KKRRH9C
 KM
 KO 0
 RD
 RC .06 .04 .06 7000 .0040
 RX 0 1480 2580 2735 2770 2870 3130 3300
 RY 3746 3740 3743.8 3740 3737 3744 3744.9 3746
 KKHWEIR1
 KM Diversion Downstream of Weir 1
 KO 0
 DRDWEIR1
 KKRRH9D
 KM Diversion: Weir 1 to Rocky Arroyo
 KO 0
 RD
 RC .06 .045 .06 23800 .005
 RX 0 240 1491 1499 1501 1509 1880 3000
 RY 3788 3787 3786 3783 3783 3786 3787.2 3788
 KKCRA1
 KM Confluence of Rio Hondo and Rocky Arroyo
 KO 0
 HC 3
 KKRRH9A
 KM Rio Hondo: Rocky Arroyo to Weir 3
 KO 0
 RD
 RC .06 .045 .06 8270 .003
 RX 0 360 2870 2900 2910 3330 3940 4500
 RY 3722 3717 3718.4 3714 3716 3720.5 3717.7 3722
 KKBRH9
 KM Rio Hondo
 KO 0
 BA 4.05
 LS 77.6
 UD 3.46
 KKCRH9
 KM Rio Hondo at Diversion Weir #3
 KO 0
 HC 2
 *
 KKDWEIR3



(1" = 500')

LEGEND

- 107.1 SUBBASIN DESIGNATION
- SUBBASIN BOUNDARY
- EXISTING PLATTING
- EXISTING ARROYO FLOW PATH
- ANALYSIS POINT AND EXISTING CONDITION FLOW RATE
- 3 FLOW RATE NOT BULKED FOR SEDIMENT
- 2 POTENTIAL AVULSION LOCATION
- MUNICIPAL LIMITS

NORTH ALBUQUERQUE ACRES
MASTER DRAINAGE PLAN
EXISTING CONDITION
FIGURE 3B
CITY OF ALBUQUERQUE
PUBLIC WORKS DEPARTMENT

City of Albuquerque

Resource Technology, Inc.

1720 N. 8th Street, Suite 100, Albuquerque, NM 87102
Tel: 505-243-7700 Fax: 505-243-7701
E-mail: info@resource-tech.com

R D

13 2005

HYDROLOGY SECTION

FIGURE 1B

KM Overflows toward South Spring downstream of Weir #3

KO									
DTDWEIR3									
DI	0	93.5	744	954	1177	1792	3509	4125	4537
4966									
DQ	0	0	8	55	129	221	1811	2066	2327
2600									

KKRRH8

KM Rio Hondo (RAS): Weir 3 to Bypass

KO									
RD									
RC	.06	.04	.06	15019	.0020				
RX	1000	3400	4950	4989	4991	5015	6090	8000	
RY	3696	3685	3689	3676	3676	3689	3686	3696	

*

KKBRH8

KM Rio Hondo

KO									
BA	10.13								
LS		78.9							
UD	3.08								

*

KKCRH8

KM Combine routed flow from upstream with Basin BRH8

HC 2

*

KKBRH7

KM Rio Hondo

KO									
BA	4.34								
LS		85.9							
UD	1.96								

*

KKDRH7

KM Divert flows from BRH7 - flows north and into a sinkhole

DTDRH7									
DI	0	10000							
DQ	0	10000							

*

KKCBYPAS

KM Rio Hondo at the Roswell Bypass

KO									
HC	2								

* A nomograph from NMSHTD was used to calculate the flow
* split at the Roswell Relief Route Bypass. Flows that exceed the
* capacity of the Rio Hondo culverts flows north along the Bypass and then
* encounters the culverts from the old Rio Hondo channel.

KKDBP_IN

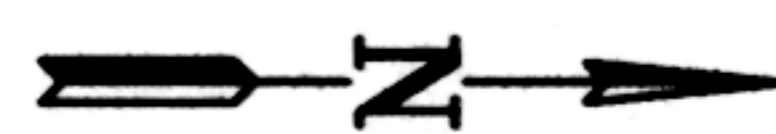
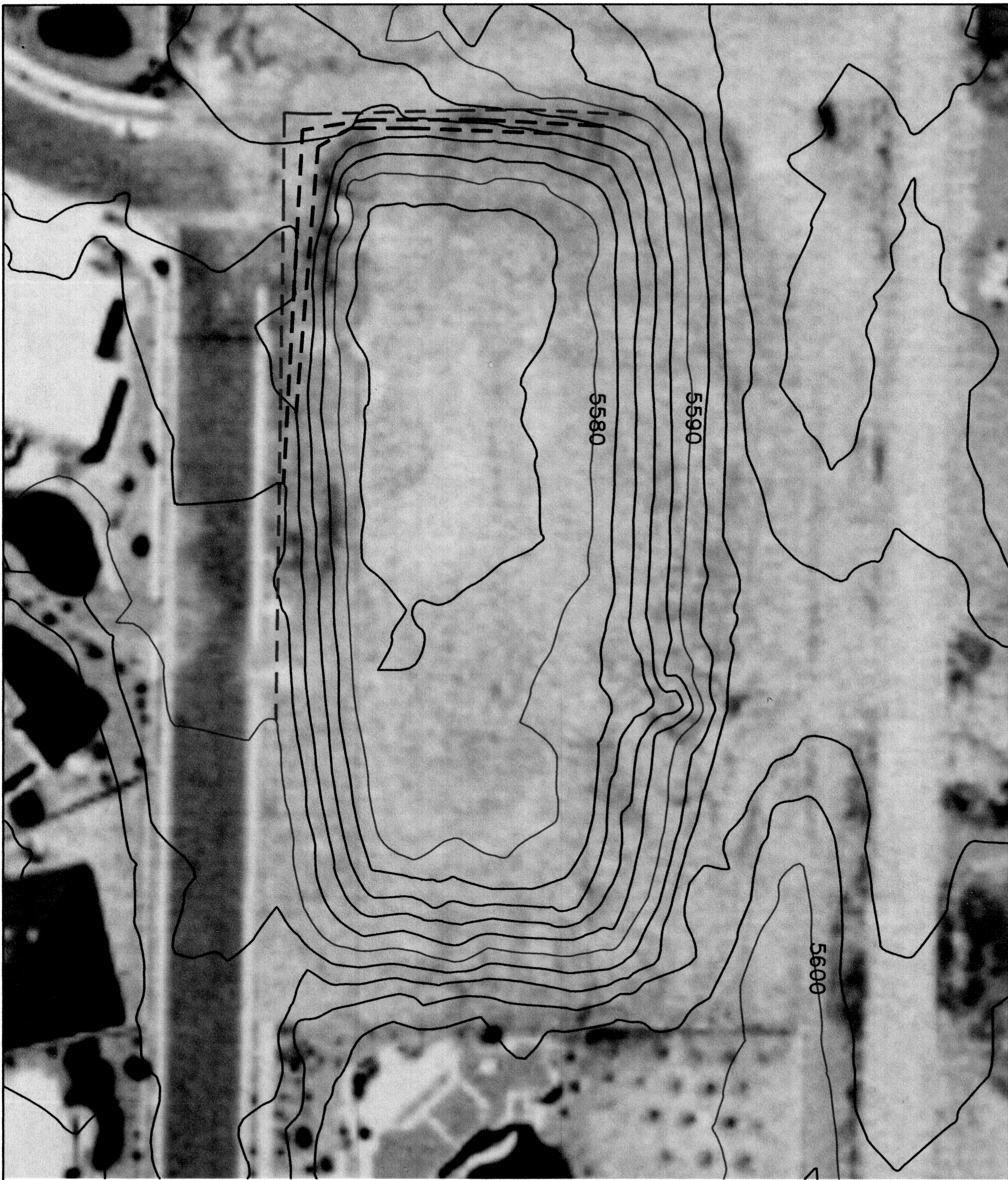
KM Flow split at the Bypass Crossing

KO									
DTDBPOUT									
DI	0	500	1000	1500	1900	2000	2200	2500	3000
4000									
DQ	0	500	1000	1500	1900	1900	1900	1900	1900
1900									

*

* A nomograph from NMSHTD was used to calculate the flow
* split at the Roswell Relief Route Bypass and the old RH channel culverts.
* Flows that exceed the capacity of the old Rio Hondo channel culverts flow
* north along the Bypass and then encounters the culverts at Brasher Road.

KKDBP_2I



SCALE: 1" = 50'

Resource Technology, Inc.
Civil Engineering Water Resources
Environmental Sciences Landscape Architecture
Planning

**EXISTING CONDITIONS
VINTNER COURT POND
05-190 VINTNER**

FIGURE

1/2

Table 1
EXISTING CONDITION HYDROLOGIC DATA
VINTNER COURT SUBDIVISION

	Location	Analysis Point in RTI AHYMO Model	Contributing Drainage Basins in RTI AHYMO Model	Total Drainage Area	RTI EXISTING CONDITIONS				CCE 12/03 Design Flows
					100-yr Peak Flow	100-yr Total Volume	10-yr Peak Flow	10-yr Total Volume	
					sq. mile	cfs	ac.ft	cfs	ac.ft
1	N. side Wilshire St.	920.19	913.1, 917, 920.1	0.0968	86	4.61	33	1.78	92
2	Lots 6, 7, 8 & 9	922.1	None	0.007	11	0.39	5	0.18	
3	Lots 6 & 7 (50% of 2)			0.004	6	0.2	3	0.09	
4	Vintner Dr.	920	920	0.043	59	2.15	24	0.89	100
5	Exist inflow volume to pond (1+3+4)					6.96		2.76	

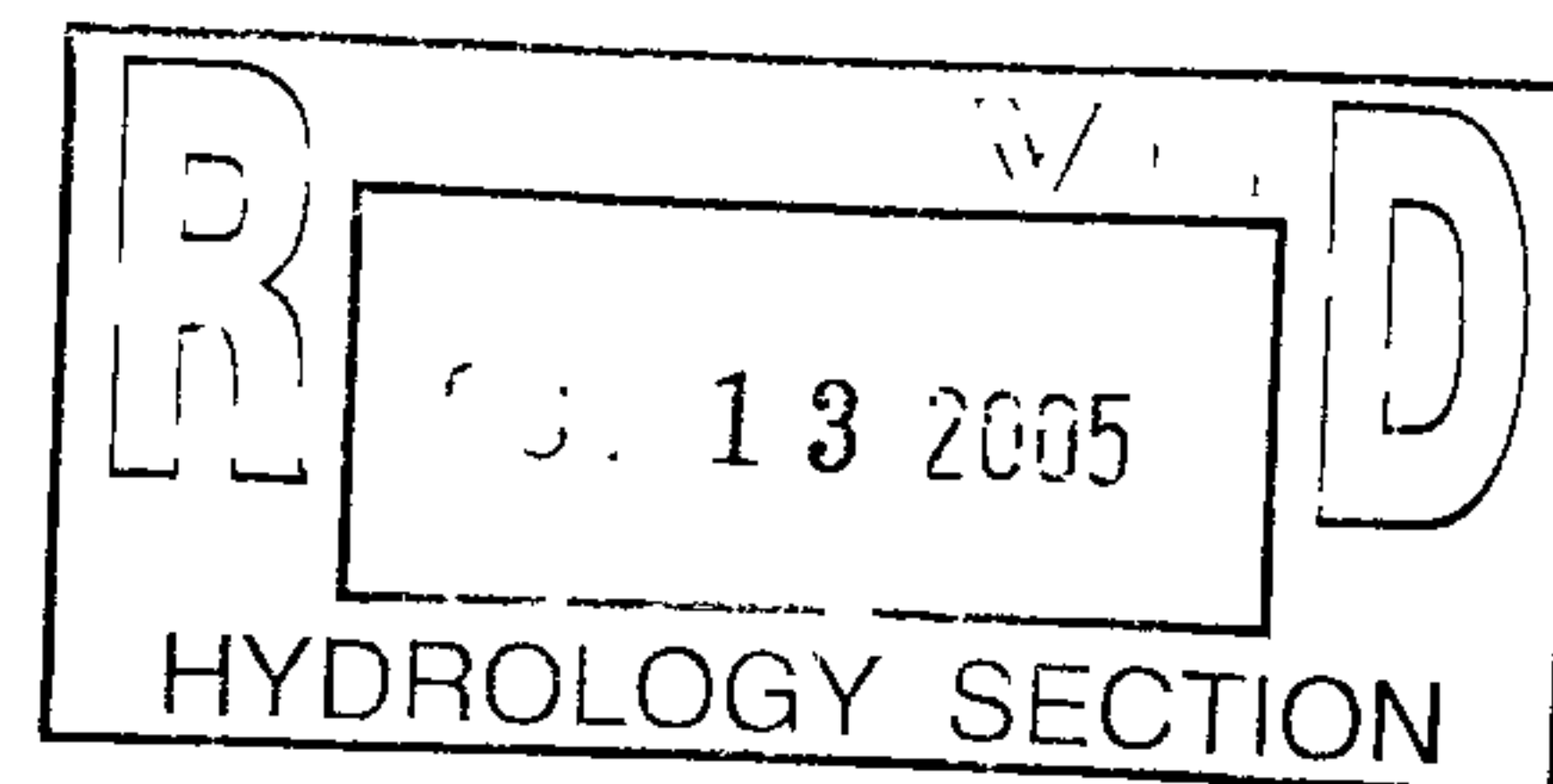
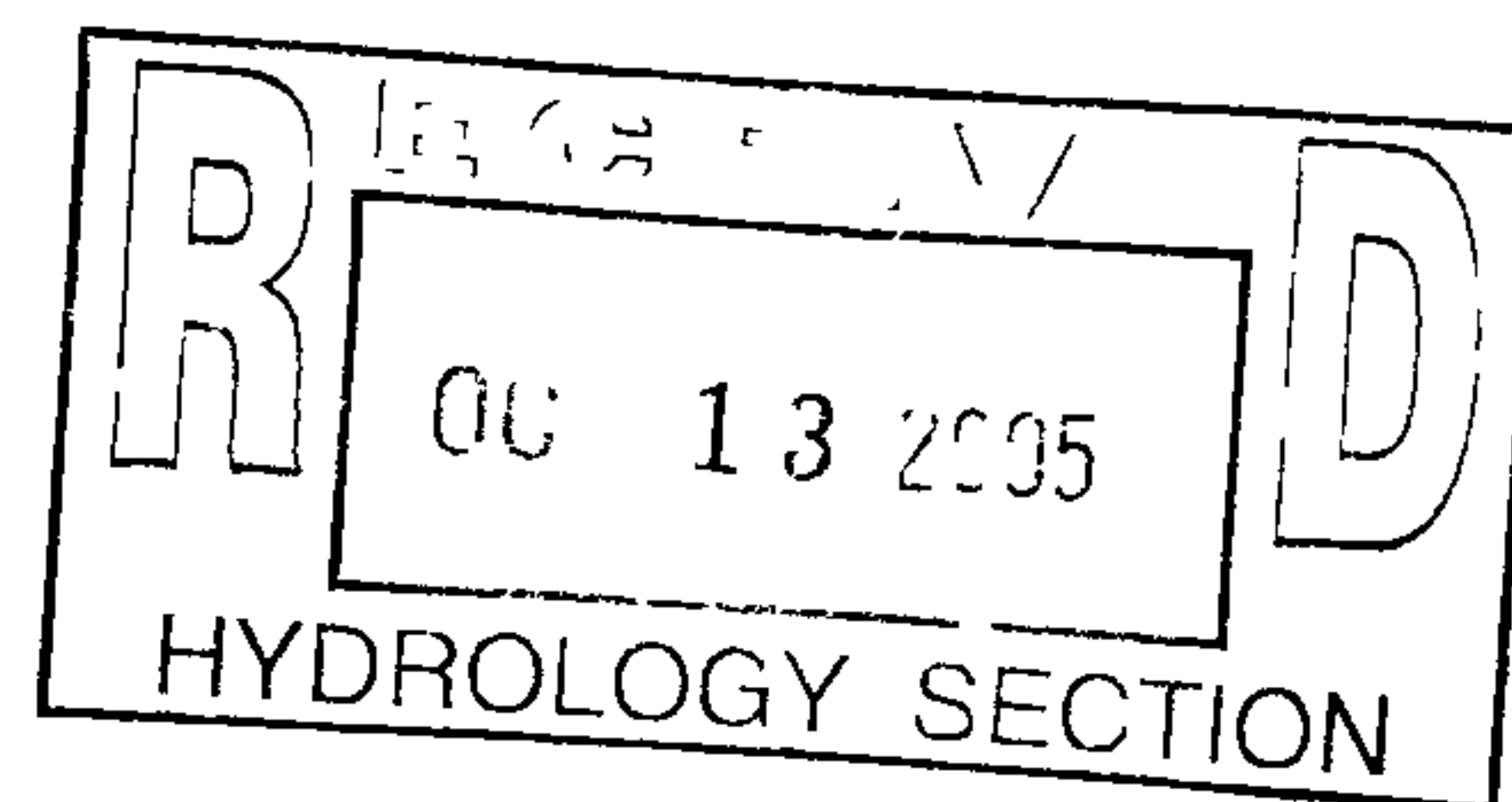


TABLE 2
EXISTING CONDITIONS
ELEVATION-VOLUME DATA
VINTNER COURT POND

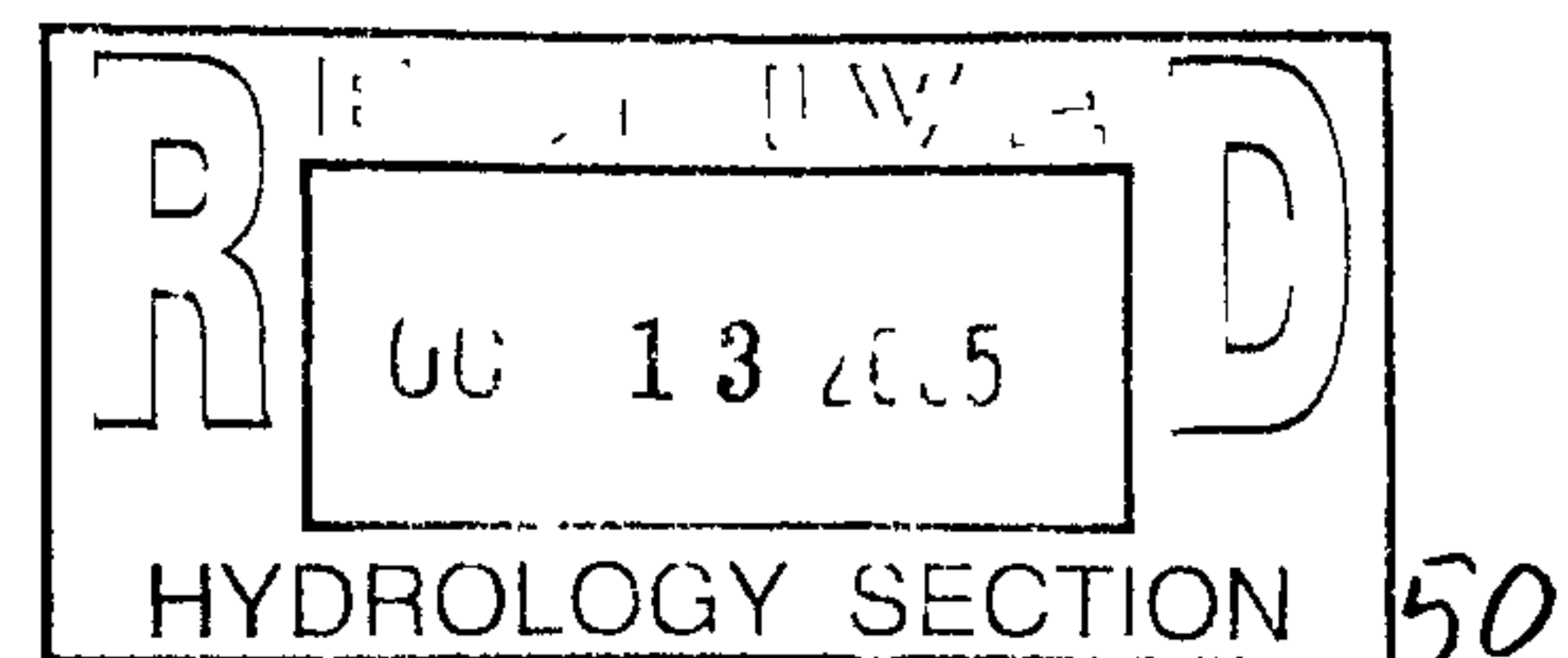
Elevation	ft	5577.50	5578.00	5580.00	5582.00	5584.00	5586.00	5588.00	5590.00
Area	Sq.ft	4390.00	8780.00	19368.00	25121.00	29653.00	33649.00	38023.00	42504.00
Volume	Cu.ft		3229.73	27458.91	44364.49	54711.40	63259.92	71627.47	80485.41
Cum. Vol.	Cu.ft		3229.73	30688.64	75053.13	129764.53	193024.44	264651.91	345137.32
Cum. Vol.	Ac.ft	0.00	0.07	0.70	1.72	2.98	4.43	6.08	7.92

44 57



100-yr Existing

COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	PAGE - 3	NOTATION
ROUTE MCUNGE	919.27	1	3	.09530	104.88	4.984	.98063	1.600	1.720	CCODE =	.1
ROUTE MCUNGE	919.28	3	2	.09530	102.24	4.984	.98063	1.750	1.676	CCODE =	.1
*S N D B ARROYO BETWEEN HOLBROOK AND VENTURA. (AP 919.38) - NOT FINAL											
ADD HYD	919.38	2& 5	6	2.98763	297.04	211.527	1.32752	1.800	.155		
RAINFALL TYPE= 2										RAIN24=	3.100
COMPUTE NM HYD	921.30	-	7	.02840	42.42	1.418	.93639	1.500	2.334	PER IMP=	10.00
RAINFALL TYPE= 2										RAIN24=	3.200
COMPUTE NM HYD	913.20	-	1	.01700	27.83	.919	1.01321	1.500	2.558	PER IMP=	10.00
ROUTE MCUNGE	913.18	1	2	.01700	23.51	.911	1.00464	1.800	2.160	CCODE =	.1
RAINFALL TYPE= 2										RAIN24=	3.150
COMPUTE NM HYD	918.00	-	1	.11400	113.10	6.050	.99514	1.650	1.550	PER IMP=	10.00
*S HOLBROOK (AP 918.99) - FINAL											
ADD HYD	918.99	1& 2	1	.13100	122.15	6.961	.99636	1.700	1.457		
ROUTE MCUNGE	918.88	1	2	.13100	119.88	6.940	.99337	1.800	1.430	CCODE =	.1
RAINFALL TYPE= 2										RAIN24=	3.100
COMPUTE NM HYD	921.10	-	1	.02920	43.61	1.458	.93639	1.500	2.334	PER IMP=	10.00
*S UPSTREAM OF END OF VINYARD ESTATES SUBDIVISION UNIT IV BETWEEN											
*S HOLBROOK & VENTURA											
*S ANAHEIM (AP 921.19) - FINAL											
ADD HYD	921.19	1& 2	1	.16020	131.96	8.399	.98298	1.800	1.287		
*S *****											
*S BEGIN: VINYARD ESTATES - UNIT IV											
*S *****											
*S ROUTE 921.19											
*S (48-INCH RCP)											
ROUTE	921.80	1	12	.16020	132.16	8.399	.98298	1.800	1.289		
*S (54-INCH RCP)											
ROUTE	921.89	12	16	.16020	132.32	8.399	.98298	1.800	1.291		
*S COMBINE 921.89 & 921.3 AS 921.98											
ADD HYD	921.98	16& 7	14	.18860	146.39	9.817	.97596	1.750	1.213		
ROUTE RESERVOIR	921.99	14	15	.18860	118.11	6.724	.66844	1.950	.979	AC-FT=	4.011
*S *****											
*S END: VINYARD ESTATES - UNIT IV											
*S *****											
*S N D B ARROYO BETWEEN HOLBROOK & VENTURA (AP 921.78) - FINAL											
ADD HYD	921.78	6&15	1	3.17623	402.87	218.250	1.28838	1.900	.198		
ROUTE MCUNGE	921.80	1	2	3.17623	399.35	218.244	1.28834	1.950	.196	CCODE =	.1
RAINFALL TYPE= 2										RAIN24=	3.070
COMPUTE NM HYD	921.20	-	1	.08740	134.09	4.415	.94707	1.500	2.397	PER IMP=	10.00
*S N D B ARROYO AT VENTURA (AP 921.9) - NOT FINAL											
ADD HYD	921.90	1& 2	26	3.26363	418.92	222.658	1.27920	1.950	.201		
RAINFALL TYPE= 2										RAIN24=	3.250
COMPUTE NM HYD	913.10	-	1	.03600	52.68	1.688	.87916	1.500	2.287	PER IMP=	5.00
ROUTE MCUNGE	913.88	1	2	.03600	46.53	1.688	.87916	1.800	2.020	CCODE =	.1
RAINFALL TYPE= 2										RAIN24=	3.150
COMPUTE NM HYD	917.00	-	1	.04060	58.42	1.863	.86049	1.500	2.248	PER IMP=	5.00
*S HOLBROOK (AP 917.99) - FINAL											
ADD HYD	917.99	1& 2	1	.07660	66.30	3.551	.86926	1.750	1.352		
ROUTE MCUNGE	917.88	1	2	.07660	65.96	3.551	.86927	1.850	1.345	CCODE =	.2
RAINFALL TYPE= 2										RAIN24=	3.100
COMPUTE NM HYD	920.10	-	1	.02020	31.87	1.058	.98186	1.500	2.465	PER IMP=	10.00
*S INTO VINEYARD ESTATES SUBDIVISION UNIT IV AT NORTH END											
*S BETWEEN HOLBROOK AND VENTURA (AP 920.19) - FINAL (UNBULKED)											
ADD HYD	920.19	2& 1	1	.09680	86.13	4.609	.89275	1.600	1.390		
RAINFALL TYPE= 2										RAIN24=	3.100
COMPUTE NM HYD	920.00	-	5	.04310	58.97	2.152	.93639	1.500	2.138	PER IMP=	10.00
*S INTO VINEYARD ESTATES SUBDIVISION UNIT IV AT EAST (NORTH END)											
*S BETWEEN HOLBROOK AND VENTURA (AP 920) - FINAL (UNBULKED)											
*S *****											
*S BEGIN: VINYARD ESTATES - UNIT IV											
*S *****											
RAINFALL TYPE= 2										RAIN24=	3.030
COMPUTE NM HYD	922.10	-	2	.00700	11.18	.394	1.05631	1.500	2.496	PER IMP=	17.00
COMPUTE NM HYD	923.00	-	12	.00700	11.32	.370	.99230	1.500	2.526	PER IMP=	10.00
COMPUTE NM HYD	922.20	-	9	.01480	35.69	1.396	1.76805	1.500	3.767	PER IMP=	40.00
COMPUTE NM HYD	922.30	-	16	.04150	100.03	3.913	1.76805	1.500	3.766	PER IMP=	40.00
*S COMBINE 920.1 + 922.1 AS 922.19											
ADD HYD	922.19	1& 2	3	.10380	94.57	5.003	.90378	1.600	1.424		
*S ROUTE 920 TO POND AS 920.8											
ROUTE MCUNGE	920.80	5	4	.04310	58.82	2.150	.93541	1.550	2.133	CCODE =	.1
*S ROUTE 922.19 THRU POND AS 920.88											
ROUTE RESERVOIR	920.88	3	7	.10380	42.57	5.003	.90378	2.050	.641	AC-FT=	2.296
*S ADD 920.88 920.8 AS 920.9 (POND Q + 920)											



10-Yr Existing

MMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	PAGE = 3	NOTATION
UTE MCUNGE	919.28	3	2	.09530	46.02	2.039	.40123	1.750	.754	CCODE =	.1
N D B ARROYO BETWEEN HOLBROOK AND VENTURA. (AP 919.38) - NOT FINAL											
D HYD	919.38	2 & 5	6	2.98763	141.75	102.536	.64351	1.850	.074		
INFALL TYPE= 2										RAIN24=	2.070
MPUTE NM HYD	921.30	-	7	.02840	17.12	.587	.38728	1.500	.942	PER IMP=	10.00
INFALL TYPE= 2										RAIN24=	2.130
MPUTE NM HYD	913.20	-	1	.01700	11.75	.387	.42635	1.500	1.080	PER IMP=	10.00
UTE MCUNGE	913.18	1	2	.01700	11.72	.387	.42637	1.700	1.077	CCODE =	.2
INFALL TYPE= 2										RAIN24=	2.100
MPUTE NM HYD	918.00	-	1	.11400	45.61	2.545	.41856	1.650	.625	PER IMP=	10.00
HOLBROOK (AP 918.99) - FINAL											
D HYD	918.99	1 & 2	1	.13100	54.81	2.931	.41956	1.700	.654		
UTE MCUNGE	918.88	1	2	.13100	54.45	2.931	.41957	1.750	.649	CCODE =	.2
INFALL TYPE= 2										RAIN24=	2.070
MPUTE NM HYD	921.10	-	1	.02920	17.60	.603	.38728	1.500	.942	PER IMP=	10.00
UPSTREAM OF END OF VINYARD ESTATES SUBDIVISION UNIT IV BETWEEN HOLBROOK & VENTURA ANAHEIM (AP 921.19) - FINAL											
D HYD	921.19	1 & 2	1	.16020	60.69	3.534	.41368	1.750	.592		

BEGIN: VINYARD ESTATES - UNIT IV											

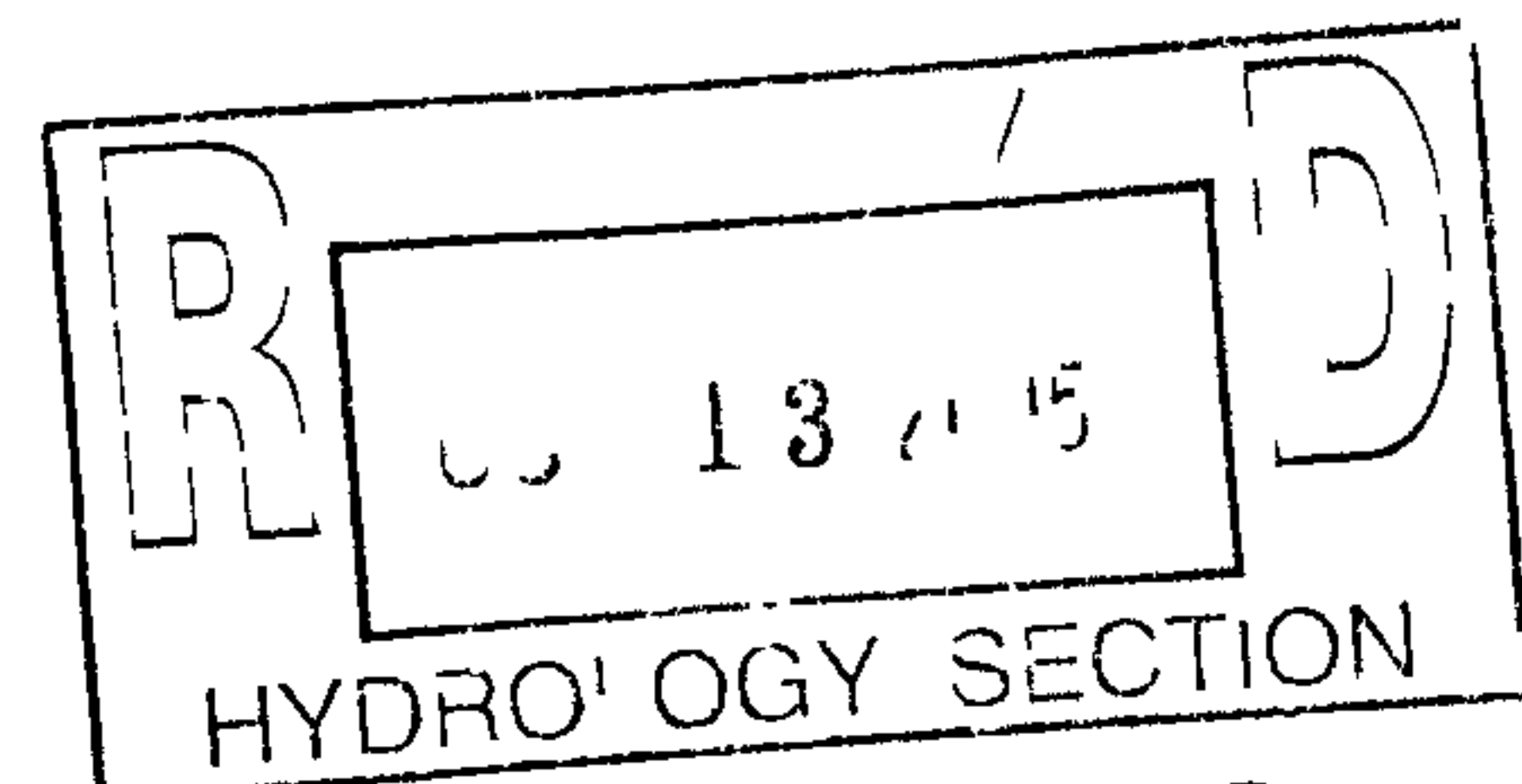
ROUTE 921.19											
(48-INCH RCP)											
UTE	921.80	1	12	.16020	60.70	3.534	.41368	1.750	.592		
(54-INCH RCP)											
UTE	921.89	12	16	.16020	60.66	3.534	.41368	1.750	.592		
COMBINE 921.89 & 921.3 AS 921.98											
DD HYD	921.98	16 & 7	14	.18860	66.73	4.121	.40970	1.750	.553		
UTE RESERVOIR	921.99	14	15	.18860	10.53	1.028	.10223	2.400	.087	AC-FT=	3.190

END: VINYARD ESTATES - UNIT IV											

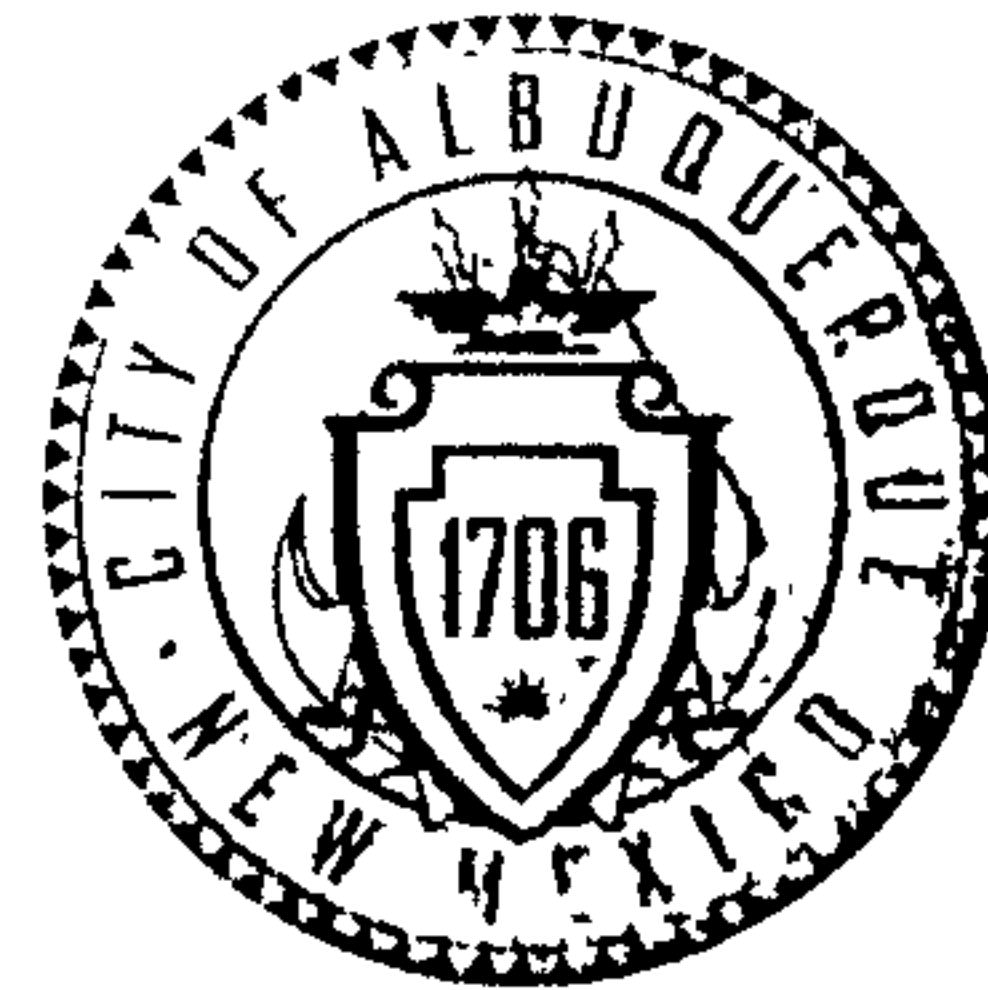
N D B ARROYO BETWEEN HOLBROOK & VENTURA (AP 921.78) - FINAL											
DD HYD	921.78	6 & 15	1	3.17623	146.51	103.565	.61137	2.450	.072		
UTE MCUNGE	921.80	1	2	3.17623	146.36	103.558	.61133	2.550	.072	CCODE =	.1
INFALL TYPE= 2										RAIN24=	2.050
MPUTE NM HYD	921.20	-	1	.08740	54.31	1.827	.39203	1.500	.971	PER IMP=	10.00
N D B ARROYO AT VENTURA (AP 921.9) - NOT FINAL											
DD HYD	921.90	1 & 2	26	3.26363	149.84	105.386	.60546	1.900	.072		
INFALL TYPE= 2										RAIN24=	2.170
MPUTE NM HYD	913.10	-	1	.03600	19.96	.634	.33028	1.500	.866	PER IMP=	5.00
UTE MCUNGE	913.88	1	2	.03600	17.36	.634	.33028	1.800	.754	CCODE =	.1
INFALL TYPE= 2										RAIN24=	2.100
MPUTE NM HYD	917.00	-	1	.04060	21.97	.693	.32022	1.500	.845	PER IMP=	5.00
HOLBROOK (AP 917.99) - FINAL											
DD HYD	917.99	1 & 2	1	.07660	24.10	1.327	.32494	1.750	.492		
UTE MCUNGE	917.88	1	2	.07660	23.96	1.328	.32495	1.850	.489	CCODE =	.2
INFALL TYPE= 2										RAIN24=	2.070
MPUTE NM HYD	920.10	-	1	.02020	13.52	.448	.41586	1.500	1.045	PER IMP=	10.00
INTO VINEYARD ESTATES SUBDIVISION UNIT IV AT NORTH END											
BETWEEN HOLBROOK AND VENTURA (AP 920.19) - FINAL (UNBULKED)											
DD HYD	920.19	2 & 1	1	.09680	33.38	1.775	.34391	1.600	.539		
MPUTE NM HYD	920.00	-	5	.04310	23.68	.890	.38728	1.500	.858	PER IMP=	10.00
INTO VINEYARD ESTATES SUBDIVISION UNIT IV AT EAST (NORTH END)											
BETWEEN HOLBROOK AND VENTURA (AP 920) - FINAL (UNBULKED)											

BEGIN: VINYARD ESTATES - UNIT IV											

INFALL TYPE= 2										RAIN24=	2.010
MPUTE NM HYD	922.10	-	2	.00700	4.91	.180	.48193	1.500	1.096	PER IMP=	17.00
MPUTE NM HYD	923.00	-	12	.00700	4.92	.158	.42326	1.500	1.098	PER IMP=	10.00
MPUTE NM HYD	922.20	-	9	.01480	21.04	.781	.98968	1.500	2.221	PER IMP=	40.00
MPUTE NM HYD	922.30	-	16	.04150	58.97	2.190	.98968	1.500	2.220	PER IMP=	40.00
S COMBINE 920.1 + 922.1 AS 922.19											
DD HYD	922.19	1 & 2	3	.10380	36.97	1.955	.35321	1.600	.556		
S ROUTE 920 TO POND AS 920.8											
UTE MCUNGE	920.80	5	4	.04310	23.61	.890	.38725	1.550	.856	CCODE =	.1
S ROUTE 922.19 THRU POND AS 920.88											
UTE RESERVOIR	920.88	3	7	.10380	18.83	1.955	.35321	2.000	.283	AC-FT=	.850
S ADD 920.88 920.8 AS 920.9 (POND Q + 920)											
DD HYD	920.90	4 & 7	7	.14690	28.11	2.845	.36319	1.600	.299		
S ROUTE POND DISCHARGE & 920 THRU 30" PIPE AS 920.98											



CITY OF ALBUQUERQUE



May 7, 2009

FEMA Depot
3601 Eisenhower Ave
Alexandria VA, 22304

Re: Request for a Letter of Map Revision based on Fill (LOMR-F) for the Vintner Court Subdivision, City of Albuquerque, New Mexico, Community No. 350002, FIRM Panel 35001C0141G

Dear FEMA review engineer,

The purpose of this submittal is to convey the documents in order to request a Letter of Map Revision – Fill for the Vintner Court subdivision. This request is to remove floodplain on a temporary pond that was reclaimed by constructing a storm drain to accept flows that used to enter this pond.

Enclosed with this letter are the Application and analysis of the project. Also enclosed is review fee of \$800.

As the Community official, I have reviewed the application and endorse this revision request. Our Community would greatly appreciate your prompt response and approval.

If you have any questions, you can contact me at (505) 924-3986.

Sincerely,

Bradley L. Bingham, PE, CFM
City Floodplain Administrator

denied

C: Elvidio Diniz, Resource Technologies
Bill Blanton, FEMA Headquarters
Jack Quarles, FEMA Region VI
file



City of Albuquerque

P.O. BOX 1293 ALBUQUERQUE, NEW MEXICO 87103

June 14, 2004

Philip W Clark, PE
Clark Consulting Engineers
19 Ryan Rd.
Edgewood, NM 87015

Re: Vintner Court Subdivision Drainage Report
Engineer's Stamp dated 4-20-04 (C20/D50)

Dear Mr. Clark,

Based upon the information provided in your submittal dated 2-18-04, the above referenced report is approved for Preliminary Plat action by the DRB. The Work Order must be under way prior to grading the pond to the proposed elevations.

This project requires a National Pollutant Discharge Elimination System (NPDES) permit. Refer to the attachment that is provided with this letter for details. If you have any questions please feel free to call the Municipal Development Department, Hydrology section at 768-3654 (Charles Caruso) or 768-3645 (Bryan Wolfe).

If you have any questions, you can contact me at 924-3986.

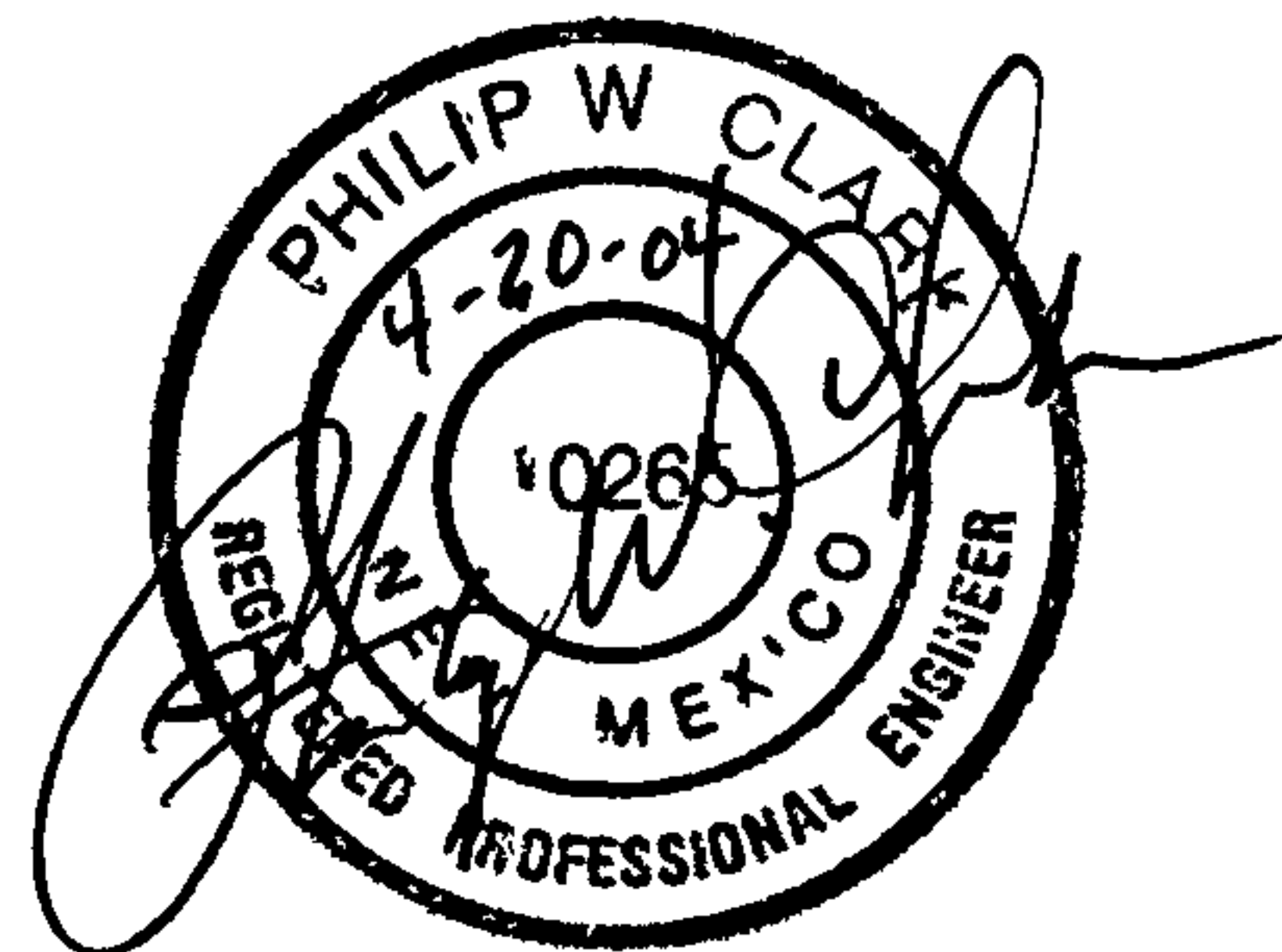
Sincerely,

Bradley L. Bingham, PE
Principal Engineer, Planning Dept.
Development and Building Services

C: Chuck Caruso, DMD
file

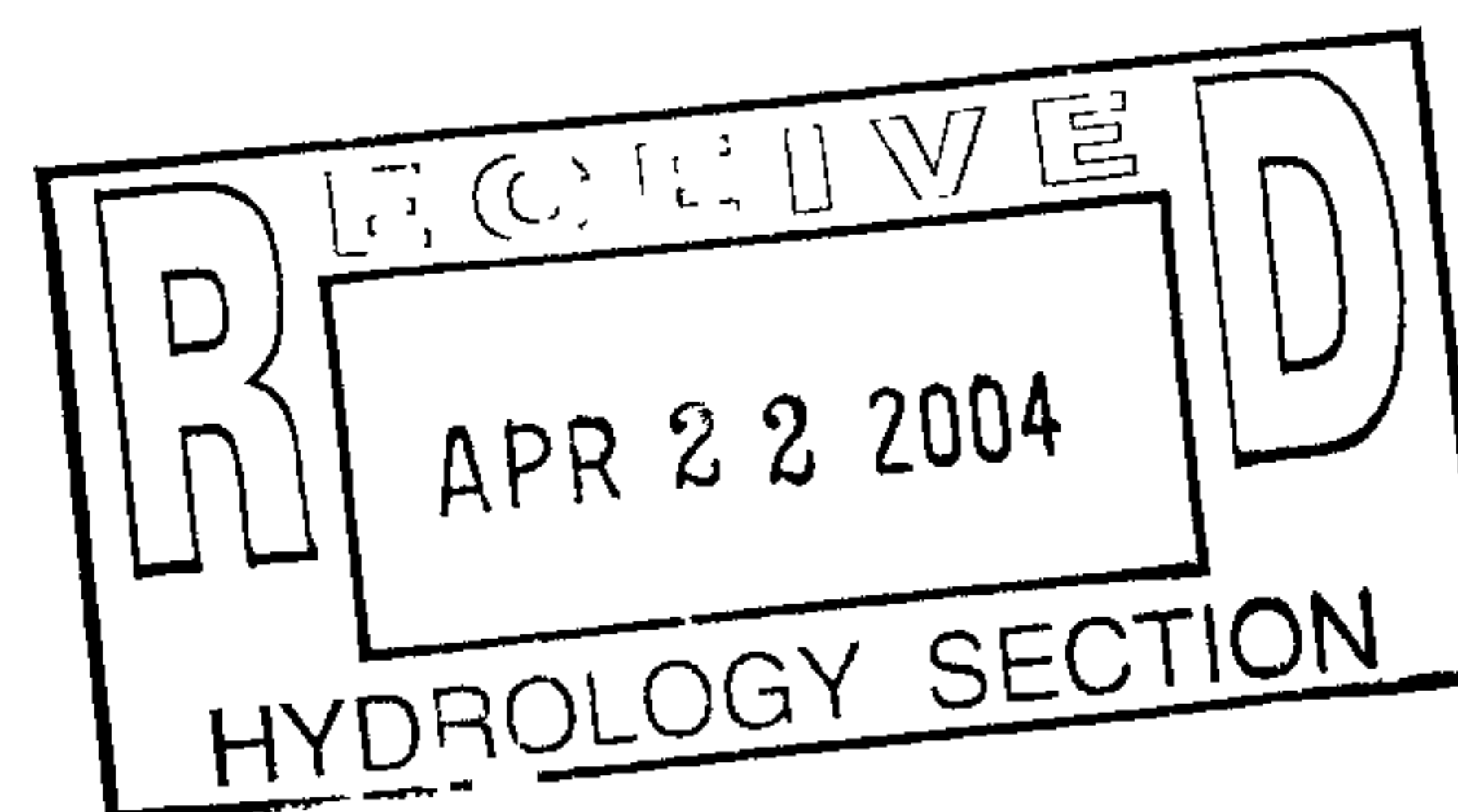
LISTING OF CONTENTS

- EXECUTIVE SUMMARY
- LOCATION AND DESCRIPTION – See Plan/ Text
- EXISTING DRAINAGE CONDITIONS – See Plan/ Text
 - A. Offsite Drainage Basins (Pocket)
 - B. Onsite Drainage Basins (See Plan)
- DRAINAGE ANALYSIS/PROPOSED CONDITIONS – See Plan
 - A. Drainage Structures
 - B. Preliminary Storm Sewer Plan & Profile Sheets – Wilshire Ave.
- APPENDIX
 - A. Calculations
 - B. **Cost Allocation Plan** for Off-site Storm Sewer Improvements
 - 1. Cost Allocation Table
 - 2. Parcel Benefactor Map
 - 3. Engineer's Estimate
 - 4. Ownership (AGIS)



LIST OF FIGURES and EXHIBITS

FIRM MAP – Panel 141, Map # 35001C0141 E, Dated 4/2/2002
EXISTING STORM SEWER FACILITIES MAP, 2004
SUBDIVISION GRADING & DRAINAGE PLAN - Located in Pocket
UPSTREAM BASIN MAP – Located in Pocket
PRELIMINARY PLAN & PROFILE SHEETS – Wilshire Ave., Pocket



EXECUTIVE SUMMARY

The purpose of the following analysis is to establish storm drainage management for the proposed 6-lot residential subdivision located within the City of Albuquerque. The plan shows free-discharge of developed on-site storm run-off directed to the new street, and then conveyed via a 30-inch diameter pipe to the existing underground conduit system located within Vineyard Estates Subdivision, Unit IV (the south adjoiner to project).

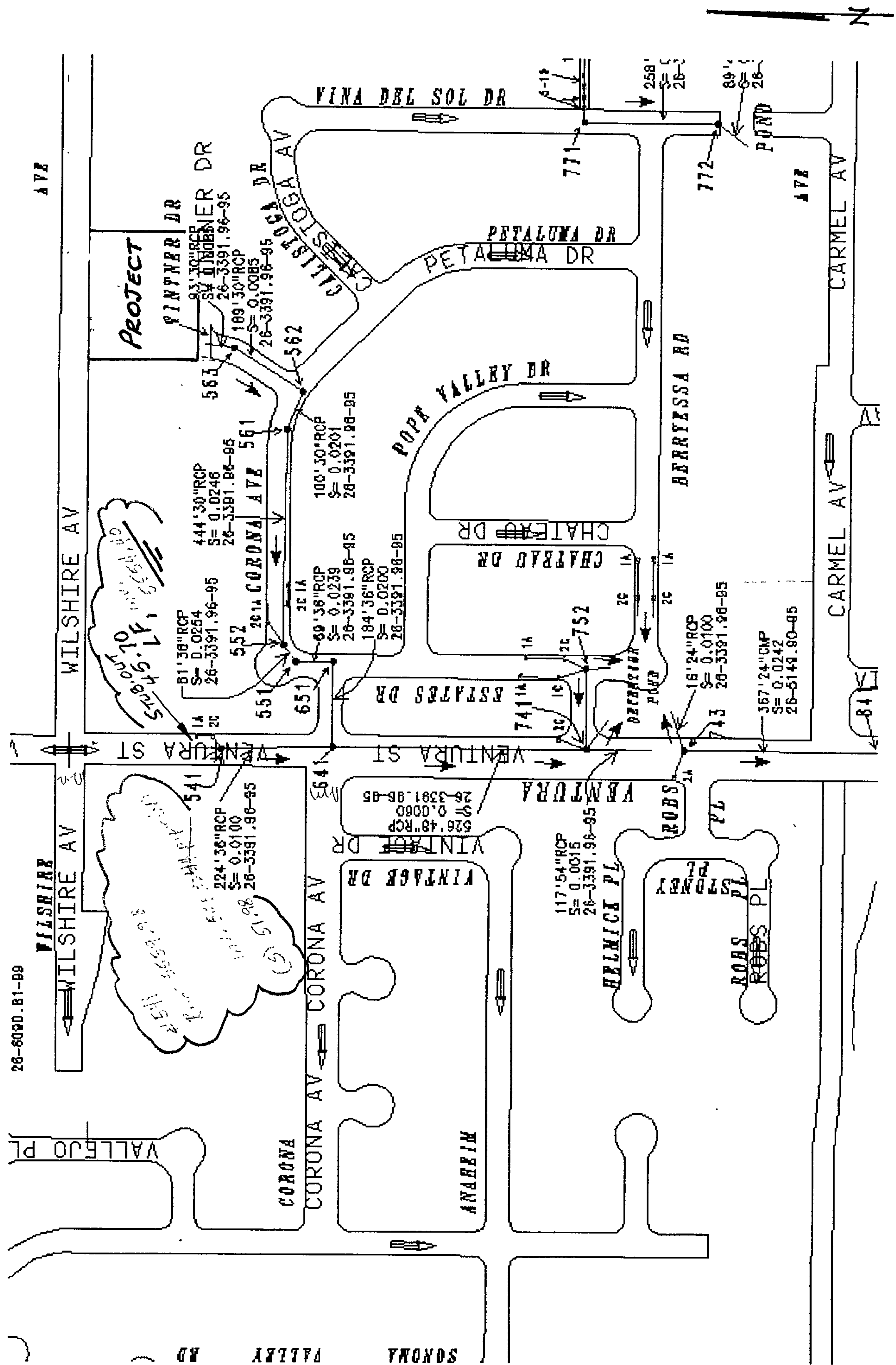
Off-site surface drainage impacts the site from the north generated by a minor drainage basin, 59 acres in size. The Vineyard Estates Master Drainage Plan, approved by the City of Albuquerque in 1995, indicates that the design year (100-year frequency), 6-hr. discharge to be 104 cfs. The analysis contained herein substantiates that figure using 101 cfs.

Currently, the site is used as an earthen detention facility (or pond) impounding the required volume associated with the 104 cfs, with release at the historic undeveloped rate as determined in the master plan. FEMA has identified the subject facility as a 100-year floodplain, therefore this project will require removal of the Zone A floodplain utilizing the Letter of Map Revision (LOMR) process prior to build-out of residential homes.

The plan as proposed will intercept the northern off-site flow with a collection of drop inlets located within Wilshire Avenue, and convey the 101 cfs westerly to Ventura Street, and then south to the existing storm sewer system which is located approximately 250 feet south of the intersection of Ventura and Wilshire.

The developer will enter into agreement with the City to construct the required storm sewer improvements using the Work Order process, construct the improvements, gain City acceptance, and coordinate the submission of the final LOMR.

578 km: 2-28



1"=300'

EXIST. COA- STORM SEWER FACILITIES MAP . 2004



Clark Consulting Engineers

19 Ryan Road
Edgewood, New Mexico 87015
E-Mail ccealbq@aol.com

SHEET / OF 5

Tele: (505) 281-2444

Fax: (505) 281-2444

CALCULATIONS

DATE: 4/16/04 Bw
PROJECT: Vintner Ct.
Subd.

QUANTIFY UPSTREAM RUNOFF IMPACTING THE PROPERTY @ Wilshire Ave.
REFERENCE DPM SECTION 22.2 - PART A.7 "RATIONAL METHOD FOR WATERSHEDS"

$$Q_p = CIA \dots \text{WHERE: } I = 0.726 \log_{10} (24.6 t_c) 1/t_c \times P_{60}$$

$$P_{60} = 2.14 \text{ INCHES (NOAA ISOPLETHALS)}$$

REACH LENGTHS BETWEEN 4000-12000'

$$t_c = ((12000-L)/(72000 \cdot K \cdot S^{0.5})) + ((L-4000) \cdot K_N \cdot (L_{CA}/L)^{0.33} / (552.2 \cdot S^{0.165}))$$

$$= 5700/30766 + 60.38/312.44$$

$$t_c = 0.38 \text{ HRS.}$$

WHERE:
K = CONVEYANCE FACTOR
(TABLE B-1)
K_N = CONVEYANCE FACTOR
(TABLE B-2)
L = 6300'

$$S = 200'/6300 = 0.0317'/\text{Ft.}$$

$$K = 6300 / ((250/0.7 + 1500/2 + 4550/3))$$

$$= 2.4$$

$$K_N = 0.033$$

$$L_{CA} = 0.5$$

C = 'CC' = COMPOSITE RATIONAL METHOD COEFFICIENT
17% D, 20% C, 20% B, 43% A.....A-1 ZONING
THEN: C = 0.53

BASIN 50% DEVELOPED, THEREFORE:

$$CC = .5(0.53) + 0.5(0.35) = 0.44$$

$$Q = 0.44(3.9) 59 \text{ AC.} = \underline{101 \text{ CFS}}$$

Manning Pipe Calculator

Given Input Data:

Shape	Circular
Solving for	Flowrate
Diameter	30.00 in ✓
Depth	28.00 in
Slope	0.0100 ft/ft ✓
Manning's n	0.0130

Computed Results:

Flowrate	44.115 cfs
Area	4.91 ft ²
Wetted Area	4.77 ft ²
Wetted Perimeter	78.58 in
Perimeter	94.25 in
Velocity	9.25 fps
Hydraulic Radius	8.74 in
Percent Full	93.33 %
Full flow Flowrate	41.017 cfs
Full flow velocity	8.36 fps

Critical Information

Critical depth	26.52 in
Critical slope	0.0104 ft/ft
Critical velocity	9.61 fps
Critical area	4.59 ft ²
Critical perimeter	73.41 in
Critical hydraulic radius	9.01 in
Critical top width	19.21 in
Specific energy	3.66 ft
Minimum energy	3.32 ft
Froude number	0.83
Flow condition	Subcritical

Exist. 30" SD $\frac{1}{2}$ New 30" @ Vintner
@ Vintner

$$Q_{c.a.p.} = 44 \text{ cfs}$$

$$Q_{100} \text{ developed} = 56 \text{ cfs}$$

$$\therefore Q_{\text{street, S. on Vintner}} = 12 \text{ cfs}$$

$$\text{depth} = 0.35 @ 2\%$$

per PL. D-1

1-01-04

Manning Pipe Calculator

Given Input Data:

Shape	Circular
Solving for	Flowrate
Diameter	36.00 in
Depth	36.00 in
Slope	0.0200 ft/ft
Manning's n	0.0130

Computed Results:

Flowrate	94.326 cfs ✓
Area	7.07 ft ²
Wetted Area	7.07 ft ²
Wetted Perimeter	113.10 in
Perimeter	113.10 in
Velocity	13.34 fps
Hydraulic Radius	9.00 in
Percent Full	100.00 %
Full flow Flowrate	94.326 cfs
Full flow velocity	13.34 fps

36" @ 2%
Ventura/Wilshire
South
Ultimate Develop.

Manning Pipe Calculator

Given Input Data:

Shape	Circular
Solving for	Flowrate
Diameter	36.00 in
Depth	36.00 in
Slope	0.0270 ft/ft
Manning's n	0.0130

on Wilshire @ Venture
Q_{cap} = 110 cfs

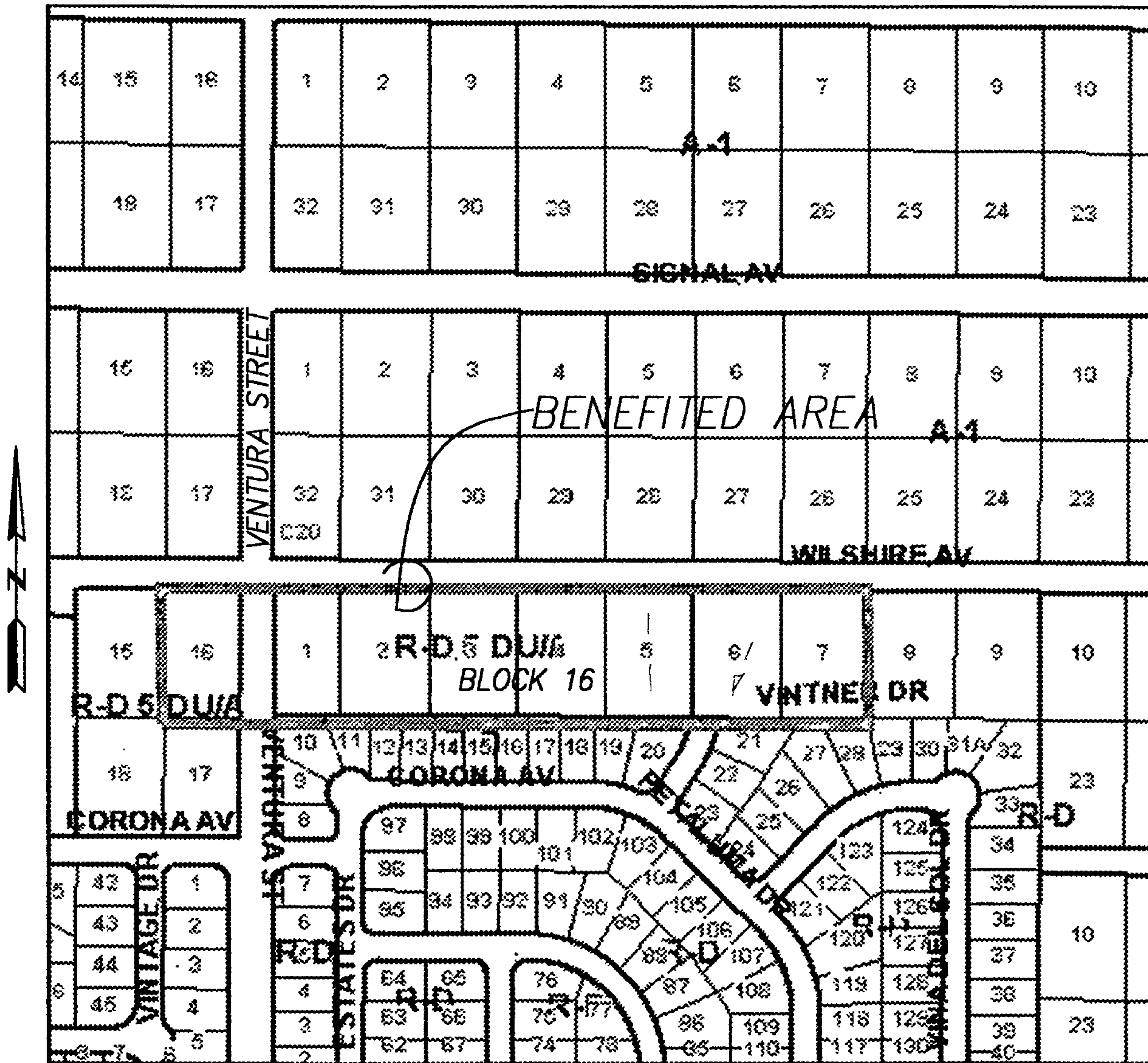
Computed Results:

Flowrate	109.597 cfs ←
Area	7.07 ft ²
Wetted Area	7.07 ft ²
Wetted Perimeter	113.10 in
Perimeter	113.10 in
Velocity	15.50 fps
Hydraulic Radius	9.00 in
Percent Full	100.00 %
Full flow Flowrate	109.597 cfs
Full flow velocity	15.50 fps

BENEFITED PARCEL	OWNER NAME	OWNER ADDRESS	*ALLOCATED COST
LOT 16, BLK 6, TR3, U3 N ABQ ACRES	ARGYRES, PETE & MARY P. & ARGYRES, GEORGE K & KATHERINE	3911 CENTRAL AVE ALBUQUERQUE, NM 87108	17,708
LOT 1, BLK 16, TR3, U3 N ABQ ACRES	KRUGER, STANLEY & ARLYNE H.	11007 BERMUDA DUNES ALBUQ., NM 87111	17,708
LOT 2, BLK 16, TR3, U3 N ABQ ACRES	KRUGER, STANLEY & ARLYNE H.	11007 BERMUDA DUNES ALBUQ., NM 87111	17,708
LOT 3, BLK 16, TR3, U3 N ABQ ACRES	KRUGER, STANLEY & ARLYNE H.	11007 BERMUDA DUNES ALBUQ., NM 87111	17,708
LOT 4, BLK 16, TR3, U3 N ABQ ACRES	AL-SABASSI, ABDUL FATTAH	PO BOX 65028 MANSORIA ALBUQ., NM	17,708
LOT 5, BLK 16, TR3, U3 N ABQ ACRES	AL-SABASSI, ABDUL FATTAH	PO BOX 65028 MANSORIA KUWAIT	17,708
LOT 6, BLK 16, TR3, U3 N ABQ ACRES	LLAVE CONSTRUCTION, INC.	PO BOX 93642 ALBUQ., NM 87199	17,708
LOT 7, BLK 16, TR3, U3 N ABQ ACRES	LLAVE CONSTRUCTION., INC.	PO BOX 93642 ALBUQ., NM 87199	17,708

* PRO-RATED BENEFIT OF DEVELOPED DISCHARGE = 1/8
OF THE TOTAL BENEFITED AREA

COST ALLOCATION TABLE



PARCEL BENEFACTOR MAP

ZAP C-20

CLARK CONSULTING ENGINEERS
281-2444

PROCEDURE 'B'
COA PROJECT NO.

4/1/04

VINTNER COURT-STORM SEWER ALLOCATION

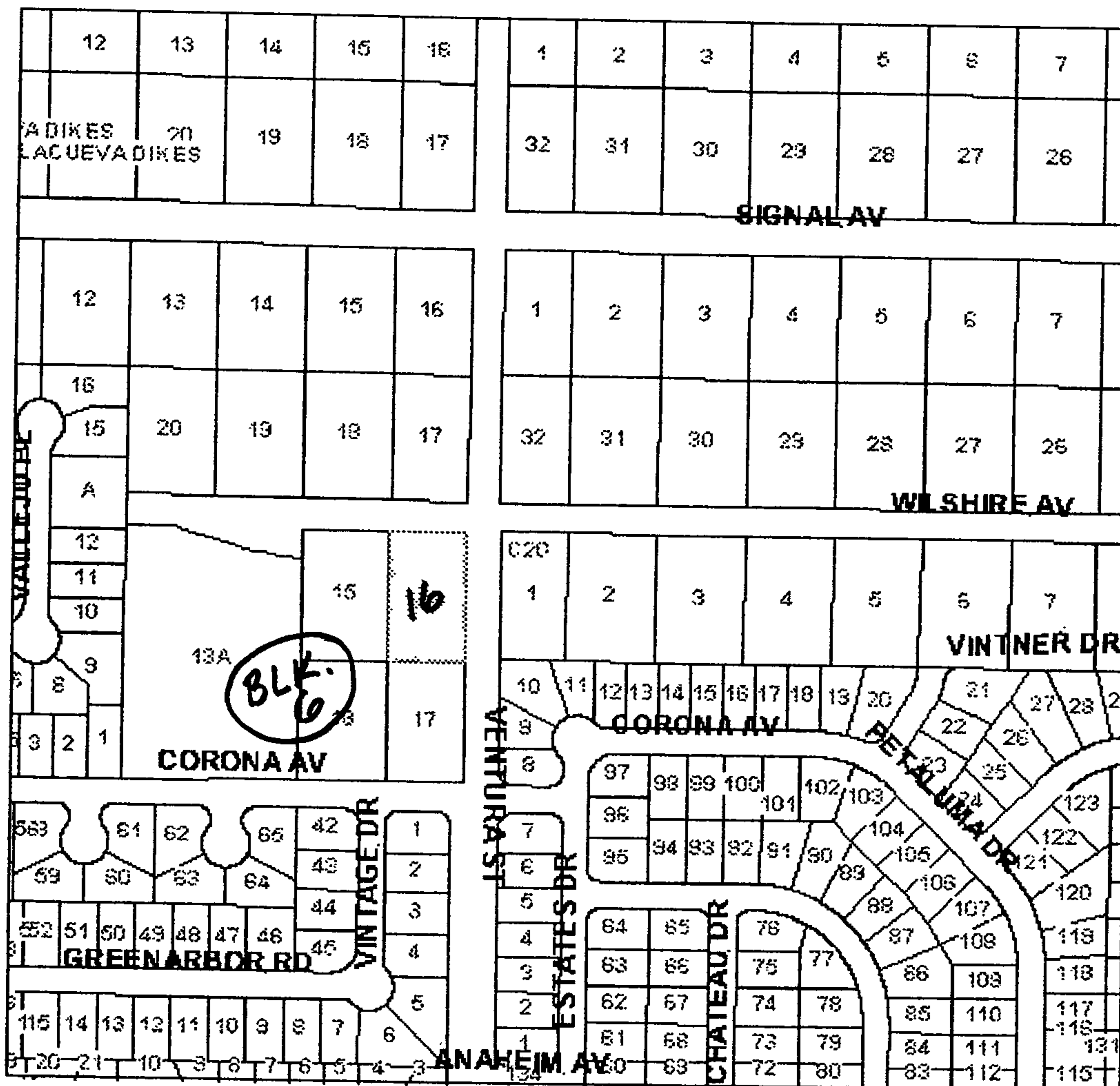
					ESTIMATED
<u>ITEM</u>	<u>DESCRIPT.</u>	<u>UNIT PRICE</u>	<u>QUANTITY</u>		<u>\$ AMOUNT</u>
4.01	STKG	~2%			2000.00
6.05/6.06	MOB/DEMOB	~3%			3000.00
201.01	SITE CLEAR/GRUB	0.3	627	AC	188.10
204.01	EXCAV & DISP>2	3.93	1000	CY	3930.00
301.01	GRADING <2'	1.62	850	SY	1377.00
343.132	ART PVMT, R&R, W/M	30.93	245	SY	7577.85
701.1	TRCH, BF, 18-36"SWR	12.9	1284		16563.60
910.005	18" RCP, III	21.38	100	LF	2138.00
910.009	24" RCP, III	26.07	40	LF	1042.80
910.017	36" RCP, III	43.48	1244	LF	54089.12
915.01	CTH BSN A	2202.19	4	EA	8808.76
915.04	CTH BSN C, DG	2756.05	2	EA	5512.10
920.07	MH, 4' DIA., E	1461.18	4	EA	5844.72
	SUB-TOTAL ESTIMATED CONSTRUCTION				\$112,072.05
	CONTIGENCY 10%				11207.21
	NM GR TAX, 5.8125%				7165.61
	DESIGN REVIEW, 6.6%				8609.36
	TESTING, 2%				2608.90
	ENG'G DESIGN/ADMIN 6%+4%				<u>13044.49</u>
	TOTAL CONSTRUCTION COST, ESTIMATED				\$141,663.12

ACTIVATE BUTTONS BY 'CLICKING' ON THE MAP

☐ Zoom In
 ☒ Id Address
 ☐ Id ZM
 ☐ Pan
 ☐ Zoom Out

LAYER LEGEND

- ☒ STREET NAMES
- ☐ PARKS
- ☐ CITY LIMITS
- ☒ ZONE MAP GRID
- ☐ NBR BOUNDARY
- ☐ COMMUNITY PLAI
- ☐ WATER LINES
- ☐ SEWER LINES
- ☐ STORM DRAINS
- ☐ ZONING
- ☒ LOT NUMBERS
- ☐ ZIP CODES
- ☐ COUNCIL DISTRI
- ☐ FLOOD ZONES (di
- ☒ PARCELS
- ☐ CONTROL STATIO
- ☐ SENATE DIST.
- ☐ REPRESENTATIVE
- ☐ COUNTY COMMIS
- ☐ PARCEL ADDRESS
- ☐ CRIMINAL ACTIV
- ☐ PUBLIC FACILITI
- ☐ LAND USE
- ☐ 1960 CITY LIMIT
- ☐ LANDFILLS/BUFF
- ☐ CRP LOCATIONS



Redraw Screen

SHOW LOCATION

SHOW 1999 AERI

Selected Address: 9050 WILSHIRE AV NE

Zoning: R-D 5 DU/A **Lot/Block/Subd: 16 , 6 , N ABQ ACRES TR 3 UNIT 3**

Council District/Name: FOUR , WINTER **County Commission: 4** **Rep District/Sen District: 27 , 10**

Nbr Assoc: VINEYARD ESTATES R

Sector Plan: VINEYARD Comp. Plan: Developing Urban Design Overlay: AMOLE ARROYO

Voter Pct: 425

High Sch District: LA CUEVA **Mid Sch District: DESERT RIDGE** **Elem Sch District: DOUBLE EAGLE**

ZoneMap Page: C20 **Jurisdiction: CITY**

Police Beat: 434/NORTHEAST

Flood Zone: ZONE X

Comm Plan Area: NORTH ALBUQUERQUE

UPC #: 102006425425030917

Owner Name: ARGYRES PETE & MARY P & ARGYRES GEORGE K & KATHERINE G

Owner Street Address: 3911 CENTRAL AVE

Owner City/State/Zip: ALBUQUERQUE / NM / 87108 NM

Note: Accuracy for Owner info cannot be guaranteed correct.

Please check with the Bernalillo County Assessor for official data.

SEARCH FOR OTHER SERVICES AROUND THIS ADDRESS

ZOOM LEVEL **MEDIUM**

TEXT SIZE **MEDIUM**

Zoom In **Id Address** **Id ZM** **Pan** **Zoom Out**

- ☒ STREET NAMES
- ☐ PARKS
- ☐ CITY LIMITS
- ☒ ZONE MAP GRID
- ☐ NBR BOUNDARY
- ☐ COMMUNITY PLAI
- ☐ WATER LINES
- ☐ SEWER LINES
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- ☐ CRP LOCATIONS

SHOW 1999 AERI

Please check with the Bernalillo County Assessor for official data.

SEARCH FOR OTHER SERVICES AROUND THIS ADDRESS

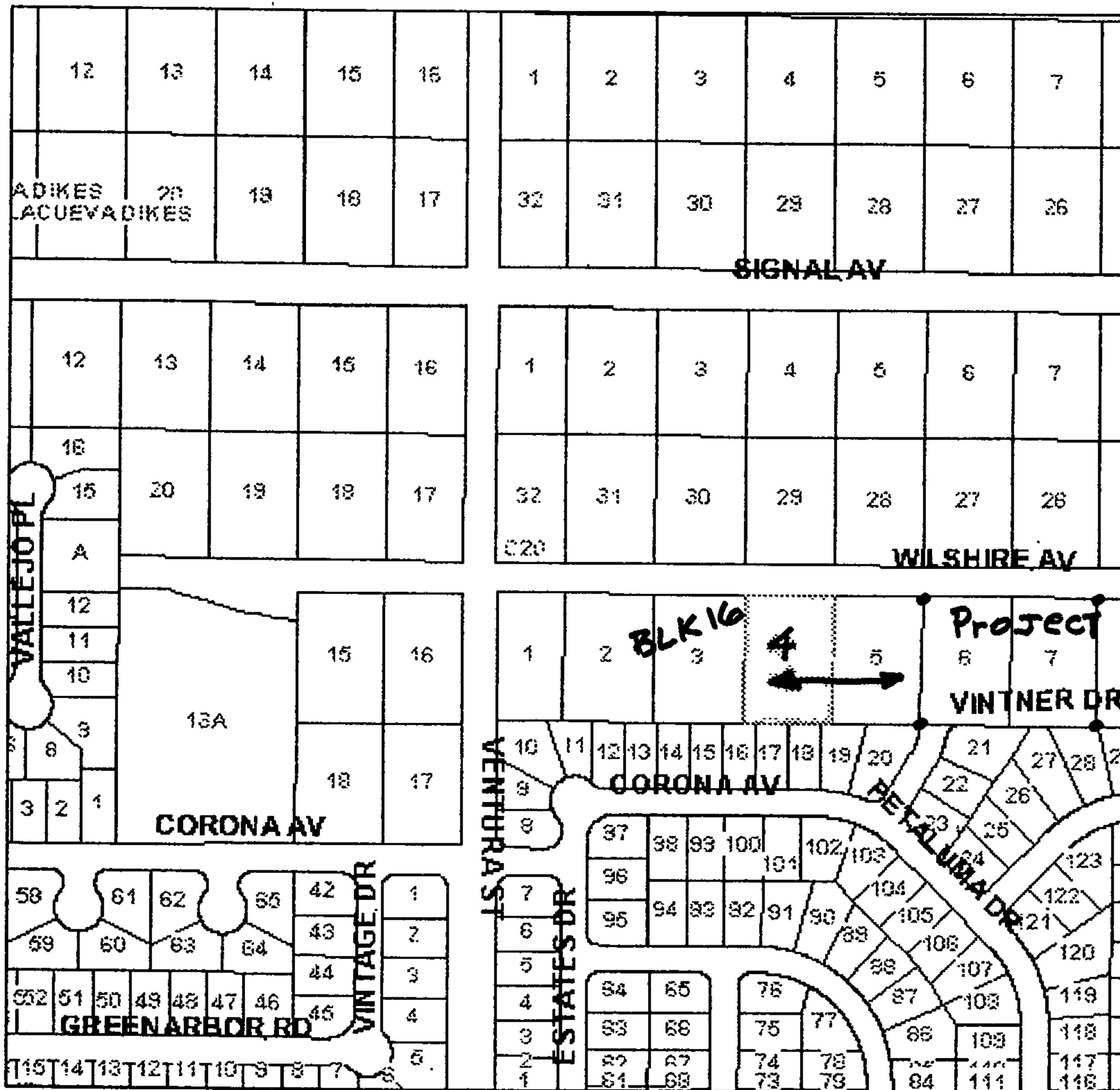
TEXT SIZE MEDIUM 

ACTIVATE BUTTONS BY 'CLICKING' ON THE MAP

☐ Zoom In
 ☒ Id Address
 ☐ Id ZM
 ☐ Pan
 ☐ Zoom Out

LAYER LEGEND

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- ☐ CRP LOCATIONS



Redraw Screen

SHOW LOCATION

SHOW 1999 AERI

Selected Address: ~~99999~~ WILSHIRE AV NE

Zoning: R-D 5 DU/A **Lot/Block/Subd:** 4 , 16 , N ABQ ACRES TR 3 UNIT 3

Council District/Name: FOUR , WINTER **County Commission:** 4 **Rep District/Sen District:** 27 , 21

Nbr Assoc: VINEYARD ESTATES R/N ALBUQUERQUE ACRES R

Sector Plan: VINEYARD Comp. Plan: Developing Urban Design Overlay: AMOLE ARROYO

Voter Pct: 426B

High Sch District: LA CUEVA

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Elem Sch District: DOUBLE

EAGLE

ZoneMap Page: C20 **Jurisdiction:** CITY

Police Beat: 434/NORTHEAST

Flood Zone: ZONE X

Comm Plan Area: NORTH ALBUQUERQUE

UPC #: 102006432125040529

Owner Name: AL-SABASSI ABDUL FATTAH PO BOX 65028 MANSORIA

Note: Accuracy for Owner info cannot be guaranteed correct.

Please check with the Bernalillo County Assessor for official data.

SEARCH FOR OTHER SERVICES AROUND THIS ADDRESS

ZOOM LEVEL **MEDIUM**

TEXT SIZE **MEDIUM**

NEW GIS QUERY