\* ZONE 4, ON-SITE (7001-110-198) WITH 3 DU/A \* Lot 15 Block 2, Unit 3, Tract 3, North Albuquerque Acres is located @ 2.93% SLOPE NYLOPLAST DRÁIN south side of Eagle Rock and west of Ventura Boulverad contains 100-YEAR, 6-HR STORM (UNDER HISTORICAL CONDITIONS) ALBUQUERQUE 7001-110-273 EAGLÆ ROCK ÄVENUE, NE +/- 0.89. See attached portion of the Vicinity Map for exact GRATE=51.50 52.66 EAGLE ROCK START INV. IN=<del>47.18</del> RAINFALL TYPE=1 RAIN QUARTER=0.0 IN INV.  $OUT = 47.08_{48.52}$ RAIN ONE=2.23 IN RAIN SIX=2.90 IN RAIN DELAY=3.65 IN DT=0.03333 HR The purpose of this drainage report is to present a grading and R-D 5551.87 × × 5.85 5 555.28 INV.=50.00 \* ON-SITE drainage solution for the proposed buildings. COMPUTE NM HYD ID=1 HYD NO=101.1 AREA=0.001385 SQ MI ACRES DRIVEWAY DRIVEWAY PER A=100.0 PER B=0.0 PER C=0.0 PER D=0.0 **Existing Drainage Conditions** TP=0.1333 HR MASS RAINFALL=-1 This site is undeveloped and falls within the NAA Master Drainage OAKLAND AVE Plan prepared by RTI. The site is within Basin 111.0 of the RTI POND C - RETENTION report and is subject to 108.83 cfs which passes throught the lot at 10-YEAR, 6-HR STORM (UNDER HISTORICAL CONDITIONS) VOLUME = 488.74 CF\* the notherly portion of the site. Another 1.71 cfs enters the mid Cap. 3 DU/A FIRST FLUSH PONDING START portion of the lot from the east. Based on the FIRM Map TYPE=1 RAIN QUARTER=0.0 IN RAINFALL 35001C0141G (revised September 26, 2008) the site does not fall 634.49 CF RAIN ONE=1.49 IN RAIN SIX=1.93 IN within a 100-year floodplain. RAIN DELAY=2.43 IN DT=0.03333 HR INV.=46.00 \* ON-SITE Proposed Conditions and On-Site Drainage Management Plan ID=1 HYD NO=111.1 AREA=0.001385 SQ MI The developed runoff generated from this site will have to comply COMPUTE NM HYD 5353.28 PER A=100.0 PER B=0.0 PER C=0.0 PER D=0.0 with the land treaments set as part of the NAA Master Drainage **VICINITY MAP:** TP=0.1333 HR MASS RAINFALL=-1 Plan. Additional runoff volume generated by this development will USE SHOTCRETE FOR TRW=54.33 \* be retained on site. Therefore, a retention pond with a volume of 5353.45 AREAS SHOWN ON THE LEGAL DESCRIPTION: 2,467.38 cf is desinged to retain the additional volume. The total 100-YEAR, 6-HR STORM (UNDER ALLOWABLE CONDITIONS) retention volume required is only 2,395.08 cf. The offsite flow of Lot 15 Block 2, Unit 3, Tract 3, North Albuquerque Acres START 1.71 cfs will enter the site to a pond and then intercepted by the BURGS REPLAT, PERFECTO ARMIJO, CONTAINING 0.89 ACRE RAINFALL TYPE=1 RAIN QUARTER=0.0 IN Beehive inlets and then discharged to the west via 10" pipe. The PROVIDE RIPRAP AT THE 108.83 cfs offsite rumoff from Basin 111.0, NAA Master Drainage RAIN ONE=2.23 IN RAIN SIX=2.90 IN TRW=55.64 æ0TIØM (D50 OF 9")∕ RAIN DELAY=3.65 IN DT=0.03333 HR Plan Basin #101.10 will be intercepted by 2-36" CMPs and will be BENCH MARK DESCRIPTION: \* ON-SITE carried acrros the property. INV.<del>≤47.00</del> ′ COMPUTE NM HYD ID=1 HYD NO=102.1 AREA=0.001385 SQ MI X=1553078.775, Y=1524900.435, ELEV.=5566.658 PER A=43.0 PER B=20.00 PER C=20.0 PER D=17.0 TP=0.1333 HR MASS RAINFALL=-1 City of Albuquerque, Development Process Manuel, Section 22.2, \* OFFSITE 111.0-A Hydrology Section, was used for runoff calculations. See this plan ID=1 HYD NO=103.2 AREA=0.000839 SQ MI COMPUTE NM HYD for AHYMO input and Summary output files. PER A=43.0 PER B=20.00 PER C=20.0 PER D=17.0 5353.05 1: CONTOUR INTERVAL IS HALF (1.00) FOOT. TP=0.1333 HR MASS RAINFALL=-1 2: ELEVATIONS ARE BASED ON CITY OF ALBUQUERQUE CONTROL STATION FIRST FLUSH PONDING 7\_B20, HAVING AN ELEVATION OF <u>5566.658</u> FEET ABOVE SEA LEVEL 10-YEAR, 6-HR STORM (UNDER ALLOWABLE CONDITIONS) VOLUME REQUIRED = EAGLE ROCK 3: UTILITIES SHOWN HEREON ARE IN THEIR APPROXIMATE LOCATION BASED \* 634.49 CF ONLY ON ABOVE GROUND EVIDENCE FOUND IN THE FIELD AND AS-BUILT INFORMATION PROVIDED BY THE CLIENT. UTILITIES SHOWN HEREON, WHETHER RAINFALL TYPE=1 RAIN QUARTER=0.0 IN INDICATED AS ABANDONED OR NOT, SHALL BE VERIFIED BY OTHERS FOR RAIN ONE=1.49 IN RAIN SIX=1.93 IN FF=5553.00 EXACT LOCATION AND / OR DEPTH PRIOR TO EXCAVATION OR DESIGN CON-RAIN DELAY=2.43 IN DT=0.03333 HR \* ON-SITE 8" NYLOPLAST DRAIN COMPUTE NM HYD 4: THIS IS <u>NOT</u> A BOUNDARY SURVEY, BEARINGS ARE ASSUMED, DISTANCES ID=1 HYD NO=112.1 AREA=0.001385 SQ MI 7001-110-272 AND FOUND PROPERTY CORNERS ARE FOR INFORMATIONAL PURPOSES ONLY. PER A=43.0 PER B=20.0 PER C=20.0 PER D=17.0 GRATE=52.67 5: SLOPES ARE AT 3:1 MAXIMUM. TP=0.1333 HR MASS RAINFALL=-1 INV. OUT=49.67 100-YEAR, 6-HR STORM (UNDER PROPOSED CONDITIONS) `6" PVC @ 1.66% 10" GRATE (7001-110-198) WITH NOTICE TO CONTRACTORS START NYLOPLAST DRAIN TYPE=1 RAIN QUARTER=0.0 IN RAINFALL 1. AN EXCAVATION/CONSTRUCTION PERMIT WILL BE REQUIRED BEFORE BEGINNING ANY 3ASIN 111.0-A 7001-110-273 5353.60 RAIN ONE=2.23 IN RAIN SIX=2.90 IN GRATE=49.50 51.25 WORK WITHIN CITY RIGHT-OF-WAY. RAIN DELAY=3.65 IN DT=0.03333 HR NV.⇒<del>48.01</del> 49.05 TRW=56.29\ TRW=52.82 \* ON-SITE 5353.74 . ALL WORK DETAILED ON THESE PLANS TO BE PERFORMED, EXCEPT AS OTHERWISE ID=1 HYD NO=101.2 AREA=0.001385 SQ MI COMPUTE NM HYD STATED OR PROVIDED HEREON, SHALL BE CONSTRUCTED IN ACCORDANCE WITH CITY OF PER A=0.0 PER B=32.0 PER C=10.0 PER D=58.0 ALBUQUERQUE INTERIM STANDARD SPECIFICATIONS FOR PUBLIC WORKS CONSTRUCTION, TRW=57.61 TP=0.1333 HR MASS RAINFALL=-1 5357.44 5353.60 3. TWO WORKING DAYS PRIOR TO ANY EXCAVATION, CONTRACTOR MUST CONTACT LINE 10-YEAR, 6-HR STORM (UNDER PROPOSED CONDITIONS) LOCATING SERVICE, 765-1234, FOR LOCATION OF EXISTING UTILITIES. \* START 4. PRIOR TO CONSTRUCTION, THE CONTRACTOR SHALL EXCAVATE AND VERIFY THE RAINFALL TYPE=1 RAIN QUARTER=0.0 IN HURIZUNTAL AND VERTICAL LOCATIONS OF ALL CONSTRUCTIONS. SHOULD A CONFLICT RAIN ONE=1.49 IN RAIN SIX=1.93 IN 358.26 EXIST, THE CONTRACTOR SHALL NOTIFY THE ENGINEER SO THAT THE CONFLICT CAN RAIN DELAY=2.43 IN DT=0.03333 HR 5353.78 BE RESOLVED WITH A MINIMUM AMOUNT OF DELAY. 5353.77 \* ON-SITE ID=1 HYD NO=111.2 AREA=0.001385 SQ MI CONC-W COMPUTE NM HYD 5. BACKFILL COMPACTION SHALL BE ACCORDING TO TRAFFIC/STREET USE. PER A=0.0 PER B=32.0 PER C=10.0 PER D=58.0 TRW=58.91 TP=0.1333 HR MASS RAINFALL=-1 OAKLAND AVE 6. MAINTENANCE OF THESE FACILITIES SHALL BE THE RESPONSIBILITY OF THE OWNER \* 5358.20 OF THE PROPERTY SERVED. G-W 5353.46 NEENAH R-4350-1 \* PORTION OF BASIN 111.0 FROM RTI'S NAA MASTER DRAINAGE PLAN 7. WORK ON ARTERIAL STREETS SHALL BE PERFORMED ON A 24-HOUR BASIS. OFFSITE BASIN MAP BEEHIVE GRATE TRW≠53.60 GRATE = 50.50 51.98LEGEND TRW=58.89 INV.=49.75 50.04 AHYMO PROGRAM SUMMARY TABLE (AHYMO\_97) -RUN DATE (MON/DAY/YR) =02/12/2015VERSION: 1997.02d 5353.842 CONC-W USER NO.= AHYMO-I-9702c01000R31-AH INPUT FILE = sant.txtEXISTING CONTOUR (MINOR) TRW=57.25 FROM TO CFS PEAK RUNOFF TIME TO PAGE = 150.50 × 5351.29 TRW=59.57 352.64 5353.26 CONC HYDROGRAPH ID **AREA** DISCHARGE VOLUME RUNOFF PEAK PROPOSED SPOT ELEVATION ¥ *85.46* COMMAND IDENTIFICATION NO. NO. (SQ MI) (CFS) (AC-FT) (INCHES) (HOURS) ACRE 5360. 5359.1: EXISTING GRADE 5265.16 5357 13 START RAINFALL TYPE= 1 RAIN6= 2.900 TRW=59.24 TRW=59.89 ∄ŔW=59.88 × 5284.43 EXISTING FLOWLINE ELEVATION COMPUTE NM HYD 101.10 .00139 .059 1.500 2.213 PER IMP= .00 POND A - RETENTION TRW=57.91 TRW=58.57 START TIME= VOLUME = 790.78 CFRAINFALL TYPE= 1 RAIN6= 1.930  $\times$  5284.43 EXISTING GROUND ELEVATION FIRST FLUSH PONDING .00139 .020 1.533 COMPUTE NM HYD 111.10 .71 .796 PER IMP= .00 VOLUME REQUIRED = START TRW=57.25 RAINFALL TYPE= 1 634.49 CF RAIN6= 2.900 PROPOSED RETAINING WALL DRAINAGE CERTIFICATION COMPUTE NM HYD 102.10 -.00139 2.81 .094 1.27203 1.500 3.168 PER IMP= 17.00 GRAPHIC SCALE 3.184 PER IMP= 17.00 COMPUTE NM HYD 103.20 .00084 1.71 1.27203 1.500 BC=89.08 BOTTOM OF CHANEL START I, REZA AFAGHPOUR , NMPE 11814, OF SBS CONSTRUCTION AND RAINFALL TYPE= 1 RAIN6= 1.930 ENGINEERING, LLC, HEREBY CERTIFY THAT THIS PROJECT HAS 112.10 -.00139 COMPUTE NM HYD 1.44 .045 1.500 1.624 PER IMP= 17.00 BEEN GRADED AND WILL DRAIN IN SUBSTANTIAL COMPLIANCE TRW=91.50 START TOP OF RETAINING WALL \*\* BASIN 111.0 FROM RTI'S NAA MASTER DRAINAGE PLAN WITH AND IN ACCORDANCE WITH THE DESIGN INTENT OF THE RAINFALL TYPE= 1 RAIN6= 2.900 TF=88.00 SCALE: 1"=20' APPROVED PLAN DATED 05-12-2015 THE RECORD COMPUTE NM HYD 101.20 .00139 3.87 2.01759 1.500 4.367 PER IMP= 58.00 TOP OF FOOTING INFORMATION EDITED ONTO THE ORIGINAL DESIGN DOCUMENT START RAINFALL TYPE= 1 RAIN6= HAS BEEN OBTAINED BY NMPS 9801, OF SBS CONSTRUCTION AND HIGH POINT COMPUTE NM HYD 111.20 .00139 2.40 1.500 2.709 PER IMP= 58.00 ENGINEERING, LLC. I FURTHER CERTIFY THAT I HAVE PERSONALLY **FINISH** VISITED THE PROJECT SITE ON AND HAVE DETERMINED BY VISUAL TRW=55.64 AS-BUILT RET. WALL INSPECTION THAT THE SURVEY DATA PROVIDED IS REPRESENTATIVE OF ACTUAL SITE CONDITIONS AND IS TRUE AND MATCH GRADE AT 6" SHOTCRETE (OVER 4X4 CORRECT TO THE BEST OF MY KNOWLEDGE AND BELIEF. THIS × 5353.05 POND 6" SHOTCRETE (OVER 4X4 (11814)PIPE & GRATE CAPACITY CALULATIONS WELDED WIRE MESH) PROPERTY LINE AS-BUILT GRADES CERTIFICATION IS SUBMITTED IN SUPPORT OF A REQUEST FOR WELDED WIRE MESH) FINAL CERTIFICATE OF OCCUPANCY CALCULATING PIPE CAPACITY UTING ORFICIE EQUATION:  $Q = CA(2gh)^{0.50}$ 5/12/2015 SBS CONSTRUCTION THE RECORD INFORMATION PRESENTED HEREON IS NOT RIP RAP RIP RAP **NEENAH R-4350-1 BEEHIVE GRATE:** NECESSARILY COMPLETE AND INTENDED ONLY TO VERIFY AND ENGINEERING, LLC  $Q = 0.6 \times 0.30 (2 \times 32.2 \times 2.0)^{0.50} = 2.04 \text{ cfs}$ INV.=46.00 · SUBSTANTIAL COMPLIANCE OF THE GRADING AND DRAINAGE ASPECTS OF THIS PROJECT. THOSE RELYING ON THIS RECORD **REZA AFAGHPOUR** 10209 SNOWFLAKE CT., NW NYLOPLAST 10" GRATE (7001-110-198): P.E. #11814 DOCUMENT ARE ADVISED TO OBTAIN INDEPENDENT VERIFICATION ALBUQUERQUE, NEW MEXICO 87114  $Q = 0.6 \times 0.20 (2 \times 32.2 \times 1.0)^{0.50} = 0.95 \text{ cfs}$ OF ITS ACCURACY BEFORE USING IT FOR ANY OTHER PURPOSE. 2-36" CMP (505)899-5570 8" PVC: LOT 15, BLOCK 2, UNIT3, TRACT 3, NAA  $Q = 0.6 \times 0.35 (2 \times 32.2 \times 1.50)^{0.50} = 2.06 \text{ cfs}$ GRANULAR **GRADING AND DRAINAGE PLAN BEDDING** BEDDING  $Q = 2 \times 0.6 \times 0.55 (2 \times 32.2 \times 2.0)^{0.50} = 3.79 \text{ cfs}$ 

SECTION B-B

NTS

SECTION A-A

NTS

2-36" PVC:

 $Q = 2 \times 0.6 \times 7.07 (2 \times 32.2 \times 4.0)^{0.50} = 136.07 \text{ cfs}$ 

C-20-Z

DRAWN BY:

SH-B

DATE:

2-16-2015

SHEET#

DRAWING:

LAST REVISION: 5/12/201

201423-GR.DWG

REZA AFAGHPOUR, NMPE 11814