CITY OF ALBUQUERQUE



March 17, 2015

Reza Afaghpour, PE through SBS Construction and Engineering, LLC 10209 Snowflake Court NW Albuquerque, NM 87114

RE: Lot 15, Block 2, Unit 3, Tract 3, Eagle Rock Avenue Grading and Drainage Plan Engineer's Stamp Date 2-22-2015 (File: C20-D056B)

Dear Mr. Afaghpour:

retention at this location.

Based upon the information provided in your submittal received 2-24-15, the above referenced plan cannot be approved for Building Permit until the following comments are addressed:

1) Provide a survey benchmark and existing spot elevations along the entire site perimeter.

2) Label the first flush volumes on the plan view. Show the roof drain locations. If part of the roof is draining to the north, locate some of this retention for the first flush immediately to the north of the proposed building, and label the volume of

- Albuquerque
- New Mexico 87103

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- 3) How do the retaining wall heights match existing contour elevations? In addition to providing the top of retaining wall, provide the bottom of wall elevations in lieu of the top of footing elevations. (The elevations for the wall labeled along the southeast corner of the site look incorrect.)
- 4) Provide new contours tie-ins to existing contours at the site boundaries. Existing contours do not appear to represent improvements on Lot 18 to the south. On Lot 18, there is already an existing wall along the boundary, and the proposed wall construction conflicts with this. Also, show boundary line for subject property according to the legend.
- 5) Show calculations for the 1.71 cfs of off-site flow discharged to the site from the east. How is the flow getting to each of the on-site inlets? (The listed beehive grate elevations appear to be too high relative to the surrounding areas.) Provide more

proposed contours. Include discussion of off-site flows within the "Proposed Conditions" write-up, and also include a very brief description of Basin 111.0 from the NAA Report, especially since it is a huge contributor of off-site flow.

6) On the downstream end of the pipe where the riprap is shown, provide calculations for determination of both riprap dimensioning and sizing. Specifically for the sizing, provide a "d50" of 9 inches or else provide a computation for determination of sizing. For the riprap dimensioning, there is a concern that there is not as long of a riprap section downstream of the two 36" pipes to handle the amount and the velocity of flow.

Based upon these calculations, show the size and depth of riprap that is placed at the downstream and upstream ends of the pipe. If a longer section of riprap is required based upon these calculations, shorten the two 36-inch pipes to make more room for the riprap.

- There is concern about scour where the shotcrete meets the riprap. Either provide a scour wall that extends a minimum of two feet below grade, or provide computations for the required depth of the scour wall.
- 8) Label the slope of the two 36" pipes on the plan view. If these are CMPs, call out the gauge of the pipes.
- 9) Show the proposed driveway location for the site and the proposed grades. Show adequate pipe cover at the point where the proposed driveway crosses over the 2-36" pipes.
- 10) Show proposed pipe run slopes for the proposed 8" pipe and the 100-year flow each individual pipe run is conveying. Provide capacity calculations for each of the individual pipe runs.
- 11) To reduce on-site flow from discharging to the site immediately to the west, provide an inlet for the 8" storm drain system to capture the flow from the southwest corner of the site.
- 12) Place headwall at the inlet location for the two 36" pipes.
- 13)The property owner is unable to directly access the beehive grate that is proposed on the east side of the site on the other side of the retaining wall. In lieu of this configuration for collection of off-site flows, it is recommended to collect the 1.71 cfs on the opposite side of the wall. Grade the site to make sure the flow gets into the grate, and ensure that there is flow away from proposed building.
- 14) In order to make the backyard area more usable, it is permissible to slightly raise the bottom grade of the backyard detention pond.

CITY OF ALBUQUERQUE



If you have any questions, you can contact me at 924-3924.

Sincerely,

Jeanne Wolfenbarger, P.E. Senior Engineer, Planning Dept. Development Review Services

Orig: Drainage file c.pdf Addressee via Email

PO Box 1293

Albuquerque

New Mexico 87103

www.cabq.gov



City of Albuquerque

Planning Department

Development & Building Services Division

DRAINAGE AND TRANSPORTATION INFORMATION SHEET

(REV 02/2013)

Project Title:	Building Permit #:	City Drainage #:
DRB#: EPC#:		Work Order#:
Legal Description:		
City Address:		
Engineering Firm:		Contact:
Address:		
Phone#: Fax#:		E-mail:
Owner:		Contact:
Address:		
Phone#: Fax#:		E-mail:
Architect:		Contact:
Address:		
Phone#: Fax#:		E-mail:
Surveyor:		Contact:
Address:		
Phone#: Fax#:		E-mail:
Contractor:		Contact:
Address:		
Phone#: Fax#:		E-mail:
TYPE OF SUBMITTAL:	CHECK TYPE OF APPROV	AL/ACCEPTANCE SOUGHT:
DRAINAGE REPORT	SIA/FINANCIAL GUARAN	TEE RELEASE
DRAINAGE PLAN 1st SUBMITTAL	PRELIMINARY PLAT APPI	ROVAL
DRAINAGE PLAN RESUBMITTAL	S. DEV. PLAN FOR SUB'D	APPROVAL
CONCEPTUAL G & D PLAN	S. DEV. FOR BLDG. PERMI	IT APPROVAL
GRADING PLAN	SECTOR PLAN APPROVAL	_
EROSION & SEDIMENT CONTROL PLAN (ESC)	FINAL PLAT APPROVAL	
ENGINEER'S CERT (HYDROLOGY)	CERTIFICATE OF OCCUPA	ANCY (PERM)
CLOMR/LOMR	CERTIFICATE OF OCCUPA	ANCY (TCL TEMP)
TRAFFIC CIRCULATION LAYOUT (TCL)	FOUNDATION PERMIT AP	PROVAL
ENGINEER'S CERT (TCL)	BUILDING PERMIT APPRO	DVAL
ENGINEER'S CERT (DRB SITE PLAN)	GRADING PERMIT APPRO	VAL SO-19 APPROVAL
ENGINEER'S CERT (ESC)	PAVING PERMIT APPROV	AL ESC PERMIT APPROVAL
SO-19	WORK ORDER APPROVAL	ESC CERT. ACCEPTANCE
OTHER (SPECIFY)	GRADING CERTIFICATION	N OTHER (SPECIFY)
WAS A PRE-DESIGN CONFERENCE ATTENDED:	Yes No Co	ppy Provided
DATE SUBMITTED:	By:	

Requests for approvals of Site Development Plans and/or Subdivision Plats shall be accompanied by a drainage submittal. The particular nature, location, and scope to the proposed development defines the degree of drainage detail. One or more of the following levels of submittal may be required based on the following

1. Conceptual Grading and Drainage Plan: Required for approval of Site Development Plans greater than five (5) acres and Sector Plans

2. Drainage Plans: Required for building permits, grading permits, paving permits and site plans less than five (5) acres

3. **Drainage Report**: Required for subdivision containing more than ten (10) lots or constituting five (5) acres or more

4. Erosion and Sediment Control Plan: Required for any new development and redevelopment site with 1-acre or more of land disturbing area, including project less than 1-acre than are part of a larger common plan of development

* ZONE 4, ON-SITE

* ZUNE 4, UN-SHE	
******	*********
* 100-YEAR 6	

START	
RAINFALL	TYPE=1 RAIN QUARTER=0.0 IN
	RAIN ONE=2.23 IN RAIN SIX=2.90 IN
	RAIN DELAY=3.65 IN DT=0.03333 HR
* ON-SITE	
COMPUTE NM HYD	ID=1 HYD NO=101.1 AREA=0.001385 SQ MI
	PER A=100.0 PER B=0.0 PER C=0.0 PER D=0.0
	TP=0.1333 HR MASS RAINFALL=-1
*****	***************************************
* 10-YEAR. 6	-HR STORM (UNDER HISTORICAL CONDITIONS) *
******	***************************************
START	
RAINFALL	TYPE=1 RAIN QUARTER=0.0 IN
	RAIN ONE=1.49 IN RAIN SIX=1.93 IN
	RAIN DELAY=2.43 IN DT=0.03333 HR
* ON-SITE	
COMPUTE NM HYD	ID=1 HYD NO=111.1 AREA=0.001385 SQ MI
	PER A=100.0 PER B=0.0 PER C=0.0 PER D=0.0
	TP=0.1333 HR MASS RAINFALL=-1
******	***************************************
* 100-YEAR,	6-HR STORM (UNDER ALLOWABLE CONDITIONS) *

START	
RAINFALL	TYPE=1 RAIN QUARTER=0.0 IN
	RAIN ONE=2.23 IN RAIN SIX=2.90 IN
	RAIN DELAY=3.65 IN DT=0.03333 HR
* ON-SITE	
COMPUTE NM HYD	ID=1 HYD NO=102.1 AREA=0.001385 SQ MI
	PER A=43.0 PER B=20.00 PER C=20.0 PER D=17.0
	TP=0.1333 HR MASS RAINFALL=-1
* OFFSITE 111.0-A	
COMPUTE NM HYD	ID=1 HYD NO=103.2 AREA=0.000839 SQ MI
	PER A=43.0 PER B=20.00 PER C=20.0 PER D=17.0
~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ 	TP=0.1333 HR MASS RAINFALL=-1
	6-HR STORM (UNDER ALLOWABLE CONDITIONS) *

START RAINFALL	TYPE=1 RAIN QUARTER=0.0 IN
RAINFALL	RAIN ONE=1.49 IN RAIN SIX=1.93 IN
	RAIN $ORE = 1.49$ IN RAIN $SIX = 1.95$ IN RAIN DELAY=2.43 IN DT=0.03333 HR
* ON-SITE	RAIN DELAT=2.45 IN DT=0.03335 HR
COMPUTE NM HYD	ID=1 HYD NO=112.1 AREA=0.001385 SQ MI
	PER A=43.0 PER B=20.0 PER C=20.0 PER D=17.0
	TP=0.1333 HR MASS RAINFALL=-1
*****	**************************************
* 100_YEAR	6-HR STORM (UNDER PROPOSED CONDITIONS) *

START	
RAINFALL	TYPE=1 RAIN QUARTER=0.0 IN
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	RAIN DELAY= 3.65 IN DT= 0.03333 HR
* ON-SITE	
COMPUTE NM HYD	ID=1 HYD NO=101.2 AREA=0.001385 SQ MI
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	TP=0.1333 HR MASS RAINFALL= -1
*****	***************************************
* 10-YEAR	6-HR STORM (UNDER PROPOSED CONDITIONS) *

START	
RAINFALL	TYPE=1 RAIN QUARTER=0.0 IN
	RAIN ONE=1.49 IN RAIN SIX=1.93 IN
	RAIN DELAY=2.43 IN DT= 0.03333 HR
* ON-SITE	
COMPUTE NM HYD	ID=1 HYD NO=111.2 AREA=0.001385 SQ MI
	PER A=0.0 PER B=32.0 PER C=10.0 PER D=58.0
	TP=0.1333 HR MASS RAINFALL= -1

*****	***********************

FINISH

AHYMO PROGRAM SUMMARY TABLE (AHYMO_97) -INPUT FILE = sant txt

Location

Lot 15 Block 2, Unit 3, Tract 3, North Albuquerque Acres is located south side of Eagle Rock and west of Ventura Boulverad contains +/-0.89. See attached portion of the Vicinity Map for exact location.

Purpose

The purpose of this drainage report is to present a grading and drainage solution for the proposed buildings.

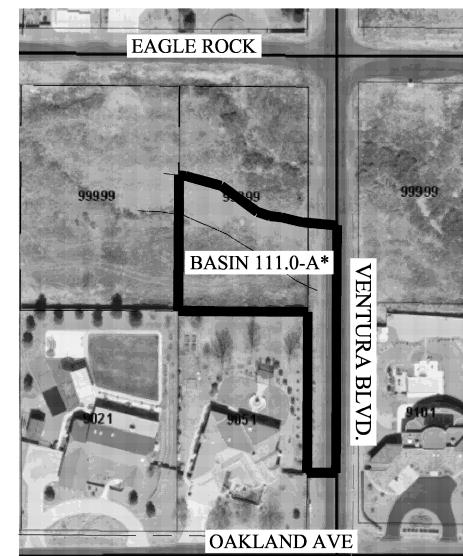
Existing Drainage Conditions

This site is undeveloped and falls within the NAA Master Drainage Plan prepared by RTI. The site is within Basin 111.0 of the RTI report and is subject to 108.83 cfs which passes through the lot at the notherly portion of the site. Another 1.71 cfs enters the mid portion of the lot from the east. Based on the FIRM Map 35001C0141G (revised September 26, 2008) the site does not fall within a 100-year floodplain.

Proposed Conditions and On-Site Drainage Management Plan The developed runoff generated from this site will have to comply with the land treaments set as part of the NAA Master Drainage Plan. Additonal runoff volume generated by this development will be retained on site. Therefore, a retention pond with a volume of 3,545.01 cf is desinged to retain the additional volume. The total retention volume required is only 2,395.08 cf. The offsite flow of 1.71 cfs will intercepted by a beehive inlet and then routed throught the site. The 108.83 cfs will be intercepted by 2-36" CMPs and will be carried across the property.

Calculations

City of Albuquerque, Development Process Manuel, Section 22.2, Hydrology Section, was used for runoff calculations. See this plan for AHYMO input and Summary output files.



* PORTION OF BASIN 111.0 FROM RTI'S NAA MASTER DRAINAGE PLAN OFFSITE BASIN MAP

INPUT FILE = sant.txt		AIT IMO_07)		VERSION.			HYMO-I-9702c0100	
	(DROGRAPH	ROM TO ID ID O. NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC—FT)	RUNOFF (INCHES) (H	TIME TO PEAI HOURS)		GE = 1 ON
START RAINFALL TYPE= 1 COMPUTE NM HYD START RAINFALL TYPE= 1	101.10	- 1	.00139	1.96	.059	.79828	1.500	TIME= RAIN6= 2.213 PER IMP= TIME= RAIN6=	.00 2.900 .00 .00 1.930
COMPUTE NM HYD START RAINFALL TYPE= 1	111.10	- 1	.00139	.71	.020	.26998	1.533	.796 PER IMP= TIME= RAIN6=	.00 .00 2.900
COMPUTE NM HYD COMPUTE NM HYD START	102.10 103.20	— 1 — 1	.00139 .00084	2.81 1.71	.094 .057	1.27203 1.27203	1.500 1.500	3.168 PER IMP= 3.184 PER IMP= TIME=	17.00
RAINFALL TYPE= 1 COMPUTE NM HYD START	112.10	- 1	.00139	1.44	.045	.61083	1.500	RAIN6= 1.624 PER IMP= TIME=	1.930 17.00 .00
RAINFALL TYPE= 1 COMPUTE NM HYD START RAINFALL TYPE= 1	101.20	- 1	.00139	3.87	.149	2.01759	1.500	RAIN6= 4.367 PER IMP= TIME= RAIN6=	2.900 58.00 .00 1.930
COMPUTE NM HYD FINISH	111.20	- 1	.00139	2.40	.087	1.18437	1.500	2.709 PER IMP=	58.00

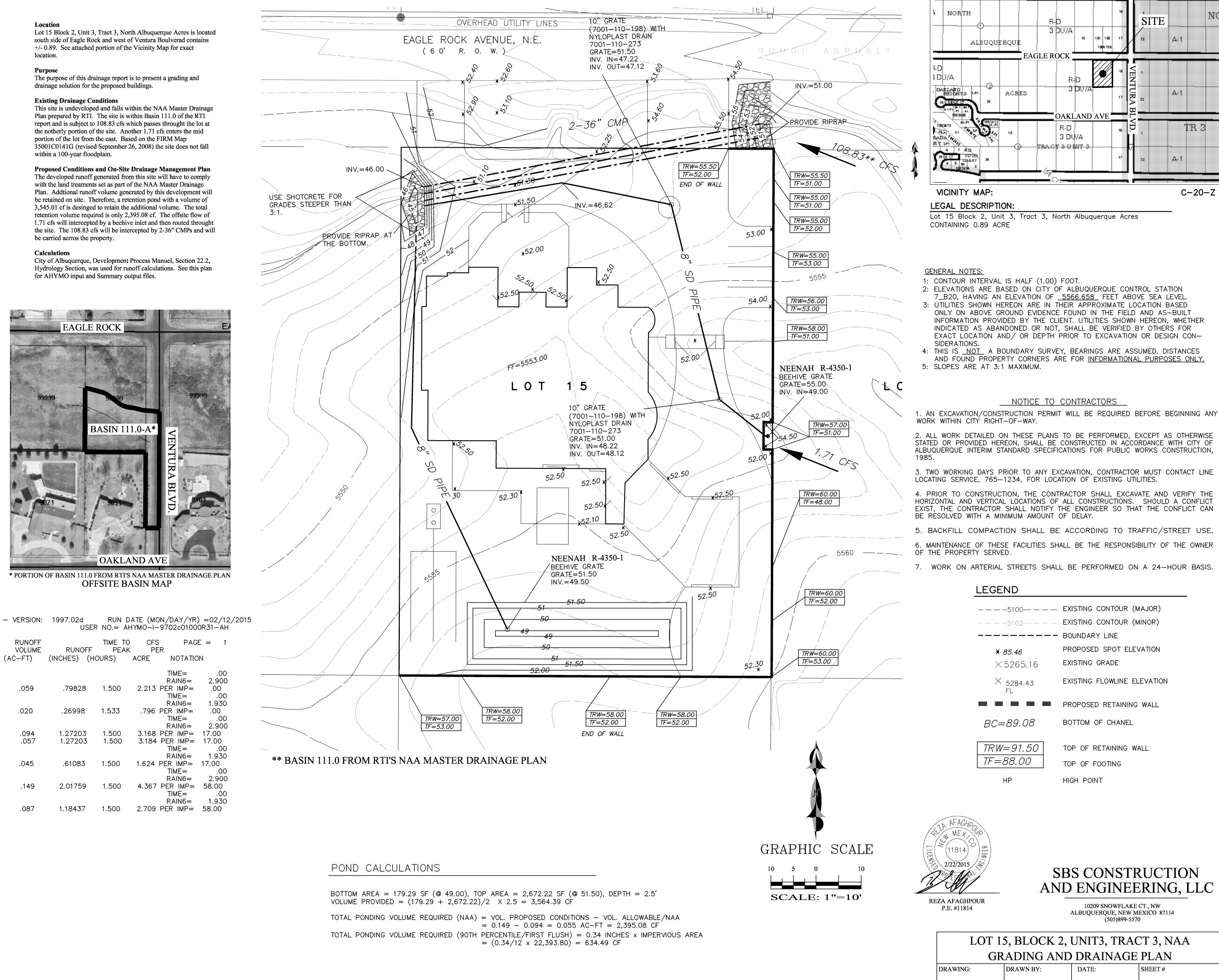
PIPE & GRATE CAPACITY CALULATIONS

CALCULATING PIPE CAPACITY UTING ORFICIE EQUATION: $Q = CA(2gh)^{0.50}$

 $\frac{\text{NEENAH R-4350-1 BEEHIVE GRATE:}}{Q = 0.6 \times 0.30 (2 \times 32.2 \times 2.0)^{\circ} 0.50 = 2.04 \text{ cfs}}$ $\frac{\text{NYLOPLAST 10" GRATE (7001-110-198):}}{Q = 0.6 \times 0.20 (2 \times 32.2 \times 1.0)^{\circ} 0.50 = 0.95 \text{ cfs}}$ 8" PVC: $Q = 0.6 \times 0.35 (2 \times 32.2 \times 2.88)^{0.50} = 2.85 \text{ cfs}$

2-36" PVC:

 $Q = 2 \times 0.6 \times 7.07 (2 \times 32.2 \times 4.0)^{0.50} = 136.07 \text{ cfs}$



201423-GR.DWG

LAST REVISION: 2/22/2015

SH-B

2-16-2015